



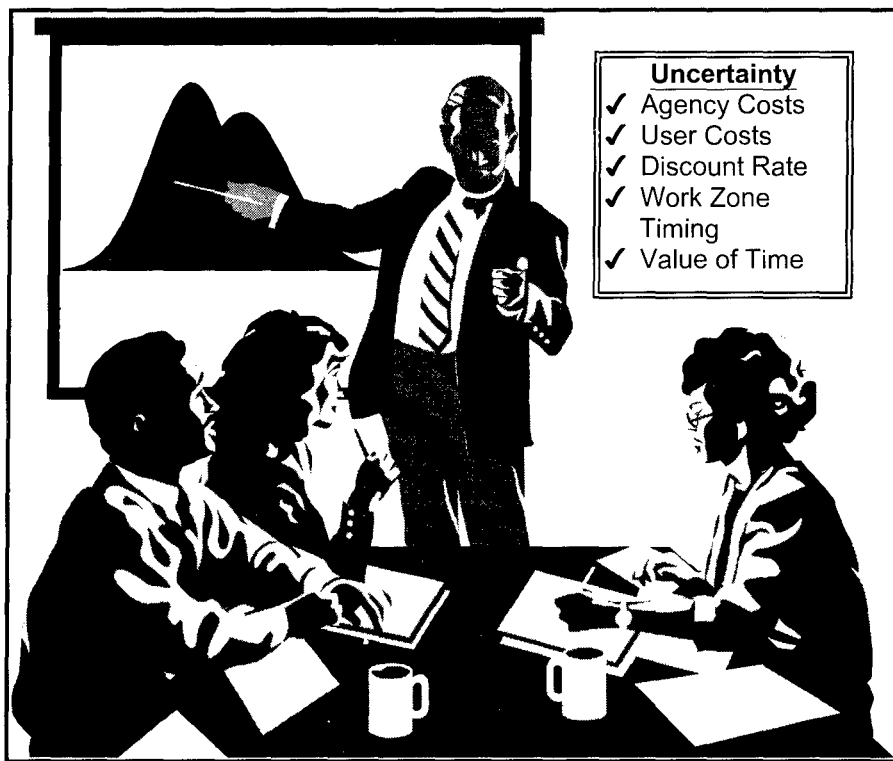
U. S. Department
of Transportation
**Federal Highway
Administration**

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August 1998



PB99-107419

Life-Cycle Cost Analysis in Pavement Design Participant's Notebook




Demonstration Project No. 115

Foreword

This participant's notebook was developed by FHWA staff to compliment a 2-day workshop on life cycle cost analysis in pavement design. This workshop will be of interest to State highway agency personnel responsible for conducting and/or reviewing pavement design LCCAs.

The FHWA Office of Engineering, Pavement Division, in cooperation with the Office of Technology Applications, offers LCCA technical support through Demonstration Project No. 115 *Probabilistic LCCA in Pavement Design* (DP-115). DP-115 is a free 2-day workshop that demonstrates best practices in performing life-cycle cost analyses for pavement design. This workshop is available, upon request, to State highway agencies.


Henry H. Rentz, Director
Office of Engineering

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**Life-Cycle Cost Analysis in Pavement Design
Participant's Notebook**

Demonstration Project No. 115



TABLE OF CONTENTS

Course Evaluation

Agenda

COURSE MODULES

Module I: Overview

Module II: Background

Module III: LCCA Process Overview

Module IV: Components and Issues

Module V: Introduction to Project Level User Costs

Module VI: Work Zone User Costs: Calculation Steps

Module VII: Basic Statistics

Module VIII: Risk Analysis Approach

Module IX: Software Demonstration

Module X: Class Exercise Revisited

Module XI: Presentation Techniques

Module XII: Benefits and Implementation

Module XIII: Summary

Class Exercises

Class Exercises - Solutions



Participant Evaluation Form

Course Title: Life Cycle Cost Analysis in Pav't Design **Dates:** _____

Instructor(s): _____ **Location:** _____

Please help us improve the training by evaluating the training course and workshop in which you participated. Your input is appreciated and needed. You may use the back of this form for additional comments.

<i>Please rate this workshop in the following areas:</i>	Excellent	Very Good	Good	Fair	Poor
▪ Adequate coverage of subject matter	5	4	3	2	1
▪ Applicable to your current job	5	4	3	2	1
▪ Meeting room location	5	4	3	2	1
▪ Course materials	5	4	3	2	1
▪ Knowledge gained from this workshop	5	4	3	2	1
▪ This workshop overall	5	4	3	2	1

Please rate the instructor(s) in the following areas:

▪ Organization	5	4	3	2	1
▪ Presentation	5	4	3	2	1
▪ Clarity of instruction	5	4	3	2	1
▪ Encouraging participation	5	4	3	2	1

Would you recommend this training to other department employees? _____ **Yes** _____ **No**

- Comments: (Please note exceptional points and/or clarify fair or poor ratings below)



Two Day Agenda

	Time	Title
Day 1	08:00 am	<i>Welcome</i>
	08:15 am	Workshop Overview
	08:30 am	Background
	09:00 am	LCCA Process Overview
	09:30 am	<i>Break</i>
	09:45 am	Components & Issues
	10:45 am	<i>Break</i>
	11:00 am	Class Exercise No. 1 or 2
	12:00 am	<i>Lunch</i>
	01:00 pm	Introduction to Work Zone User Costs
	01:30 pm	Work Zone User Costs: Calculation Steps
	02:45 pm	<i>Break</i>
	03:00 pm	Class Exercise No. 3
	04:00 pm	Class Exercise No. 4
	05:00 pm	<i>Close for Day</i>
	Day 2	08:00 am
09:00 am		Risk Analysis Approach
10:00 am		<i>Break</i>
10:15 am		Software Demonstration
12:00 pm		<i>Lunch</i>
01:00 pm		Class Exercise Revisited
02:00 pm		Presentation Techniques
02:30 pm		<i>Break</i>
02:45 pm		Benefits & Implementation
03:30 pm		Workshop Summary
04:00 pm		Question & Answers – Workshop Evaluations
05:00 pm		<i>Closeout</i>

Three Day Agenda

	Time	Title
Day 1	01:00 pm	<i>Welcome</i>
	01:15 pm	Workshop Overview
	01:30 pm	Background
	02:00 pm	LCCA Process Overview
	02:30 pm	<i>Break</i>
	02:45 pm	Components & Issues
	03:45 pm	<i>Break</i>
	04:00 pm	Class Exercise No. 1 or 2
	05:00 pm	<i>Close for Day</i>
<hr style="border-top: 1px dashed black;"/>		
Day 2	08:00 am	Introduction to Work Zone User Costs
	08:30 am	Work Zone User Costs: Calculation Steps
	09:45 am	<i>Break</i>
	10:00 am	Class Exercise – No. 3
	11:00 am	Class Exercise – No. 4
	12:00 pm	<i>Lunch</i>
	01:00 pm	Class Exercise - No. 4 Continued
	02:00 pm	Basic Statistics
	03:00 pm	<i>Break</i>
	03:15 pm	Risk Analysis Approach
	04:15 pm	Software Demonstration
05:00 pm	<i>Close for the Day</i>	
<hr style="border-top: 1px dashed black;"/>		
Day 3	08:00 am	Class Exercise Revisited
	09:00 am	Presentation Techniques
	09:30 am	<i>Break</i>
	09:45 am	Benefits & Implementation
	10:30 am	Workshop Summary
	11:00 am	Question & Answers – Workshop Evaluations
	12:00 pm	<i>Closeout</i>

1



2

Project Team

Max Grogg, 518-431-4224 x 223

Keith Herbold, 708-283-3548

Michael Smith, 202-366-4057

James Walls, 202-366-1339

3


A diagram titled "Expert Task Group" in a large, 3D, slanted font. To the left of the title is a vertical list of seven boxes, each containing the name of an organization and a person's name. To the right of the title is a line drawing of four people sitting around a table, engaged in a discussion or meeting.

Arizona DOT Larry Scofield
Penn DOT Gaylord Cumberledge (retired)
Ohio DOT Roger Green
Montana DOT Dick Clark
ACPA James Mack
NAPA Ken Hansen
University Washington Joe Mahoney

4

DP-115 Phases


- Traditional approach and Introduce probabilistic concepts
- SHA Case Studies
- Application of Probabilistic Approach



5

Phase I Objective


- Provide training and practice on traditional LCCA
- Introduce probabilistic concepts



6

Workshop Outline

- Background
- Process Overview
- Components and Issues
- User Costs
- Class Exercises
- Basic Statistics
- Probabilistic Approach
- LCCA Probabilistic Example
- Benefits and Implementation



7

Major Focus ...
<ul style="list-style-type: none">■ LCCA process overview■ Components and issues■ User cost procedure■ <u>Introduce</u> Probabilistic approach

8

End Session



1



2

Session Overview
<ul style="list-style-type: none">■ Definitions■ Levels of Application■ Driving Forces■ Implementing Guidance■ National Pavement Design Review

3

Definitions
<ul style="list-style-type: none">■ Life Cycle Cost Analysis■ Agency Cost■ User Cost■ Deterministic Approach■ Risk Analysis Approach

4

LCCA Defined (NHS)

“A process for evaluating the total economic worth of a useable project segment by analyzing initial costs and discounted future costs, such as maintenance, reconstruction, rehabilitation, restoring, and resurfacing costs, over the life of the project segment.”

5

Useable Project Segment

A portion of a highway that when completed could be opened to traffic independent of some larger overall project.

6

TEA 21 (98) LCCA Defined

Life-cycle cost analysis is a process for evaluating the total economic worth of a usable project segment by analyzing initial costs and discounted future costs, such as maintenance, user costs, reconstruction, rehabilitation, restoration, and resurfacing costs, over the life of the project segment.

7

Agency Costs
<ul style="list-style-type: none">■ Design and Engineering■ Initial Construction■ Maintenance of Traffic■ Maintenance■ Rehabilitation

8

User Costs
Costs incurred by users of a highway facility including excess costs to those who cannot use the facility because of agency or self-imposed detour requirements.

9

Deterministic Approach
The application of accepted LCCA procedures and techniques without regard for the variability of input factors.

10

Risk Analysis Approach

A technique which identifies the variability associated with LCCA input factors and carries this variability through the computation process to generate results in the form of a probability distribution.

11

Session Overview

- Definitions
- Levels of Application
- Driving Forces
- Implementing Guidance
- National Pavement Design Review

12

Levels of Application

- Funding levels
- Program allocation
- Project selection
- Design selection ←

13

Session Overview

- Definitions
- Levels of Applications
- Driving Forces
 - ISTEA (91)
 - FHWA-AASHTO Symposium (93)
 - Executive Order 12893 (94)
 - NHS Designation Act (95)
 - TEA 21 (98)
 - NQI Survey (95)

14

ISTEA (1991)

- Sections: 1024 & 1025

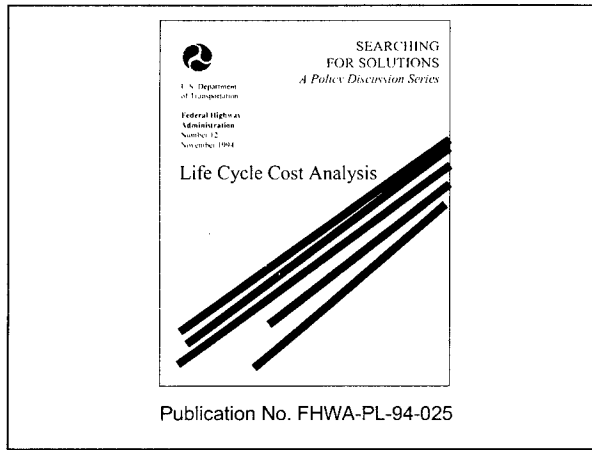
Factors to be *considered* ...
the use of life-cycle costs in the
design and engineering of bridges,
tunnels, or pavements.

15

LCCA Symposium (93)

- AASHTO Survey
- Focused attention on LCCA
- Spotlighted issues
- No Resolutions

16



17

Executive Order 12893 (94)

“Principles for Federal Infrastructure Investments”

- Directed at Federal Agencies
- Grant Programs
- Initiated FHWA Policy Statement

18

NHS Designation Act (95)

Section 303, “Quality Improvement,”

... requires States to conduct LCCA of each NHS high cost (\$25M or more) useable project segment.

19

TEA 21 (98)

- LCCA no longer mandated
- Adds Users Costs to LCCA def.
- Directs DOT to develop LCCA procedures based on principals contained in Exec. Order 12893
- Transportation Research Program addresses analysis period, discount rates, user costs, ...

20

National Quality Initiative

- National Policy on Quality of Highways (92)
- Survey conducted (11/95)

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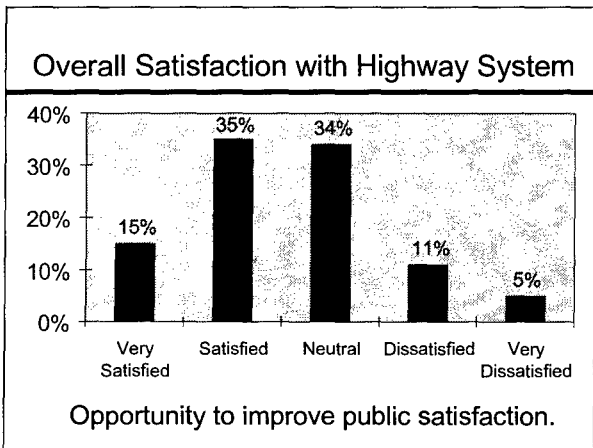
	NQI SURVEY
National Highway User Survey	1 Introduction
	2 Background of Survey
	4 Survey Design and Methodology
	6 Profile of Respondents
<small>National Quality Initiative</small> NQI <small>Steering Committee</small>	8 Major Findings
	17 Additional Findings
	20 Summary
	<small>Coopers & Lybrand L.L.P.</small>

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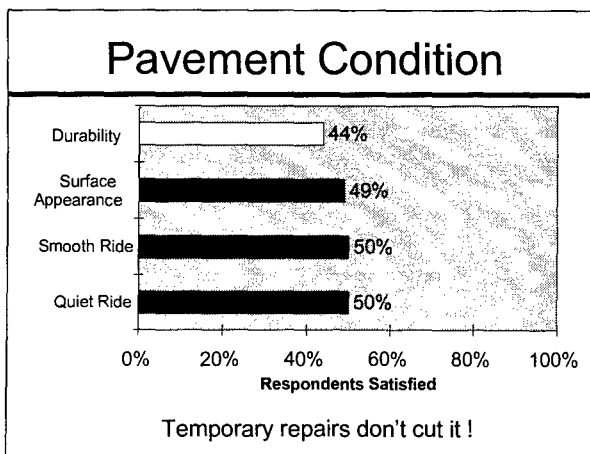
Survey Highlights

- Overall satisfaction
- Pavement condition
- Maintenance response time
- Traffic flow

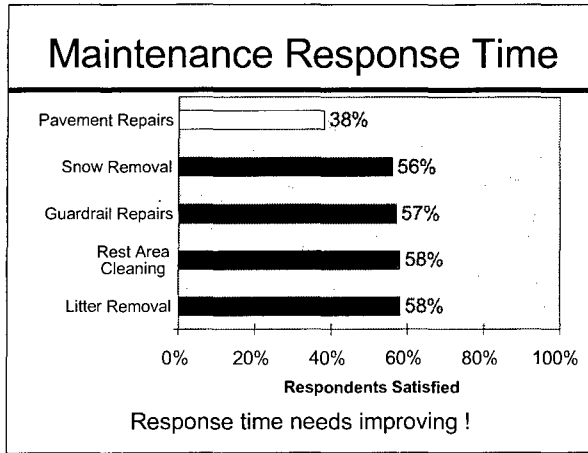
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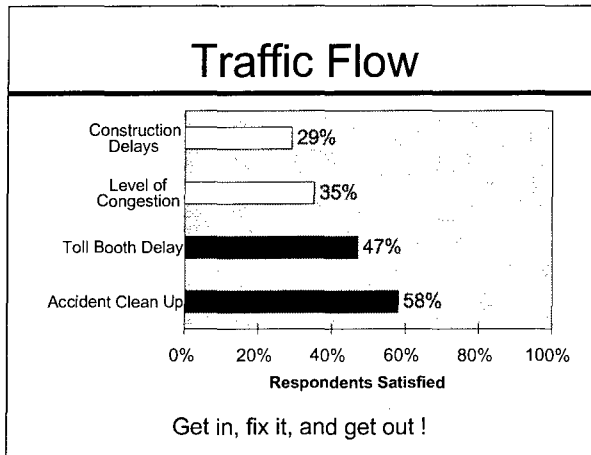
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27

- ### NQI Survey Priorities ...
- ❶ Pavement Condition
 - ❷ Safety
 - ❸ Traffic Flow

28

Session Overview

- Definitions
- Levels of Applications
- Driving Forces
- Implementing Guidance
 - NHS - FHWA memo (4/96)
 - LCCA Policy Statement (9/96)
 - Technical Bulletin (97)
 - DP 115

29

FHWA Memo (4/19/96)

- Federal-aid eligibility contingent on LCCA for \$25 Million + NHS projects
- Defines useable project segment
- LCCA procedures not prescribed
- Focus on "good" practice

30

LCCA Policy Statement (9/96)

- FHWA Philosophy ...
- Decision support tool
 - Results are not decisions
 - Use process to improve maintenance and rehabilitation strategies
 - Logical evaluation process is as important as results

31

LCCA Policy Statement (9/96)

- LCCA important consideration in all highway investment decisions
- Level of detail commensurate with level of investment
- Long analysis periods
 - Pavements - min. 35 years
 - Bridges - min. 75 years

(More)

32

Policy Statement Con't ...

- Agency and user costs should be included

- Future costs should be discounted to their *net present value (NPV)*

33

Technical Bulletin

- State of the practice
 - Traditional approach
- User costs (work zone)
 - VOC
 - Delay
- Introduce risk analysis (probabilistic approach)

34

DP 115

- Workshop
- Case studies
- Future activities

35

Additional Resources

- NCHRP
 - Synthesis reports
 - MicroBencost software
- AASHTO
 - Red Book
 - Pavement Design Guide
 - Darwin

36

Session Overview

- Definitions
- Levels of Application
- Driving Forces
- Implementing Guidance
- National Pavement Design Review

37

National Pavement Design Review

- Background
- Purpose
- General Findings (LCCA)

38

Background

- OIG/GAO reviews
- FHWA reviews 1995 - 1997
- 52 SHAs
- Areas addressed:
 - ✓ LCCA
 - ✓ Design procedures
 - ✓ Traffic

39

LCCA General Findings

- Procedures
- Analysis Periods
- Performance Periods
- Discount Rates
- User Costs

40

LCCA Procedures

- Number Reporting 52
- Documented Procedures 33
 - New Location 33
 - Reconstruction 33
 - Major Rehabilitation 22

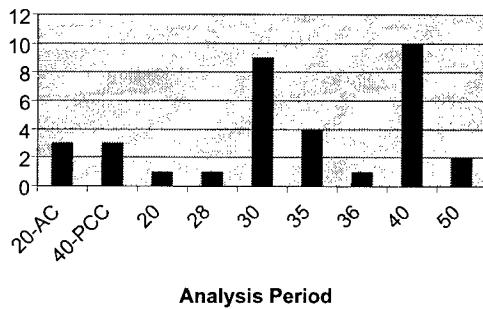
41

LCCA Procedures Cont.'d

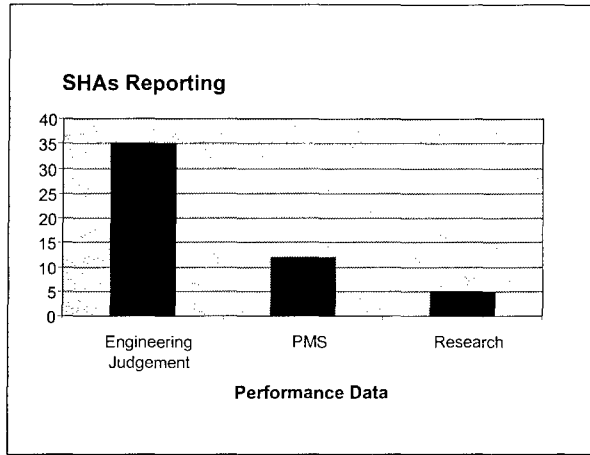
- Not Documented19
 - Informal Procedures11
 - Plan to Develop Procedure 7
 - Has no Plan to Develop Procedure 1

42

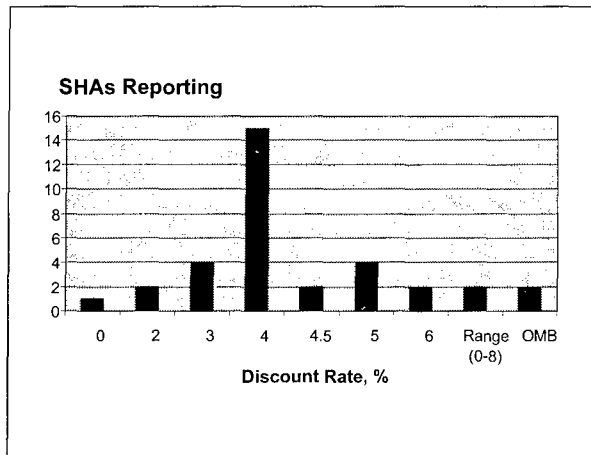
SHAs Reporting



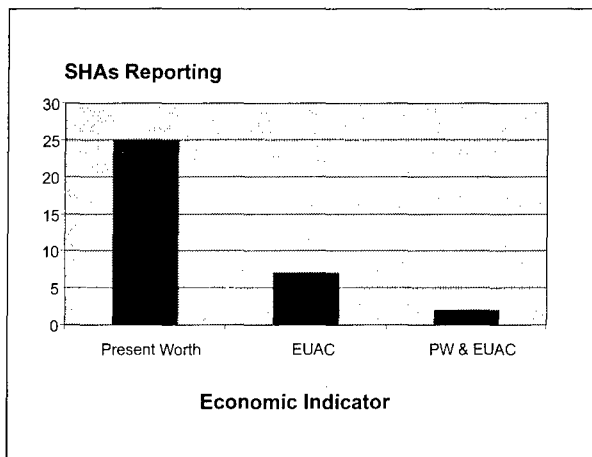
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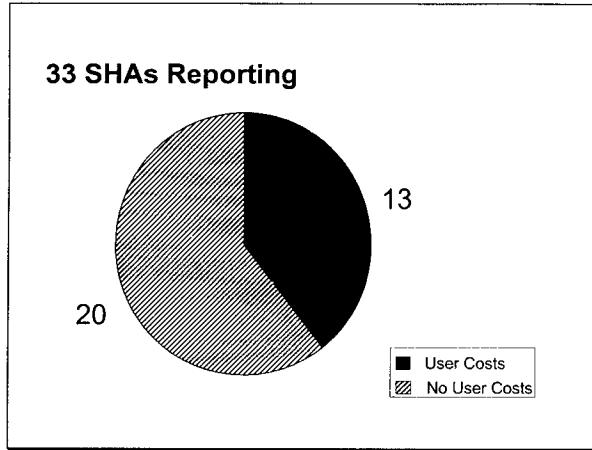
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Session Summary

- Definitions
- Driving Forces
- Implementing Guidance
- National Pavement Design Review

48

End Session

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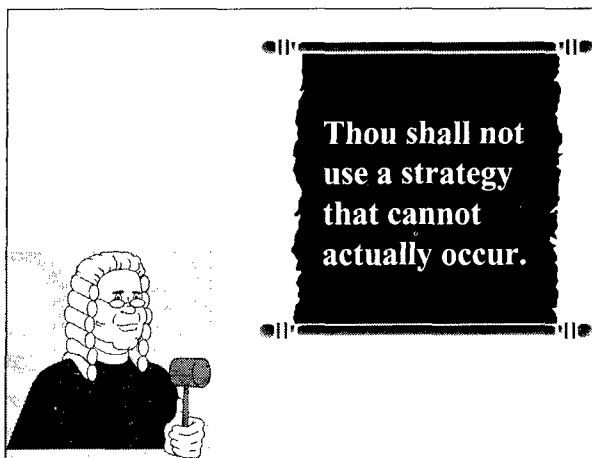
Process
Overview

2

LCCA Process Steps

- 1 Establish strategies for analysis period
- 2 Establish activity timing
- 3 Estimate agency costs
- 4 Estimate user costs
- 5 Develop expenditure streams
- 6 Compute NPV
- 7 Analyze results
- 8 Reevaluate strategies

3



Thou shall not use a strategy that cannot actually occur.

4

1. Establish Strategies for Analysis Period

- Initial pavement designs
- Subsequent maintenance and rehabilitations

5

Example: PCC M&R Activities

Yr.	Activity
5	Clean & Seal Jnts, Seal Coat Shoulders
10	Same as 5
15	Same as 5
20	Same plus Patch & Slab Stabilization Diamond Grind & Spall Repair
25	Same as 5
30	Patch, Overlay, Saw & Seal Joints, Pave Shoulders, Adjust Guardrail & Drainage
35	Seal Coat Shoulders

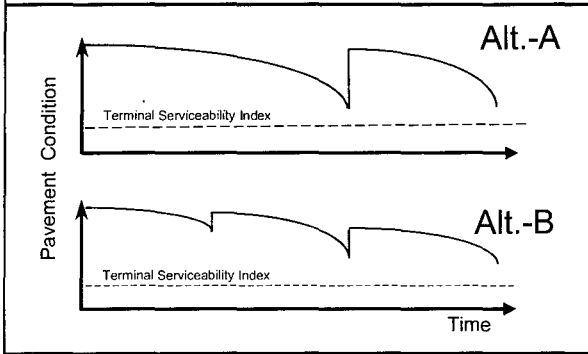
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LCCA Process Steps

- ① Establish strategies for analysis period
- ② Establish activity timing

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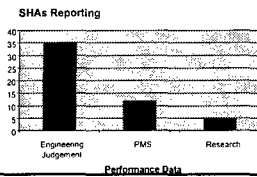
2. Establish Activity Timing



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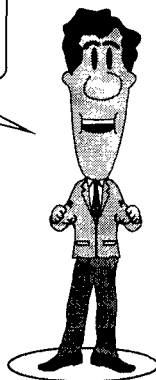
Data Sources ...

- Local Research Studies
- Agency Personnel - Experience
- Pavement Management Systems
- National Research Studies
- Other ...



9

Let's Consider
the following ...

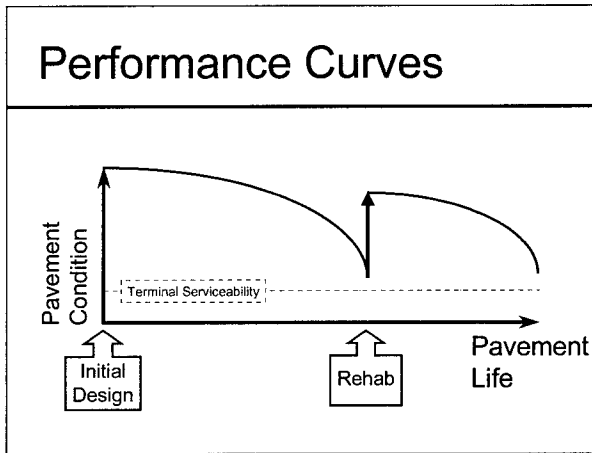


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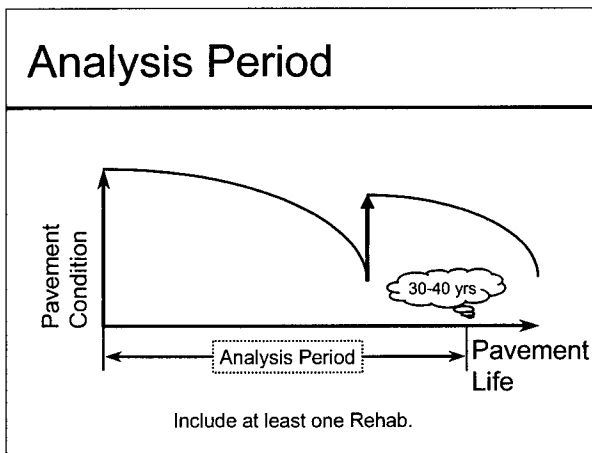
Example

- Initial Construction
- Single Future Construction

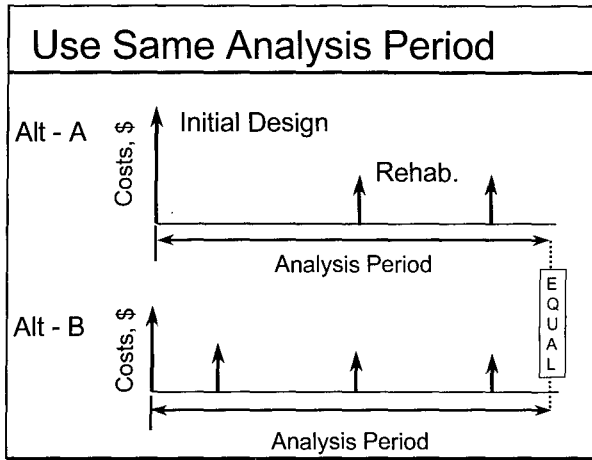
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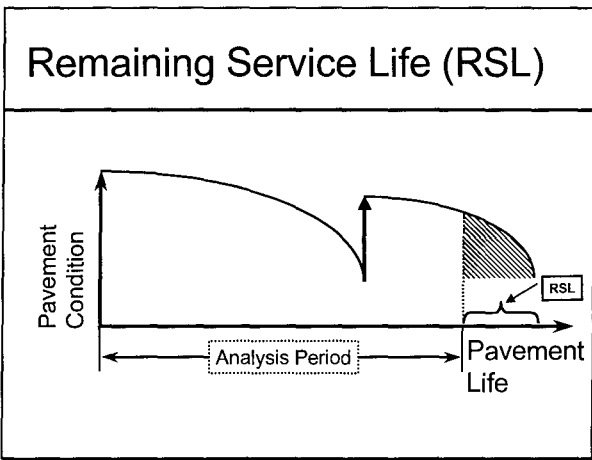
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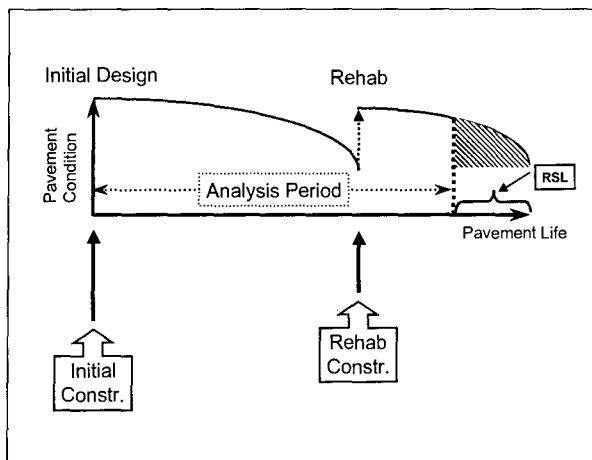
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16

LCCA Process Steps
<ul style="list-style-type: none">● Establish strategies for analysis period● Establish activity timing● Estimate agency costs

17

3. Estimate Agency Costs
Agency Costs Defined ... <ul style="list-style-type: none">■ Costs associated with roadway improvements■ Born by Agency

18

Agency Cost Include ...
<ul style="list-style-type: none">■ Initial Construction Cost■ Future Rehab and Preventive Maint.■ Project Overhead ...<ul style="list-style-type: none">● Preliminary Engineering,● Contract Administration,● Construction Supervision and Inspection■ Traffic Control

19

Data Sources ...

