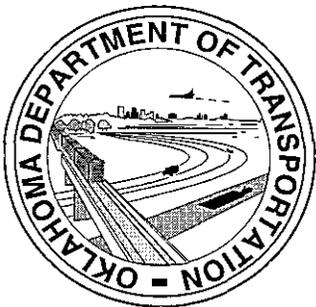


# **NOVACHIP- ULTRATHIN BONDED WEARING COURSE**

**Construction Report  
October 2005**

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**Wilson B. Brewer, Jr.  
Project Manager  
Gary Williams, P.E.  
Engineering Manager**



**Planning & Research Division  
Oklahoma Department of Transportation  
200 N.E. 21<sup>st</sup> Street, Room 3A7  
(405)522-3207  
FAX (405)521-6948**

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<p>Open Graded Friction Course (OGFC) pavement sheds water and provides good skid resistance. A problem with OGFC surfaces is short life before raveling begins. The Oklahoma Department of Transportation (ODOT) is testing several alternative surfaces. Surfaces being tested and compared include Novachip placed over new and old asphalt mixes, an OGFC and a Permeable Friction Course (PFC), each over new mixes. The report looks at construction techniques and compares each of the alternative surfaces.</p> <p>Other state DOT's report that Novachip has a proven track record in the maintenance field. Uses include repairing of oxidized surfaces, extending life of the surface and restoring skid resistance. ODOT needs to know if the cost and performance of NovaChip is equal to/or better than OGFC, ODOT's standard surface treatment.</p> <p>Based on ease of construction, NovaChip and OGFC were approximately equal, yet the requirement of the special NovaChip laydown machine makes the that product unique. The PFC also needed a special machine (at the plant) to introduce the fiber into the mix.</p>			
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## SI (METRIC) CONVERSION FACTORS

<i>Approximate Conversions to SI Units</i>				<i>Approximate Conversions from SI Units</i>					
Symbol	When you know	Multiply by	To Find	Symbol	Symbol	When you know	Multiply by	To Find	Symbol
<b>LENGTH</b>				<b>LENGTH</b>					
in	inches	25.40	millimeters	mm	mm	millimeters	0.0394	inches	in
ft	feet	0.3048	meters	m	m	meters	3.281	feet	ft
yd	yards	0.9144	meters	m	m	meters	1.094	yards	yd
mi	miles	1.609	kilometers	km	km	kilometers	0.6124	miles	mi
<b>AREA</b>				<b>AREA</b>					
in <sup>2</sup>	square inches	645.2	square millimeters	mm <sup>2</sup>	mm <sup>2</sup>	square millimeters	0.00155	square inches	in <sup>2</sup>
ft <sup>2</sup>	square feet	0.0929	square meters	m <sup>2</sup>	m <sup>2</sup>	square meters	10.764	square feet	ft <sup>2</sup>
yd <sup>2</sup>	square yards	0.8361	square meters	m <sup>2</sup>	m <sup>2</sup>	square meters	1.196	square yards	yd <sup>2</sup>
ac	acres	0.4047	hectares	ha	ha	hectares	2.471	acres	ac
mi <sup>2</sup>	square miles	2.590	square kilometers	km <sup>2</sup>	km <sup>2</sup>	square kilometers	0.3861	square miles	mi <sup>2</sup>
<b>VOLUME</b>				<b>VOLUME</b>					
fl oz	fluid ounces	29.57	milliliters	mL	mL	milliliters	0.0338	fluid ounces	fl oz
gal	gallons	3.785	liters	L	L	liters	0.2642	gallons	gal
ft <sup>3</sup>	cubic feet	0.0283	cubic meters	m <sup>3</sup>	m <sup>3</sup>	cubic meters	35.315	cubic feet	ft <sup>3</sup>
yd <sup>3</sup>	cubic feet	0.7645	cubic meters	m <sup>3</sup>	m <sup>3</sup>	cubic meters	1.308	cubic feet	yd <sup>3</sup>
<b>MASS</b>				<b>MASS</b>					
oz	Ounces	28.35	grams	g	g	grams	0.0353	Ounces	oz
lb	pounds	0.4536	kilograms	kg	kg	kilograms	2.205	pounds	lb
T	short tons (2000 lb)	0.907	megagrams	Mg	Mg	megagrams	1.1023	short tons (2000 lb)	T
<b>TEMPERATURE (exact)</b>				<b>TEMPERATURE (exact)</b>					
°F	degrees Fahrenheit	(°F-32)/1.8	degrees Celsius	°C	°C	degrees Celsius	9/5+32	degrees Fahrenheit	°F
<b>FORCE and PRESSURE or STRESS</b>				<b>FORCE and PRESSURE or STRESS</b>					
lbf	poundforce	4.448	Newtons	N	N	Newtons	0.2248	poundforce	lbf
lbf/in <sup>2</sup>	poundforce per square inch	6.895	kilopascals	kPa	kPa	kilopascals	0.1450	poundforce per square inch	lbf/in <sup>2</sup>

# **NOVACHIP- ULTRATHIN BONED WEARING COURSE**

## **CONSTRUCTION REPORT**

**Wilson B. Brewer, Jr.**

**Project Manager**

**Gary Williams, P.E.**

**Engineering Manager**

Under the Supervision

of

Dawn R. Sullivan, P.E.

Division Engineer

Planning & Research Division

Oklahoma Department of Transportation

200 Northeast 21<sup>st</sup> Street RM 3A7

Oklahoma City, Oklahoma 73105

October 2005

# TABLE OF CONTENTS

INTRODUCTION.....	1
BACKGROUND.....	1
OBJECTIVES.....	3
INFORMATION ON PROJECTS.....	3
SPECIFICATIONS.....	4
NOVACHIP CONSTRUCTION .....	5
INVESTIGATION.....	8
CONCLUSIONS.....	8
REFERENCES.....	10
APPENDIX A. PAVEMENT MANAGEMENT PROJECT REPORTS.....	A-1 thru A-4
APPENDIX B. TYPICAL SECTION DRAWINGS.....	B-1 thru B-4
APPENDIX C. SPECIFICATIONS.....	C-1 thru C-11

## LIST OF FIGURES

Figure 1. A small pop out and cracking.....	2
Figure 2. Centerline raveling and cracking.....	2
Figure 3. An asphalt transfer machine mixing NovaChip and placing it into the hopper.....	5
Figure 4. A Novachip laydown machine.....	6
Figure 5. A special membrane tack sprayed in front of the mat.....	6
Figure 6. Placing a NovaChip mat.....	7
Figure 7. Cornell Construction Company rolls NovaChip.....	7

## **INTRODUCTION**

Open Graded Friction Course (OGFC) surfaces are placed on roadways for reasons associated with increased friction (traction) for motorist safety. In the past years, raveling problems typically occur in a short period of time (6 to 8 years) after the OGFC is placed on many of the roadways(1). The Oklahoma Department of Transportation (ODOT) is experimenting with several products and procedures to achieve longer life of the surface and provide the same level of safety as OGFC. NovaChip, trademark name, is a proprietary surface treatment which can be utilized to enhance skid properties, resist rutting, seal raveling in existing roads which have permeability issues, and reduce the water spray during rainy weather. NovaChip is being used on interstate roads in other states (2). NovaChip is an ultrathin bonded wearing surface with coarse aggregates in a hot mix placed over a special liquid asphalt membrane using a special machine to place the membrane without tire contact. In this project, an OGFC and an experimental Permeable Friction Course (PFC), placed on interstate roads, will have a performance comparison to the NovaChip surface.