- SHA historical bid data
- Bid Analysis Management System (BAMS)

20

LCCA Process Steps

- ① Establish strategies for analysis period
- ② Establish activity timing
- ③ Estimate agency costs
- ④ Estimate user costs



21

4. Estimate User Costs

User Costs Defined ...

- Costs incurred by users of a highway facility including excess costs to those who do not use the facility because of agency or self-imposed detour requirements.

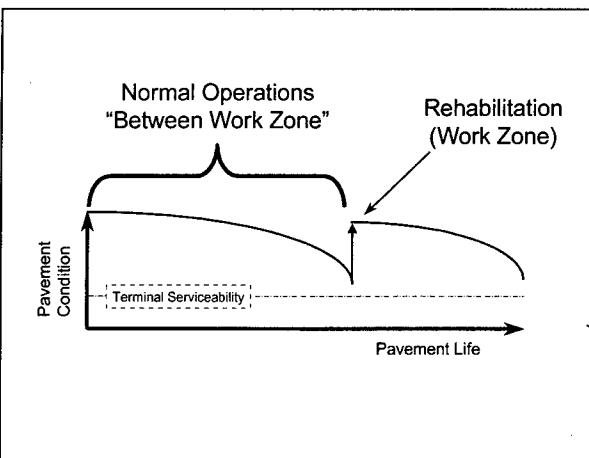
22

User Costs Include ...
<ul style="list-style-type: none">■ Vehicle operating■ User delay■ Crash

23

User Costs
Can occur during ... <ul style="list-style-type: none">■ Normal operations■ Work Zone

24



25


User Costs (Normal Operations)

- Difficult to quantify
- Data suggest may be minimal if roughness is "small"

26


Our Primary Focus ...

Vehicle Operating User Delay



Work Zone

Crash Costs



27

Work Zone User Cost

Defined ...

- Excess VOC and delay costs to users of the facility during work zone operations.
- Additional VOC and delay costs to users who do not use the facility because of agency or self imposed detours.

28

LCCA Process Steps

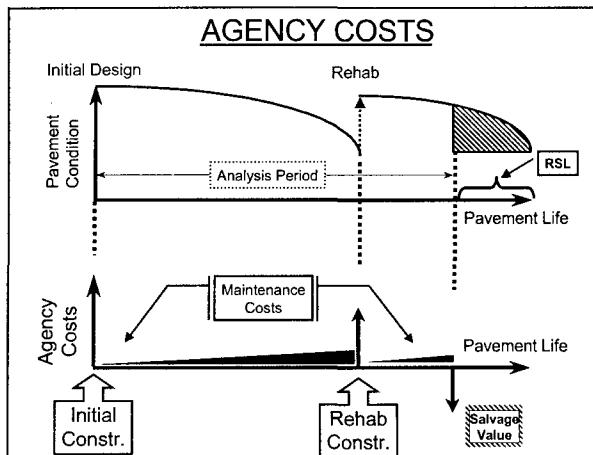
- ❶ Establish strategies for analysis period
- ❷ Establish activity timing
- ❸ Estimate agency costs
- ❹ Estimate user costs
- ❺ Develop expenditure streams

29

5. Develop Expenditure Streams

- Agency Costs
- User Costs

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31

Salvage Value

Salvage Value = $\left(\frac{RSL}{Life\ Rehab} \right)$ Rehab Cost

32

Routine Reactive Maintenance

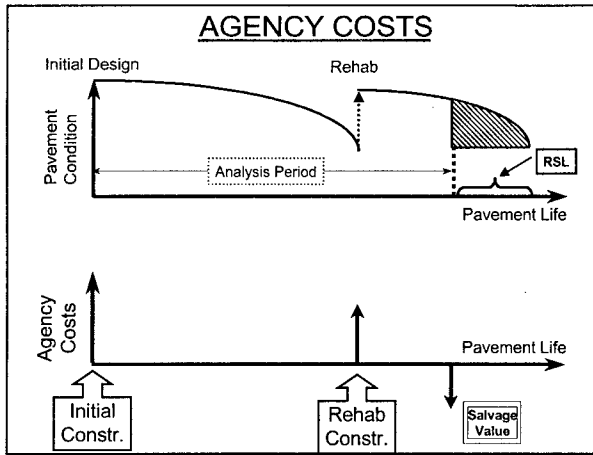
- Usually small
- When discounted its even smaller
- Can be ignored in the analysis

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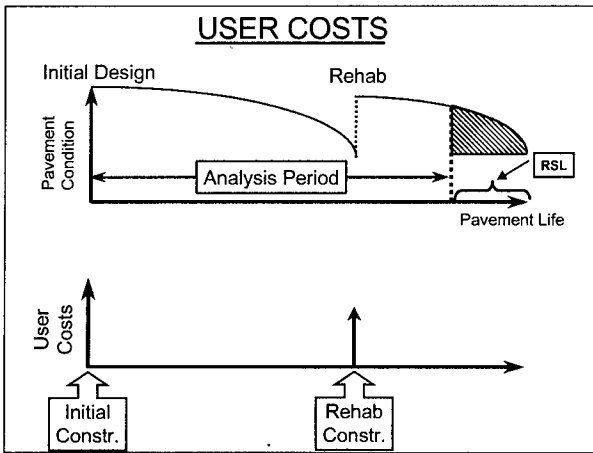
Inflation

- Use constant dollars
- Use real discount rate

34



35



36

LCCA Process Steps

- ❶ Establish strategies for analysis period
- ❷ Establish activity timing
- ❸ Estimate agency costs
- ❹ Estimate user costs
- ❺ Develop expenditure streams
- ❻ Compute NPV

37

Definition

Discounted present value of benefits less discounted present value of costs.

38

6. NPV Equation

NPV = Initial Cost +

$$\sum_{k=1}^N \text{Future Cost}_k \times \underbrace{\left[\frac{1}{(1+i)^{n_k}} \right]}_{\text{Present Value Factor}}$$

i = discount rate

n = year of expenditure

Present Value Factor

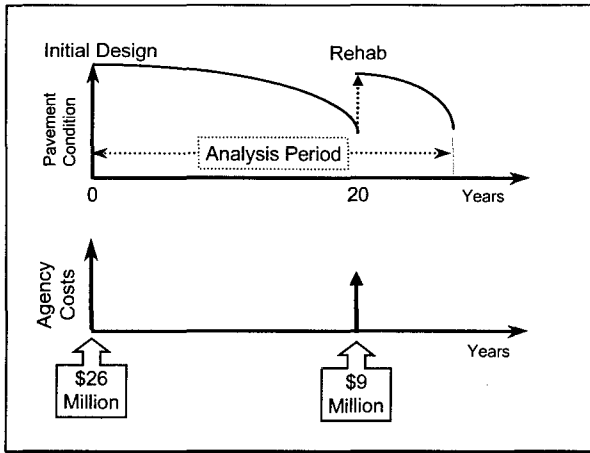
39

Present Value Factors

Year	Discount Rate (i)				
	4.0%	4.5%	5.0%	5.5%	6%
0	1.0000	1.0000	1.0000	1.0000	1.0000
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
⋮	⋮		⋮		⋮
⋮	⋮		⋮		⋮

NPV = (Future Cost) x (Present Value Factor)

40



41

Deterministic Approach

\$ 30.1 M \$ 26 M
 NPV = Initial Cost +
 Future Cost x $\left[\frac{1}{(1+i)^n} \right]$
 \$ 9 M

4% 20 yrs

42

❶ Agency NPV
 ❷ User NPV

Since User Costs May Dominate
 Separate Agency and User Costs

43

LCCA Process Steps

- ❶ Establish strategies for analysis period
- ❷ Establish activity timing
- ❸ Estimate agency costs
- ❹ Estimate user costs
- ❺ Develop expenditure streams
- ❻ Compute NPV
- ❼ Analyze results

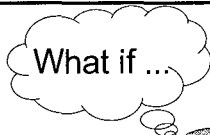
44

7. Analyze Results

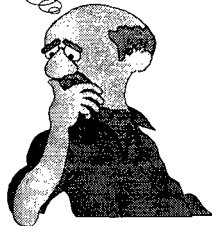
- Weigh qualitative advantages and disadvantages of alternatives
- Determine and explain LCCA implications

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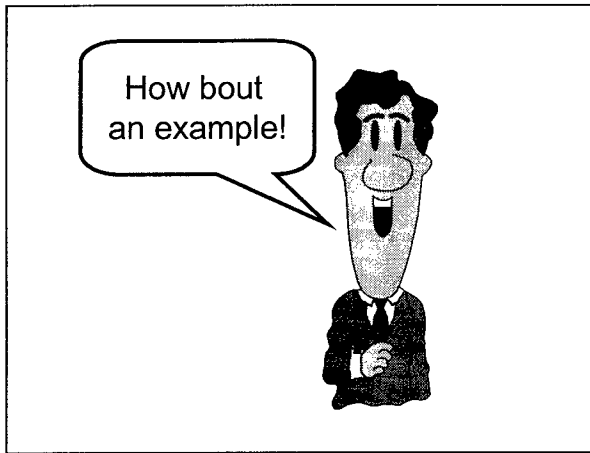
Sensitivity Analysis



- ✓ *Best Case*
- ✓ *Most Likely Case*
- ✓ *Worst Case*



46



47

Example Sensitivity Analysis

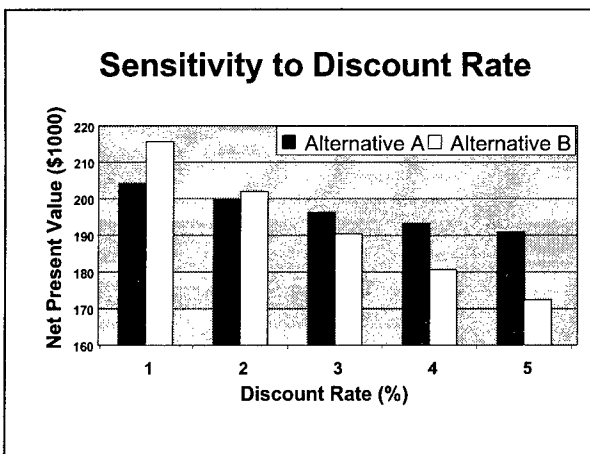
Alternative - A

Activity	Year	Cost	Discounted Cost				
			1%	2%	3%	4%	5%
Constr.	0	\$177.0	\$177.0	\$177.0	\$177.0	\$177.0	\$177.0
Rehab.	10	\$ 10.0	\$ 9.1	\$ 8.2	\$ 7.4	\$ 6.8	\$ 6.1
Rehab.	20	\$ 15.0	\$ 12.3	\$ 10.1	\$ 8.3	\$ 6.8	\$ 5.7
Rehab.	30	\$ 15.0	\$ 11.1	\$ 8.3	\$ 6.2	\$ 4.6	\$ 3.5
Salvage	35	\$(7.5)	\$(5.29)	\$(3.75)	\$(2.67)	\$(1.90)	\$(1.36)
NPV			\$204.2	\$199.8	\$196.3	\$193.3	\$190.9

Alternative - B

Activity	Year	Cost	Discounted Cost				
			1%	2%	3%	4%	5%
Constr.	0	\$125.0	\$125.0	\$125.0	\$125.0	\$125.0	\$125.0
Rehab.	15	\$ 80.0	\$ 68.9	\$ 59.4	\$ 51.3	\$ 44.4	\$ 38.5
Rehab.	30	\$ 80.0	\$ 59.4	\$ 44.2	\$ 33.0	\$ 24.7	\$ 18.5
Salvage	35	\$(53.3)	\$(37.6)	\$(26.7)	\$(18.9)	\$(13.5)	\$(9.7)
NPV			\$215.6	\$202.0	\$190.4	\$180.6	\$172.3

48



49

Sensitivity Analysis

■ Advantages

- Shows effect of changing input variable on outcome
- Easy to perform

50

Sensitivity Analysis

■ Disadvantages

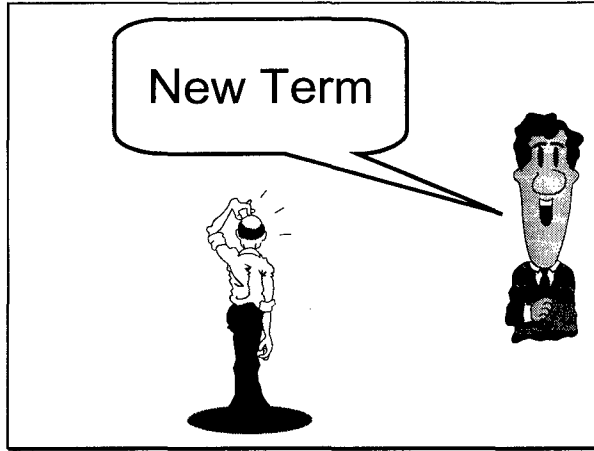
- One input variable changes (others held constant)
- Limited analysis
- Does not ...
 - ➔ account for simultaneous change of *ALL* inputs on outcome
 - ➔ account for likelihood of input value actually occurring
 - ➔ reflect reality

51

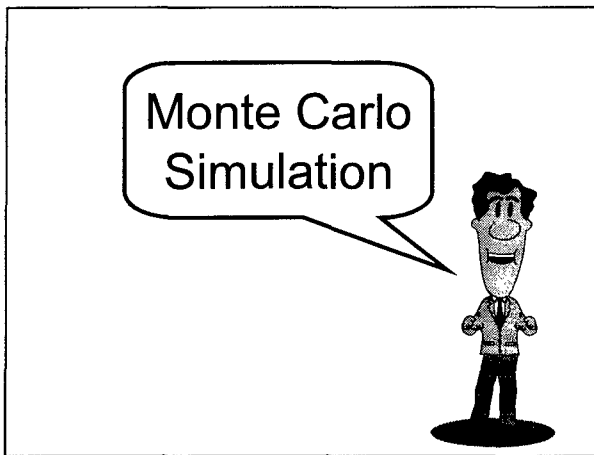
We need a technique that incorporates simultaneous changes of input variables into our results.



52



53



54

Monte Carlo Simulation

- Input variables described using probability distribution
- Samples randomly drawn from input distributions to calculate results
- Hundreds, even thousands, of samples may be drawn to form a distribution of results

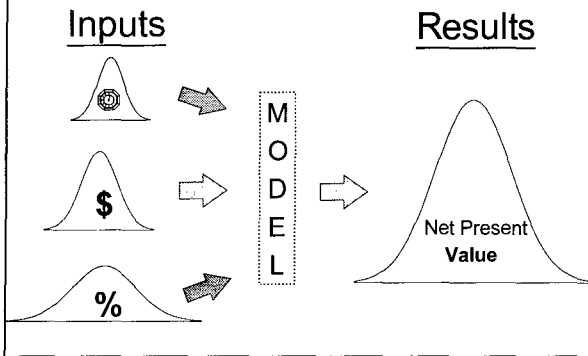
55

Monte Carlo Con't

- Rigorous extension of ...
 - Best Case
 - Most Likely
 - Worst Case
- Data Input values based on likelihood of occurrence

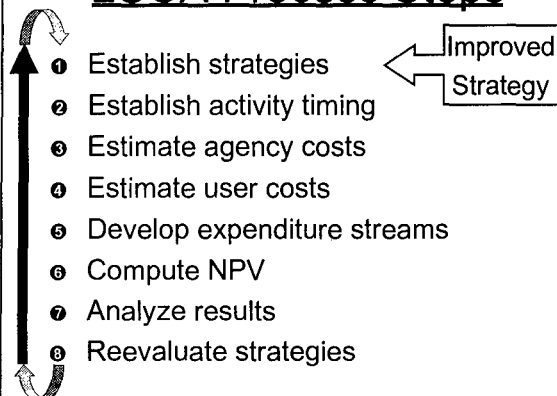
56

Monte Carlo Simulation



57


LCCA Process Steps



58

8. Re-evaluate Strategies
<ul style="list-style-type: none">● Modify alternatives<ul style="list-style-type: none">✓ Design lives✓ Strengthen shoulders✓ New technologies● Revise maintenance of traffic plan<ul style="list-style-type: none">✓ Reduce construction period✓ Restrict contractor work hours✓ Examine alternative modes of travel

59

Implications	
<ul style="list-style-type: none">■ Lengths and times of queues■ Agency versus user costs■ Reliability of LCCA outcome■ Practical Realities	

60

Practical Realities ...
<ul style="list-style-type: none">■ Local politics■ Availability of funding■ Industry support to perform the required construction■ Agency experience with a particular strategy■ Accuracy of pavement design and rehabilitation models

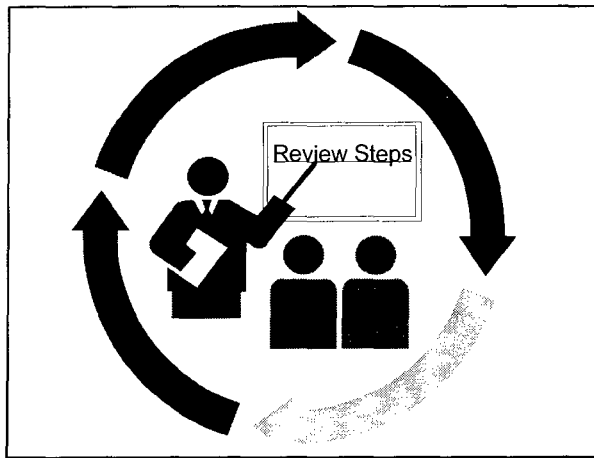
61

In Closing

LCCA ...

- Decision support tool
- Results are not decisions
- Use process to improve maintenance and rehabilitation strategies
- Logical evaluation process is as important as results

62



63

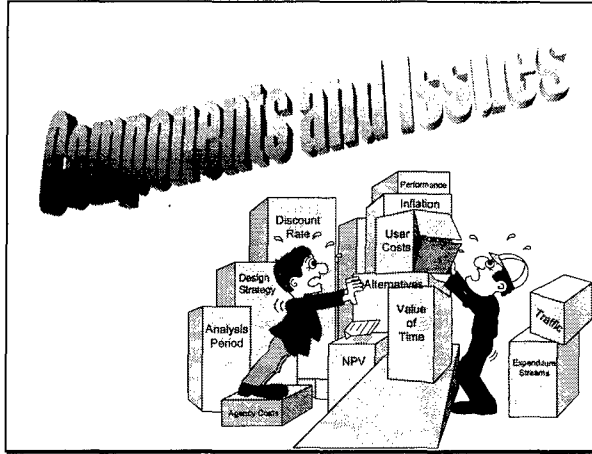
LCCA Process Steps

- ① Establish strategies for analysis period
- ② Establish activity timing
- ③ Estimate agency costs
- ④ Estimate user costs
- ⑤ Develop expenditure streams
- ⑥ Compute NPV
- ⑦ Analyze results
- ⑧ Reevaluate strategies

64

End Session

1



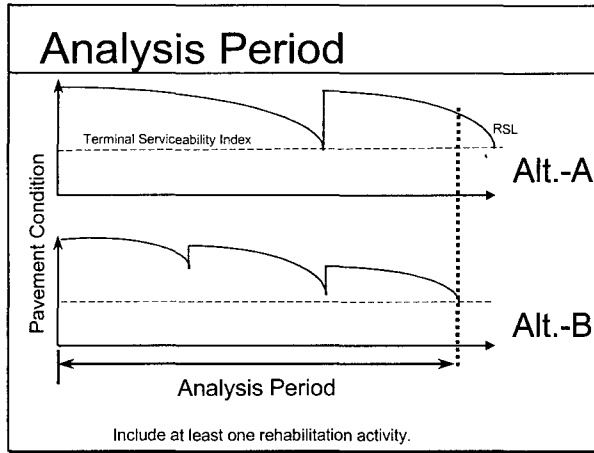
2

<h3>Session Overview</h3>
<ul style="list-style-type: none">■ Analysis periods■ Traffic■ Design strategy■ Performance estimates■ Expenditure streams

3

<h3>Session Overview Cont'd</h3>
<ul style="list-style-type: none">■ Costing■ Discounting■ Economic indicators■ Agency cost■ User cost

4



5

- Analysis Period**
- Capture alternative differences
 - Include one rehabilitation
 - FHWA LCCA Policy
 - Pavements 35 Yrs
 - Bridges..... 75 Yrs

6

- Traffic - Characteristics**
- AADT
 - Traffic mix
 - Growth rates
 - Directional hourly volume

7

Traffic - Projections

- Volumes - User costs
- Classification - User costs
- Load factors - Design
- ESALs - Design

8

Traffic - Hourly Demand

Sources:

- Traffic data
- Typical default values
 - PennDOT
 - MicroBencost

9

Example

PennDOT AADT Distribution - Hourly Percentages

Hour	Traffic Pattern Group					
	Interstate		Prin. Arterial		Min. Arterial	
	Urban	Rural	Urban	Rural	Urban	Rural
0 - 1	1.3	1.7	0.9	0.9	0.8	0.7
1 - 2	0.9	1.4	0.5	0.5	0.4	0.4
2 - 3	0.8	1.3	0.4	0.5	0.3	0.3
3 - 4	0.8	1.3	0.4	0.5	0.3	0.4
4 - 5	1.1	1.4	0.6	0.9	0.4	0.8
5 - 6	2.1	2.1	1.8	2.3	1.3	2.2
6 - 7	4.7	3.7	4.4	4.9	4.0	4.5
7 - 8	6.4	4.9	6.2	6.2	6.4	5.5
8 - 9	5.6	4.9	5.7	5.5	5.7	5.3
9 - 10	5.1	5.2	5.1	5.3	4.8	5.4
10 - 11	5.2	5.5	5.2	5.4	4.9	5.8
11 - 12	5.4	5.8	5.6	5.6	5.5	6.0
12 - 13	5.4	5.8	5.6	5.6	5.5	6.0
13 - 14	5.4	5.8	5.6	5.6	5.5	6.0
14 - 15	5.4	5.8	5.6	5.6	5.5	6.0
15 - 16	5.4	5.8	5.6	5.6	5.5	6.0
16 - 17	5.4	5.8	5.6	5.6	5.5	6.0
17 - 18	5.4	5.8	5.6	5.6	5.5	6.0
18 - 19	5.4	5.8	5.6	5.6	5.5	6.0
19 - 20	5.4	5.8	5.6	5.6	5.5	6.0
20 - 21	5.4	5.8	5.6	5.6	5.5	6.0
21 - 22	5.4	5.8	5.6	5.6	5.5	6.0
22 - 23	5.4	5.8	5.6	5.6	5.5	6.0
23 - 24	2.0	2.4	1.7	1.5	1.6	1.4

10

Performance Estimates

- Performance periods affect timing of rehabilitation
 - Frequency
 - Expenditure timing
 - Traffic levels
 - User costs

11

Design Strategy

- Initial design
- Identify supporting rehabs
- Viable and competitive

12

PCC Design Strategy

Activity	Year						
	5	10	15	20	25	30	35
■ Clean and Seal Joints	X	X	X	X	X	X	X
■ Seal Coat Shoulders	X	X	X	X	X		X
■ CPR - Patch				X		X	
- Spall Repair				X			
- Slab Stabilization				X			
- Diamond Grinding				X			
■ Overlay							X
■ Saw and Seal Joints							X
■ Pave Shoulders							X
■ Adjust Guard Rail and Drainage Structures							X

13

Example

- 6 Lane Facility (3 Lane per dir.)
- Work Zone 1 Lane Open
- 30 Year Analysis Period
- Initial AADT = 110,000 vpd
- 2 Rehabs including maint. plan

14

Options

- Plan to add capacity in the outyears
- Strengthen shoulders
- Examine use of alternative routes, modes of transportation

15

Options Cont'd

- Use materials with greater performance lives thereby reducing number of rehabs
- Initiate programs, such as preventive maintenance, that preserve pavement life

16

Costing and Discounting

- Costing - Type of Dollars
 - Constant (real)
 - Inflated (nominal)

17

Can You Assume Inflation?

1947 1997

Top-selling car

\$8,890 (Chevrolet) \$18,545 (Ford Taurus)

Average time to earn that much

4.8 months 5.3 months

Gallon of gas

\$1.70 \$1.22

1997 \$Dollars\$

18

Example: Deflation

Computer Cost:

- 1989 - \$2,500
- 1998 - \$1,200

19

Costing and Discounting

- Costing - Type of Dollars
 - Constant (real)
 - Inflated (nominal)
- Discounting - Type of Rates
 - Real
 - Nominal

20

Discounting - Rate Factors

- 4.0% - Real
- 3.5% - Inflation
- 4.0% - Risk premium
-
- 11.5% - Nominal

21

Discounting - Matching Dollars & Rates

- Real dollars and rates
- Nominal dollars and rates
- Never mix nominal and real

22

Discounting "True-isms"

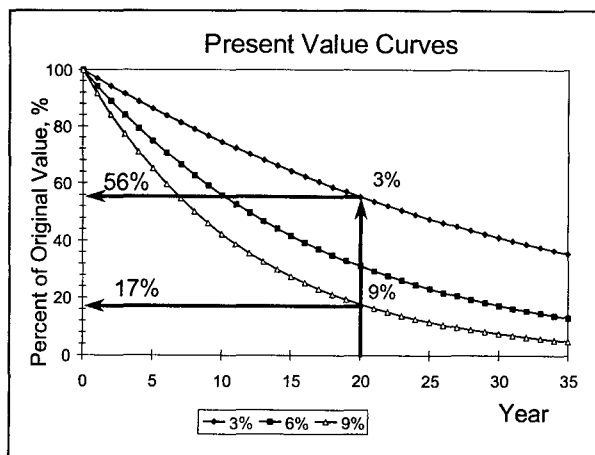
- Present costs valued higher
- Out year costs worth less

23

Discounting "True-isms"

- Low Rates -
Favor alternatives with higher initial costs and lower future costs
- High Rates -
Favor alternatives with lower initial costs and higher future costs

24




25

Discount Rate Selection

- Opportunity Cost
- Office of Management and Budget Circular A-94

26

Opportunity Cost - Personal 


■ 401K - IRA	28% - 35%+
■ Credit Cards	15% - 22%
■ Signature Loans	12% - 15%
■ Car Loans	8% - 12%
■ Home Equity	7% - 11%
■ Mortgage	5% - 8%
■ Savings/Checking	0% - 3%

Nominal rates of return

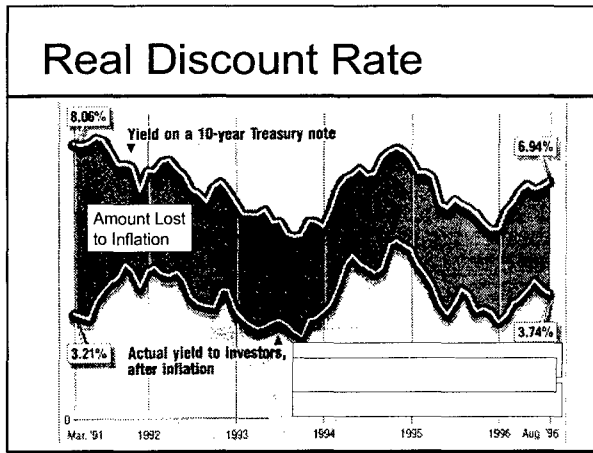
27

Opportunity Cost - Gov't

- J. "Queue" Public
- Other Investments
- Old Bonds
- New Bonds



28



29

Real Discount Rates

Source: OMB Circular A-94

YEAR	Investment Maturity					
	3	5	7	10	30	
Nov 92	2.7	3.1	3.3	3.6	3.8	
Feb 93	3.1	3.6	4.0	4.3	4.5	
Feb 94	2.1	2.3	2.5	2.7	2.8	
Feb 95	4.2	4.5	4.6	4.8	4.9	
Feb 96	2.7	2.7	2.8	2.8	3.0	
Feb 97	3.2	3.3	3.4	3.5	3.6	
Jan 98	3.4	3.5	3.5	3.6	3.8	
Avg	3.1	3.3	3.4	3.6	3.8	(No Inflation Premium)
Std	0.6	0.7	0.7	0.7	0.7	

30

Circular A-94

Discount Rates Web Address:

<http://www.whitehouse.gov/WH/EOP/OMB/html/circulars/>

Select A-94 and see appendix C.

31

Recommend
<ul style="list-style-type: none">➔ 3 to 5 %➔ Real rates with real dollars

32

Economic Indicators
<ul style="list-style-type: none">■ Internal rate of return■ Benefit cost ratio■ Equivalent uniform annual costs■ Net present value

33

Economic Indicator of Choice
<u>Net</u> <u>Present</u> <u>Value</u>

34

Agency Costs

- Design and Engineering
 - Initial Construction
 - Maintenance of Traffic
 - Preventive Maintenance
 - Rehabilitation
 - Reactive Maintenance/Operating Cost
- Salvage Value Sunk Costs

35

Salvage Value

- Remaining service life
 - % of last rehab cost
- Residual value
 - Value of recycled material

36

Sunk Costs

... costs that are not relevant to the decision at hand

37

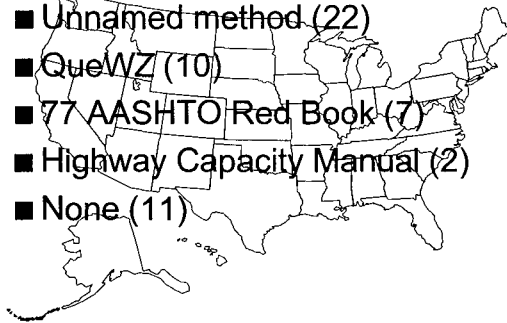
User Costs: 2B or not 2B

- Pro - User costs drive transportation investments.
 - User fees collected for public investment
- Con - Can't recoup costs
 - "Not in my budget"

38

AASHTO User Cost Survey for Innovative Contracting

- Unnamed method (22)
- QueWZ (10)
- 77 AASHTO Red Book (7)
- Highway Capacity Manual (2)
- None (11)



39

User Cost Components

- Vehicle crash cost
- Vehicle operating cost
- User delay

40

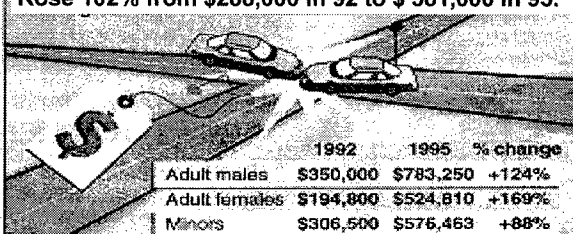
User Cost Components

- Vehicle crash cost
 - Property damage only - \$
 - Injury - \$\$
 - Fatality - \$\$\$\$\$
- Data sources

41

USA SNAPSHOTS®
 A look at statistics that shape the nation

Higher Traffic Fatality Awards
 Median Jury Awards for all Traffic Crash Fatalities
 Rose 102% from \$288,000 in 92 to \$ 581,000 in 95.



	1992	1995	% change
Adult males	\$350,000	\$783,250	+124%
Adult females	\$194,800	\$524,810	+169%
Minors	\$306,500	\$576,463	+88%

Source: Jury Verdict Research
 By Anne R. Carey and Genevieve Lynn, USA TODAY

42

User Cost Components

- Vehicle crash cost
- Vehicle operating cost
 - Normal operations
 - Work zone

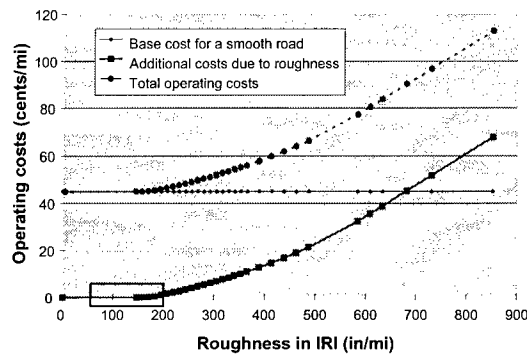
43

VOC Normal Operations

- Function of ...
 - Pavement performance
 - VOC - IRI relationship
- May be significant but ...
 - ..Not quantifiable at this time

44

Effect of Roughness on Road User Costs in New Zealand



45

VOC in Work Zones

- Speed change cost
- Stopping cost
- Idling cost

46

User Cost Components

- Vehicle crash cost
- Vehicle operating cost
- User delay
 - WZ reduced speed delay
 - Congestion delay

47

USA SNAPSHOTS®
A look at statistics that shape the nation

Bumper-to-bumper gridlock
Commuters in 1/3 of Metro Areas Spend more than 40 Hours / Year Stuck in Traffic Jams

Metro Area	Hours / Year Stuck in Traffic Jams
San Bernardino-Riverside	76
Washington, D.C.	70
San Francisco-Oakland	66
Los Angeles	65
Houston	60

Source: TTI
Tombrow, Least congested metro areas.

48

USA SNAPSHOTS®
A look at statistics that shape the nation

Driving the Open Road
Least Congested Cities. Annual Hours Stuck in Traffic Jams

City	Annual Hours Stuck in Traffic Jams
Corpus Christi	7
El Paso	11
Indianapolis	12
Salt Lake City (2e)	13
Memphis	13

Source: TTI

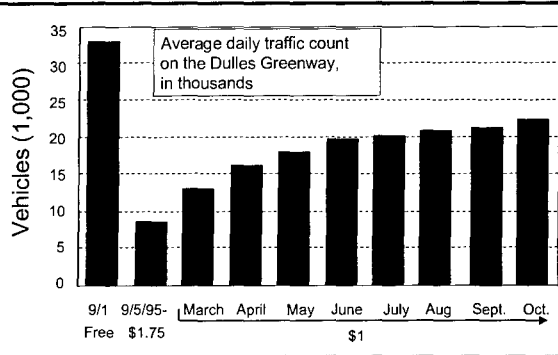
49

Value of Time: Sources

- A + B Bidding - Lane rentals
- Toll facilities
- Congestion pricing
- High Occupancy Toll (HOT)
- US DOT - FHWA HERS
- Research studies

50

Toll Facilities



51

Congestion Pricing

- Houston 1990
- Hardy toll road experiment
- \$1.00 peak - \$.50 off peak
- Too successful
- Lost \$500,000 in 90 days

52

HOT Lanes

- San Diego I-15
 - \$10 Million experiment 12/96
 - Started at \$50/mo., now at \$70
 - 700 permits with 400 waiting
- Orange Co - Riverside Freeway
 - \$2.25 saves 20 minutes

53

HOT Lanes Cont'd

- Houston Katy Freeway
 - 2 for 3 @ \$2.00

54

Value of Time (USDOT 1997)

Travel Category	\$ / Person Hour			
	Local		Inter City	
	Low	High	Low	High
Business	15.00	22.60	15.00	22.60
Personal	6.00	10.20	10.20	15.30
Mixed	6.40	10.70	10.40	15.70
Trucks	16.50	16.50	16.50	16.50

1995 Dollars

55

Value of Time (HERS 1997)

Source: Highways Economic Requirement System 9/97

Travel Category	\$ / Veh. Hour		
	Autos	Trucks	
		Single Unit	Combination
Business	28	26	31
Personal	13	NA	NA
% AADT			
Personal	90%	0%	0%
Wt. Avg.	14	26	31

1995 Dollars

56

Value of Time (Research Studies)

1996 Dollars

Vehicle Class	\$ / Vehicle Hour Value
Passenger Vehicles	11.58
Single Unit Trucks	18.54
Combination Trucks	22.31

Average of ...
AASHTO Red Book and MicroBENCOST Default Values

57

Value of Time Recommendations

Vehicle Class	\$ / Vehicle Hour Range
Passenger Vehicles	10.00 - 13.00
Single Unit Trucks	17.00 - 20.00
Combination Trucks	21.00 - 24.00

1996 Dollars

58

Further Information ...

LCCA Tech Bulletin ...

- Fundamental principles
- Good practice
- LCCA issues
- Case studies
- Uncertainty and variability
- Computer software

59

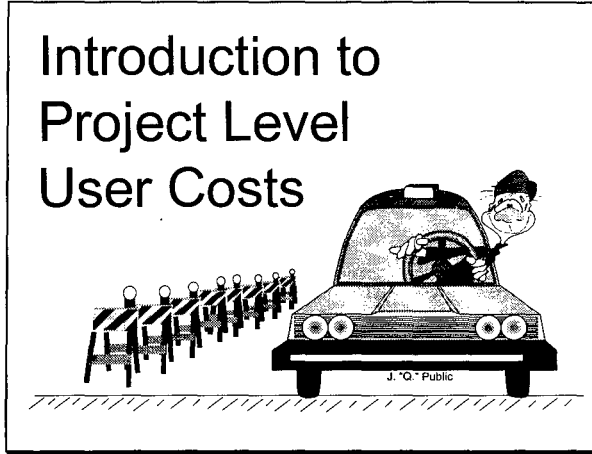
Parting Caveat

- Defending LCCA results
 - Justify all assumptions
 - Address all issues
(even if not relevant to the analysis at hand)

60

End Session

1



2

Session Overview

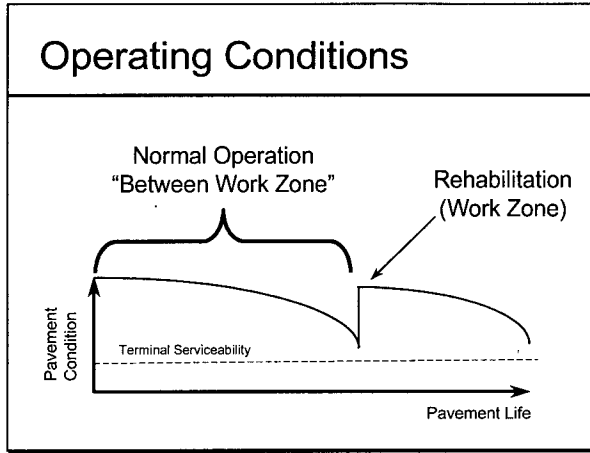
- Components
- Operating Conditions
- Work zone

3

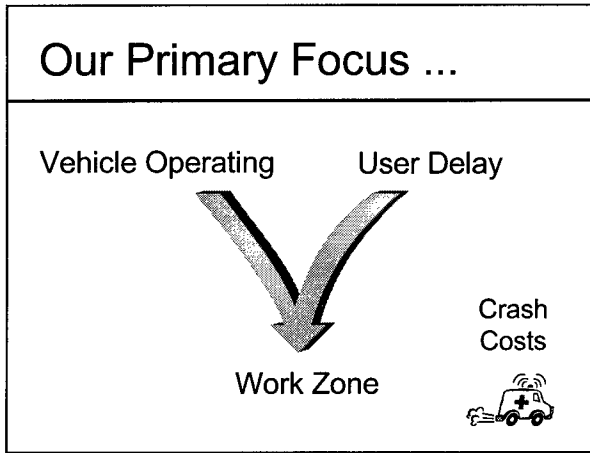
User Cost Components

- Vehicle operating
- User delay
- Circuity
- Crash

4



5



6

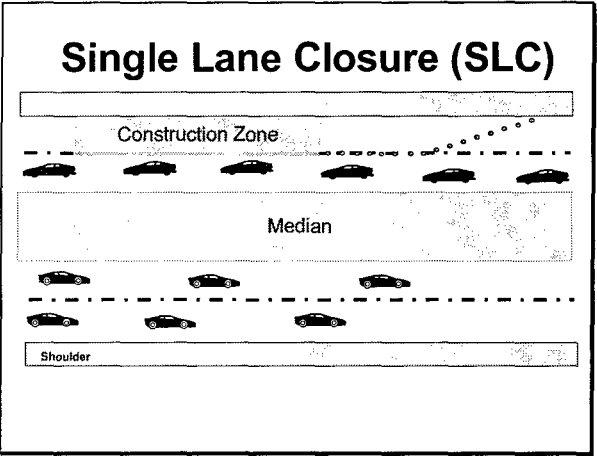
- ### WZ User Costs Function of ...
- Type
 - Characteristics
 - Duration
 - Frequency
 - Timing
 - Traffic Operations

7

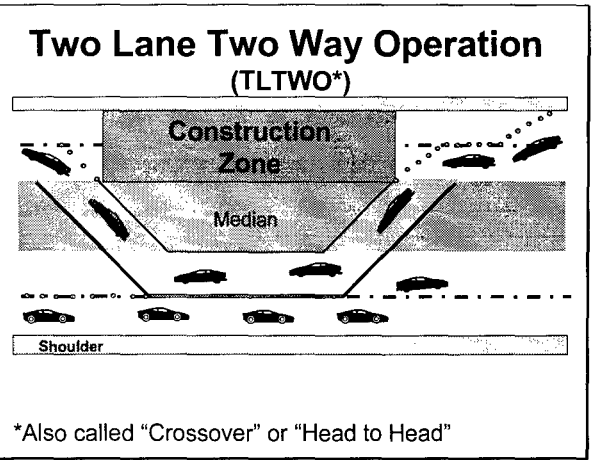
Work Zone Types

- Road closures
- Single lane closure
- Two lane two way operation

8



9



10

WZ Characteristics
<ul style="list-style-type: none">■ Length■ Posted Speed■ Hours of Operation■ Capacity■ Alternate Routes/Detours

11

Work Zone Duration
Includes: <ul style="list-style-type: none">■ Hours per day■ Number of days

12

Work Zone Frequency
<ul style="list-style-type: none">■ Number of times rehab work zones need to be established over the analysis period■ The more rehabilitations the more work zones

13

Work Zone Timing

- Refers to the year the work zone is in place
- Impacts user cost NPV
 - Out-year traffic levels
 - Discount factor

14

Net Present Value (NPV)

NPV = Initial Cost +

$$\sum_{k=1}^N \text{Future Cost}_k \times \underbrace{\left[\frac{1}{(1+I)^{n_k}} \right]}_{\text{Present Value Factor}}$$

I = discount rate

n = year of expenditure

Present Value Factor

15

Work Zone Analysis

- Different work zone types must be analyzed separately.
- Work zones with different characteristics, including traffic demand, must also be analyzed separately.

16

Work Zone Analysis Con't

- Manual approach
 - Capacity analysis
 - AASHTO Red Book
- Automated programs
 - MicroBenCost
 - QueWZ

17

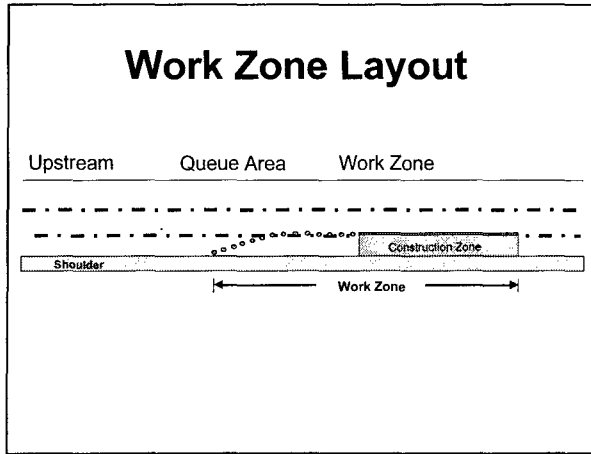
McTrans Ph: 1-800-226-1013

- MicroBenCost: ~ \$110
 - QueWZ: ~ \$20
- Includes: software, documentation,
and shipping

18

Work Zone Traffic Operations

19



20

WZ Operations

- Free Flow (Base Case)
WZ Capacity Exceeds Demand
- Forced Flow (Congestion)
Demand Exceeds WZ Capacity

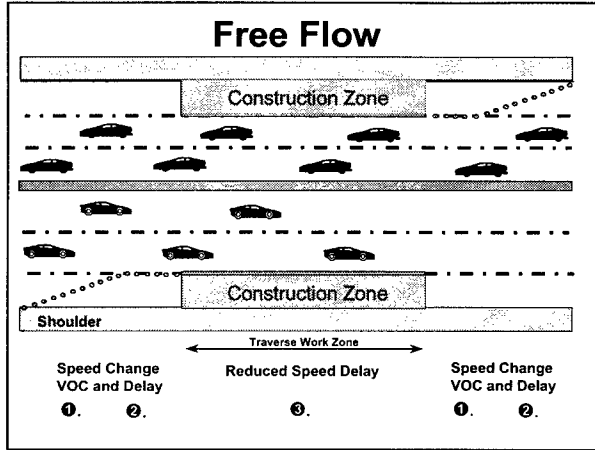
21

Free Flow Cost Components

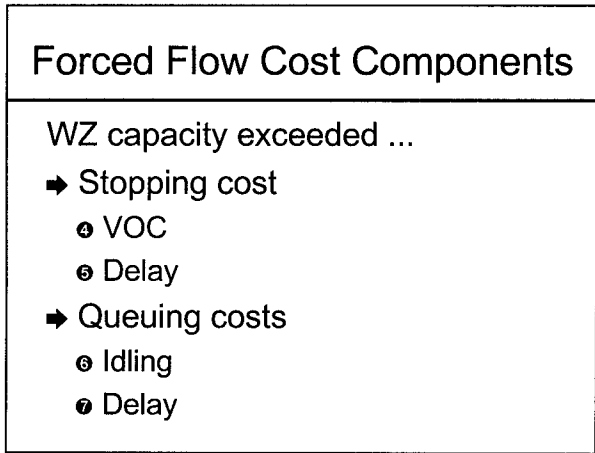
WZ capacity not exceeded ...

- ➔ Speed change costs
 - VOC
 - Delay
- ➔ Reduced speed costs
 - Delay

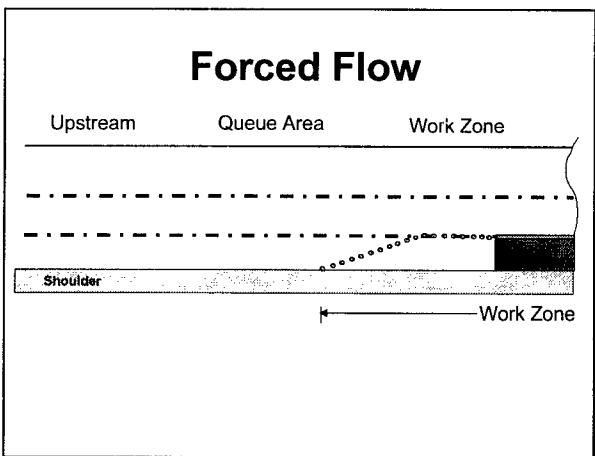
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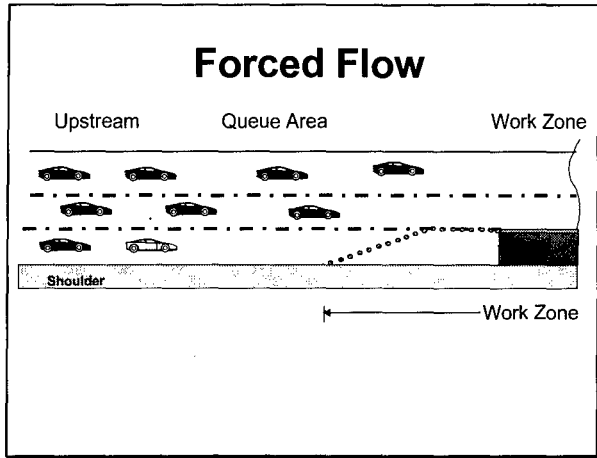
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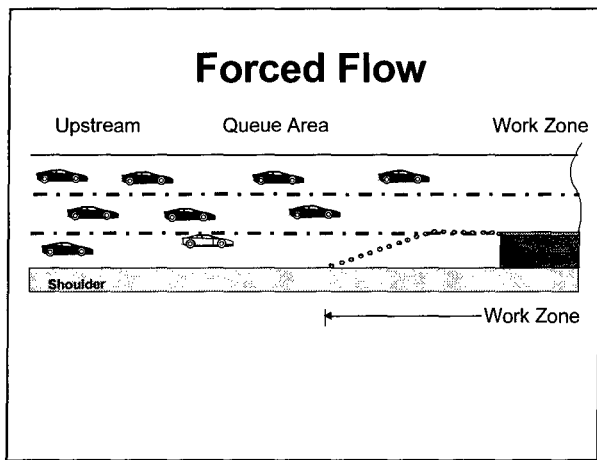
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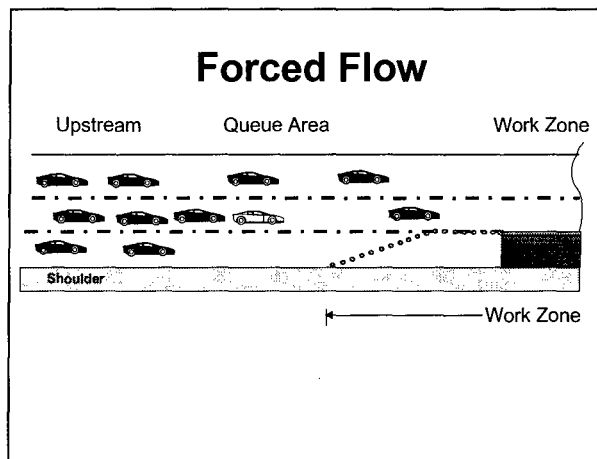
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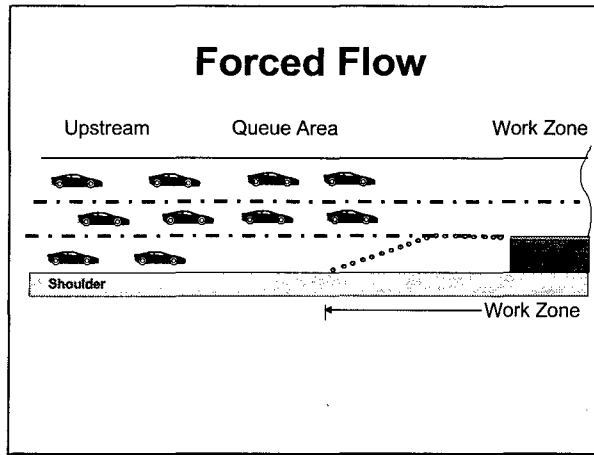
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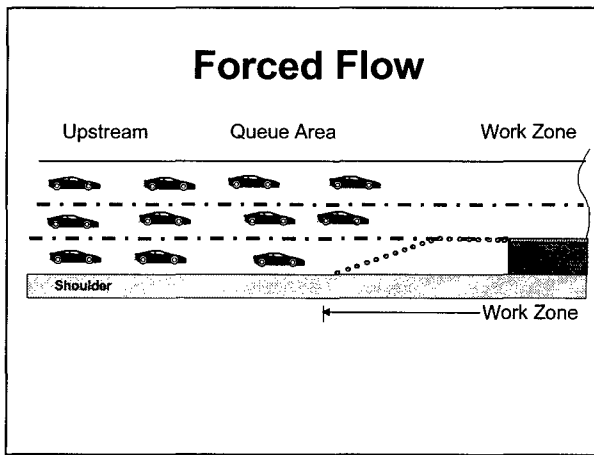
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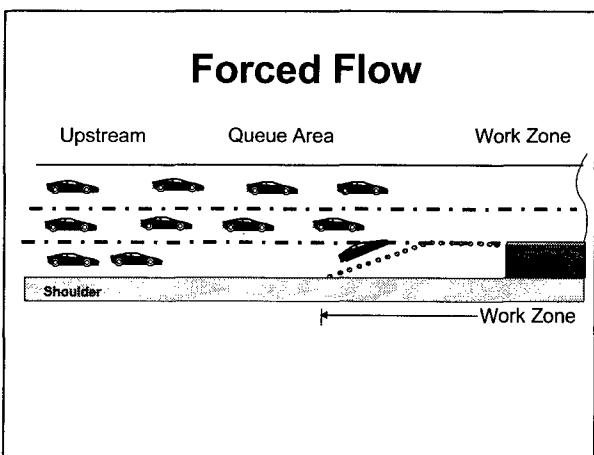
28



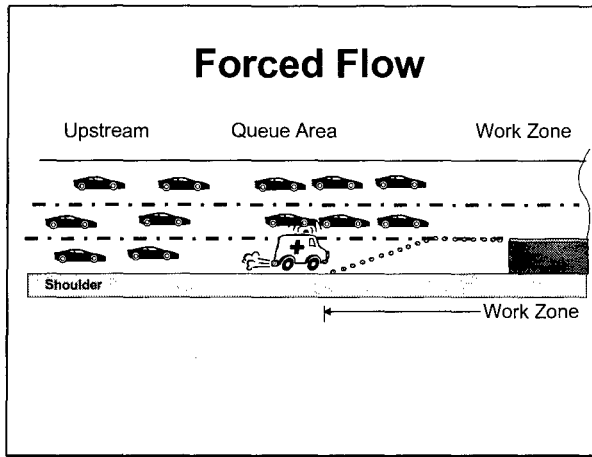
29



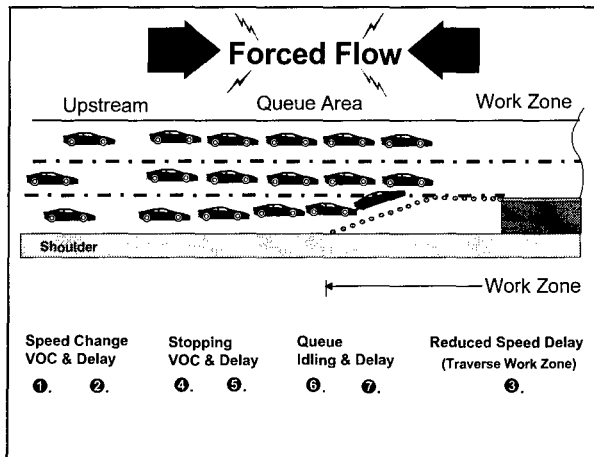
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31



32

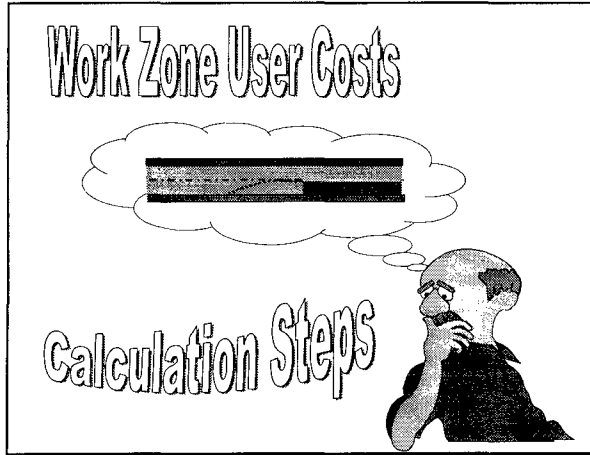


33

End Session



1



2

WZ User Costs Steps

1. Determine Capacity
2. Calculate Directional Hourly Demand
3. Identify User Cost Components
4. Quantify Traffic Affected by each Component
5. Compute Reduced Speed Delay
6. Assign VOC Cost Rates

(More)

3

WZ User Costs Steps (Cont'd)

7. Assign Delay Cost Rates
8. Assign Traffic to Vehicle Classes
9. Compute User Costs by Vehicle Class
10. Determine Circuitry
11. Compute Crash Costs
12. Sum Total User Costs

4

1. Determine Capacity
<ul style="list-style-type: none">■ With and without WZ■ Resources:<ul style="list-style-type: none">● Research Studies● Highway Capacity Manual

5

Free Flow Capacity
<ul style="list-style-type: none">■ Ideal<ul style="list-style-type: none">● 2200 to 2300 pvplph■ Mixed flow reductions<ul style="list-style-type: none">● Trucks present● Lane width● Hazard offset● Recreational drivers

6

Mixed Traffic Capacity
<p>1000 - 2300 vplph</p> <p>See Table 3.4-3.6 of Technical Bulletin</p>

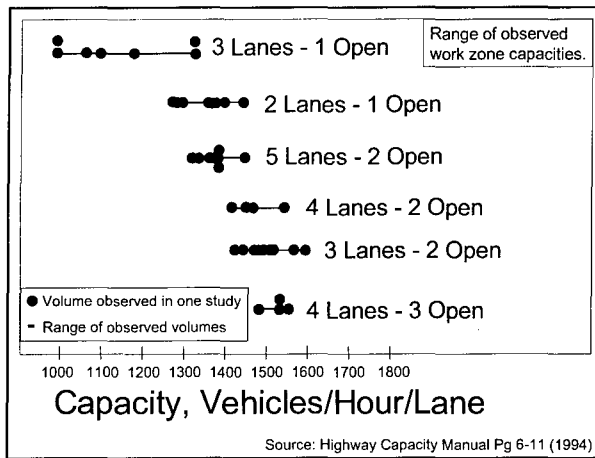
7

Work Zone Capacity

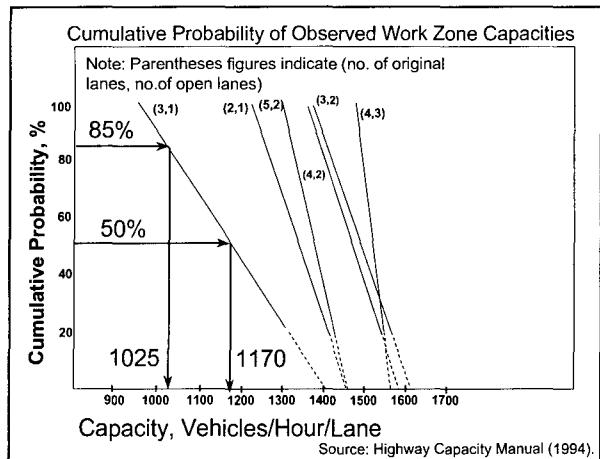
Directional Lanes		No of Studies	Capacity	
Normal Operations	WorkZone Operations		(vph)	(Veh/Ln-Hr.)
3	1 Open	7	1170	1170
2	1 Open	8	1340	1340
5	2 Open	8	2740	1370
4	2 Open	4	2960	1480
3	2 Open	9	2980	1490
4	3 Open	4	4560	1520

Source: 1994 Highway Capacity Manual - Table 6.1

8



9



10

Queue Dissipation Cap.