## **BACKGROUND**

Data regarding pavement conditions were recorded by ODOT's Pavement Management Branch of the Planning & Research Division. The data for stretches of roadway where the NovaChip was placed was recorded in 2001, before rehabilitation construction. Pavement condition ratings referred to in this report have a scale indices from 0 to 100, with 100 being the best. The indices are as follows. Ride is the measure of pavement smoothness in International Ride Index (IRI). Rutting is a measure of permanent wheel path deformation in asphalt concrete (AC) pavements. The functional index is a measure of non-load related distresses. The structural index is a measure of load-related distresses.

ODOT's Pavement Management branch had pavement condition summary sheets for the areas where the NovaChip project was located, based on data collected in the fall of 2001, the last data filed prior to rehabilitation construction. See Figures 1 & 2 for examples of distresses.



Figure 1 A small pop out and cracking.



Figure 2. Centerline raveling and cracking.

The NovaChip project in Custer county had a ride index of 99, a rut index of 100, a functional index of 97, and a structural index of 99. The surface material was constructed with a porous mix, which gave it a functional index of 97. A second NovaChip project in Washita county had a pavement condition summary sheet that was based on data collected in 2001. A high level of deformation was evident, as shown by the rut index of 62. Other indices were ride at 89, the functional index at 76 and the structural index at 87. See Appendix A for pavement management project reports.

The PFC project area in Custer county was rated in September of 2001. The ride index was the lowest of all the ratings with an index of 83. The other ratings were a rut index of 97, a functional index of 93 and a structural index of 99. Beckham county is where the OGFC is located. It had a rutting problem that was measured and recorded on the pavement condition summary sheets in September 2001. The rut index was 51, the ride index was 80, the functional index was 91 and the structural index was 98. See Appendix A for a complete detail report of ODOT's pavement management project reports.

## **OBJECTIVES**

The objectives of this project are as follow:

- Document existing pavement conditions.
- Monitor construction, document specifications and construction procedures.
- Evaluate and assess NovaChip properties under field conditions.
- Compare performance of NovaChip to a PFC and an OGFC.
- Write reports 1) after construction and 2) after several years of monitoring.

## **INFORMATION ON PROJECTS**

The Novachip surface was placed on two projects. The first Novachip project was number IM-40-3(066)069, located on Interstate 40 in Custer County. It begins at mile marker 69.19 and extends east to mile marker 76.75. This was a resurface job done for the purpose of enhancing skid properties and sealing the existing Type BH asphaltic concrete pavement surface. The second project was

number IMY-0040-2(127)045, on Interstate 40 in Washita County. It begins at mile marker 45.17 and extends east to mile marker 52.97. The project was a deep (1 ½ inch ) coldmilling operation and included several full depth removal locations (underneath overpasses and through a lake area). The remaining part of the contract included placing three inches of Type S-3 PG 76-28 asphalt concrete, four inches of Type S-4 PG 76-28 asphalt and a ¾ inch layer of NovaChip as a wearing surface. See Appendix B for typical sections drawings.

The PFC project was located on Interstate 40 in Custer County, project number IMY-40-3(063)076. It begins at 83.63 and extends west to mile marker 86.27. Three inches of existing pavement was removed by coldmilling. It was then overlaid with six inches of Type S-3 PG 76-28 asphalt, two inches of Type S-5 PG 76-28 asphalt and ¾ inch layer of PFC as a wearing surface.

The standard OGFC project was located on Interstate 40 in Beckham County. The project number was IMY-40-2(115)033, it begins at mile marker 33.02 and extends east to mile marker 40. The project is a deep mill (2 inches)and overlay project with five inches of Type S-3 PG 76-28 asphalt, two inches of Type S-4 PG 76-28 asphalt and a ¾ inch layer OGFC as a wearing surface. See Appendix B for typical sections drawings.

## **SPECIFICATIONS**

Specifications for each of the surfacing operations have similar characteristics. Mixing temperature for the NovaChip was between 320°F and 350°F. Compaction temperature was between 195°F and 330°F. Asphalt content was 5.0% of mix weight. See Appendix C, NovaChip Mix Design Sheet.

The PFC mixing temperature was 340° F to 345°F. This mix included polypropene fibers that were introduced and mixed into the PFC with a machine built by High Tech (a trademark name). The compaction temperature was 300°F. Its asphalt content was 6.0% +/- 0.4% of mix weight. See Appendix C, Special Provision, Bases and Surfaces.

The OGFC mixing temperature was between 275° F and 350° F, and the asphalt content was 5.7% +/- 0.3% of mix weight. See Appendix C for an ODOT Materials Division Mix Design Sheet. Immediately following placement of the OGFC material, the surface was rolled for 2 or 3 passes with the static steel wheel, self-propelled roller, according to ODOT specification number 405.04(I), (7).

## NOVACHIP CONSTRUCTION

NovaChip's plant operation consisted of preparing a two bin mix, that included a 3/4 inch gap graded aggregate processed by a dryer drum mixer. The aggregates were blended with a PG 76-28 OK liquid asphalt. The mix was then transported to the job site, after loading into dump trucks.

At the site, the asphalt transfer machine would receive the material, remix it and convey it into the front hopper of the Novachip machine. See Figure 3.



Figure 3. An asphalt transfer machine mixing NovaChip and placing it into the hopper.

The Novachip laydown machine moves the asphaltic concrete material to the back hopper, from where it is placed on the roadway. See Figure 4.



Figure 4. A NovaChip laydown machine.

This machine carried a special membrane tack with it that was sprayed on. After spraying, the spray sets up to form a waterproof membrane. The spraying operation takes place between the back wheels and the screed. This system allowed for the tack to meet the surface without the back wheels of the laydown machine tracking through it. See Figure 5.



Figure 5. A special membrane tack sprayed in front of the screed.

The asphalt material was laid  $\frac{7}{8}$  inch thick with a  $\frac{1}{8}$  inch roll down, making it  $\frac{3}{4}$  inch mat and resulting in an average weight of 72 lbs per sq ft. See Figure 6.



Figure 6. Placing a NovaChip mat.

Mat temperature was 285°F, measured behind the laydown machine. A steel wheel roller moved as close as 10ft behind. A second steel wheel roller followed the first. A rolling pattern was established although no density specification was used. On each project, the outside lanes were placed, then the inside lanes See Figure 7.



Figure 7. Connell Construction Company rolls Novachip.

## **INVESTIGATION**

The Custer County NovaChip project was evaluated after the Field Division personnel's final inspection. Several isolated areas were noticed to have small amounts of segregation. These were located at the start of the project and made up less than 1% of the total surface area. The Custer County project was generally easy to construct. Work on the Washita County project resulted in no problems in placement. No segregation was found. The following observations were made on both projects during the investigation. Both projects had a minimum amount of traffic delay, a quiet ride, shed water and very little tire spray in wet weather.

The Custer County PFC project had some constructability issues. The fibers needed to be hand placed into the machine that introduced them into the drum plant. The yard where the plant was located had a noticeable amount of fiber on the ground. The asphalt concrete's drain down of liquid asphalt was low and its performance was excellent. The mix was stiffer than an OGFC in placement. The completed PFC surface has a quiet ride, sheds water and very little water spray in wet weather.

The OGFC is not a difficult mix to construct. It is laid at 69 lbs per sq ft. The Beckham County project had about 1% of segregation of the total surface area when completed. It also has a quiet ride, shed water and very little water spray in wet weather. The construction cost was less the other mixes.