- Has significant impact on time to clear queue
- Ranges from 1500 to 2000 vphpl
- See HCM Page 2-32

11

2. Calculate Directional Hrly Demand

Directional Hourly Demand =

- (AADT) x
- (% Hourly Demand) x
- (Directional Factor)

12

Sources

- AADT & directional factor
 - Traffic counts
- Hourly distributions
 - MicroBENCOST
 - Traffic counts

13

Hour	Interstate			Principal Arterials			Minor Arterials			Major Collectors		
	ADT	% In	% Out	ADT	% In	% Out	ADT	% In	% Out	ADT	% In	% Out
0 - 1	1.8	48	52									
1 - 2	1.5	48	52									
2 - 3	1.3	45	55									
3 - 4	1.3	53	47									
4 - 5	1.5	53	47									
5 - 6	1.8	53	47									
6 - 7	2.5	57	43	Same			Same			Same		
7 - 8	3.5	56	44									
8 - 9	4.2	56	44									
9 - 10	5.0	54	46									
10 - 11	5.4	51	49									
11 - 12	5.6	51	49									
⋮												

14

Hour	Interstate			Principal Arterials			Minor Arterials			Major Collectors		
	ADT	% In	% Out	ADT	% In	% Out	ADT	% In	% Out	ADT	% In	% Out
12 - 13	5.7	50	50									
13 - 14	6.4	52	48									
14 - 15	6.8	51	49									
15 - 16	7.3	53	47									
16 - 17	9.3	49	51									
17 - 18	7.0	43	57									
18 - 19	5.5	47	53	Same			Same			Same		
19 - 20	4.7	47	53									
20 - 21	3.8	46	54									
21 - 22	3.2	48	52									
22 - 23	2.6	48	52									
23 - 24	2.3	47	53									

15

Hour	Traffic Pattern Group					
	Interstate		Prin. Arterial		Min. Arterial	
	Urban	Rural	Urban	Rural	Urban	Rural
0 - 1	1.3	1.7	0.9	0.9	0.8	0.7
1 - 2	0.9	1.4	0.5	0.5	0.4	0.4
2 - 3	0.8	1.3	0.4	0.5	0.3	0.3
3 - 4	0.8	1.3	0.4	0.5	0.3	0.4
4 - 5	1.1	1.4	0.6	0.9	0.4	0.8
5 - 6	2.1	2.1	1.8	2.3	1.3	2.2
6 - 7	4.7	3.7	4.4	4.9	4.0	4.5
7 - 8	6.4	4.9	6.2	6.2	6.4	5.5
8 - 9	5.6	4.9	5.7	5.5	5.7	5.3
9 - 10	5.1	5.2	5.1	5.3	4.8	5.4
10 - 11	5.2	5.5	5.2	5.4	4.9	5.8
11 - 12	5.4	5.8	5.6	5.6	5.5	6.0

16

Example



PennDOT AADT Distribution - Hourly Percentages

Hour	Traffic Pattern Group					
	Interstate		Prin. Arterial		Minor Arterial	
	Urban	Rural	Urban	Rural	Urban	Rural
12 - 13	5.5	5.7	6.0	5.7	6.0	6.2
13 - 14	5.5	5.9	5.9	5.9	5.7	6.4
14 - 15	6.1	6.3	6.4	6.6	6.3	7.2
15 - 16	7.3	6.9	7.4	7.7	7.6	8.1
16 - 17	7.8	7.2	7.8	8.0	8.3	8.0
17 - 18	7.2	6.6	7.5	7.4	8.0	7.1
18 - 19	5.4	5.3	5.9	5.5	6.2	5.4
19 - 20	4.3	4.4	4.8	4.3	5.1	4.4
20 - 21	3.7	3.8	4.0	3.6	4.3	3.6
21 - 22	3.2	3.4	3.3	3.0	3.4	2.9
22 - 23	2.6	2.9	2.4	2.3	2.4	2.1
23 - 24	2.0	2.4	1.7	1.5	1.6	1.4

17

3. Identify User Cost Components

- ➔ Free Flow
 - Speed Change
 - Reduced Speed
- ➔ Forced Flow
 - Stopping
 - Queuing



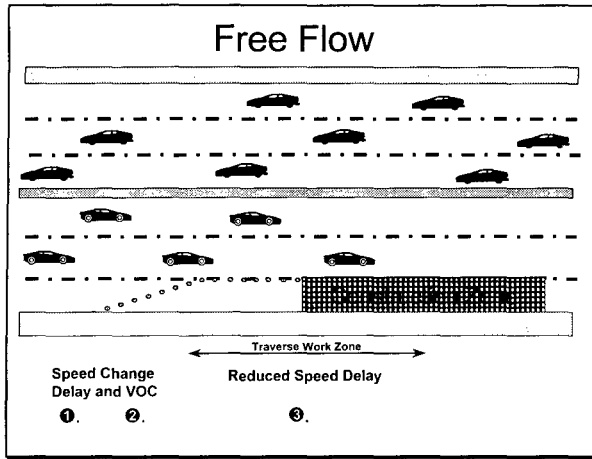
18

Free Flow Cost Components

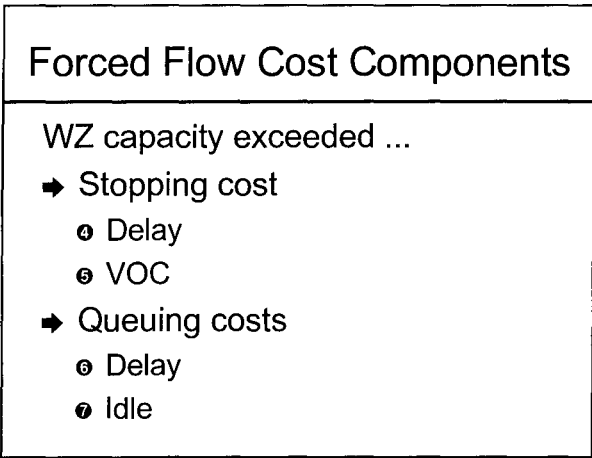
WZ capacity not exceeded ...

- ➔ Speed change costs
 - ① Delay
 - ② VOC
- ➔ Reduced speed costs
 - ③ Delay

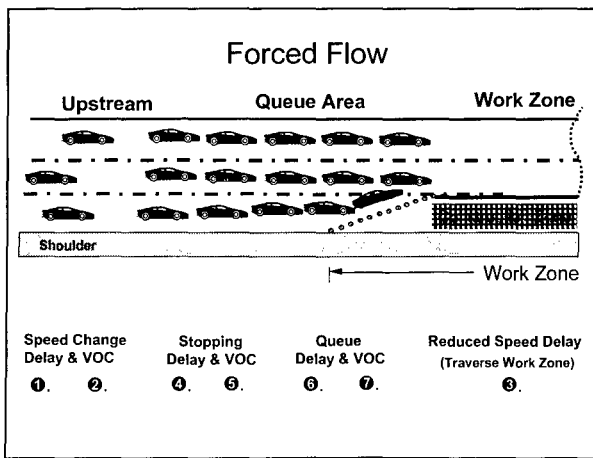
19



20



21



22

4. Quantify Traffic Affected by Each Component
<ul style="list-style-type: none">■ Lane closure hours■ 24 Hours analysis period

23

Example
<ul style="list-style-type: none">■ AADT = 67,406 veh per day. (SouthBound)■ Traff. Mix 90% Auto, 5.4% SU, 4.6% Combo■ 3 Lane Open Non Work Zone (Cap. 6285 vph)■ 2 Lane Open - 1 Lane Closed for Work Zone (Cap. 3027 vph)■ Work Zone Hours 8 pm - 5 am, 9 am - 3 pm■ Length = 5.25 miles■ Approach Speed = 55 mph■ Work Zone Speed = 40 mph■ Work Zone In Place 60 Days

24

Twenty Four Hour Analysis Period - SouthBound

Hour	Hourly Distr. (%)	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queued Veh. (e _i +f _{i-1})	Vehicles that ...		
						Stop 55-0-55	Traverse WZ @40 mph	Slowdown 55-40-55
		(AADT)(b)		(c-d)	(e _i +f _{i-1})	IF f > 0,c,0	See Note**	IF g=0, h,0
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)
0 - 1	0.9	607	3,027	(2,420)	0	0	607	607
1 - 2	0.5	337	3,027	(2,690)	0	0	337	337
2 - 3	0.4	270	3,027	(2,757)	0	0	270	270
3 - 4	0.4	270	3,027	(2,757)	0	0	270	270
4 - 5	0.6	404	3,027	(2,623)	0	0	404	404
5 - 6	1.8	1,213	6,285	(5,072)	0	0	0	0
6 - 7	4.4	2,966	6,285	(3,319)	0	0	0	0
7 - 8	6.2	4,179	6,285	(2,106)	0	0	0	0
8 - 9	5.7	3,842	6,285	(2,443)	0	0	0	0
9 - 10	5.1	3,438	3,027	411	411	3,438	3,027	0
10 - 11	5.2	3,505	3,027	478	889	3,505	3,027	0
11 - 12	5.6	3,775	3,027	748	1,637	3,775	3,027	0
⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮

** If(d=3027, If (c>d,d,c),0)

25

Table Continued

Hour	Hourly Distr. (%)	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queued Veh.	Vehicles that ...		
						Stop 55-0-55	Traverse WZ at 40 mph	Slowdown 55-40-55
		(AADT)(b)		(c-d)	(e+f _{i-1})	IF f > 0, c, 0	See Note**	IF g=0, h, 0
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)
12 - 13	6.0	4,044	3,027	1,017	2,654	4,044	3,027	0
13 - 14	5.9	3,977	3,027	950	3,604	3,977	3,027	0
14 - 15	6.4	4,314	3,027	1,287	4,891	4,314	3,027	0
15 - 16	7.4	4,988	5,454	(466)	4,425	4,988	0	0
16 - 17	7.8	5,258	5,454	(196)	4,229	5,258	0	0
17 - 18	7.5	5,055	5,454	(399)	3,830	5,055	0	0
18 - 19	5.9	3,977	5,454	(1,477)	2,353	3,977	0	0
19 - 20	4.9	3,303	5,454	(2,151)	202	3,303	0	0
20 - 21	4.0	2,696	3,027	(331)	0	1,646 *	2,898	1,051
21 - 22	3.3	2,224	3,027	(803)	0	0	2,224	2,224
22 - 23	2.4	1,618	3,027	(1,409)	0	0	1,618	1,618
23 - 24	1.7	1,146	3,027	(,1881)	0	0	1,146	1,146
Total						47,279	27,936	7,926

*Prorated based on portion of hour required to clear queue.

26

Traffic Affected - Free Flow

- Speed Change (55-40-55)
 - 7,926 vpd (60 days) = 475,569 veh.

- Traverse Workzone
(at reduced speed of 40 mph)
 - 27,936 vpd (60 days) = 1,676,152 veh.

27

Traffic Affected - Forced Flow

- Stopping (55-0-55)
 - 47,279 vpd (60 day) = 2,836,762 veh.

- Queuing
 - Same as above

28

5. Compute Reduced Speed Delay

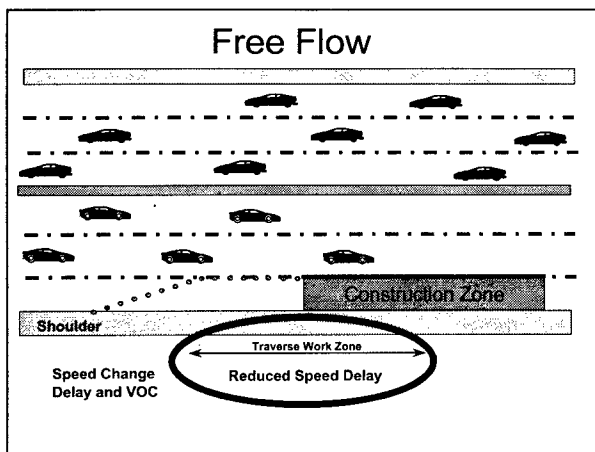
- ✓ Traverse Work Zone



- ✓ Que



29

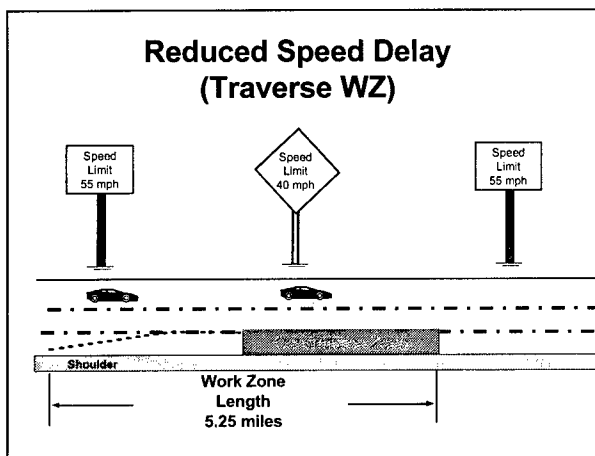


30

Reduced Speed Delay (Traverse WZ)

- Reduced speed delay is ...
 - increased travel time necessary to traverse the work zone at the posted speed compared to the up stream posted speed

31



32

Reduced Speed Delay (WZ)

Reduced Speed Delay =

$$\frac{\text{WZ Length}}{\text{WZ Speed}} - \frac{\text{WZ Length}}{\text{Upstream Speed}}$$

33

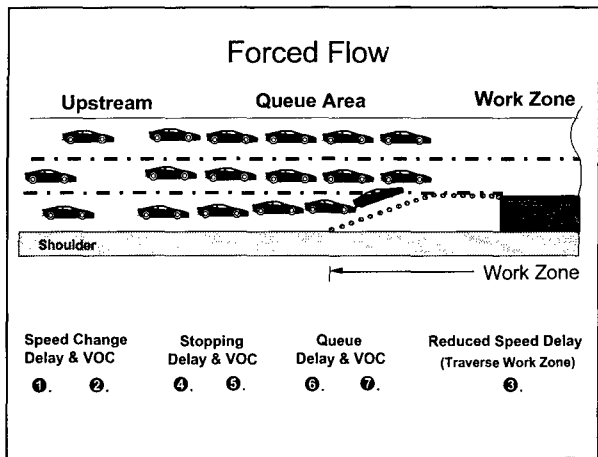
Reduced Speed Delay (WZ)

$$\frac{\text{WZ Length}}{\text{WZ Speed}} - \frac{\text{WZ Length}}{\text{Upstream Speed}}$$

$$\frac{5.25 \text{ Miles}}{40 \text{ mph}} - \frac{5.25 \text{ Miles}}{55 \text{ mph}}$$

➔ Delay / Vehicle = 0.0358 Hours

34



35

Reduced Speed Delay (Queue)

Reduced Speed Delay =

$$\frac{\text{Queue Length}}{\text{Queue Speed}} - \frac{\text{Queue Length}}{\text{Upstream Speed}}$$

36

Reduced Speed Delay (Queue)

Reduced Speed Delay =

$$\frac{\text{Queue Length}}{\text{Queue Speed}} - \frac{\text{Queue Length}}{\text{Upstream Speed}}$$

 ? Miles
-
 ? Miles

 ? Mph
-
 55 mph

37

Reduced Speed Delay (Queue)

- Determine queue speed
- Determine average queue length

38

Reduced Speed Delay (Queue)

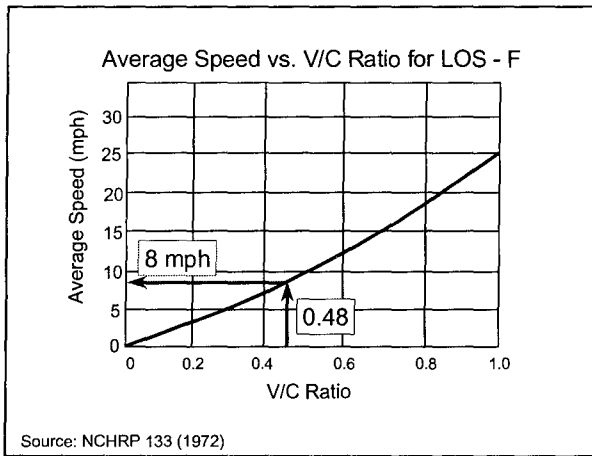
- Determine queue speed

$$V/C = \frac{\text{Vol}_{\text{Queue}}}{\text{Capacity}}$$

WZ Capacity
Upstream

$$V/C = 3,027/6,285 = 0.48$$

39



40

Reduced Speed Delay Queue

$$\frac{\text{Queue Length}}{\text{Queue Speed}} - \frac{\text{Queue Length}}{\text{Upstream Speed}}$$

$$\frac{? \text{ Miles}}{8 \text{ Mph}} - \frac{? \text{ Miles}}{55 \text{ mph}}$$

41

Reduced Speed Delay (Queue)

✓ Determine queue speed

■ Determine average queue length

42

Reduced Speed Delay (Queue)

Average Queue Length =
Maximum Queue Length / 2

43

Twenty Four Hour Analysis Period - SouthBound

Hour	Hourly Distr. (%)	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queued Veh. (e*1.1)	Vehicles that ...			
						Stop 55-0-55	Traverse WZ @40 mph	Slowdown 55-40-55	
(a)	(b)	(c)	(d)	(e)	(f)	IF f > 0, c, 0: See Note**	(h)	IF g=0, h, 0	(i)
0 - 1	0.9	607	3,027	(2,420)	0	0	607	607	
1 - 2	0.5	337	3,027	(2,690)	0	0	337	337	
2 - 3	0.4	270	3,027	(2,757)	0	0	270	270	
3 - 4	0.4	270	3,027	(2,757)	0	0	270	270	
4 - 5	0.6	404	3,027	(2,623)	0	0	404	404	
5 - 6	1.8	1,213	6,285	(5,072)	0	0	0	0	
6 - 7	4.4	2,966	6,285	(3,319)	0	0	0	0	
7 - 8	6.2	4,179	6,285	(2,106)	0	0	0	0	
8 - 9	5.7	3,842	6,285	(2,443)	0	0	0	0	
9 - 10	5.1	3,438	3,027	411	411	3,438	3,027	0	
10 - 11	5.2	3,505	3,027	478	889	3,505	3,027	0	
11 - 12	5.6	3,775	3,027	748	1,637	3,775	3,027	0	
:	:	:	:	:	:	:	:	:	
:	:	:	:	:	:	:	:	:	
:	:	:	:	:	:	:	:	:	
:	:	:	:	:	:	:	:	:	

** If(d=3027, If (c>d,d,c),0)

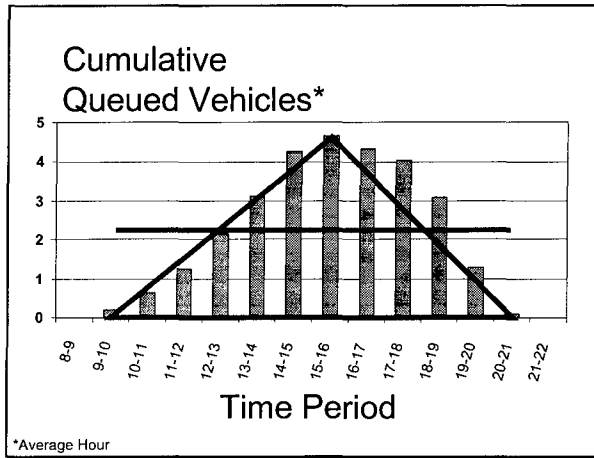
44

Table Continued

Hour	Hourly Distr. (%)	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queued Veh. (e+1,.)	Vehicles that ...		
						Stop 55-0-55	Traverse WZ at 40 mph	Slowdown 55-40-55
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)
12 - 13	6.0	4,044	3,027	1,017	2,654	4,044	3,027	0
13 - 14	5.9	3,977	3,027	950	3,604	3,977	3,027	0
14 - 15	6.4	4,314	3,027	1,287	4,691	4,314	3,027	0
15 - 16	7.4	4,988	5,454	(466)	4,425	4,988	0	0
16 - 17	7.8	5,258	5,454	(196)	4,229	5,258	0	0
17 - 18	7.5	5,055	5,454	(399)	3,830	5,055	0	0
18 - 19	5.9	3,977	5,454	(1,477)	2,353	3,977	0	0
19 - 20	4.9	3,303	5,454	(2,151)	202	3,303	0	0
20 - 21	4.0	2,696	3,027	(331)	0	1,646*	2,898	1,051
21 - 22	3.3	2,224	3,027	(803)	0	0	2,224	2,224
22 - 23	2.4	1,618	3,027	(1,409)	0	0	1,618	1,618
23 - 24	1.7	1,146	3,027	(1,881)	0	0	1,146	1,146
Total						47,279	27,936	7,926

*Prorated based on portion of hour required to clear queue. ** If(d=3027, If (c>d,d,c),0)

45



46

Reduced Speed Delay (Queue)

Caution:

- Queue could grow and stabilize for a period of time and then dissipate.
- This would support calculating queue length on a per hour basis.

47

Reduced Speed Delay (Queue)
Average Queue Length = Maximum Queue Length / 2

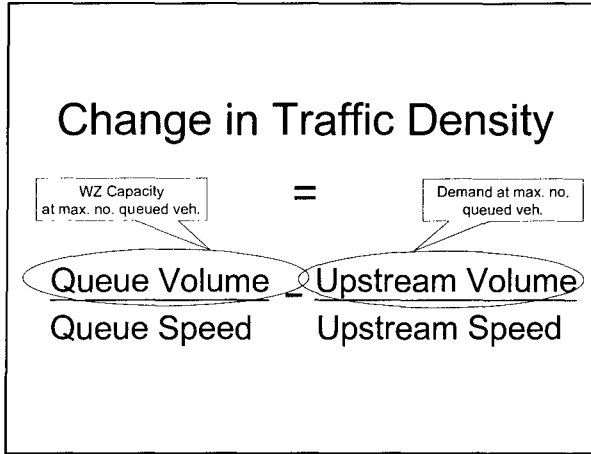
48

Reduced Speed Delay (Queue)
Maximum Queue Length = $\frac{\text{Maximum No. of Queued Vehicles}}{\text{Change in Traffic Density}}$

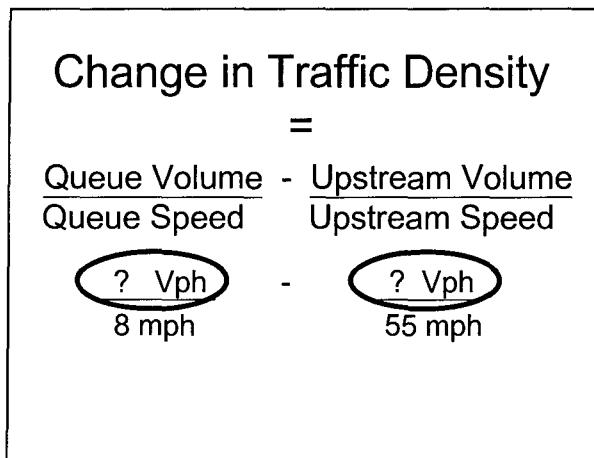
49

Traffic Density (veh./mi.)
<ul style="list-style-type: none">■ The number of vehicles on a <u>mile</u> of road.■ Computed by ...■ Volume / Speed (vph/mph)

50



51



52

Twenty Four Hour Analysis Period - SouthBound

Hour	Hourly	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queue	Stop 55-0-55	Vehicles that ...	
	Distr. (%)				Queued Veh.		Traverse WZ @40 mph	Slowdown 55-40-55
	(AADT)(b)	(c)	(d)	(e)	(f)	IF f > 0, c, 0	IF g=3027, c, 0	IF g=0, h, 0
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)
0 - 1	0.9	607	3,027	(2,420)	0	0	607	607
1 - 2	0.5	337	3,027	(2,690)	0	0	337	337
2 - 3	0.4	270	3,027	(2,757)	0	0	270	270
3 - 4	0.4	270	3,027	(2,757)	0	0	270	270
4 - 5	0.6	404	3,027	(2,623)	0	0	404	404
5 - 6	1.8	1,213	6,285	(5,072)	0	0	0	0
6 - 7	4.4	2,966	6,285	(3,319)	0	0	0	0
7 - 8	6.2	4,179	6,285	(2,106)	0	0	0	0
8 - 9	5.7	3,842	6,285	(2,443)	0	0	0	0
9 - 10	5.1	3,438	3,027	411	411	3,438	3,027	0
10 - 11	5.2	3,505	3,027	478	478	3,505	3,027	0
11 - 12	5.6	3,775	3,027	748	748	3,775	3,027	0
⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮
⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮	⋮

53

Table Continued

Hour	Hourly Distr. (%)	Demand (vph)	Cap. (vph)	Queue Rate (vph)	Queued Veh. (e+1,.)	Vehicles that ...		
						Stop 55-0-55	Traverse WZ at 40 mph	Slowdown 55-40-55
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)
12 - 13	6.0	4,044	3,027	1,017	2,054	4,044	3,027	0
13 - 14	5.9	3,977	3,027	950	2,283	3,977	3,027	0
14 - 15	6.4	4,314	3,027	1,287	4,391	4,314	3,027	0
15 - 16	7.4	4,988	3,027	(466)	4,420	4,988	0	0
16 - 17	7.8	5,258	3,027	(196)	4,229	5,258	0	0
17 - 18	7.5	5,055	3,027	(399)	3,830	5,055	0	0
18 - 19	5.9	3,977	3,027	(1,477)	2,363	3,977	0	0
19 - 20	4.9	3,303	3,027	(2,151)	202	3,303	0	0
20 - 21	4.0	2,696	3,027	(331)	0	1,646 *	2,898	1,051
21 - 22	3.3	2,224	3,027	(803)	0	0	2,224	2,224
22 - 23	2.4	1,618	3,027	(1,409)	0	0	1,618	1,618
23 - 24	1.7	1,146	3,027	(1,881)	0	0	1,146	1,146
Total						47,279	27,936	7,926

*Prorated based on portion of hour required to clear queue.

54

Change in Traffic Density

=

$$\frac{\text{Queue Volume}}{\text{Queue Speed}} - \frac{\text{Upstream Volume}}{\text{Upstream Speed}}$$

(3027 vph) / (8 mph) - (4314 vph) / (55 mph)

55

Change in Traffic Density

=

$$\frac{\text{Queue Volume}}{\text{Queue Speed}} - \frac{\text{Upstream Volume}}{\text{Upstream Speed}}$$

(3027 vph) / (8 mph) - (4314 vph) / (55 mph)

= 300 veh./mi.

56

Reduced Speed Delay (Queue)

$$\begin{aligned} \text{Maximum Queue Length} &= \\ & \frac{\text{Maximum no. of Queued Vehicles}}{\text{Change in traffic density}} \\ & \frac{4,891 \text{ vehicles}}{300 \text{ vehicles/mile}} = 16.3 \text{ Miles} \end{aligned}$$

57

Reduced Speed Delay (Queue)

$$\begin{aligned} \text{Average Queue Length} &= \\ & \frac{\text{Maximum Queue Length}}{2} \\ & = \frac{16.3 \text{ Miles}}{2} = 8.15 \text{ Miles} \end{aligned}$$

58

Reduced Speed Delay
Queue

$$\frac{\text{Queue Length}}{\text{Queue Speed}} - \frac{\text{Queue Length}}{\text{Upstream Speed}}$$

$$\frac{8.15 \text{ Miles}}{8 \text{ Mph}} - \frac{8.15 \text{ Miles}}{55 \text{ mph}}$$

59

Reduced Speed Delay Queue

$$\frac{\text{Queue Length}}{\text{Queue Speed}} - \frac{\text{Queue Length}}{\text{Upstream Speed}}$$

$$\frac{8.15 \text{ Miles}}{8 \text{ Mph}} - \frac{8.15 \text{ Miles}}{55 \text{ mph}}$$

$$1.02 \text{ Hours} - 0.15 \text{ Hours}$$

➔ Delay / Vehicle = 0.87 Hours

60

6. Assign VOC Cost Rates

Sources:

- NCHRP 133 Procedures for Estimating Highway User Costs, Air Pollution, and Noise Effects, 1972
- Economic Analysis for Highways, Winfrey 1969

61

Added Time & Veh. Running Cost / 1000 Stops and Idling Costs (Aug. 1996 values)

Initial Speed (mph)	Added Time (Hr/1000 Stops) (Excludes Idling Time)			Added Cost (\$/1000 Stops) (Excludes Idling Cost)		
	Pass Cars	Single Unit Trk	Comb. Truck	Pass Cars	Single Unit Trk	Comb. Truck
5	1.02	0.73	1.10	2.70	9.25	33.62
10	1.51	1.47	2.27	8.83	20.72	77.49
15	2.00	2.20	3.48	15.16	33.89	129.97
20	2.49	2.93	4.76	21.74	48.40	190.06
25	2.98	3.67	6.10	28.67	63.97	256.54
30	3.46	4.40	7.56	36.10	80.23	328.21
35	3.94	5.13	9.19	44.06	96.88	403.84
40	4.42	5.87	11.09	52.70	113.97	482.21
45	4.90	6.60	13.39	62.07	130.08	562.14
50	5.37	7.33	16.37	72.31	145.96	642.41
55	5.84	8.07	20.72	83.47	160.89	721.77
60	6.31	8.80	27.94	95.70	178.98	798.99
65	6.78	9.53	NA	109.02	195.84	NA
70	7.25	NA	NA	123.61	NA	NA
75	7.71	NA	NA	139.53	NA	NA
80	8.17	NA	NA	156.85	NA	NA
Idling Cost (\$ / Veh-Hr.)				0.6927	0.7681	0.8248

62

Example

55 mph → 40 mph → 55 mph

Initial Speed (mph)	Added Time (Hr/1000 Stops) (Excludes Idling Time)			Added Cost (\$/1000 Stops) (Excludes Idling Cost)		
	Pass Cars	Single Unit Trk	Comb. Truck	Pass Cars	Single Unit Trk	Comb. Truck
5	1.02	0.73	1.10	2.70	9.25	33.62
10	1.51	1.47	2.27	8.83	20.72	77.49
15	2.00	2.20	3.48	15.16	33.89	129.97
20	2.49	2.93	4.76	21.74	48.40	190.06
25	2.98	3.67	6.10	28.67	63.97	256.54
30	3.46	4.40	7.56	36.10	80.23	328.21
35	3.94	5.13	9.19	44.06	96.88	403.84
40	4.42	5.87	11.09	52.70	113.97	482.21
45	4.90	6.60	13.39	62.07	130.08	562.14
50	5.37	7.33	16.37	72.31	145.96	642.41
55	5.84	8.07	20.72	83.47	160.89	721.77
60	6.31	8.80	27.94	95.70	178.98	798.99
65	6.78	9.53	NA	109.02	195.84	NA
70	7.25	NA	NA	123.61	NA	NA
75	7.71	NA	NA	139.53	NA	NA
80	8.17	NA	NA	156.85	NA	NA
Idling Cost (\$ / Veh-Hr.)				0.6927	0.7681	0.8248

63

Example

Added Time & Veh. Running Cost / 1000 Stops and Idling Costs (Aug. 1996 values)

Initial Speed (mph)	Added Time (Hr/1000 Stops) (Excludes Idling Time)			Added Cost (\$/1000 Stops) (Excludes Idling Cost)		
	Pass Cars	Single Unit Trk	Comb. Truck	Pass Cars	Single Unit Trk	Comb. Truck
55	5.84	8.07	20.72	83.47	160.89	721.77
40	4.42	5.87	11.09	52.70	113.97	482.21
55-40-55	1.42	2.20	9.63	30.77	46.92	239.56

64

- Note:
- Update tables to current year
- Value of time
 - Overall current CPI to base year overall CPI
 - VOC
 - Current year transportation component CPI to base year transportation component

65

7. Assign Delay Cost Rates



Value of Time

Vehicle Class	\$ / Vehicle Hour	
	Value	Range
Passenger Vehicles	11.58	10 - 13
Single Unit Trucks	18.54	17 - 19
Combination Trucks	22.31	21 - 24

Aug. 1996 Dollars

66

8. Assign Traffic to Vehicle Classes

Vehicle classification:

- Passenger vehicles
 - Personal
 - Commercial
- Single unit trucks
- Combination trucks

67

	Cost Component	Vehicle Class	No. Vehicles
Free Flow	① WZ Speed Change Delay (55-40-55) (475,569)(%Class)=	Pass	428,012
		SU	25,681
		Comb	21,786
Forced Flow	② WZ Speed Change VOC (55-40-55) (475,569)(%Class)=	Pass	428,012
		SU	25,681
		Comb	21,786
Forced Flow	③ WZ Reduced Speed Delay (Traverse WZ at 40 mph) (1,676,152)(%Class)=	Pass	1,508,537
		SU	90,512
		Comb	77,103
Forced Flow	④ Queue Stopping Delay (55-0-55) (2,836,762)(%Class)=	Pass	2,553,086
		SU	153,185
		Comb	130,491
Forced Flow	⑤ Queue Stopping VOC (55-0-55) (2,836,762)(%Class)=	Pass	2,553,086
		SU	153,185
		Comb	130,491
Forced Flow	⑥ Queue Added Travel Delay (Traverse Queue at 8 mph) (2,836,762)(%Class)=	Pass	2,553,086
		SU	153,185
		Comb	130,491
Forced Flow	⑦ Queue Idle VOC (2,836,762)(%Class)	Pass	2,553,086
		SU	153,185
		Comb	130,491

68

9. Compute User Costs by Vehicle Class



69

Cost Component	Vehicle Class	No. Vehicles	Added Time (h/v)	Cost Factor	Cost (\$)	% of Total
① WZ Speed Change Delay 55 → .40 → .55 mph	Pass	428,012	0.00142	11.58	7,038	0.02
	SU	25,681	0.00220	18.54	1,047	0.00
	Comb	21,876	0.00963	22.31	4,700	0.01
② WZ Speed Change VOC 55 → .40 → .55 mph	Pass	428,012		0.03077	13,170	0.04
	SU	25,681		0.04692	1,205	0.00
	Comb	21,876		0.23956	5,241	0.02
③ WZ Reduced Speed Delay 40 vs 55 mph	Pass	1,508,537	0.0358	11.58	625,385	1.85
	SU	90,512	0.0358	18.54	60,076	0.18
	Comb	77,103	0.0358	22.31	61,582	0.18
④ Queue Stopping Delay 55 → .0 → .55 mph	Pass	2,553,086	0.00584	11.58	172,658	0.51
	SU	153,185	0.00807	18.54	22,919	0.07
	Comb	130,491	0.02072	22.31	60,321	0.18
⑤ Queue Stopping VOC 55 → .0 → .55 mph	Pass	2,553,086		0.08347	213,106	0.63
	SU	153,185		0.16089	24,646	0.07
	Comb	130,491		0.72177	94,185	0.28
⑥ Queue Idle Delay 55 → .0 → .55 mph	Pass	2,553,086	0.87	0.6927	1,538,615	4.55
	SU	153,185	0.87	0.7681	102,366	0.30
	Comb	130,491	0.87	0.8248	93,637	0.28
Total WZ User Cost = \$33,826,855						

70

**Queue Added Travel Time Delay
8 vs 55 mph**

Vehicle Class	No. Vehicles	Added		% of Total
		Time (h/v)	Cost Factor	
Pass	2,553,086	0.87	11.58	25,721,320 76.04
SU	153,185	0.87	18.54	2,470,846 7.30
Combo	130,491	0.87	22.31	2,532,792 7.49

71

- 10. Determine Circuity**
- Detours - 31cents per mile
 - Road closures
 - Self-imposed diversions
 - Driver type

72

- Driver Types**
- Hang Toughers
 - Time Shifters
 - Detourees
 - Trip Swappers
 - Trip Avoiders

73

11. Compute Crash Cost

$$\text{Crash Cost} = (\text{Crash Rate}) (\text{Exposure}) (\text{Crash Cost})$$

[units] [crash/100 M VMT] [VMT] [\$/crash]

Crash rate: Studies, MicroBENCOST

Exposure: Traffic data

Crash cost: MicroBENCOST defaults, Insurance, SHA

Note:

Crash rate and cost is determined by type of crash.