## **CONCLUSION**

All three treatments were similar in their constructability performance evaluation. Water spray during and after a rain storm was reduced and each of the surfaces provided good visibility. This results in an improved safety factor for the motoring public.

The NovaChip project in Custer County was constructed directly on the surface of an oxidized, well traveled, porous surface. The other surface courses were over new mixes. This makes the NovaChip project difficult to evaluate against any of the other projects in this study. Novachip laydown represents an added cost for the average contractor because it requires special equipment. Novachip has a good track record with other state Department of Transportation agencies as a maintenance surface repair treatment (8). All of the surfaces will be observed for at least three years and their performance will be compared. If an accurate life cycle cost analysis is to be done, it should take place after failure of the surfaces.

## REFERENCES

1. Gerald Huber, "Performance Survey on Open-Graded Friction Course Mixes," National Cooperative Highway Research Program, Synthesis of Highway Practice 284, Washington, D.C., 2000.
2. "NovaChip Ultrathin Bonded Wearing Course," KOCH Pavement Solutions, Tulsa, Oklahoma, March 2004.
3. Donald E. Watson, Kathryn Ann Moore, Kevin Williams, and L. Allen Cooley, Jr., "Refinement of New-Generation Open-Graded Friction Course Mix Design," Transportation Research Record, No. 1832, p. 78, October 2003.
4. Vittorio Ranieri, "Runoff Control in Porous Pavements," Transportation Research Record, No. 1789, p. 46. September 2002.
5. Jeff S. Uhlmeyer, Linda M. Pierce, Jim T. Weston, "NOVACHIP," Washington State Department of Transportation, SR-17 City of Soap Lake, June 2003.
6. L. Allen Cooley, Jr., E. Ray Brown, Donald E. Watson, "Evaluation of OGFC Mixtures Containing Cellulose Fibers," NCAT Report No. 2000-05, December 2000.
7. "Standard Specifications for Highway Construction," Oklahoma Department of Transportation, Sections 406 and 708, Oklahoma, August 1999.
8. Ted Keiter, "Novachip," Final Report, Commonwealth of Pennsylvania Department of Transportation, October 2002.

# **APPENDIX A**

## **PAVEMENT MANAGEMENT PROJECT REPORTS**

# Pavement Management Project Report

Oklahoma Department of Transportation

September 2003

J/P Number: 20258(04)  
Project Number: IMY-040-3(066)069  
Proposed Let Date: 4/2004  
Highway/County: I-40 in Custer County  
Location: From MM 69 to 77  
Control: 20-04, 0.00-7.59  
Milemarkers: 69.16-76.75  
Length: 7.59 mi

Original Construction: 1962 - I-40-3(12)069 - 4.5" Asphalt Concrete on 8" FABB & 6" Select

Treatments: 1979 - I-IR-40-3(40)069 - OGFC  
1997 - 2" AC Type B, 3" AC Type A, 2" coldmilling & fabric

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## Pavement Condition Summary (Data Collected Fall 2001)

Condition Indices (0-100 scale with 100 being best):

Ride Index	99	A measure of pavement smoothness (IRI converted to 0-100 scale)
Rut Index	100	A measure of rutting in AC pavements
Functional Index	97	A measure of non-load related distresses
Structural Index	99	A measure of load-related distresses

### Distresses:

Average IRI (in/mi)	64	<60=v. good;61-90=good;91-120=fair;120-170=poor;>170=v.poor
Average Rut Depth (in)	0.09	<0.15=good; 0.15-0.35=fair; >0.35=poor
Maximum Rut Depth (in)	0.15	<0.4=good; 0.4-1.0=fair; >1.0=poor
# of Low Sev. Transv. Cracks/0.01mi	0.2	<2.0=good; 2.0-4.0=fair; >4.0=poor
# of M/H Sev. Transv. Cracks/0.01 mi	0.0	<0.5=good; 0.5-1.0=fair; >1.0=poor
% Length w/ Low Sev. Allig. Crack	1%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ M/H Sev. Allig. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Low Sev. Misc. Crack	3%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ M/H Sev. Misc. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Raveling	0%	<5%=good; 5%-15%=fair; >15%=poor
% Area w/ AC Patching	0.2%	<0.6%=good; 0.6%-1.3%=fair; >1.3%=poor

### Discussion:

There is little observable distress on this pavement. Ride is good and rutting is low.

# Pavement Management Project Report

## Oklahoma Department of Transportation

Jan. 24, 2003

J/P Number: 19574(04)  
Project Number: IMY-40-2(127)045  
Proposed Let Date: 02/2004  
Highway/County: I-40 Washita County  
Location: W of Clinton, MP 45 to 53  
Control: 75-02, 3.18-11.01  
Milemarkers: 45.17-53.00  
Length: 7.83 mi  
Type: Rural Interstate

Original Construction: 1960 - I-40-2(17)047 - 4.5" AC, 8" Soil Cement, 6" Select Borrow

Treatments: 1967 - IMC-75(12) - 1.5" Overlay  
1977 - I-40-2(85)047 - AC Leveling Course and OGFC  
1988 - IR-40-2(105)045 - 2" AC Type B w/ 1.25" Coldmill, Fabric & OGFC

---

### Pavement Condition Summary (Data Collected 9/2001)

#### Condition Indices (0-100 scale with 100 being best):

Ride Index	89	A measure of pavement smoothness (IRI converted to 0-100 scale)
Rut Index	62	A measure of rutting in AC pavements
Functional Index	76	A measure of non-load related distresses
Structural Index	87	A measure of load-related distresses

#### Distresses:

Average IRI (in/mi)	86	<60=v. good;61-90=good;91-120=fair;120-170=poor;>170=v.poor
Average Rut Depth (in)	0.31	<0.15=good; 0.15-0.35=fair; >0.35=poor
Maximum Rut Depth (in)	0.51	<0.4=good; 0.4-1.0=fair; >1.0=poor
# of Low Sev. Transv. Cracks/0.01mi	0.4	<2.0=good; 2.0-4.0=fair; >4.0=poor
# of M/H Sev. Transv. Cracks/0.01 mi	0.6	<0.5=good; 0.5-1.0=fair; >1.0=poor
% Length w/ Low Sev. Allig. Crack	10%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ M/H Sev. Allig. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Low Sev. Misc. Crack	35%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ M/H Sev. Misc. Crack	1%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Raveling	0%	<5%=good; 5%-15%=fair; >15%=poor
% Area w/ AC Patching	1.7%	<0.6%=good; 0.6%-1.3%=fair; >1.3%=poor

#### Discussion:

The predominant pavement distresses seen on this project are significant rutting, extensive miscellaneous (non-wheelpath) cracking and depressed thermal cracks that are beginning to reflect through the last overlay. There is also a significant amount of blade patching. Ride is good.