74

Crash Costs

- Commonly assumed that Crashes triple in work zone compared to normal operation of facility
- Not much statistical data to support rule
- WZ Crash rates?

75

Crash Injury Rates Computed From 1995 Highway Statistics

Persons Injured per 100 Million VMT

Functional Class	Rural		Urban	
	Fatalities	Non-Fatal Injury	Fatalities	Non-Fatal Injury
Interstate	1.0	25.0	0.6	74.3
Other Freeways	-	-	0.9	57.2
Other Principal Arterial	2.1	55.8	1.6	208.8
Minor Arterial	2.8	108.6	1.3	175.8
Major Collector	2.7	100.8	-	-
Minor Collector	3.1	120.4	-	-
Collectors	-	-	2.0	172.4
Local	3.7	224.8	2.2	292.1

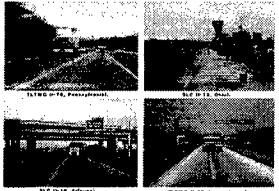
76

Construction Costs and Safety Impacts of Work Zone Traffic Control Strategies
Volume II Informational Guide

"Construction Cost and Safety Impacts of Work Zone Traffic Control Strategies"

Publication No. FHWA-RD-89-210 (Dec. 1989)

December 1989



Publication No. FHWA-RD-89-210 (Dec. 1989)

U.S. Department of Transportation
Federal Highway Administration

Research, Development, and Technology
Lester B. Yeates Highway Research Center
6200 Georgetown Pike
McLean, Virginia 22101-2298

77

Constraints of Study

- Single lane closure (SLC) versus two-lane two-way operation (TLTWO) Rural, 4-lane divided highways
- ADT: 10,000 to 30,000
- 51 projects in 11 states
- 3 had traffic delays

78

Primary Findings

- Many variables influence WZ cost control strategy
- No statistical difference in Crash rates for SLC vs. TLTWO
- Fatal + injury Crashes had a signif. increase for both SLC and TLTWO
- No significant accidents to construction workers

79

Annual Crashes / mi / 10k ADT

Single Lane Closure			
	Before	During	Change
Average	2.256	3.414	1.159
StdDev	1.525	2.372	

Based on data from 26 projects.

Two Lane Two Way Oper.			
	Before	During	Change
Average	3.241	3.057	-0.184
StdDev	2.191	1.384	

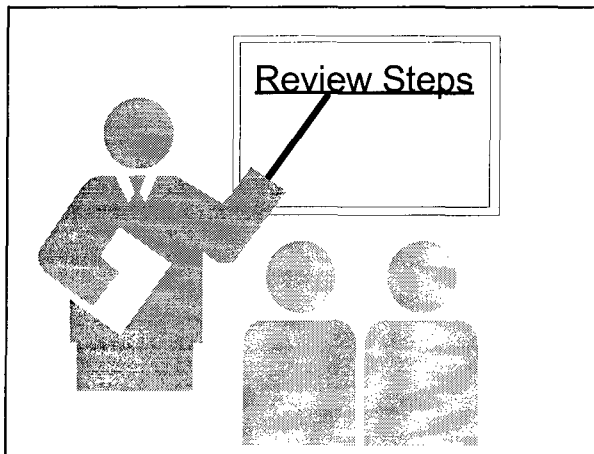
Based on data from 22 projects.

80

12. Sum Total Work Zone User Costs

Delay & VOC
 + Circuity
 + Crash
 Total User Costs

81



82

WZ User Costs Steps

1. Determine Capacity
2. Calculate Directional Hourly Demand
3. Identify User Cost Components
4. Quantify Traffic Affected by Each Component
5. Compute Reduced Speed Delay Times
6. Assign VOC Cost Rates

83

WZ User Costs Steps (Con't)

7. Assign Delay Cost Rates
8. Assign Traffic to Vehicle Classes
9. Compute User Costs by Vehicle Class
10. Circuity
11. Crash Costs
12. Sum Total User Costs

84

End Session

1

Basic Statistics

2

Session Overview

- Probability Concepts
- Probability Distributions
- Measures of Central Tendency
- Measures of Variability
- Interpreting Results

3

Probability Concepts

- Probability is the likelihood of an event occurring
- Probabilities sum to 100%

4

Two Same Birthdays

$$P = 1 - \left[\frac{365!}{(365-n)! (365^n)} \right]$$

n = number of people

23 people ... P = 51%

30 people ... P = 71%

5

Probability Development

- Theoretical
 - Math
- Empirical
 - Experience

6

Variables

- Discrete
- Continuous

7

Discrete

- Countable
- Examples - rolling dice
 - birthday
 - # of accidents

8

DICE COMBINATIONS

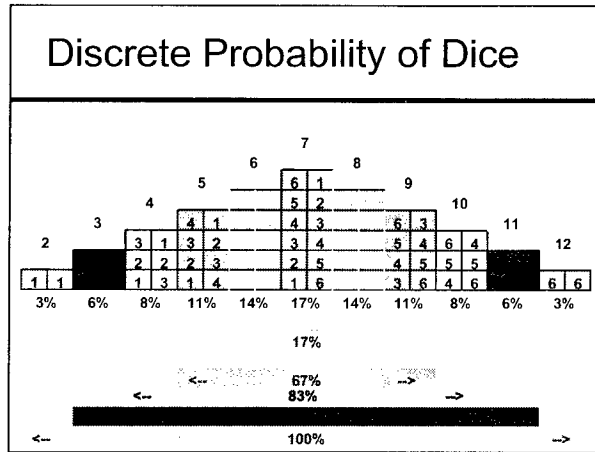
Dice	1	2	3	4	5	6
1	2	3	4	5	6	7
2	3	4	5	6	7	8
3	4	5	6	7	8	9
4	5	6	7	8	9	10
5	6	7	8	9	10	11
6	7	8	9	10	11	12

9

Discrete Probability of Dice

7
 6 1 8
 5 2 9
 4 3 6 3 10
 3 4 5 4 6 4 11
 2 2 2 3 2 5 4 5 5 5 12
 1 1 1 3 1 4 1 6 3 6 4 6 6 6
 3% 6% 8% 11% 14% 17% 14% 11% 8% 6% 3%

10



11

Continuous

■ Uncountable

12

Probability Distributions

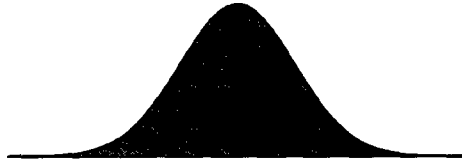
■ Common Varieties

- Normal
- Uniform
- Triangular

13

Normal Distribution

- Bell shaped curve
 - Intelligence tests
 - Defined by mean and std. dev.



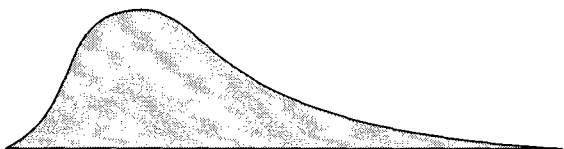
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Skewed Distribution



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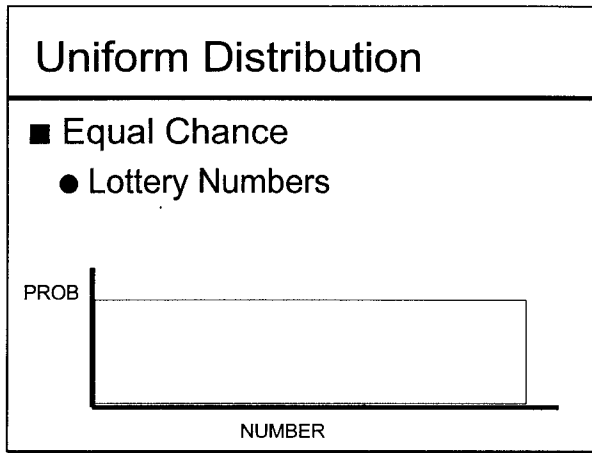
Skewed Distribution



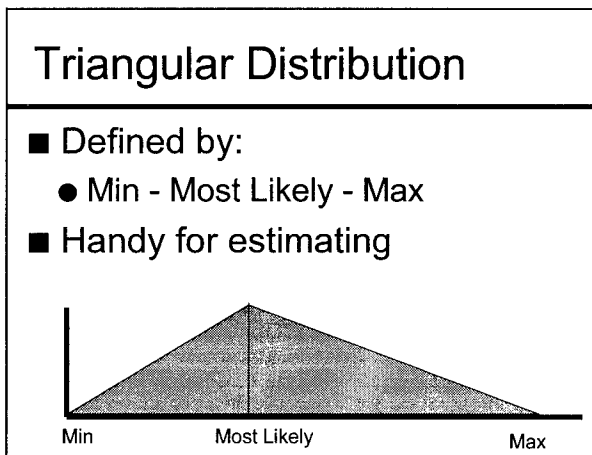
Salaries

Michael
Jordan

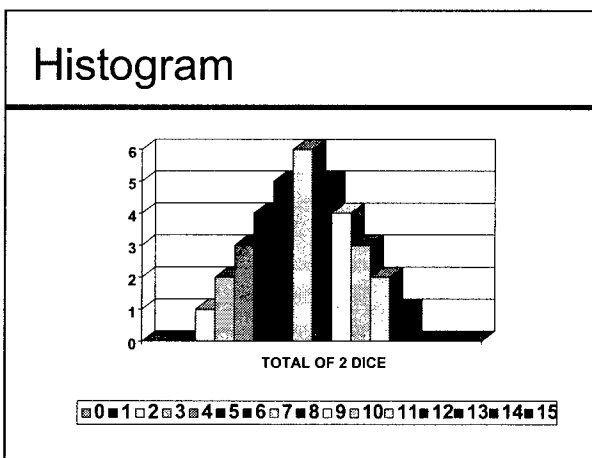
16



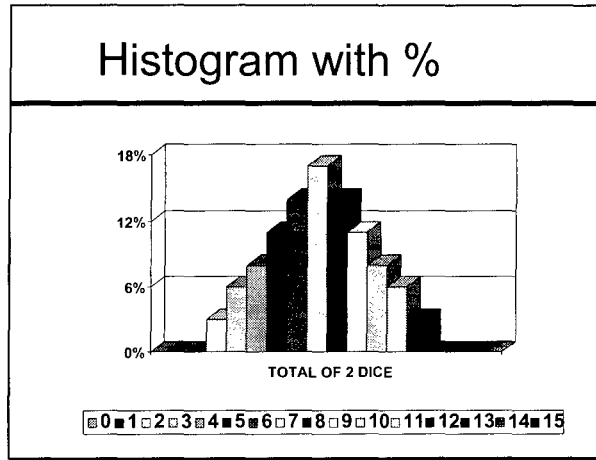
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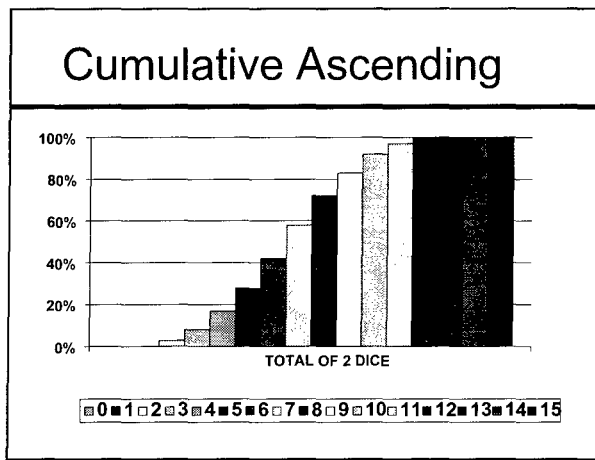
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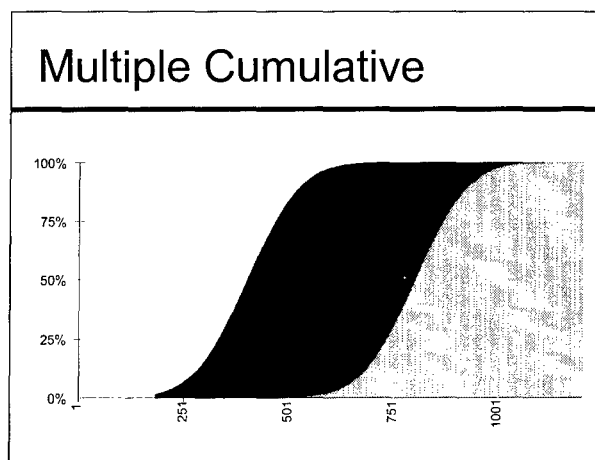
19



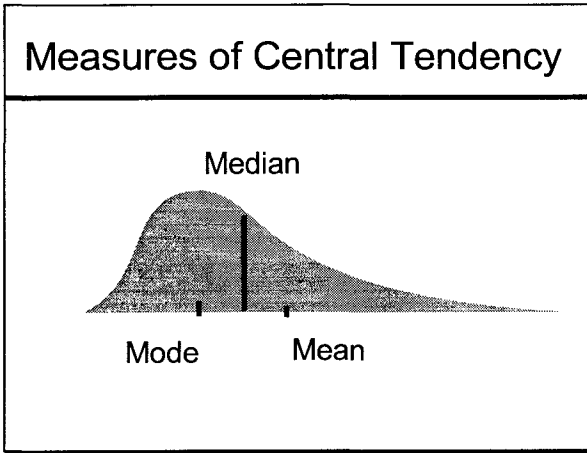
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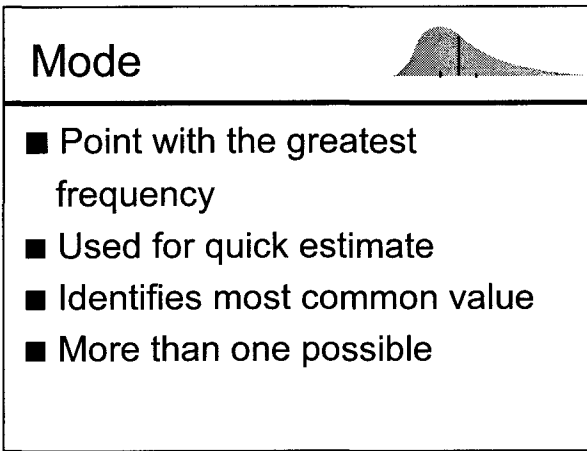
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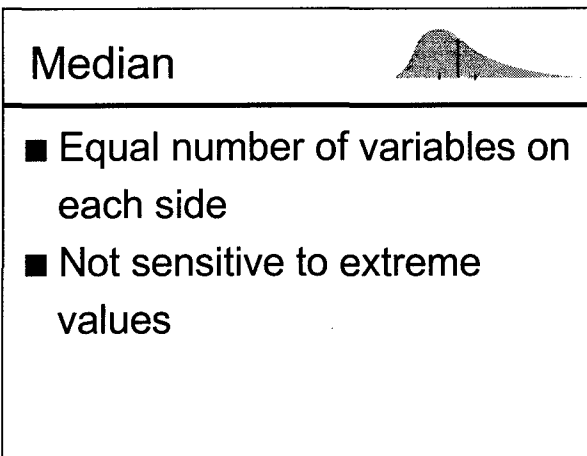
22



23



24



25

Mean



- Balance point of distribution
- Sensitive to all scores
- Extreme values can effect

26

Measures of Variability

- Range
- Standard Deviation
- Coefficient of Variation

27

Range

- Difference between largest and smallest measurement in a set
- In our dice example the range should be $12 - 2 = 10$

28

Standard Deviation

- Root Mean Square of individual deviations from the mean
- Sensitive to all values

29

Formula

- Population

$$\sigma = \sqrt{\sum (x_i - \bar{x})^2 / n}$$

- Sample

$$s = \sqrt{\sum (x_i - \bar{x})^2 / (n - 1)}$$

30

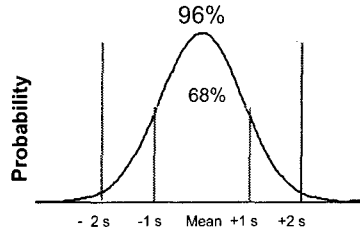
Rule of Thumb

- Standard Deviation may be estimated by dividing the range by 4 to 6

31

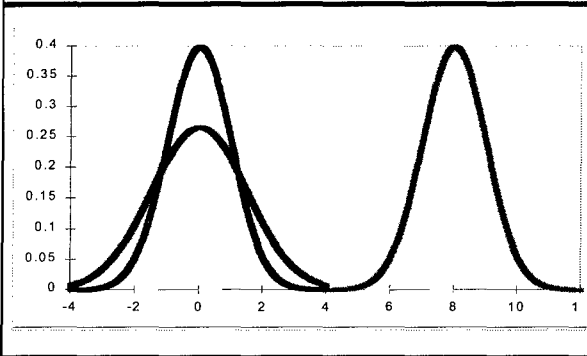
Applications

- 68% of data within $\pm 1 \sigma$
- 96% within $\pm 2 \sigma$



32

Means & Standard Deviations



33

Coefficient of Variation

- $COV = \text{Std. Dev.} / \text{Mean}$
 - Mean = 120
 - Std. Dev. = 30
 - $COV = 25\%$
- Use To Normalize Variation

34

Population - Set of all measurements of interest

Sample - Subset of measurements selected from the population

35

Random Sample

- Each part of the population has an equal chance of being included in the sample

36

Confidence Intervals

- There is a 95% probability that the mean height of class members is between 63 and 73 inches

37

Difference Between Means

- Significance levels
- Statistical versus practical significance

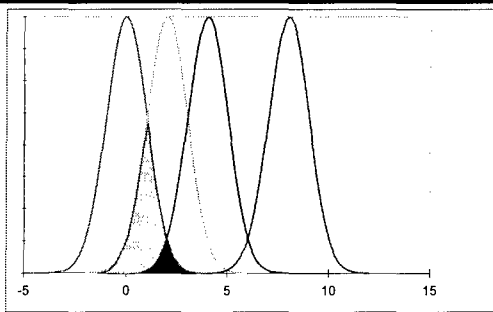
38

Interpreting Results

- Percentiles
- Distributions
 - Overlapping
 - Cumulative

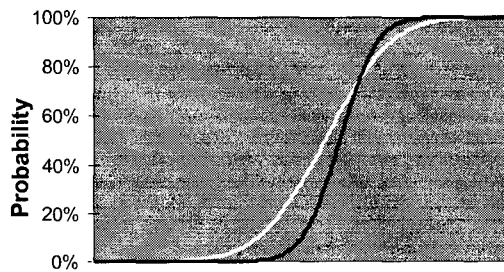
39

Overlapping Normals



40

Overlapping Cumulative



41

Correlation

- How well does a regression equation describe the data
- $R^2 = 1.0$ Perfect
- $R^2 = 0.0$ None
- Percentage of variation described by equation

42

Interpreting R

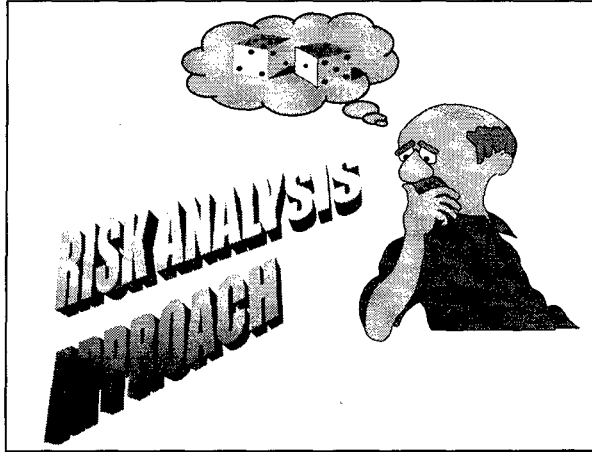
- $R > 0$
 - Positive correlation
- $R < 0$
 - Negative correlation

43

End Session



1



2

Session Overview
<ul style="list-style-type: none">■ Deterministic Approach■ Sources of Variability■ Risk Analysis Approach■ Applications■ Advantages/Disadvantages

3

Deterministic Approach
<ul style="list-style-type: none">■ Select discrete point values<ul style="list-style-type: none">● Initial cost● Future cost● Timing of future cost● Value of time● Discount rate■ Compute discrete alternative NPV

4

Deterministic Approach

\$ 29.4 M \$ 26 M

NPV = Initial Cost +

Future Cost x $\left[\frac{1}{(1+i)^n} \right]$

\$ 9 M

5% 20 yrs

5

We didn't account for the variability of the inputs!

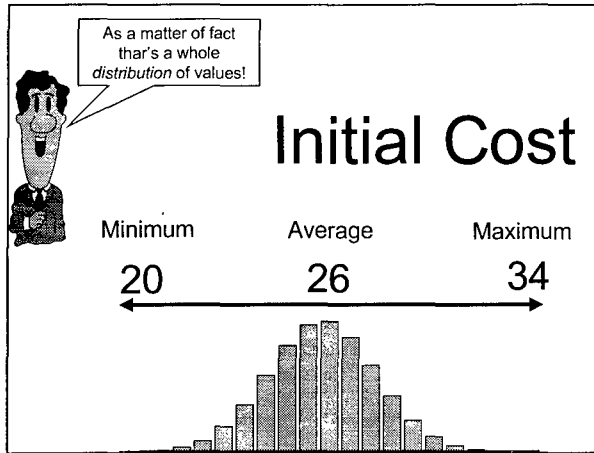
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Initial Cost

Minimum	Average	Maximum
20	26	34

Range

7



8

Sources of Variability

Assumptions and estimates in ...

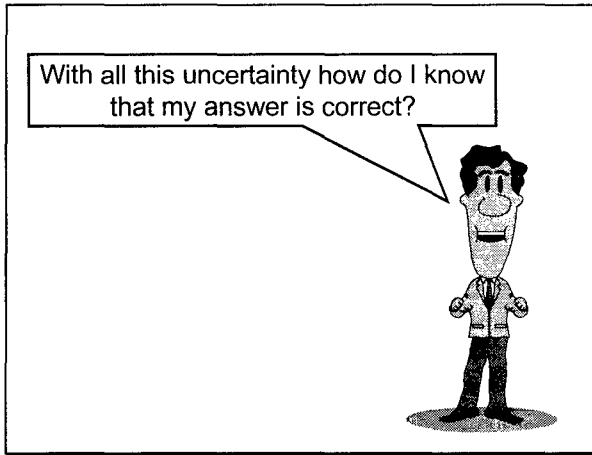
- ➔ Agency Costs
 - initial, rehab. construction, and maintenance activities --->materials, labor, overhead
- ➔ User Costs
 - Daily delay (traffic --> initial & growth rate, daily distribution), construction work days, value of time, ...

9

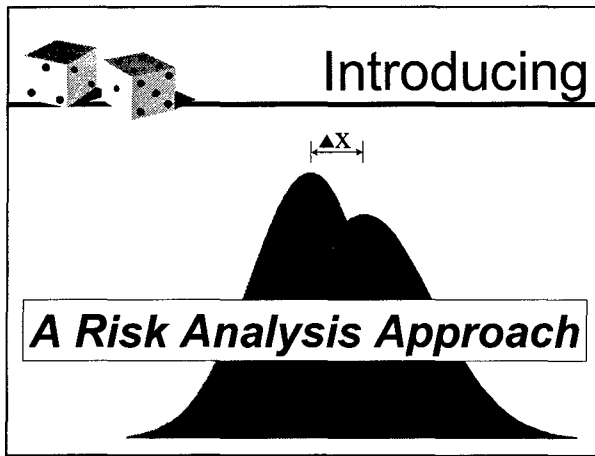
Sources of Variability Con't

- ➔ Discount Rate
- ➔ Performance
 - Environment, traffic loading, subgrade properties, materials design and construction ...

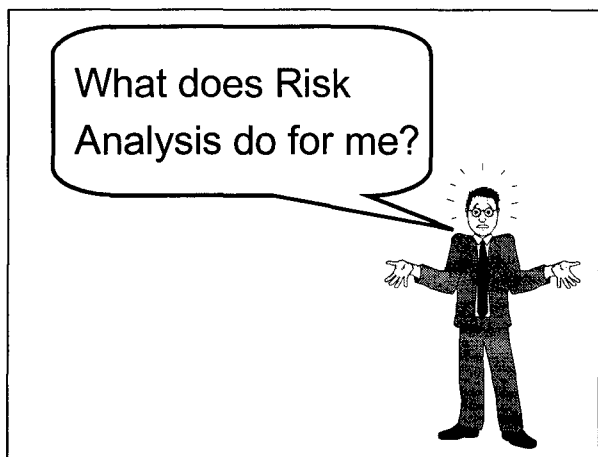
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11



12



13

Risk Analysis ...

- Exposes areas of uncertainty
- Provides an opportunity to take mitigating action to reduce risk exposure
- Provides those vested with appropriate authority the opportunity to make decisions about risk taking

14

What is Risk?

- Chance
- Uncertainty
- Deviation from the expected
- Possibility of loss or gain

15

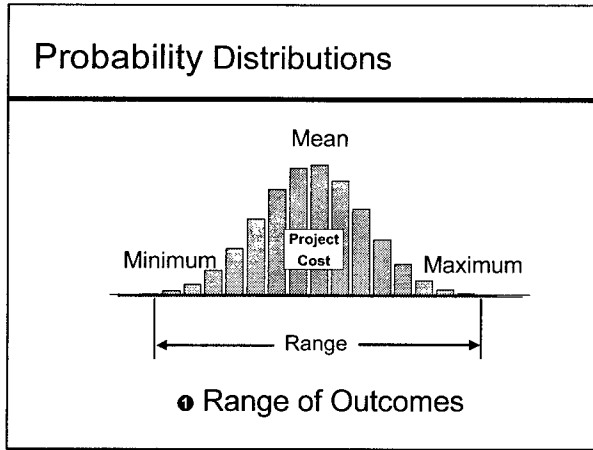
Risk ...

- Subjective
 - Intuitively deciding how risky a situation is.
 - Example: Driving vs. Flying

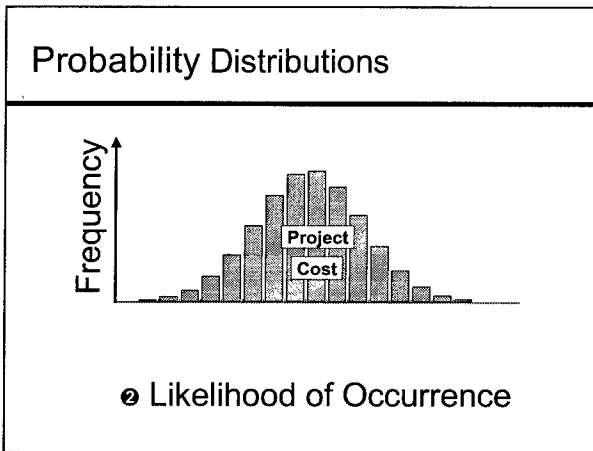


- Objective
 - Quantitatively determining risks
 - 1996 Fatalities: 1070 Aviation 40676 Highways

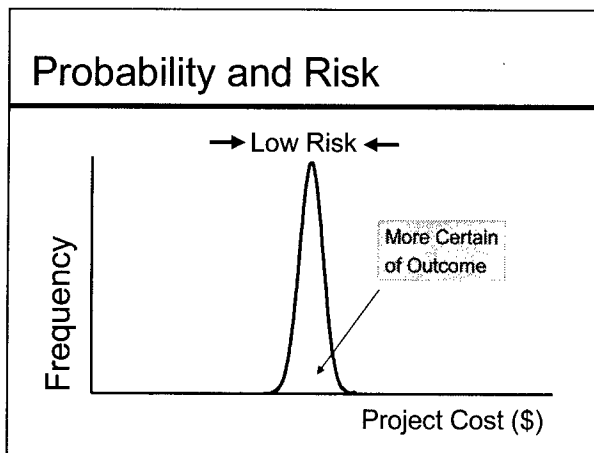
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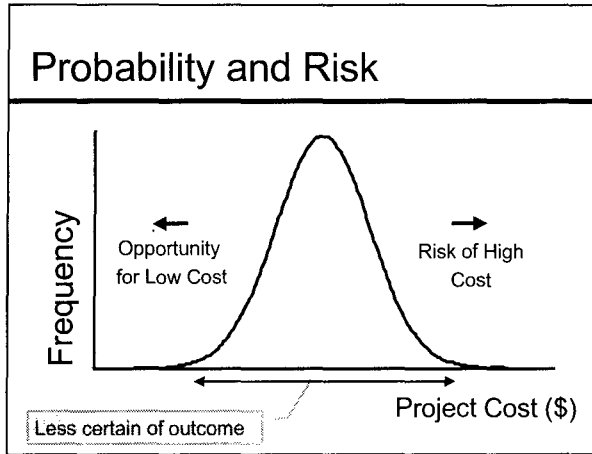
17



18



19



20

- Five Step Approach to Risk Analysis**
- ① Identify structure and logic of problem
 - ② Quantify assumptions w/ probabilistic descriptions of uncertain variables
 - ③ Simulate problem to obtain results
 - ④ Analyze and interpret results
 - ⑤ Make consensus decision

21

1. Identify Structure and Logic of Problem

NPV = Initial Cost +

$$\sum_{k=1}^N \text{Future Cost}_k \times \left[\frac{1}{(1+i)^{n_k}} \right]$$

i = discount rate
n = year of expenditure

22

2. Quantify Assumptions Using Probability
<ul style="list-style-type: none">■ Identify variables to include■ Describe uncertainty

23

Variables to Include ...
<ul style="list-style-type: none">■ Agency costs■ User costs■ Timing of costs■ Discount rate

24

Agency Costs
<ul style="list-style-type: none">■ Preliminary engineering■ Construction management■ Construction costs■ Maintenance costs

25

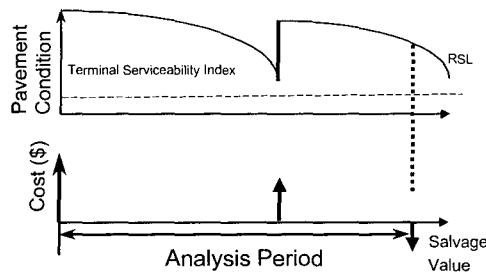
User Costs

- Current traffic
- Future traffic
- Hourly demand
- Vehicle distributions
- Dollar value of delay time
- Work zone configuration
- Work zone hours of operation
- Work zone duration
- Work zone activity years
- Accident rates

26

Timing of Costs

- Pavement performance



27

Discount Rate

NPV = Initial Cost +

$$\sum \text{Future Cost} \times \left[\frac{1}{(1 + i)^n} \right]$$



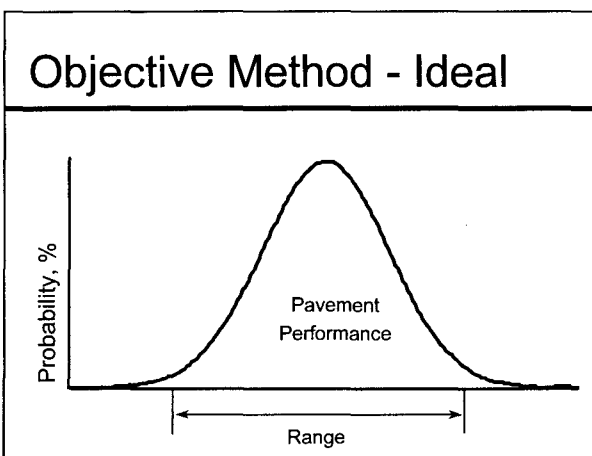
28

Describe Uncertainty
<ul style="list-style-type: none">■ Objective Method■ Subjective Method

29

Objective Method
<ul style="list-style-type: none">■ Historical data<ul style="list-style-type: none">● BAMS● PMS● Research studies● Other■ Statistical analysis software<ul style="list-style-type: none">● BestFit, ...

30



31

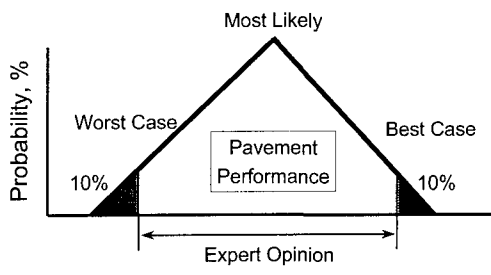
Subjective Method

- Interviews
- Group discussion
 - Facilitated
 - Consensus Building
- Surveys



32

Subjective Method



33

3. Simulate Problem to Obtain Results

- Monte Carlo Simulation ...
- Values are randomly selected from input probability distributions
 - Each randomly selected set is used to determine an outcome
 - Combination of all outcomes form a probability distribution of results

34

Monte Carlo Simulation

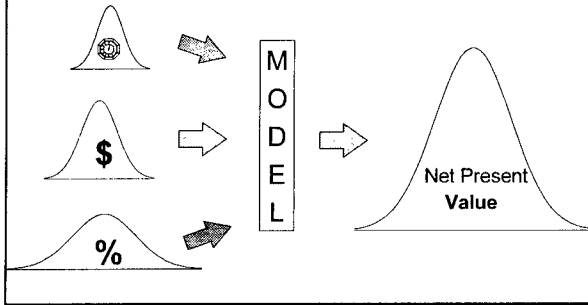
- Rigorous extension of ...
 - best case
 - most likely
 - worst case
- Use empirical or theoretical equations
- Calculation intensive

35

Monte Carlo Simulation

Inputs

Results



36

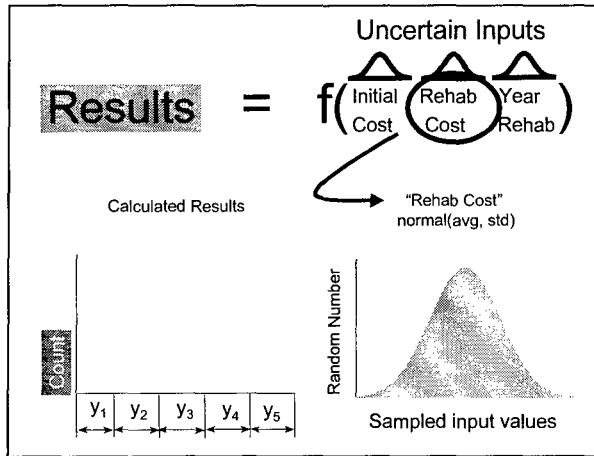
$$NPV = \text{Initial Cost} + \sum \text{Future Cost} \times \left[\frac{1}{(1+i)^n} \right]$$



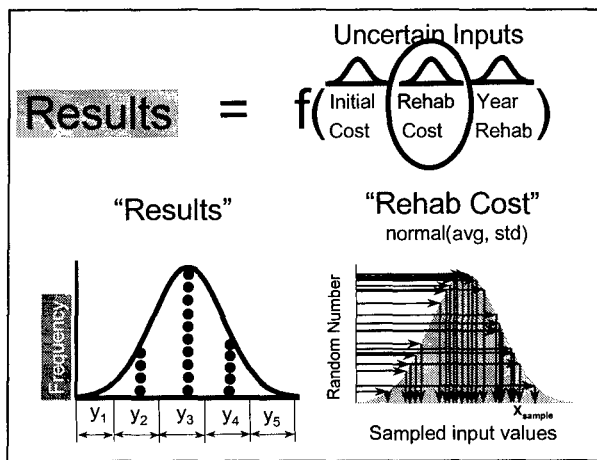
Uncertain Inputs

Results = f ($\begin{matrix} \text{Initial} & \text{Rehab} & \text{Year} \\ \text{Cost} & \text{Cost} & \text{Rehab} \end{matrix}$)

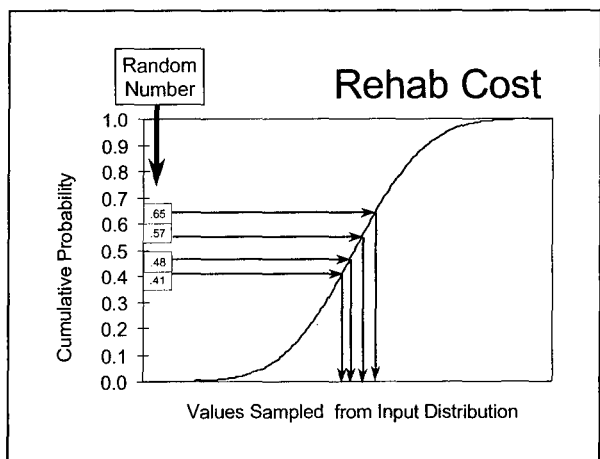
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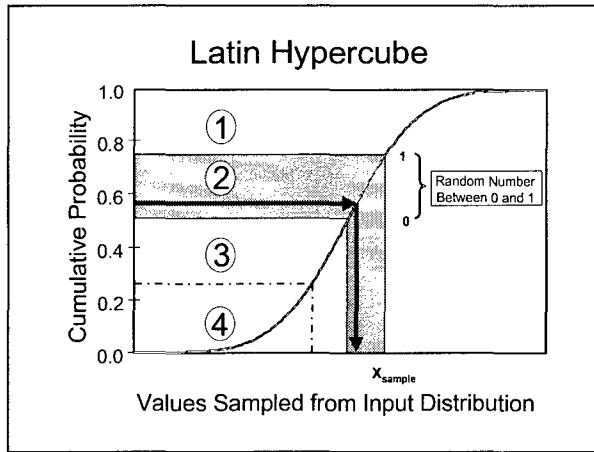
38



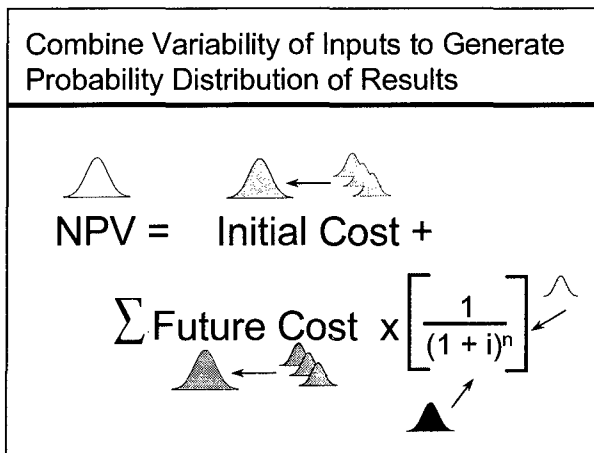
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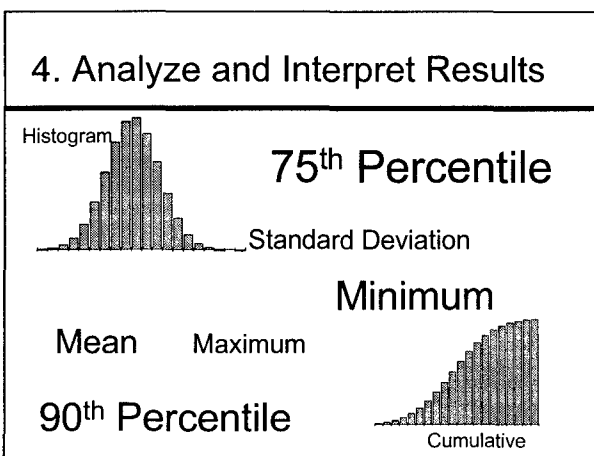
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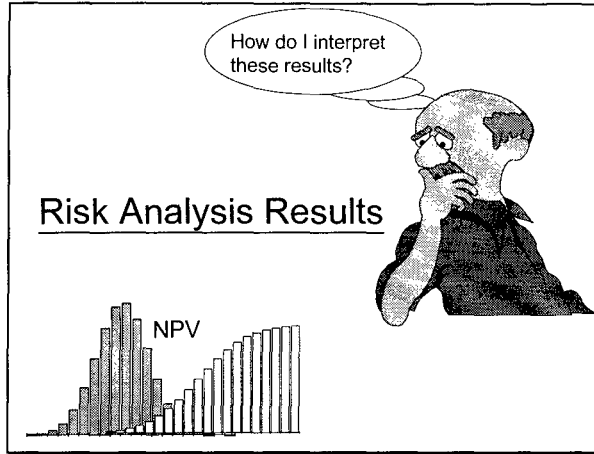
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42



43

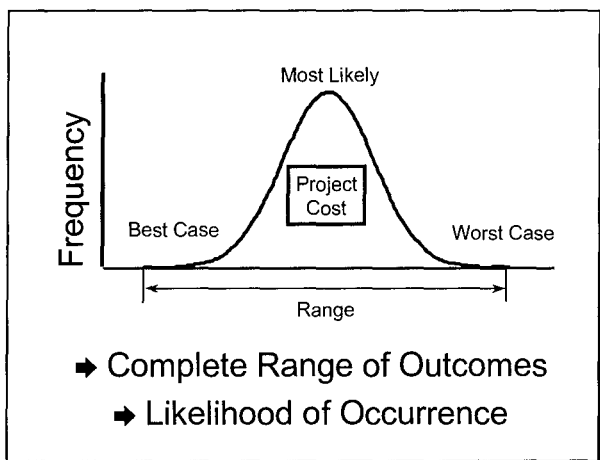


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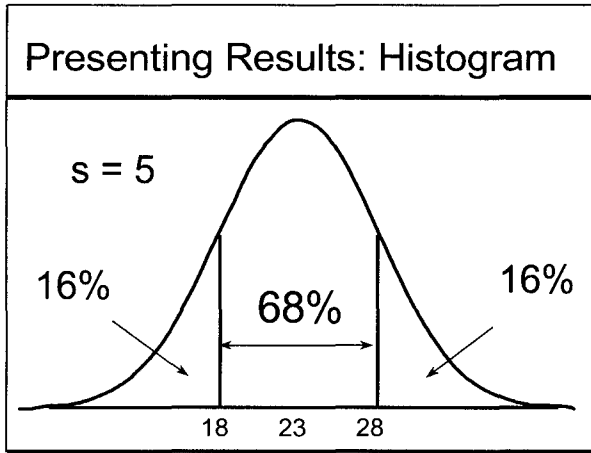
Probabilistic results provide ...

- ① Distribution of “complete” range of outcomes
- ② Probability of occurrence

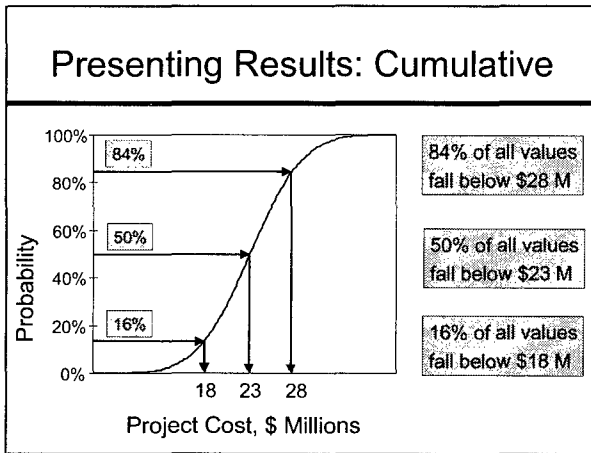
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46



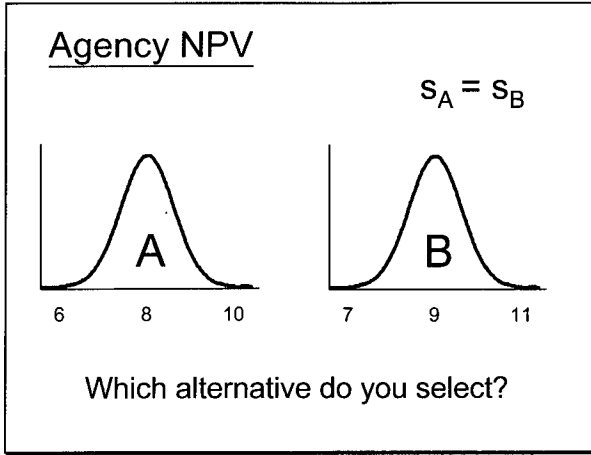
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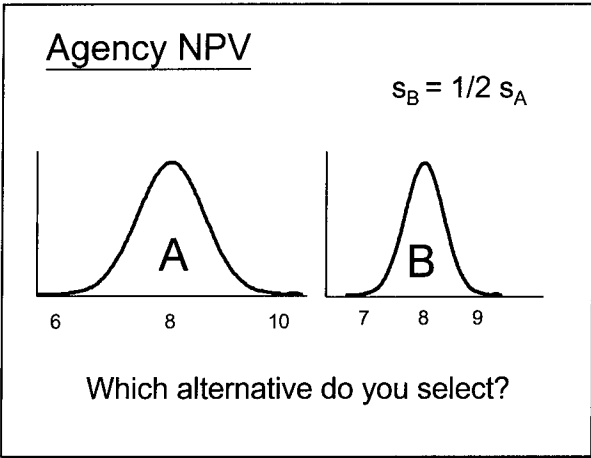
48

Examples

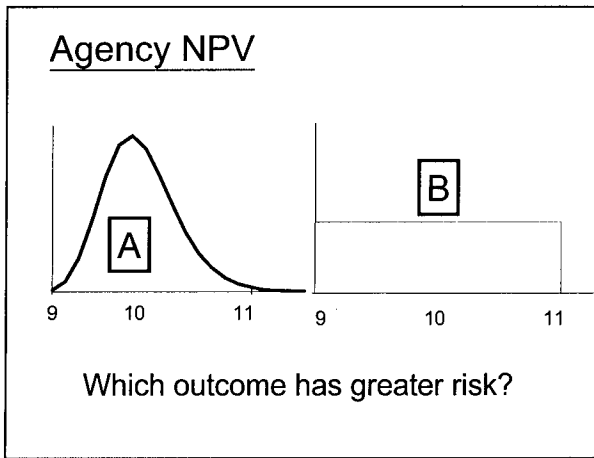
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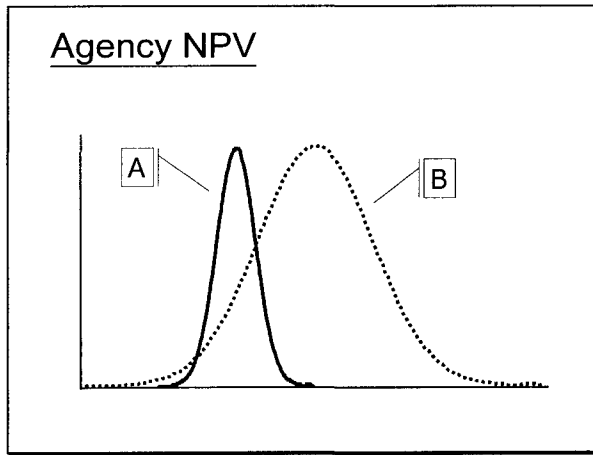
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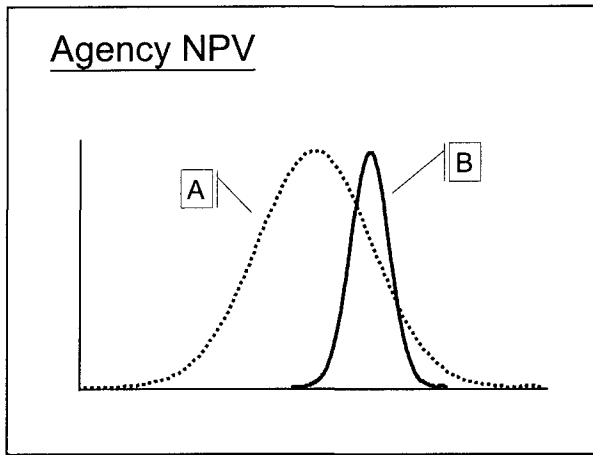
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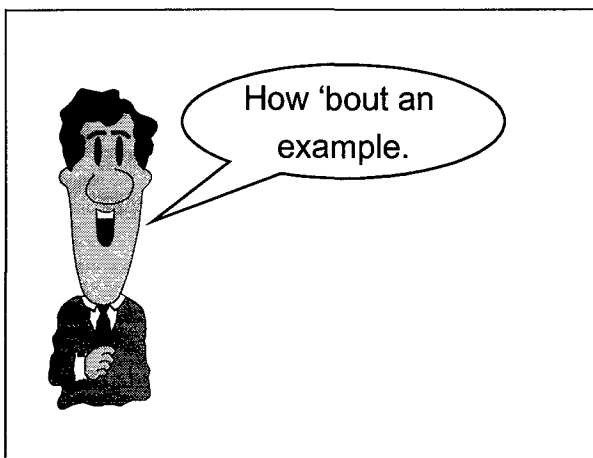
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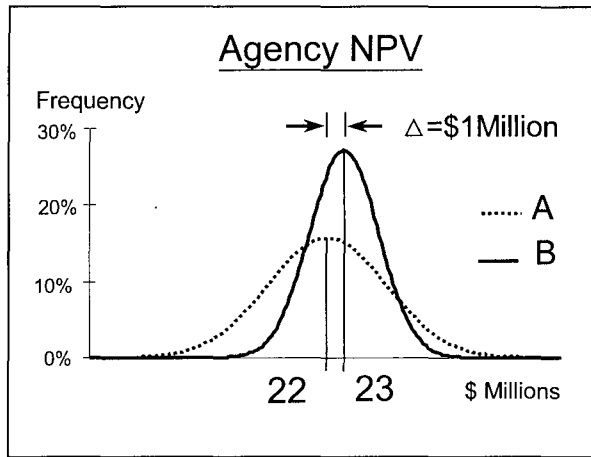
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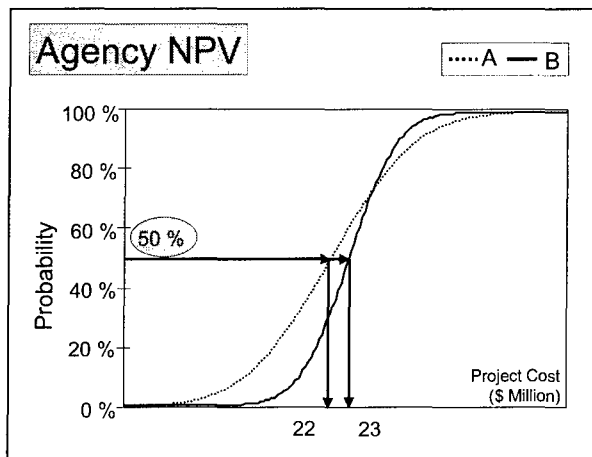
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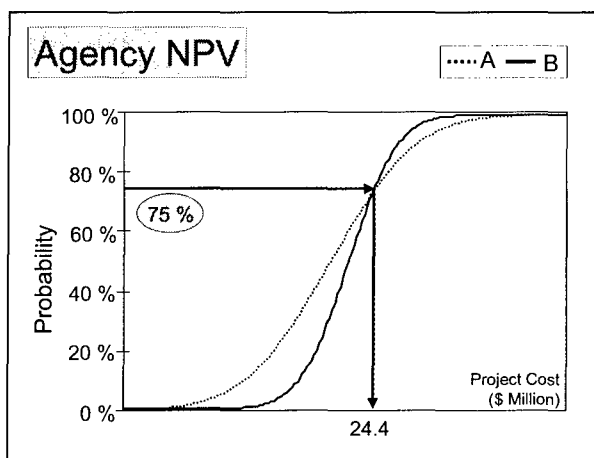
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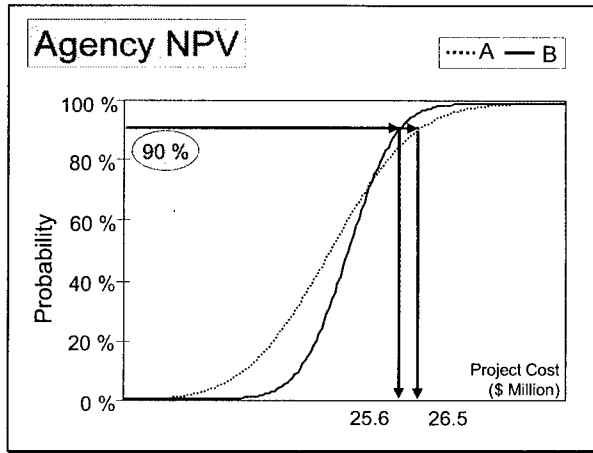
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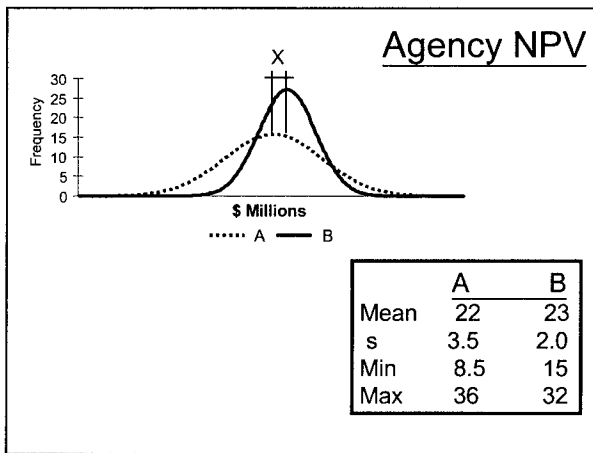
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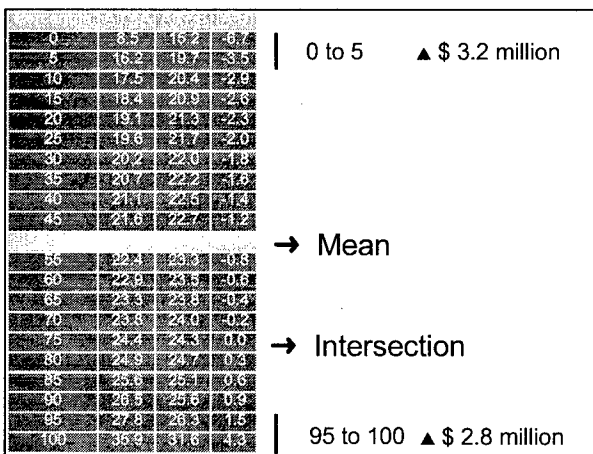
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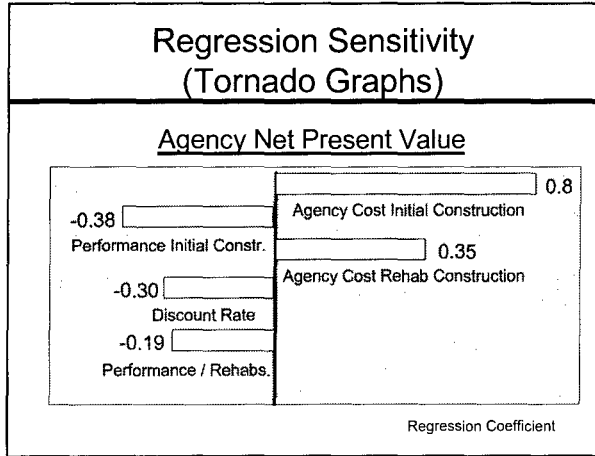
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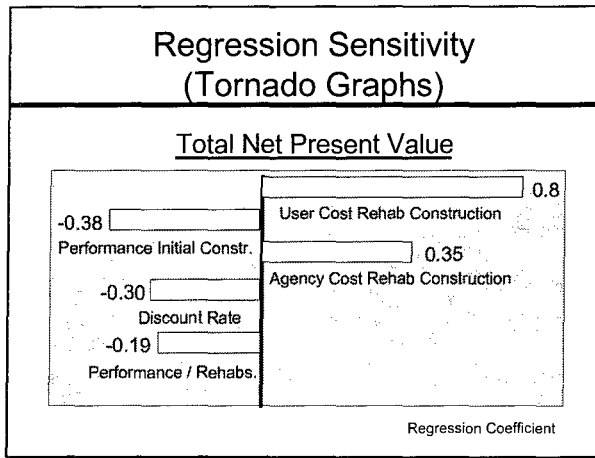
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61



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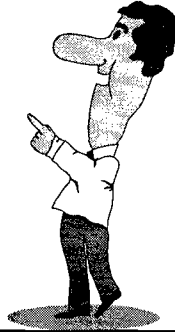
63

- ### Scenario Analysis
- Examine changes in policy variables
 - Example:
 - Closing down a lane of traffic versus
 - Keeping traffic lane open
 - Develop better alternatives

64

5. Make Consensus Decision

- Decisions about ...
 - Strategic Planning
 - Resource Allocation
 - Timing of Investments



65

Risk Analysis Approach (Review)

- Identify structure and logic of problem
- Quantify assumptions w/ probabilistic descriptions of uncertain variables
- Simulate problem to obtain results
- Analyze and interpret results
- Make consensus decision

66

Applications



- Investment & new product analysis



New Drug Research



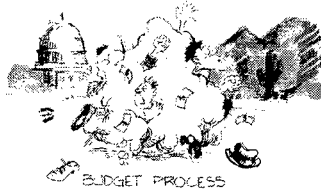
American Stock Exchange

67

Applications

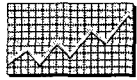


- Investment & new product analysis
- Capital budgeting

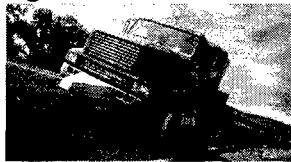


68

Applications



- Investment & new product analysis
- Capital budgeting
- Performance specifications



WesTrack

69

Applications




- Investment & new product analysis
- Capital budgeting
- Performance specifications
- Quality control




70

Applications




- Investment & new product analysis
- Capital budgeting
- Performance specifications
- Quality control
- Traffic flow analysis


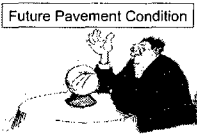


71

Applications



- Investment & new product analysis
- Capital budgeting
- Perf. spec. & quality control
- Traffic flow analysis
- Engineering design

72

Example: Flexible Pavement Design

$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN + 1) - 0.2$$

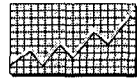
$$+ \frac{\log_{10} \Delta PSI}{4.2 - 1.5}$$

$$+ \frac{1094}{0.40 + (SN + 1)^{5.19}}$$

$$+ 2.32 \times \log_{10}(M_R) - 8.07$$

73

Disadvantages

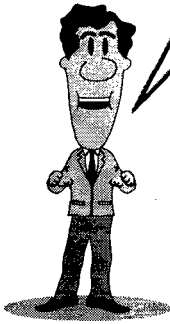


- Computer intensive
 - Proprietary software
 - Complex models
- Requires some statistical background
- Requires “buy-in” of risk management by senior executives



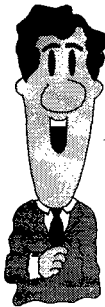
74

This won't work!
You've got to have data to develop those distributions.



75

Its easier to estimate a “range” than a single “point” value.