# Pavement Management Project Report

Oklahoma Department of Transportation

June 22, 2005

J/P Number:

Project Number:

Proposed Let Date:

Highway/County: I-40 in Custer County

Location: From Milepost 83.63 to 86.27

Control & Distance: 20-04 from 14.76 to 17.11

Project Length: 2.35 miles

Original Construction: EB—1959—I-40-3(6)082- 4.5" AC on 8" Soil Cement Base  
WB—1967—I-40-3(22)082—4.5" AC on 8" FABB

Treatments:

EB—1971—I-40-3(34)082 - 2" AC Overlay

EB & WB—1980—I-40-3(49)077—3" AC Overlay w/ 2" Coldmilling

EB & WB (outside lanes only) - year unknown - Mill & Overlay (not recorded but seen in video)

---

## Pavement Condition Summary (Data Collected Sept 2001)

### Condition Indices (0-100 scale with 100 being best):

Ride Index	83	A measure of pavement smoothness (IRI converted to 0-100 scale)
Rut Index	97	A measure of rutting in AC pavements
Functional Index	93	A measure of non-load related distresses
Structural Index	99	A measure of load-related distresses

### Distresses:

Average IRI (in/mi)	103	<60=v.good;61-94=good;95-119=fair;120-170=poor;>170=v.poor
Average Rut Depth (in)	0.11	<0.25=good; 0.25-0.50=fair; >0.50=poor
# of Low Sev. Transv. Cracks/0.01 mi	0.9	<2.0=good; 2.0-4.0=fair; >4.0=poor
# of Med/Hi Sev. Transv. Cracks/0.01 mi	0.0	<0.5=good; 0.5-1.0=fair; >1.0=poor
% Length w/ Low Sev. Allig. Crack	1%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ Med/Hi Sev. Allig. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Low Sev. Misc. Crack	1%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ Med/Hi Sev. Misc. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Raveling	0%	<5%=good; 5%-15%=fair; >15%=poor
% Area w/ AC Patching	0.0%	<0.6%=good; 0.6%-1.3%=fair; >1.3%=poor
Skid Number (3/2002)	37	<35=poor

### Discussion:

This pavement has had a mill and overlay in the outside lane since 1980 that was not recorded. The primary distress seen in this pavement was poor skid.

# Pavement Management Project Report

Oklahoma Department of Transportation

June 22, 2005

J/P Number:

Project Number:

Proposed Let Date:

Highway/County: I-40 in Beckham County

Location: From Milepost 33.02 to 40

Control & Distance: 05-04 from 7.81 to 14.99

Project Length: 7.18 miles

Original Construction: 1971—I-40-2(19)032—4.5" AC on 8" FABB w/ 6" Lime Treat Subgrade

Treatments: 1985—IR-40-2(101)032 - 2" AC Overlay w/ fabric  
Year Unknown—Mill and Overlay outside lanes only (seen in video)

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## Pavement Condition Summary (Data Collected Sept 2001)

### Condition Indices (0-100 scale with 100 being best):

Ride Index	80	A measure of pavement smoothness (IRI converted to 0-100 scale)
Rut Index	51	A measure of rutting in AC pavements
Functional Index	91	A measure of non-load related distresses
Structural Index	98	A measure of load-related distresses

### Distresses:

Average IRI (in/mi)	110	<60=v.good;61-94=good;95-119=fair;120-170=poor;>170=v.poor
Average Rut Depth (in)	0.38	<0.25=good; 0.25-0.50=fair; >0.50=poor
# of Low Sev. Transv. Cracks/0.01 mi	0.0	<2.0=good; 2.0-4.0=fair; >4.0=poor
# of Med/Hi Sev. Transv. Cracks/0.01 mi	0.5	<0.5=good; 0.5-1.0=fair; >1.0=poor
% Length w/ Low Sev. Allig. Crack	1%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ Med/Hi Sev. Allig. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Low Sev. Misc. Crack	1%	<10%=good; 10%-35%=fair; >35%=poor
% Length w/ Med/Hi Sev. Misc. Crack	0%	<5%=good; 5%-15%=fair; >15%=poor
% Length w/ Raveling	0%	<5%=good; 5%-15%=fair; >15%=poor
% Area w/ AC Patching	0.8%	<0.6%=good; 0.6%-1.3%=fair; >1.3%=poor
Skid Number (3/2002)	37	<35=poor

### Discussion:

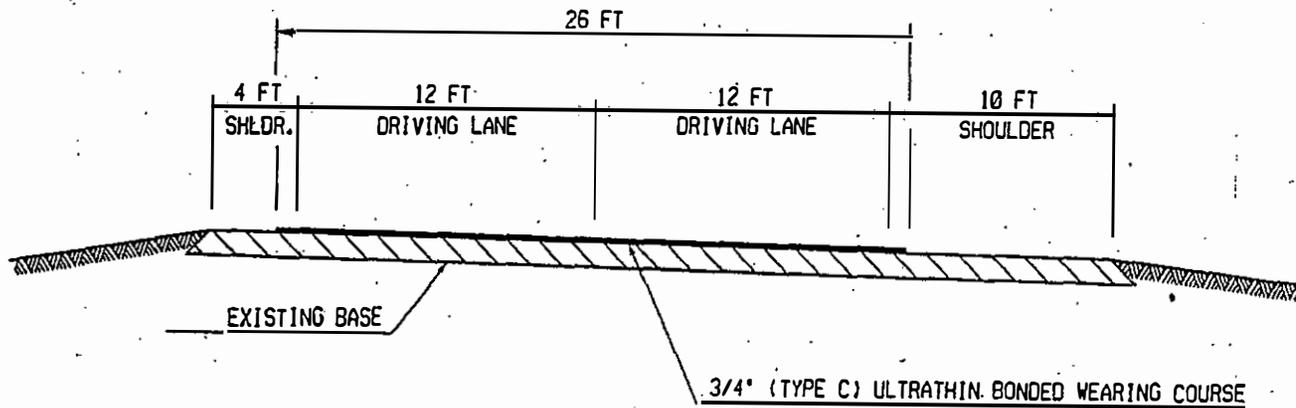
This pavement has had a mill and overlay in the outside lane since 1985 that was not recorded. The primary distresses seen in this pavement were moderate rutting, patching, depressed thermal cracks that were starting to reflect through the overlay in the outside lanes, and poor skid.

# **APPENDIX B**

## **TYPICAL SECTION DRAWINGS**

STATE OF OKLAHOMA  
DEPARTMENT OF TRANSPORTATION  
PLAN OF PROPOSED  
ULTRATHIN BONDED WEARING COARSE,  
I-40

PROJECT NO. IMY-0040-3(066)069 JP#:20258(04)