76

Advantages



- Quantify risk
- Provide decisionmaker the opportunity to take mitigating action
- Justify budget requests, pavement structural designs, ...
- Scenario analysis to create better alternatives

77

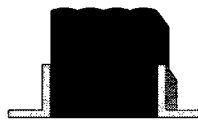
Advantages Con't



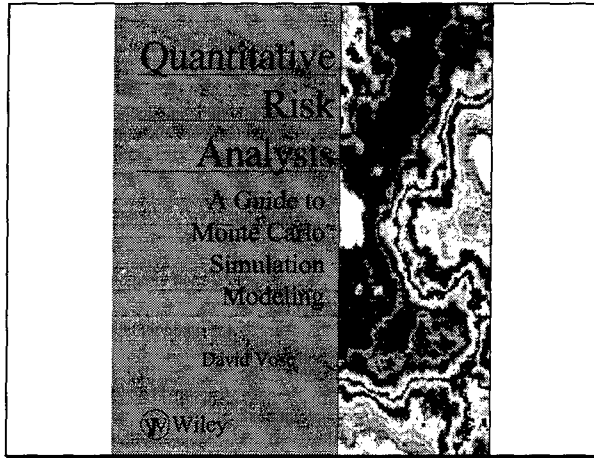
- Elevates the LCCA debate ...
 - From validity of results
 - To what is "our" best policy

78

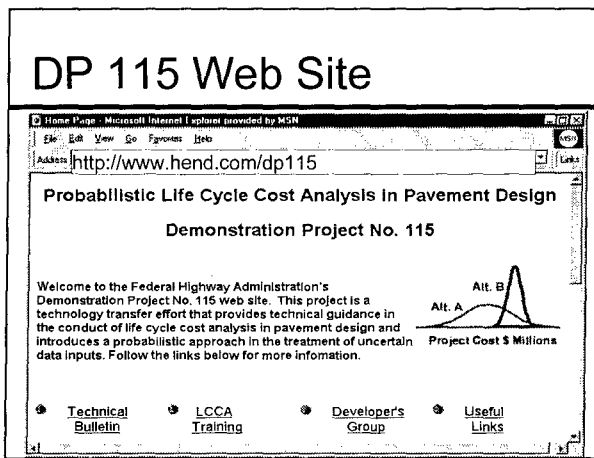
Additional Resources



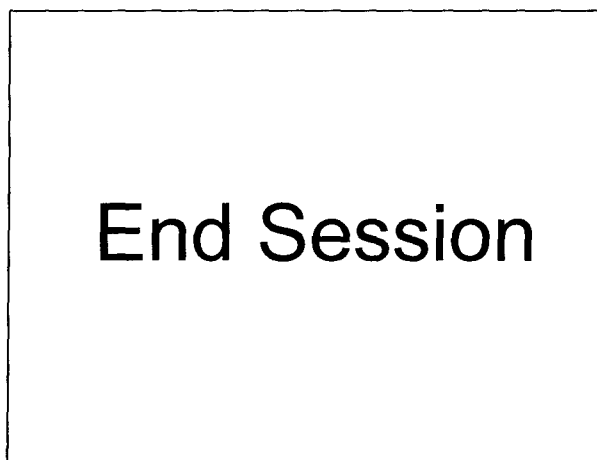
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80

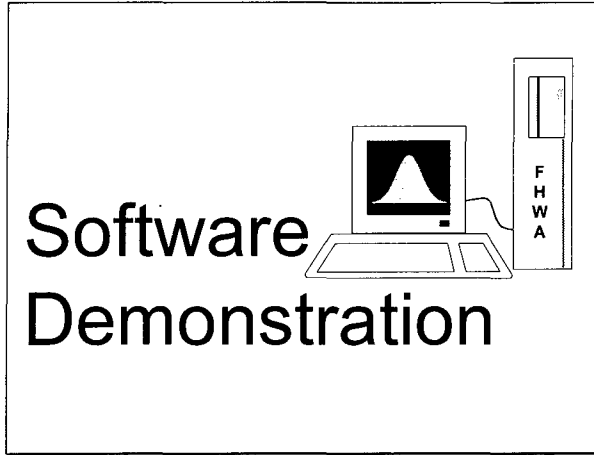


81





1



2

Session Overview

Part 1

- Adding Uncertainty to Models

Part 2

- Running a Risk Analysis
- @Risk Results

3

	A	B	C	D	E
1	Net Present Value				
2				Discounted	
3	ITEM	Year	Cost	Cost	
4	Initial Construction	0	\$ 120.0	\$ 120.0	
5	Rehab	30	\$ 30.0	\$ 12.4	
6	Salvage	35	\$ (15.0)	\$ (5.3)	
7			Net Present Value	\$ 127.0	
8					
9	Assumptions:				
10	Discount Rate		3%		
11	Initial Life		30 Years		
12	Rehab Life		10 Years		

4

ITEM	Year	Cost	Discounted Cost
Initial Construction	0	\$ 120.0	\$ 120.0
Rehab	25	\$ 30.0	\$ 11.3
Salvage	35	\$ (10.0)	\$
Net Present Value			\$ 131.3

ITEM	Year	Cost	Discounted Cost
Initial Construction	0	\$ 120.0	\$ 120.0
Rehab	25	\$ 30.0	\$ 12.4
Salvage	35	\$ (15.0)	\$ (5.3)
Net Present Value			\$ 127.0

ITEM	Year	Cost	Discounted Cost
Initial Construction	0	\$ 120.0	\$ 120.0
Rehab	25	\$ 30.0	\$ 11.3
Salvage	35	\$ (10.0)	\$
Net Present Value			\$ 123.0

5

NPV = \$132 NPV = \$150 NPV = \$97 NPV = 123
 NPV = \$76 NPV = \$200 NPV = \$156 NPV =
 \$148 NPV = \$45 NPV = \$187 NPV = (\$156)
 NPV = \$286 NPV = \$98 NPV = \$34 NPV = \$165
 NPV = \$129 NPV = \$47 NPV =
 \$27 NPV = \$175 NPV =
 = \$145 = (\$94)
 NPV = \$4) NPV =
 \$190 NPV = \$167 NPV = \$10 NPV = \$132 NPV
 = \$88 NPV = \$98 NPV = \$44 NPV = \$22 NPV =
 \$50 NPV = \$65 NPV = \$90 NPV = \$22 NPV =
 \$50 NPV = \$65 NPV = \$90

Help!

6

Solution: Modeling Uncertainty with Probability

@Risk works with Excel or Lotus

- How to add risk analysis to spreadsheet models
- How to use built-in @Risk functions

7

.Net Present Value					
Alternative A Samples					
Pavement Life	20.0	5.0	5.0	5.0	5.0
Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
Discount Rate, (%)	4				
	Initial	Rehab	Rehab	Rehab	
Year >>>	0	20	25	30	35
Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	
Present Worth Factor	1.0000	0.4564	0.3751	0.3083	
Agency Cost (Present Worth)	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894	
Total NPV (Agency Cost)	\$22,486,723				

8

Adding Variability to Spreadsheet Models

- Identify uncertain variables

9

Alternative A Samples

	Initial	Rehab	Rehab	Rehab	
Pavement Life	20.0	5.0	5.0	5.0	5.0
Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$4,375,000	\$ 4,375,000
Discount Rate, (%)	4				
Year >>>	0	20	25	30	35
Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$4,375,000	
Present Worth Factor	1.0000	0.4564	0.3751	0.3083	
Agency Cost (Present Worth)	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$1,348,894	
Total NPV (Agency Cost)	\$22,486,723				

What are the Uncertain Variables?

10

Adding Variability to Spreadsheet Models

- Identify uncertain variables
- Describe uncertain variables as probability distributions

11

Adding Variability to Spreadsheet Models

- Identify uncertain variables
- Describe uncertain variables as probability distributions
- @Risk provides over 30 built-in probability functions

12

@Risk Probability Functions

Beta	Gamma	Normal
Binomial	Geometric	Pareto
Chi-Square	General	Poisson
Cumulative	Histogram	Truncated Exponential
Dependent	Hypergeometric	Truncated Lognormal
Discrete	Independent	Truncated Normal
Discrete Uniform	Logistic	Triangle
Error Function	Lognormal	Trigen
Erlang	Lognormal2	Uniform
Exponential	Negative Binomial	Weibull

13

Functions are Similar to Excel

```
=RiskNormal(A1,A2)
=RiskNormal(3500*B7,C12/3000)
=RiskNormal(RiskLognormal(A1,A2),RiskUniform(1,5))
=If (G7>0,RiskNormal(3500,300),RiskNormal(3500,300*G8))
```

14

Microsoft Excel - SoftwareDemo3.xls

@Risk Input Parameters

Analysis Period 35 years

Variable	.Risk Analysis Input Parameters						Distribution Type
	Lower Estimate	Mean	Upper Estimate	Lower Percentile	Upper Percentile	Standard Deviation	
Discount Rate (%)	3	4	5	0	100		Trigen
Construction Costs							
Initial	\$ 15,000,000	\$ 17,500,000	\$ 20,000,000			\$ 1,250,000	Normal
Rehab	\$ 3,750,000	\$ 4,375,000	\$ 5,000,000			\$ 312,500	Normal
Performance (yrs)							
Initial	16	20	24	0	100		Trigen
Rehab	4	5	6	0	100		Trigen

Variables /

15

Microsoft Excel - SoftwareDemo3.xls

.Net Present Value

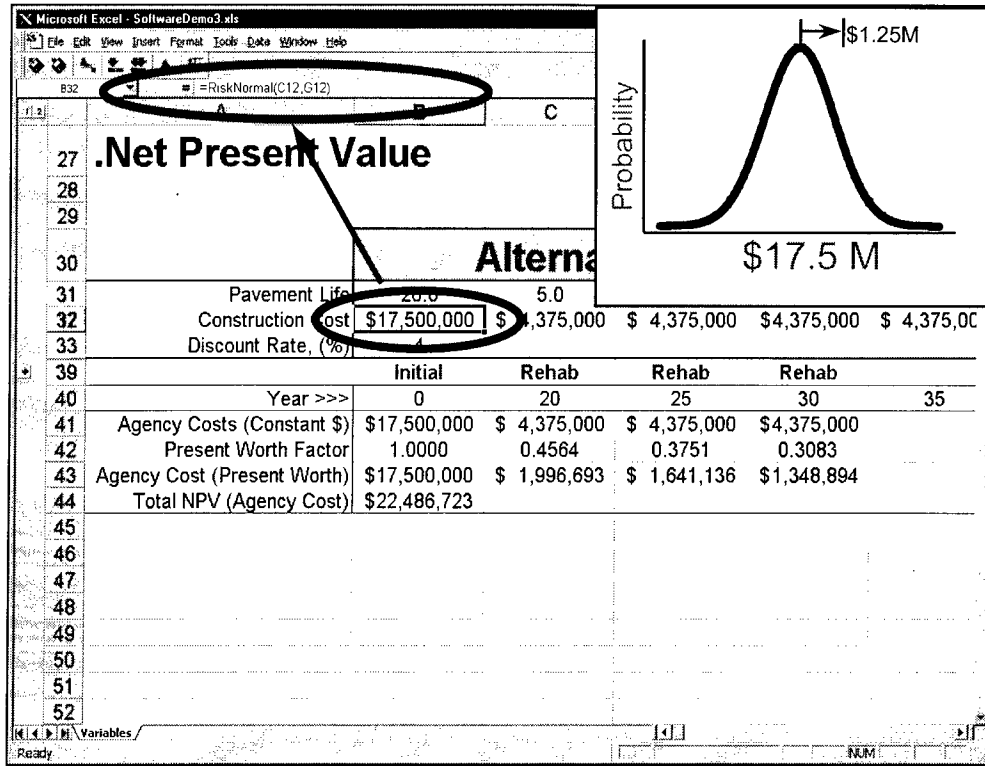
Alternative A Samples

Pavement Life

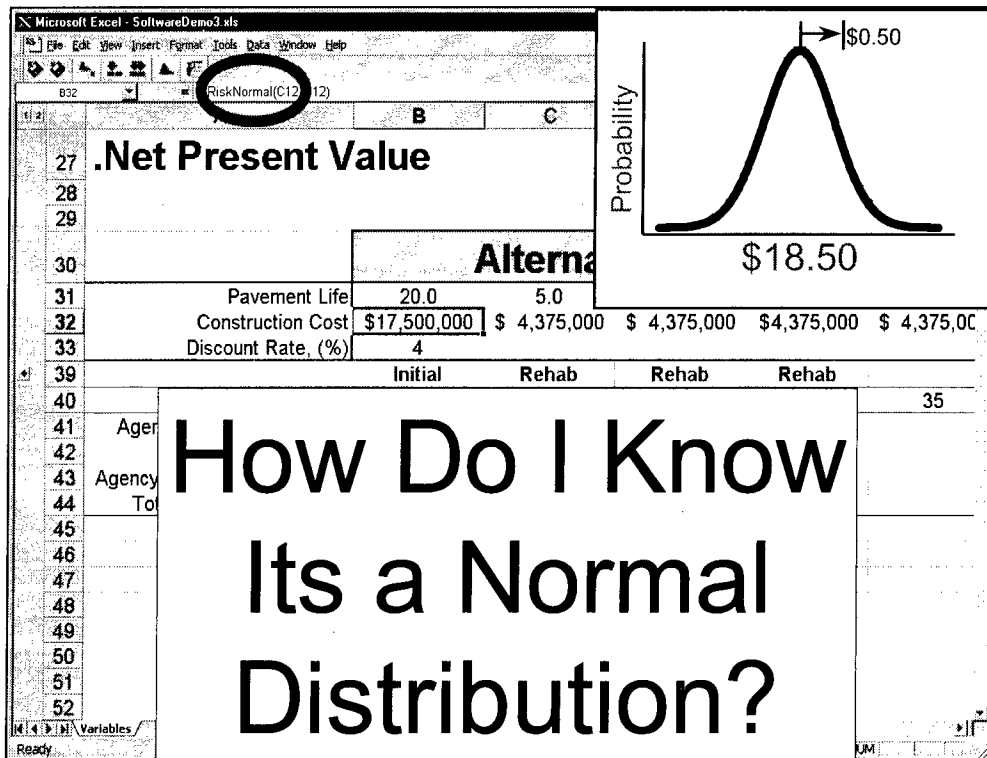
31	Pavement Life	20.0	5.0	0.0	5.0	5.0
32	Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$4,375,000	\$ 4,375,000
33	Discount Rate, (%)	4				
39		Initial	Rehab	Rehab	Rehab	
40	Year >>>	0	20	25	30	35
41	Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$4,375,000	
42	Present Worth Factor	1.0000	0.4564	0.3751	0.3083	
43	Agency Cost (Present Worth)	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894	
44	Total NPV (Agency Cost)	\$22,486,723				

Ready

16



17




18

@Risk Probability Functions

Beta	Gamma	Normal
Binomial	Geometric	Pareto
Chi-Square	<div style="border: 1px solid black; padding: 5px; display: inline-block;"> <p>Which One Should I Use?</p> </div>	
Cumulative		
Dependent	Exponential	Lognormal
Discrete	Lognormal	Trigen
Discrete	Lognormal2	Uniform
Error Function	Negative Binomial	Weibull
Erlang		
Exponential		

19

@Risk & BestFit 

- Based on historical data input distribution models are developed using BestFit
- BestFit automatically determines the "bestfit" probability distribution
- Distribution model is "copied" directly into @Risk/Excel spreadsheet

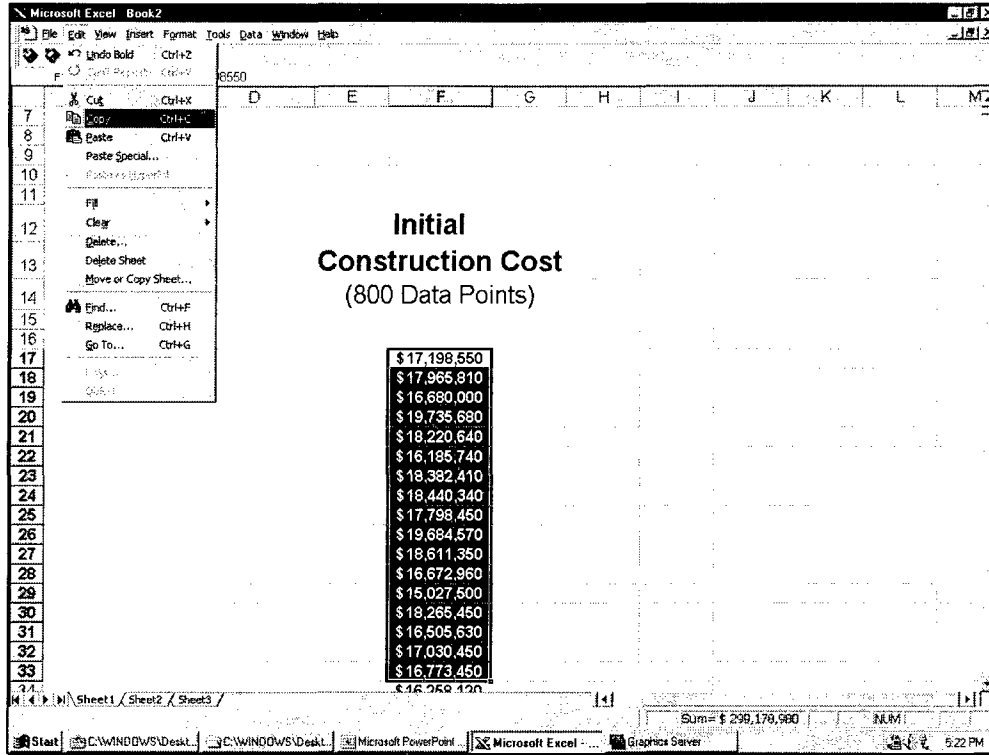
20

Row	Value
17	\$17,198,550
18	\$17,965,810
19	\$16,680,000
20	\$19,735,680
21	\$18,220,640
22	\$16,185,740
23	\$18,382,410
24	\$18,440,340
25	\$17,798,450
26	\$19,684,570
27	\$18,611,350
28	\$16,672,960
29	\$15,027,500
30	\$18,265,450
31	\$16,505,630
32	\$17,030,450
33	\$16,773,460

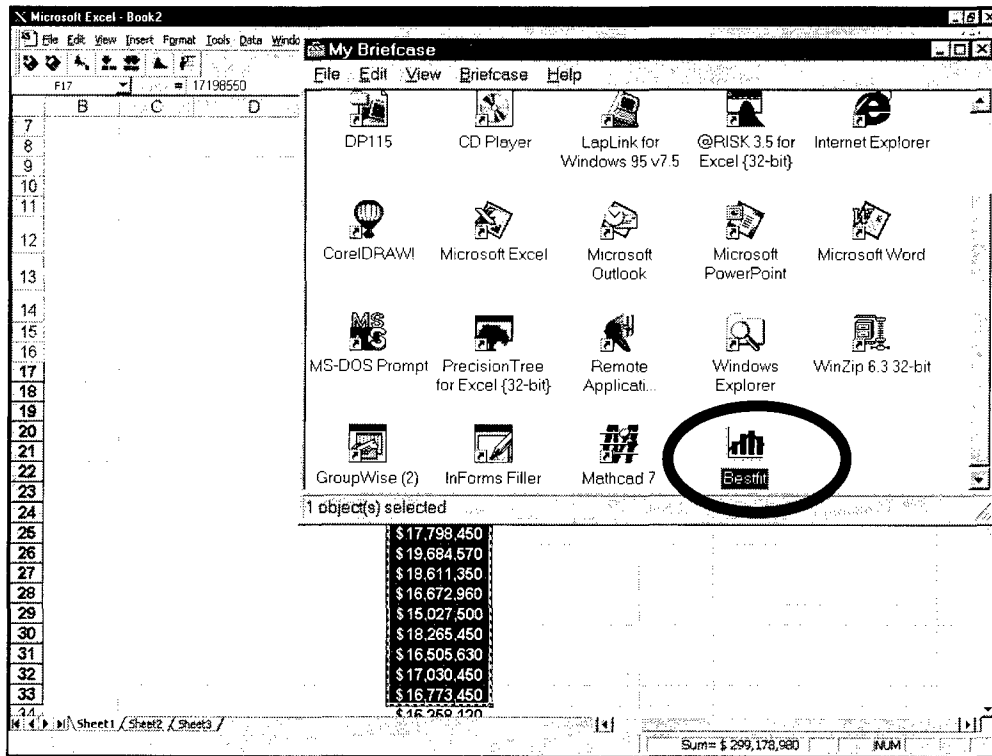
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Row	Value
17	\$17,198,550
18	\$17,965,810
19	\$16,680,000
20	\$19,735,680
21	\$18,220,640
22	\$16,185,740
23	\$18,382,410
24	\$18,440,340
25	\$17,798,450
26	\$19,684,570
27	\$18,611,350
28	\$16,672,960
29	\$15,027,500
30	\$18,265,450
31	\$16,505,630
32	\$17,030,450
33	\$16,773,460

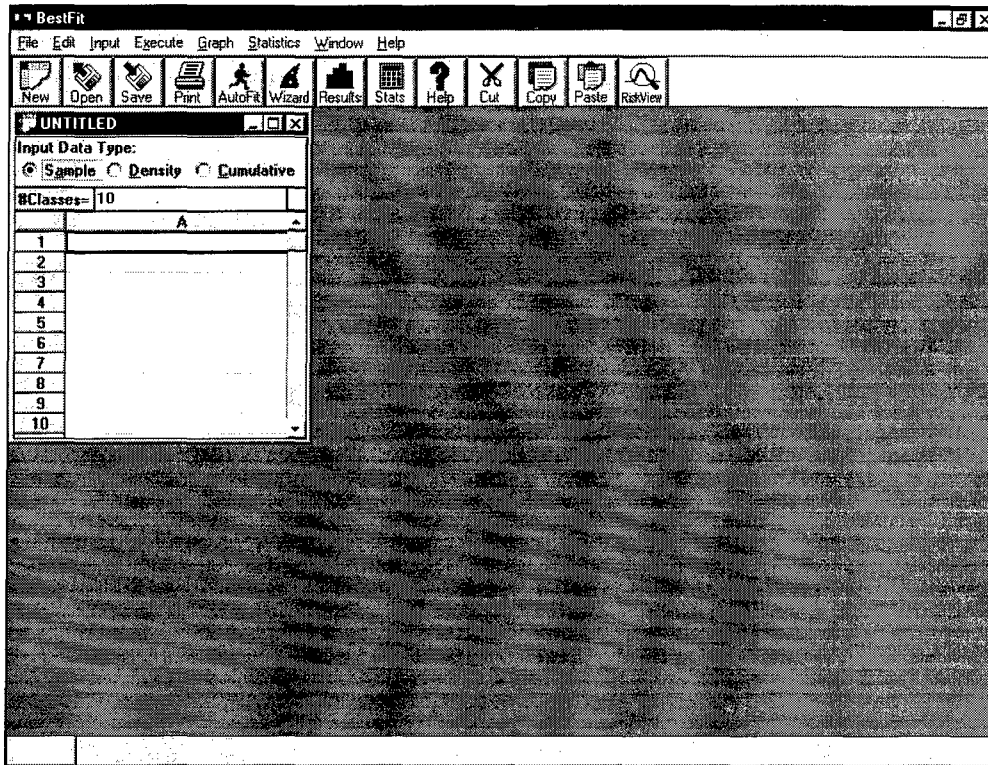
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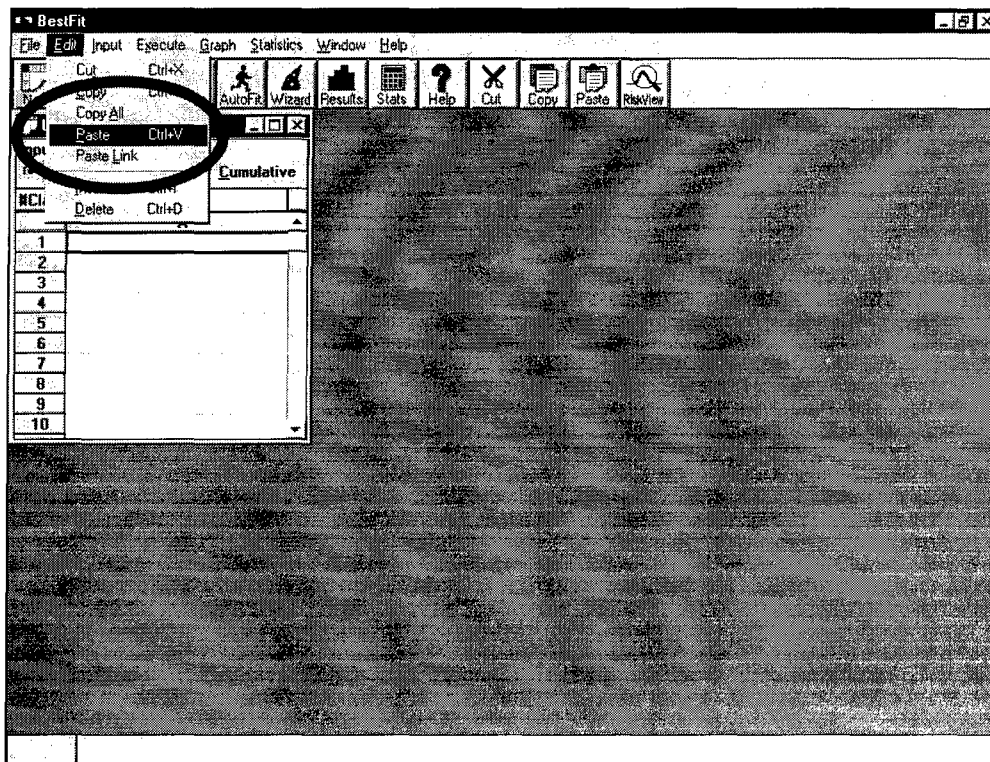
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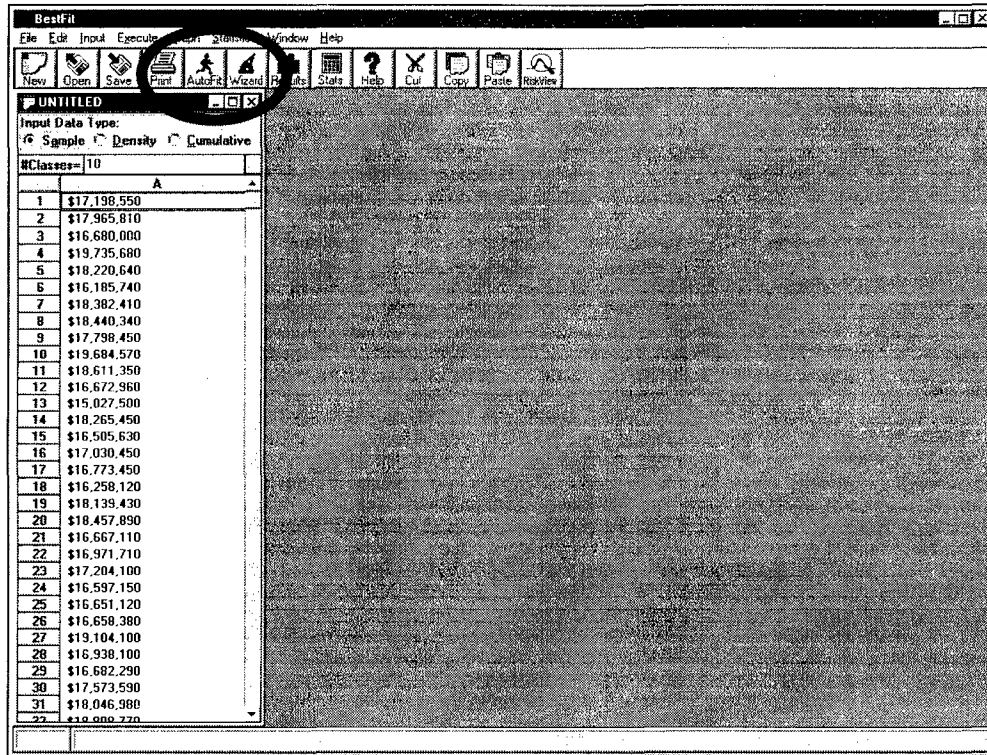
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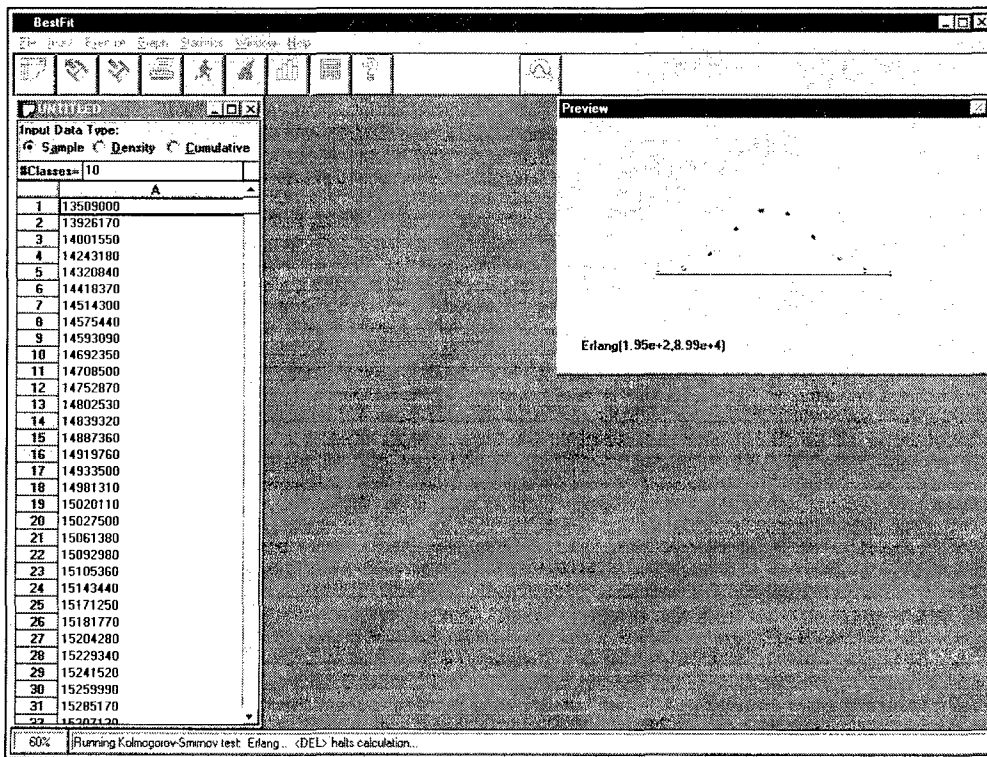
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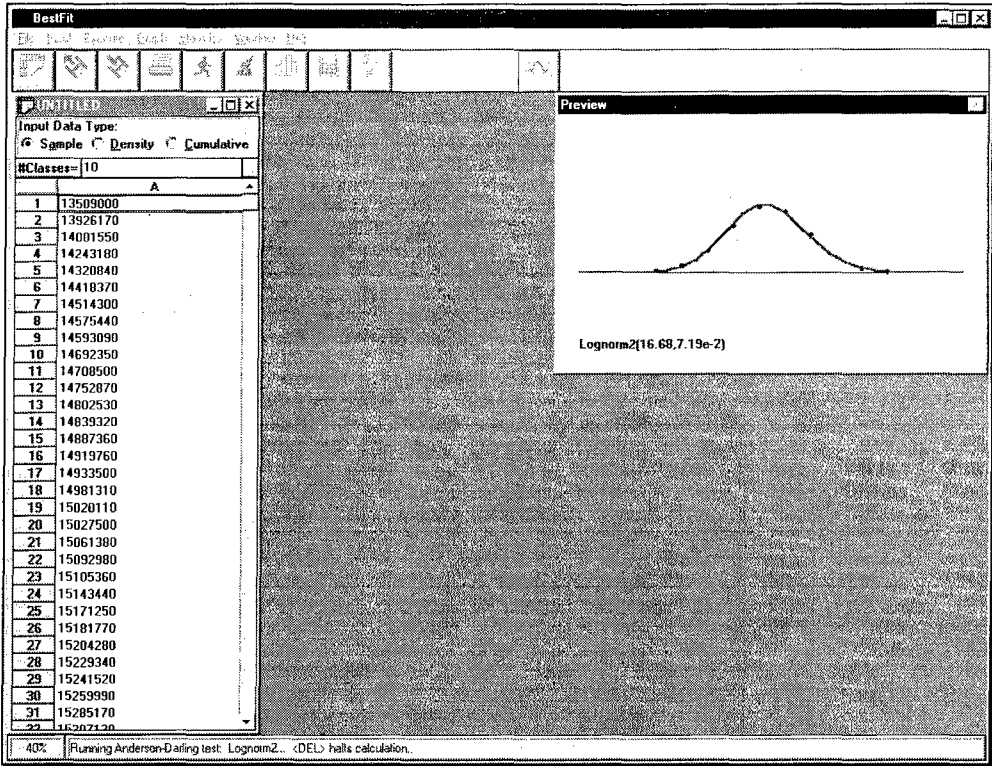
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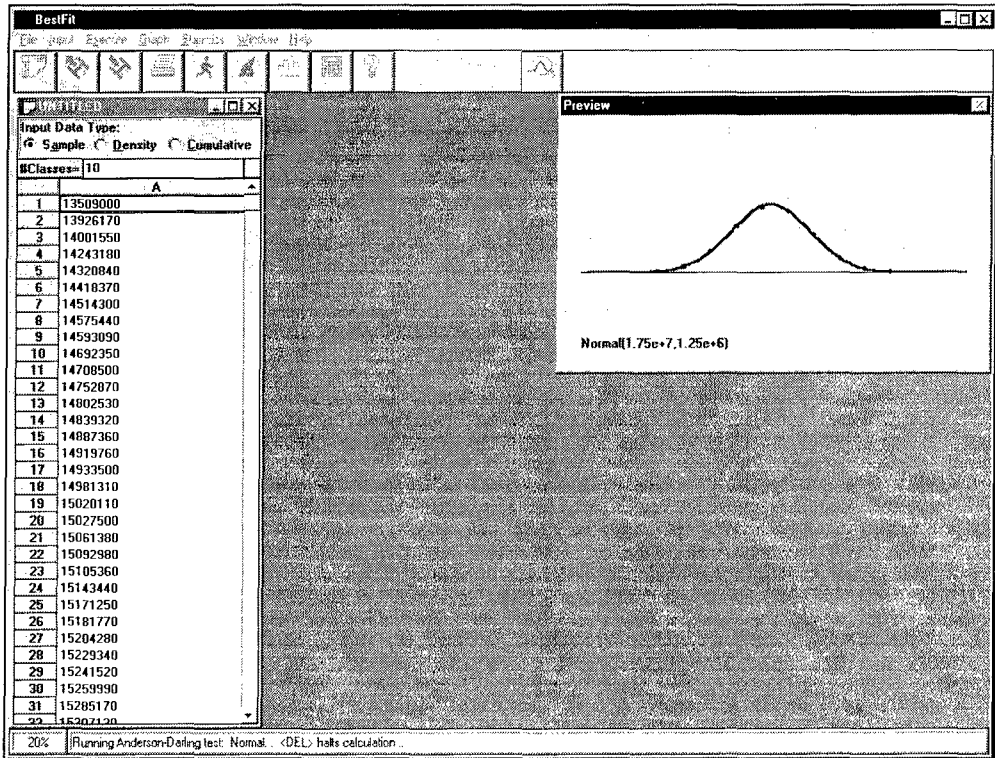
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29