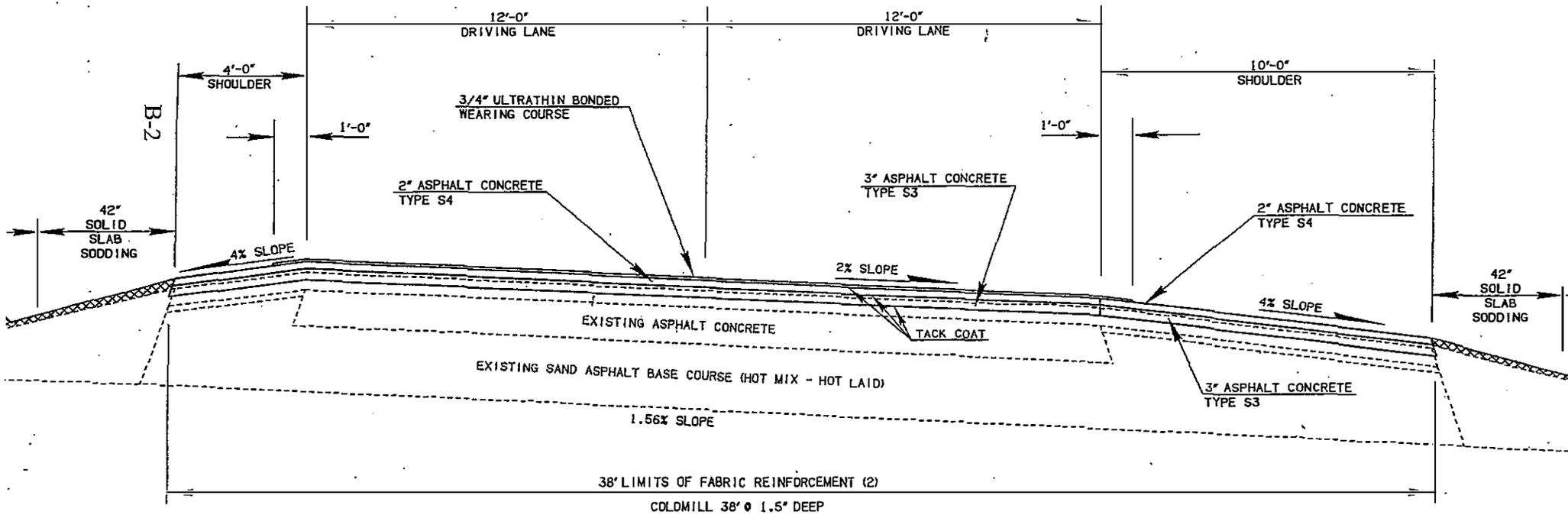
B-1

TYPICAL SECTION NUMBER

CUSTER COUNTY

STATE HIGHWAY  
 FEDERAL AID PROJECT NO. IMY-0040-20(27)045  
 INTERSTATE RESURFACING PLANS  
 INTERSTATE HIGHWAY NO. 40  
**WASHITA COUNTY**

CONTROL SECTION NO. 40-75-02  
 STATE JOB NO. 19574(04)  
 NBIS NO. 16937 & 16936 , 14359 & 14358



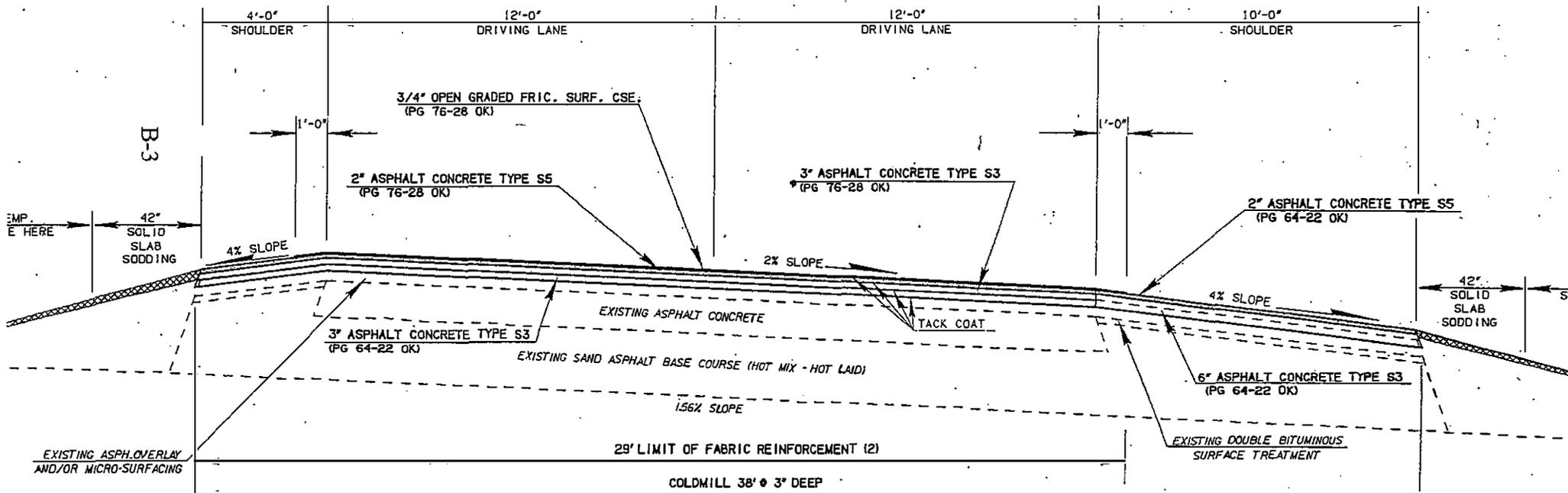
**HALF TYPICAL SECTION NO. 1**

STA. 2428+00.00 TO STA. 2474+60.82 EASTBOUND  
 STA. 2475+92.38 TO STA. 2678+34.34 EASTBOUND  
 STA. 2689+65.67 TO STA. 2708+36.00 EASTBOUND  
 STA. 2691+65.67 TO STA. 2708+36.00 WESTBOUND  
 STA. 2718+66.00 TO STA. 2840+00.00 EAST & WEST BOUND

**STATE HIGHWAY**  
**FEDERAL AID PROJECT NO. IMY-40-3 (063) 076 IR**  
**INTERSTATE RESURFACING PLANS**  
**INTERSTATE HIGHWAY NO. 40**

**CUSTER COUNTY**

**CONTROL SECTION NO. 40-20-04**  
**STATE JOB NO. 12687(05)**



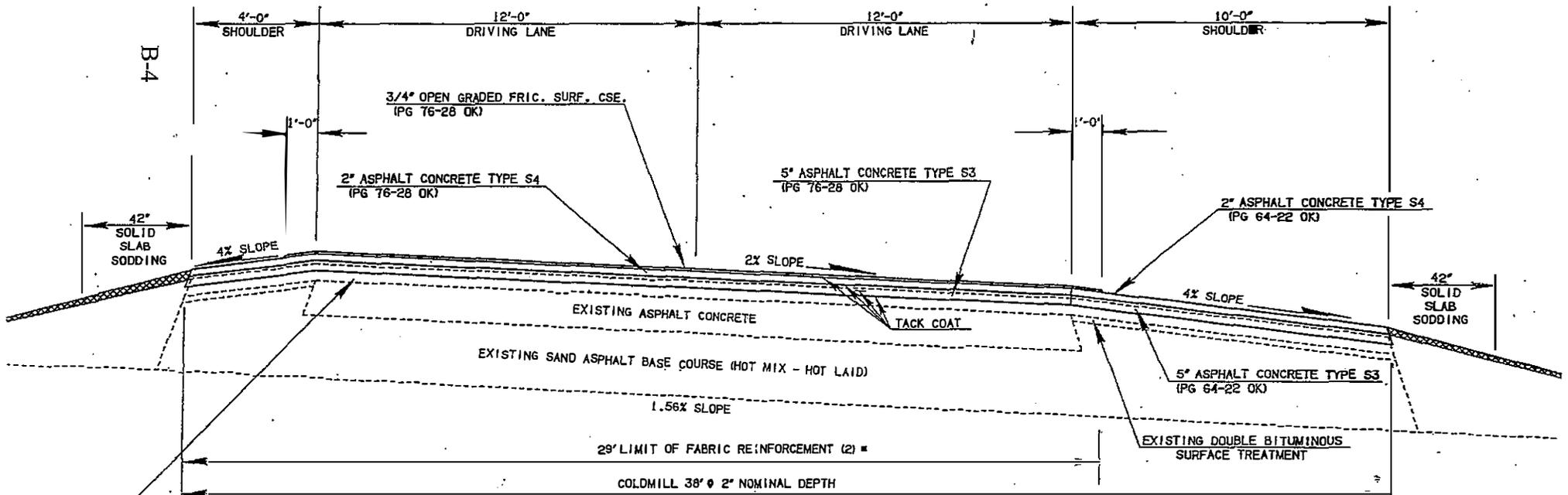
**HALF TYPICAL SECTION NO. 1**

STA. 4477+30.00 TO STA. 4528+00.00 EASTBOUND  
 STA. 4477+30.00 TO STA. 4538+00.00 WESTBOUND  
 STA. 4562+00.00 TO STA. 4601+29.42 EASTBOUND  
 STA. 4562+00.00 TO STA. 4601+29.42 WESTBOUND

**STATE HIGHWAY**  
**FEDERAL AID PROJECT NO. MY-40-2(115)033**  
**OVERLAY, SURFACING, EROSION CONTROL & TRAFFIC PLANS**  
**INTERSTATE HIGHWAY NO. 40**

**BECKHAM COUNTY**

CONTROL SECTION NO. 40-05-01  
 STATE JOB NO. 10814(04)



**HALF TYPICAL SECTION NO. 1**

- STA. 1781+46.41 TO STA. 2151+73.86 WEST BOUND

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- STA. 1779+94.75 TO STA. 2151+69.43 EAST BOUND

OVERLAY  
 IMPACTING

**APPENDIX C**  
**SPECIFICATIONS**



**NOVACHIP® MIX DESIGN SHEET**

KOCH MATERIALS LABORATORY 415 NORTH 10th STREET TERRE HAUTE, INDIANA 47807

PROJECT - I-40, Custer Co., Weatherford, OK	W.O. US.OK.NC.2004.0378
CONTRACTOR - Ritchie Paving	DATE COMPLETED: 19-Aug-04
BINDER - PG 76-28 OK $G_o = 1.025$	ENGINEER: Stephen Fain
SUPPLIER - KMC - Muskogee	TECHNICAL CONTACT: Robert Lee
SALESMAN - Joe Ridley	PHONE: (817) 504-7540

Mixing Temperature 320-350 °F  
 Compaction Temperature 290-310 °F  
 Asphalt Content Percentage 5.0 %  
 Add 0.5% Kling Beta 2912

**AGGREGATE GRADATIONS - INDIVIDUAL AND BLEND**

KMC Lab No.		2003.0696	2003.0701	2003.0702	100.0	Type C
		C-Run MMM@S	5/8" MMM@S	Scrms. MMM@S		
% in Blend		27.0	65.0	8.0	Blend	Specs
SIEVE						
1/2"	12.50 mm	98	95	100	96	75 - 100
3/8 "	9.50 mm	94	65	100	75	50 - 80
#4	4.75 mm	76	7	94	33	25 - 38
#8	2.36 mm	57	3	66	22	19 - 27
#16	1.18 mm	42	2	46	16	15 - 23
#30	0.600 mm	30	2	31	12	10 - 18
#50	0.300 mm	22	2	21	9	8 - 13
#100	0.150 mm	16	1	14	6	6 - 10
#200	0.075 mm	11.4	1.1	9.4	4.5	4.0 - 6.0
Aggregate Gsb		2.596	2.600	2.586	2.598	
FAA (T 304-96, Method A)					47	40 min
Sand Equivalency (T176-86)	60			72	57	45 min
Meth. Blue (TP57-99)	9			5	8	10 max
F & E, 3:1 (D4791-95)	39	10			19	25 max
Micro-Deval (TP58-99)	7	12			10	18 max
LA. Abrasion (T96-99)	26	25			25	35 max
Crushed Face (ASTM D 5821)						
% Crushed, single face	100	100			100	95 min
% Crushed, two faces	100	100			100	85 min
Water Absorption (T255-92)	0.8	0.6	0.9		0.8	*Producers Historical Data

Draindown (T305-97) 0.01 % Film Thickness = 11 microns

Tensile Strength Ratio (T283) 105 % Recommended min. emulsion shot rate = 0.21 gal/yd<sup>2</sup>

The rates given for the application of polymer modified emulsion do not take existing surface conditions into account. The rates need to be field adjusted to account for the field NovaChip mix properties and the existing pavement surface condition.

Test data reported herein has been secured by reliable testing procedures. As we have no knowledge of, or control over, the conditions that may affect the use of material from which the samples were taken, we assume no responsibility in furnishing this data other than to warrant that they represent reliable measurements of the properties of the sample received and tested.

**STATE OF OKLAHOMA  
DEPARTMENT OF TRANSPORTATION  
MATERIALS DIVISION**

A.D. No. 007-008-004 PFC Design No. 3077-OAEST-04075  
 Project No. IMY-40-3(63)76IR 12687(05) Hwy. I-40 ESAL 30M+  
 Contractor Western Plains Construction Co. Producer Western Plains Const. Co.

MATERIAL	SOURCE	%USED
3/4" Chips	Dolese @ Richard Spur, OK(1601)	15
5/8" Chips	Dolese @ Cyril, OK(0801)	85
Cellulose Fibers	Hi-Tech Asphalt Solutions @ Mechanicsville, VA	
Anti-Strip Add.(AD-here HP+)	ARR-Maz @ Winter Haven, FL	
Asphalt Cement (PG76-28OK)	Koch @ Muskogee, OK	

Laboratory No.	Aggregate	3/4" Chips	5/8" Chips	Combined Aggregate	Job Formula	JMF Tolerance
	Percent Passing					
	3/4"	100	100	100	100	± 0
	1/2"	37	90	82	82	± 7
	3/8"	15	55	49	49	± 7
	No. 4	5	7	7	7	± 7
	No. 8	3	3	3	3	± 5
	No. 16	2	2	2	2	± 4
	No. 30	2	2	2	2	± 4
	No. 50	1	2	2	2	± 4
	No. 100	1	2	2	2	± 3
	No. 200	1.1	1.3	1.3	1.3	± 2
	%AC (PG76-28OK)				6.0	± 0.4
	%Cellulose Fibers By Weight of Aggregate				0.3	
	%Anti-Strip Add. By Weight of Asphalt Cement				0.5	
	Mix Temperature @ discharge from Mixer, °F				325	± 20
	Optimum Roadway Compaction Temperature, °F				305	

**Tests on Asphalt Cement:**

Spec. Grav. @ 77 °F	Found	Required
	1.0241	

**Tests on Aggregates:**

	Found	Required
F.A.A. %U	N/A	N/A
Sand Equivalent	N/A	N/A
L.A. Abrasion	25.3	40 Max.
Durability (DC)	79.4	40 Min.
IOC	0.76	
Insoluble Residue	51.6	40 Min.
Fractured Faces	100/100	75/75 Min.
Gse	2.689	
Gsb	2.656	
Specimen Wt.	3850	

**Tests on Compressed Mixtures (at Design AC Content):**

	SGC	Dens. % of Gmm	Dens. % of Gmm Req'd
Nini			
Ndes	50	80.7	78-82
Nmax			

**Tests on Compressed Mixtures:**

Percent Asphalt	Gmb	Gmm	Dens. % of Gmm	Dens. % of Req'd of Gmm	V.M.A. (%)	V.M.A. (Min.%)	%VFA	%VFA Req'd	%DP	%DP Req'd
5.5	1.970	2.469	79.8		29.9		32.5		0.25	
6.0	1.978	2.450	80.7	78-82	30.0		35.8		0.23	
6.5	2.008	2.432	82.6		29.3		40.5		0.21	

Mix Layer Depth: ≤ 4"  
 Compacted Wt. 91.7 lbs./sq.yd./1" thickness  
 QA/QC Project: Tolerances shall be governed by PWL SP 411-9QA(a-y)99 Rev. 10/10/03.  
 Drain Down 0.02 (Required: 0.2 Max.)

**OKLAHOMA DEPARTMENT OF TRANSPORTATION  
SPECIAL PROVISIONS FOR  
PLANT MIX BITUMINOUS BASES AND SURFACES**

These Special Provisions revise, amend and where in conflict, supersede applicable sections of the 1999 Standard Specifications for Highway Construction, English and Metric, as applicable. Units of measurement are provided in the subsections in both English and Metric equivalents. The units applicable for this project will be those specified in the project plans.

**708.02. MINERAL AGGREGATE.** *(Add PFC column in Table 1 as follows:)*

**TABLE 1  
PHYSICAL PROPERTIES OF AGGREGATES**

<b>Test</b>	<b>PFC</b>
L.A Abrasion(a) , % wear, maximum	30
Sand Equivalent(b), % minimum	
less than 3 Million ESALs	40
3 Million ESALs to 30 Million ESALs	45
greater than 30 Million ESALs	50
Mechanically Fractured Faces(b,c,j), % minimum	100/95
Aggregate Durability Index(a), minimum	40
Insoluble Residue(d,e), % minimum	40
Micro-Deval (a), % wear, maximum	25
Flat or Elongated Pieces(b,c,f), % maximum	10
Natural Sand and Gravel(b), % maximum	0
Clay Balls and Friable Particles(g), % maximum	0
Soft Particles(a), % maximum	5
Sticks or Roots(a), % maximum	0

(a) Applies to each source except as noted.

(b) Applies to the combined aggregate except as noted.

- (c) Applies to the aggregate retained on the No.4 (4.75mm) sieve
- (d) Applies to the combined coarse aggregate.
- (e) Applies to the coarse aggregate used in the surface course. Does not apply to shoulders and temporary detours.
- (f) A flat piece is one in which the maximum width is greater than 5 times the maximum thickness. An elongated piece is one in which the maximum length is greater than 5 times the maximum width.
- (g) Applies to the combined aggregate. Provided the maximum for the combined aggregate is not exceeded, a maximum 1.5 percent will be allowed for any one source.
- (j) In the requirement format “xx/yy”, “xx” denotes the percentage of coarse aggregate requiring one fractured face and “yy” denotes the percentage of coarse aggregate requiring 2 fractured faces.
- (l) Regardless of the actual design life of the roadway, the design ESALs are based on 20 years.

**(a) Coarse Aggregate.** (Add the following:) The coarse aggregate for PFC shall be that part of the aggregate retained on the No.8 (2.36mm) sieve and shall consist of clean, tough, durable particles.

**(b) Fine Aggregate.** (Add the following:) No fine aggregate shall be used in the permeable friction course. Fine aggregate is defined as aggregate passing the No.8 (2.36mm) sieve and consisting of hard, durable grains of natural sand, crushed stone, stone dust, crushed gravel, mine chat or jig-sand or any combination of these materials.

#### 708.04. COMPOSITION OF MIXTURES. *(Change as follows:)*

- (a) Asphalt Mix Design and Initial Job-Mix Formula.** The PFC mix design and initial job-mix formula are the responsibility of the Contractor and shall be submitted to the Materials Division for review. The review of the proposed mix design will be to determine that the mix meets the design criteria.

The Contractor shall furnish one mix design for each specific PFC mixture listed on the plans or in the Contract. The mix design shall be prepared in an approved laboratory of the Contractor's choice. A request for laboratory approval may be made either by a Contractor or a Laboratory. Approval will be according to the Materials Division Policy for Asphalt Mix Design Laboratories. Mix designs will not be approved for use until the Contractor submits an acceptance letter stating acceptance status, project number and mix design designation, or signs and returns the mix design to the Materials Engineer.

The initial job-mix formula shall meet the requirements of tables 5E and 6E. The contractor shall prepare a trial mixture.

If the trial mixture, prepared at the initial job-mix formula proportions, fails to meet the requirements of Tables 5E and 6E, the Contractor may propose changes to the job-mix formula. If the changes result in a mixture meeting these Specifications, the job-mix formula will be adjusted accordingly.

The job-mix formula shall establish a single percentage of aggregate passing each required sieve, a single percentage of asphalt to be contained in the mixture, and a single temperature of the mixture at point of discharge from the plant.

The job-mix formula with the allowable tolerances shown in Table 7 shall establish the Specification limits for each mixture. The aggregate gradation shall not exceed the broad range given in table 5E.

- (c) **Recycled Bituminous Pavements.** (Delete and replace as follows:  
No reclaimed asphalt pavement shall be used in the permeable friction course.

**TABLE 5E**  
**PFC MIXTURE GRADATION AND BINDER CRITERIA**

( Percent Passing)

3/4" (19.0mm)	100
1/2" (12.5mm)	80 - 100
3/8" (9.5mm)	35 - 60
No. 4 (4.75mm)	1 - 20
No. 8 (2.36mm)	1 - 10
No. 200 (0.075mm)	1 - 4
Binder Content, % of mix weight	6.0 - 7.0
Cellulose Fiber Content, % of mix weight	0.2 - 0.5
Anti-Stripping Additive, % of binder weight	0.5

**TABLE 6E**  
**PROPERTIES OF LABORATORY MOLDED SPECIMENS**

Mixture Property	PFC Design Requirements	PFC Field Requirements
Drain-down	< 0.20 %	-
Lab-Molded Density (a)	80	78 - 82

(a) PFC specimens shall be molded on a Superpave gyratory compactor at N = 50 gyrations. Due to the inherently high air void content of PFC mixtures, the CoreLok™ apparatus according to OHD L-45 must be used to determine  $G_{mb}$  instead of the OHD L-14 method.

**708.05. TOLERANCES.** (*Add PFC column to Table 7:*)

**TABLE 7  
RANGE OF TOLERANCES FOR PFC MIXTURES**

Sieve Size	Percent Passing
No.4 (4.75mm) & larger	± 7
No.8 (2.36mm)	± 5
No.200 (0.075mm)	± 2
Asphalt Cement	± 0.4(a)
Temp. of mix as discharged from mixer, °F (°C)	± 20 (± 10)

(a) The tolerance shown for asphalt content is for individual samples. The average asphalt content by OHD L-26 shall be within ± 0.2 percent of the job-mix formula.

**708.07. SAMPLING AND TESTING FOR PFC.**

(a) **Methods.** Sampling and testing shall be done in accordance with AASHTO methods, except as noted below:

**1. Sampling and Testing Aggregates:**

Sampling	T 2
Sieve Analysis	T 27
Material Passing No.200 (0.075 mm) Sieve	T 11
Los Angeles Abrasion	T 96
Mud, Clay Balls, Sand Clusters, Sticks and Roots, Retained on No. 4 (4.75 mm) Sieve	OHD L-9
Fractured Faces	OHD L-18
Flat or Elongated Particles in Coarse Aggregate	ASTM D4791
Sand Equivalent (Clay Content)	T 176
Aggregate Durability Index	T 210
Insoluble Residue	OHD L-25
Soft Particles	OHD L-38

**2. Sampling and Testing Bituminous Mixtures:**

Mechanical Analysis of Extracted Aggregate	T 30
Sampling(a)	T 168
Bitumen Content	OHD L-26

Maximum Specific Gravity of Bituminous Paving Mixtures	T 209
Preparation and Density of Hot Mix Asphalt Specimens by SHRP Gyrotory Compactor(b)(c)	T 312
Determination of Draindown Characteristics In Uncompacted Asphalt Mixtures	T 305
Determining the Specific Gravity and Unit Weight of Compacted Bituminous Mixtures Using the CoreLok™ Apparatus	OHD L-45

- (a) The sample size of compacted bituminous pavement shall be in accordance with T 168.  
 (b) Lab molded specimens shall be compacted to N = 50 gyrations to determine the density, % of  $G_{mm}$ . All reported values shall be the average of 2 specimens.  
 (c) Design mixtures shall be mixed at 325°F (163°C), aged at 300°F (149°C) for a minimum of 2 hours and a maximum of 4 hours, and compacted at 300°F (149°C). Field samples shall be compacted at 300°F(149°C). No aging period is required for field samples, but they should be heated for a minimum of 2 hours to completely dry the sample and to ensure that the sample has stabilized at 300°F(149°C).

3. *Testing Asphalt Materials*

Performance Graded Asphalt Binder	MP 1
Accelerated Aging of Asphalt Binder Using a Pressure Aging Vessel	PP 1
Creep Stiffness by Bending Beam Rheometer	TP 1
Rheological Properties by Dynamic Shear Rheometer	TP 5
Grading or Verifying Asphalt Binder	PP 6
Solubility in Trichloroethylene	T 44
Flash and Fire Points by Cleveland Open Cup	T 48
Water	T 55
Rolling Thin Film Oven Test	T 240
Specific Gravity by Pycnometer	T 228

stormy; and when the wind or other conditions prevent proper leveling and consolidation.

Aggregate stockpiles must be reasonably dry so that drum mixing will drive out all remaining moisture.

- (g) **Spreading and Finishing.** Prior to placing PFC, clean all foreign matter from the surface of the existing roadbed. The temperature of the mixture for placement on the road shall be  $300 \pm 25^\circ\text{F}$ .

The PFC shall not be windrowed prior to spreading and finishing. A Materials Transfer Device or Materials Transfer Vehicle (MTD/MTV) shall be used for placement of the PFC. At the Engineer's discretion, isolated portions of a project may be exempted from use of the MTD/MTV.

The material shall be continuously remixed or rebled either internally in the transfer device, in a paver hopper insert, or in the paver's hopper. Remixing/reblending shall be accomplished by using remixing augers or paddles capable of continuously blending the PFC.

The MTD/MTV, haul units, and the paver shall work together to provide a continuous, uniform, segregation-free flow of material. The number of haul units, speed of the paver, plant production rate, and speed of the MTD/MTV shall be coordinated to avoid stop and go operations. The wings of the paver/receiving hopper shall not be raised (dumped) at any time during the paving operation.

If a MTD/MTV unit malfunctions during laydown operations, the Contractor may continue until any PFC in transit or stored in a silo has been laid and until such time as there is sufficient PFC placed to maintain traffic in a safe manner. Laydown operations shall cease afterward, until the equipment is operational.

Any MTD/MTV unit which exceeds 20,000 pounds per axle will be allowed to cross bridges in good condition, provided the unit's hopper is substantially empty, the vehicle travels at crawl speed, and the wheels on the vehicle travel as close as possible to the underlying beam lines. For bridges in poor condition or posted for load limits, the Engineer will consult Bridge Division to determine if any additional limitations are necessary, such as transporting the unit on a vehicle with more axles to distribute the load.

The mat shall be free from segregation, non-uniform texture, bleeding or fat spats, and cracking.

- (h) **Joints.** The location of the longitudinal joint shall be on the lane lines, and offset from the underlying joint a minimum of 3 inches. All construction joints shall be tight, smooth, butt-type joints.
- (i) **Compaction.** Immediately following placement of the PFC material, roll the surface with 2-3 passes of a static (non-vibratory) steel-wheeled, self-propelled roller of such weight as approved by the Engineer.

Finish the surface so that it is smooth and true to the dimensions shown on the Plans. Immediately correct any low or defective areas by removing them, replacing them with new material, and compacting them to conform to the remainder of the pavement. Such corrective work shall be done at the expense of the Contractor.

Trucks and all other traffic shall not be permitted on the finished PFC pavement until the surface temperature is within  $10^\circ\text{F}$  of ambient temperature or two hours time has elapsed from final rolling.

**406.05. METHOD OF MEASUREMENT.**

*Permeable Friction Course*, including aggregate, liquid asphalt, cellulose fiber, and other ingredients as specified in the job-mix formula - will be measured by the ton of combined mixture.

*Tack Coat* will be measured and paid for in accordance with Section 407.

**406.06. BASIS OF PAYMENT.**

Accepted quantities of Permeable Friction Course measured, as provided above, will be paid for at the contract unit price as follows:

PERMEABLE FRICTION COURSE.....TON

Such payment shall be full compensation for furnishing all materials, equipment, labor and incidentals to complete the work as specified.

***DRAFT***

MATERIAL	SOURCE	% USED
"D" Rock	Martin-Marietta Mat'l's @ Snyder, OK (3802)	40
Shot	The Doiese Co. @ Cooperton, OK (3801)	25
3/8" Chips	The Doiese Co. @ Cooperton, OK (3801)	35
Anti-Strip Add.(Kling Beta 2912)	Akzo-Nobel @ Waco, TX	
Asphalt Cement PG76-28OK	Koch @ Muskogee, Okla.	

Laboratory No.	"D" Rock	Shot	3/8" Chips	Combined Aggregate	Job Formula	JMF Tolerance
Aggregate Percent Passing						
1/2"	100		100	100	100	±0
3/8"	87	100	97	94	94	±7
No. 4	29	85	15	38	38	±7
No. 10	3	8	3	4	4	±4
No. 200	1.0	1.7	1.5	1.3	1.3	±2
% Asphalt Cement PG76-28OK					5.7	±0.3
Anti-Strip additive by weight of Asphalt Cement, %					0.5	
Mix Temperature @ discharge from Mixer, °F					290	±20
Optimum Roadway Compaction Temperature, °F						

**Tests on Asphalt Cement:**

	Found	Required
Abs. Visc. @ 140°F		
Kin. Visc. @ 275°F		
Spec. Grav. @ 77°F	1.0199	

**Tests on Aggregates:**

	Found	Required
Sand Equivalent	91	45 Min.
L.A. Abrasion % Wear	28.7	30 Max.
Durability (DC)	80	40 Min.
IOC	0.31	
Insoluble Residue (Cal)	0.0	30 Min.
Fractured Faces	100	75 w/2
ESG	2.710	
Hveem Wt.	1040	

**Tests on Compressed Mixtures:**

Percent Asphalt	Spec. Grav. Specimen	Max. Theo. Spec.Grav.	Dens. % of Max. Theo.	Dens. % Req'd. of Max. Theo.	V.M.A. (%)	V.M.A. (Min.%)	Hveem Stab.	Hveem Stab.(Min.)
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Retained Strength 95.6% 75% Minimum Required  
 Compacted Wt. 92.7 lbs./sq.yd./1" thickness

Tolerances for this project shall be governed by Special Provision 411-3QA(a-h)99.

<b>MIXTURE TYPE:</b>	<b>OPEN GRADED FRICTION SURFACE<sup>a</sup></b>
<b>Sieve Size</b>	
1 <sup>1</sup> / <sub>2</sub> inch(37.5 mm)	
1 inch(25.0 mm)	
3 <sup>3</sup> / <sub>4</sub> inch(19.0 mm)	
1 <sup>1</sup> / <sub>2</sub> inch(12.5 mm)	100
3 <sup>3</sup> / <sub>8</sub> inch(9.5 mm)	90-100
No. 4(4.75 mm)	25-45
No. 10(2.0 mm)	0-10
No. 40(425 μm)	
No. 80(180 μm)	
No. 200(75 μm)	0-5
Asphalt Cement <sup>d</sup> ,	
% of mix mass	_f
Viscosity Grade	
Asphalt Cement	_g