30

The screenshot shows the BestFit software interface. On the left, the 'Input Data Type' is set to 'Sample' with 10 classes. A list of 31 data points is shown in column 'A'. On the right, the 'Results' dialog box is open, showing a list of distributions: Normal, Gamma, Erlang, Lognorm, Lognorm2, InverseGaussian, Logistic, Weibull, Beta, and Trunc. The 'Normal' distribution is selected. The 'Display' options include Comparison, Difference, P-P, Q-Q, Chi-Square, K-S Test, and A-D Test. The 'Graph' button is checked.

31

The screenshot shows the 'Statistics: UNTITLED' window in BestFit. The 'Rank/Distribution' table shows the 'Normal' distribution selected with a Chi-Square value of 1.51046. Below this, a table compares the Normal distribution with other distributions (Gamma, Lognorm, Lognorm2, InverseGaussian, Beta) across various statistical parameters. A callout bubble points to the 'Normal(17.5M, 1.25M)' result. Another callout bubble says 'Cut and Paste the "Best" Result' pointing to the Normal distribution's parameters.

Parameter	Normal	Gamma	Lognorm	Lognorm2	InverseGaussian	Beta
Parameter 1	1.750053e+7	134.555873	1.750075e+7	16.675171	1.750053e+7	4.819018
Parameter 2	1.251135e+6	8.930497e+4	1.259617e+6	0.071882	3.378109e+9	5.117524
Parameter 3						
Formula	Normal(1.75e+7, 1.25e+6)	Gamma(1.95e+2, 8.99e+4)	Lognorm(1.75e+7, 1.25e+6)	Lognorm(216.68, 7.19e-2)	InverseGaussian(1.75e+7, 3.378109e+9)	Beta(4.82, 5.117524)
Minimum	1.3509e+7					1.348547e
Maximum	2.174085e+7					2.175433e
Mean	1.750053e+7	1.75005e+7	1.750075e+7	1.750075e+7	1.750053e+7	1.750053e
Mode	1.721332e+7	1.741063e+7	1.736584e+7		1.736506e+7	1.746919e
Median	1.749855e+7	1.750053e+7	1.747057e+7			1.748186e
Standard Deviation	1.251135e+6	1.251135e+6	1.254346e+6			1.251135e
Variance	1.565341e+12	1.565341e+12	1.573385e+12			1.565341e
Skewness	0.010235	0.0	0.14335			0.023318
Kurtosis	3.987764	3.0	3.030824			2.553302
Histogram	Histogram(1.3509e+7, 2.174085e+7)					
Minimum	1.3509e+7	1.3509e+7	1.3509e+7			1.3509e+7
Maximum	2.174085e+7	2.174085e+7	2.174085e+7			2.174085e
P1	5.0	3.502179	2.322685	1.650077		0.485232
P2	19.0	18.533158	16.650762	15.713029	15.713029	18.104515
P3	65.0	63.61437	65.206776	66.011069	66.011069	74.065156
P4	142.0	141.630145	148.47077	151.915063	151.915063	147.02752
P5	201.0	204.527067	207.26631	208.354558	208.354558	192.27457
P6	189.0	191.575792	185.752819	182.676196	182.676196	181.15368
P7	116.0	116.392525	111.15511	108.590269	108.590269	122.35042
P8	47.0	45.86741	45.977142	45.995399	45.995399	53.454421
P9	13.0	11.724059	13.549369	14.482022	14.482022	10.835543
P10	3.0	1.943777	2.521823	3.514174	3.514174	0.208826
#Classes	10.0					
Interval Width	8.23185e+5					
Results						
Chi-Square						
Test Value	1.51046	4.205913	7.946169	7.946169	8.050472	81.88638

32

Microsoft Excel - SoftwareDemo3.xls

File Edit View Insert Format Tools Data Window Help

B32 =RiskNormal(17500000,1250000)

27	.Net Present Value					
28			Paste Results Here.			
29						
30			Alternative A Samples			
31	Pavement Life	20.0	5.0	5.0	5.0	5.0
32	Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
33	Discount Rate, (%)	4				
39		Initial	Rehab	Rehab	Rehab	
40	Year >>>	0	20	25	30	35
41	Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	
42	Present Worth Factor	1.0000	0.4564	0.3751	0.3083	
43	Agency Cost (Present Worth)	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894	
44	Total NPV (Agency Cost)	\$22,486,723				

Variables /

Select destination and press ENTER or choose Paste

33

Microsoft Excel - SoftwareDemo3.xls

File Edit View Insert Format Tools Data Window Help

B32 =RiskNormal(17500000,1250000)

27	.Net Present Value					
28			=RiskNormal(17.5M,1.25M)			
29						
30			Alternative A Samples			
31	Pavement Life	20.0	5.0	5.0	5.0	5.0
32	Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
33	Discount Rate, (%)	4				
39		Initial	Rehab	Rehab	Rehab	
40	Year >>>	0	20	25	30	35
41	Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	
42	Present Worth Factor	1.0000	0.4564	0.3751	0.3083	
43	Agency Cost (Present Worth)	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894	
44	Total NPV (Agency Cost)	\$22,486,723				

Variables /

Select destination and press ENTER or choose Paste

34

Running a Risk Analysis

35

The screenshot shows a spreadsheet application window titled 'SoftwareDemo3.xls'. The spreadsheet contains a table with the following data:

	A	B	C	D	E	F
27	.Net Present Value					
28						
29						
30						
31	Pavement Lif					0
32	Construction Cos					5.00
33	Discount Rate, (%)					
39						
40	Year >>					5
41	Agency Costs (Constant \$					
42	Present Worth Facto					
43	Agency Cost (Present Worth					
44	Total NPV (Agency Cos					
45						
46						
47						
48						
49						
50						
51						
52						

The 'Simulation Settings' dialog box is open, showing the following configuration:

- Iterations: 150
- Simulations: 1
- Each Iteration:
 - Allow Multitasking
 - Pause on Error
 - Update Display

A 'Control Panel' label with an arrow points to the dialog box. The spreadsheet status bar at the bottom shows 'Variables / Select destination and press ENTER or choose Paste'.

36

Simulation Settings

Iterations | Sampling | Convergence | **Macro** | External

Execute Macro

Execute Macro?

Macro name:

Macro Executes When?

Before simulation

Before sampling/ worksheet recalc

After sampling/ worksheet recalc

After simulation

OK Cancel

37

Define Cell Reference

Define Total NPV as Output

		Alternative A Samples				
Year >>>		Initial	Rehab	Rehab	Rehab	
Pavement Life		20.0	5.0	5.0	5.0	5.0
Construction Cost		\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
Discount Rate, (%)		4				
Agency Costs (Constant \$)		\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
Present Worth Factor		1.0000	0.4564			
Agency Cost (Present Worth)		\$17,500,000	\$ 1,996,693			
Total NPV (Agency Cost)		\$22,486,723				

38

Net Present Value

GO!!!! Alternative A Samples

	Initial	Rehab	Rehab	Rehab
Pavement	5.0	5.0	5.0	5.0
Construction Cost	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
Discount Rate, (%)	4			
Year >>>	0	20	25	30
Agency Costs (Constant \$)	\$17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
Present Worth Factor	1.0000	0.4564	0.3751	0.3083
Worth	\$17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894
Cost	\$22,486,723			

Simulating...

Sims: 1
 # Iters: 1000
 Sim #: 1
 Iter #: 750

76%
 Cancel

39

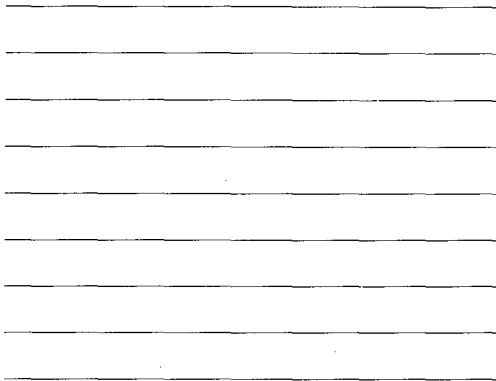
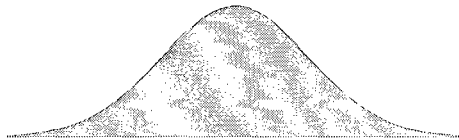
Simulation Processing

- Latin Hypercube
- 24 Input Variables
- 1 Output Variables
- 10,000 Iterations
- Run Time = 1 minute 15 seconds

40

Simulation Results

- Results from each iteration are stored and presented as probability distributions.



41

Inputs and Outputs

Simulation

Simulation #1 of SoftwareDemo3.XLS
Iterations = 10000 Simulations = 1
Input Variables = 24
Output Variables = 1
Sampling Type = Latin Hypercube
Runtime = 00:01:15
Run on 6/14/2001 11:54 AM

Summary of Results

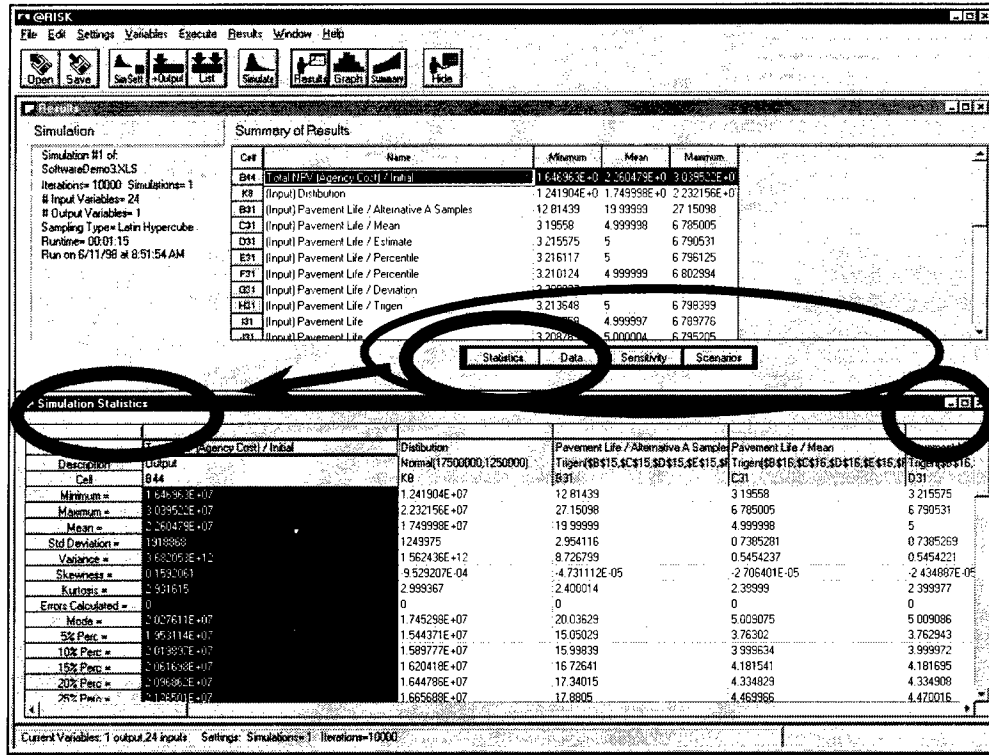
Cell	Min	Maximum	Mean
B44 Total NPV (Agency Cost) / Initial	1.646963E+0	3.039522E+0	2.260479E+0
K8 (Input) Distribution	1.241904E+0	2.232156E+0	1.749998E+0
B31 (Input) Pavement Life / Alternative A Samples	12.81439	27.15098	19.99999
C31 (Input) Pavement Life / Mean	3.19558	6.785005	4.999998
D31 (Input) Pavement Life / Estimate	3.215575	6.790531	5
F31 (Input) Pavement Life / Percentile	3.216117	6.796125	5
F31 (Input) Pavement Life / Percentile	3.210124	6.802994	4.999999
G31 (Input) Pavement Life / Deviation	3.209937	6.795606	4.999998

Simulation Statistics

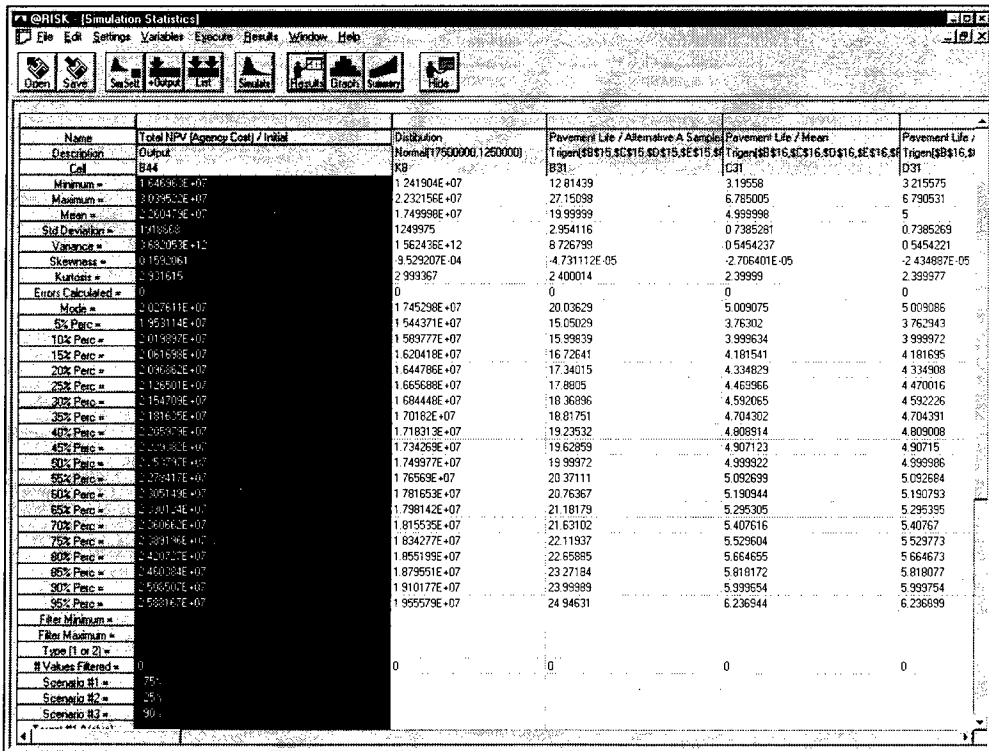
Name	Total NPV (Agency Cost) / Initial	Distribution	Pavement Life / Alternative A Samples	Pavement Life / Mean
Description	Output	Normal(1.7500000,1.250000)	Trigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15)	Trigen(\$B\$16,\$C\$16,\$D
Cell	B44	K8	B31	C31
Minimum =	1.646963E+07	1.241904E+07	12.81439	3.19558
Maximum =	3.039522E+07	2.232156E+07	27.15098	6.785005
Mean =	2.260479E+07	1.749998E+07	19.99999	4.999998
Std Deviation =	1.918868	1.249975	2.954116	0.7385281
Variance =	3.682053E+12	1.562436E+12	8.726799	0.5454237
Skewness =	0.1592061	-9.529207E-04	-4.731112E-05	-2.706401E-05
Kurtosis =	2.931615	2.999367	2.400014	2.39999
Errors Calculated =	0	0	0	0
Mode =	2.027611E+07	1.745298E+07	20.03629	5.009075
5% Perc =	1.953114E+07	1.544371E+07	15.05029	3.76302

Current Variables: 1 output, 24 inputs Settings: Simulations = 1 Iterations = 10000

42



43



44

Simulation Summary

Cell	Name	Minimum	Mean	Maximum
B4	Total NPV (Agency Cost) / Initial	1.64638E+07	2.26047E+07	3.03952E+07
K8	Input Distribution	1.24190E+07	1.74939E+07	2.23215E+07
B31	Input Pavement Life / Alternative A Sample	12.81439	19.99999	27.15098
C31	Input Pavement Life / Mean	3.19558	4.999998	6.795005
D31	Input Pavement Life / Estimate	3.215575	5	6.790531
E31	Input Pavement Life / Percentile	3.215117	5	6.795129
F31	Input Pavement Life / Percentile	3.210124	4.999999	6.802394
G31	Input Pavement Life / Deviation	3.208937	4.999998	6.799606
H31	Input Pavement Life / Trigen	3.213648	5	6.792399
I31	Input Pavement Life	3.207358	4.999997	6.799376
J31	Input Pavement Life	3.215575	5	6.795005

Simulation Statistics

Description	Output	Distribution	Pavement Life / Alternative A Sample	Pavement Life / Mean	Pavement Life / Estimate
Cell	B4	Normal(17500000,1250000)	Trigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15)	Trigen(\$B\$16,\$C\$16,\$D\$16,\$E\$16,\$F\$16)	Trigen(\$B\$17,\$C\$17,\$D\$17,\$E\$17,\$F\$17)
Minimum	1.64638E+07	K8	931	C31	D31
Maximum	3.03952E+07		12.81439	3.19558	3.215575
Mean	2.26047E+07		2.23215E+07	6.795005	6.790531
Std Deviation	1.918858		1.74939E+07	4.999998	5
Variance	3.68205E+12		1.249375	0.7385281	0.7365269
Skewness	0.1952051		1.56243E+12	0.5454237	0.5454231
Kurtosis	2.931615		9.529207E-04	-4.731112E-05	-2.434887E-05
Errors Calculated	0		2.999367	2.99999	2.999977
Mode	2.02781E+07		0	0	0
5% Perc.	1.953114E+07		1.745298E+07	20.03829	5.009075
10% Perc.	2.019897E+07		1.544371E+07	15.09029	3.76302
15% Perc.	2.061698E+07		1.589772E+07	15.99839	3.762943
20% Perc.	2.08835E+07		1.620418E+07	15.99839	3.999932
25% Perc.	2.125001E+07		1.644786E+07	16.72641	4.181639
			1.683698E+07	17.34015	4.334908
				17.8805	4.469265
					4.470016

45

Simulation Data

Name	Total NPV (Agency Cost) / Initial	Distribution	Pavement Life / Alternative A Sample	Pavement Life / Mean	Pavement Life / Estimate
Description	Output	Normal(17500000,1250000)	Trigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15)	Trigen(\$B\$16,\$C\$16,\$D\$16,\$E\$16,\$F\$16)	Trigen(\$B\$17,\$C\$17,\$D\$17,\$E\$17,\$F\$17)
Iteration# / Cell	B4	K8	B31	C31	D31
1	2.126415E+07	1.797999E+07	20.96728	4.677584	3.887964
2	2.317721E+07	1.681989E+07	18.46149	4.435757	4.841287
3	2.306772E+07	1.862183E+07	22.25286	4.96759	4.708482
4	2.434059E+07	1.711903E+07	18.5422	5.144017	5.116165
5	2.2856E+07	2.002709E+07	18.96022	5.045822	4.022956
6	2.192406E+07	1.693866E+07	23.63166	5.517721	6.158058
7	2.439176E+07	1.650325E+07	14.93987	3.529779	6.07284
8	2.152317E+07	1.705151E+07	20.68883	5.760303	6.510606
9	2.231088E+07	1.677901E+07	21.37864	6.311396	5.289359
10	1.844606E+07	1.877138E+07	25.40576	5.118252	6.678833
11	2.212229E+07	1.911357E+07	19.95281	4.614734	4.926433
12	2.411793E+07	1.442257E+07	17.61503	5.122742	6.287986
13	2.362151E+07	1.812831E+07	22.03385	5.133126	3.883309
14	2.330158E+07	1.86124E+07	18.47469	3.79127	5.31108
15	2.497353E+07	1.819874E+07	18.26341	4.185094	4.363712
16	2.217433E+07	1.988075E+07	21.81091	5.683984	5.204988
17	2.386051E+07	1.893087E+07	17.67594	6.832777	5.4975
18	2.117212E+07	1.924483E+07	23.35806	5.700949	5.315555
19	2.365437E+07	1.607503E+07	16.59205	5.777596	4.365477
20	2.353938E+07	1.650041E+07	19.12016	5.629378	4.69423
21	2.44654E+07	1.675703E+07	19.33757	5.048025	4.97445
22	2.358705E+07	1.507345E+07	21.84931	5.141917	3.411816
23	2.291301E+07	1.862431E+07	20.56302	4.301559	4.168668
24	2.438033E+07	1.675239E+07	20.13086	4.688388	6.11964
25	2.21439E+07	1.845887E+07	20.63653	5.049551	5.386622
26	2.446014E+07	1.890542E+07	21.38361	6.455077	4.100144
27	2.122457E+07	1.648737E+07	21.03812	4.780933	5.763663
28	2.052929E+07	1.629656E+07	23.80799	5.4264	5.424806

46

Agency NPV

- Probability
- Sensitivity
- Scenario Analysis

47

Summary of Results

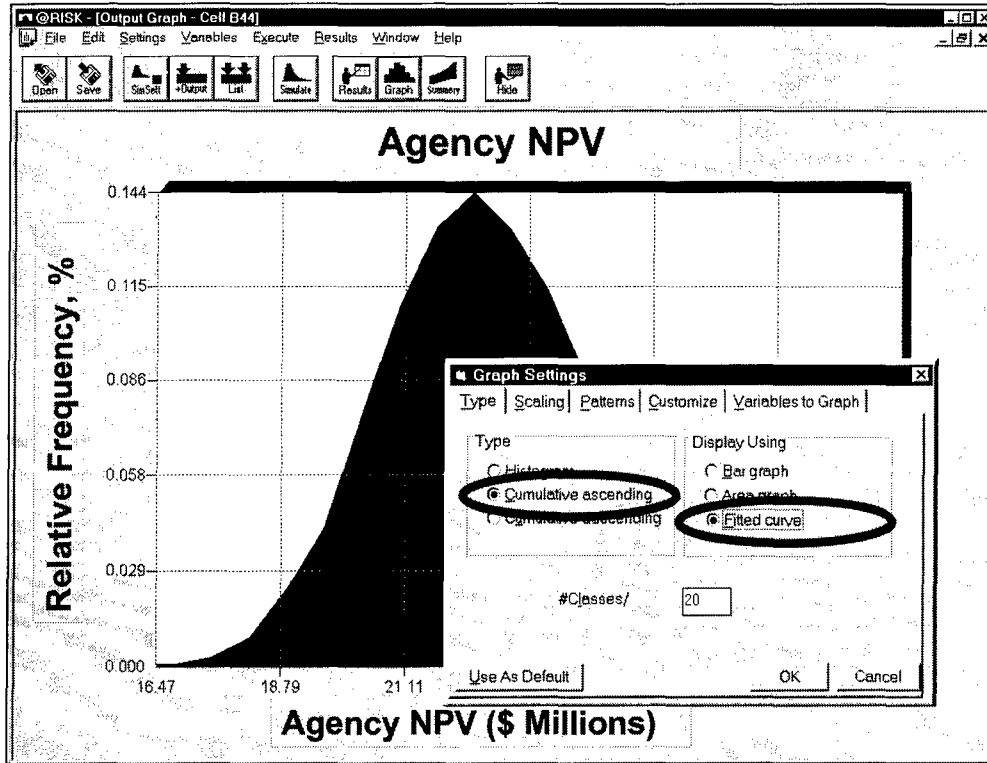
Cell	Name	Minimum	Mean	Maximum
B44	Total NPV (Agency Cost) / Initial	1.46963E+0	2.260473E+0	3.03952E+0
K8	(Input) Distribution	1.241904E+0	1.749998E+0	2.232156E+0
B31	(Input) Pavement Life / Alternative A Samples	12.81439	19.99999	27.15098
C31	(Input) Pavement Life / Mean	3.19558	4.999998	6.785005
D31	(Input) Pavement Life / Estimate	3.215575	5	6.790531
E31	(Input) Pavement Life / Percentile	3.216117	5	6.796125
F31	(Input) Pavement Life / Percentile	3.210124	4.999999	6.802994
G31	(Input) Pavement Life / Deviation	3.209837	4.999998	6.795606

Simulation Statistics

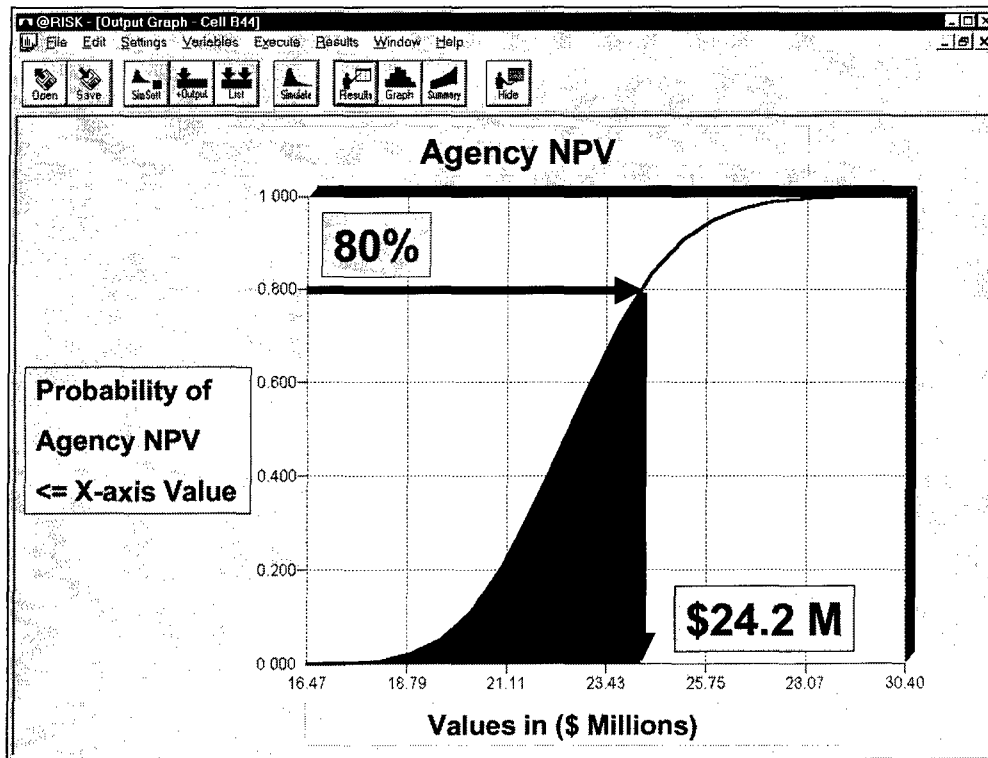
Name	Total NPV (Agency Cost) / Initial	Distribution	Pavement Life / Alternative A Samples	Pavement Life / Mean
Description	Output	Normal(17500000,1250000)	Trigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15,\$G\$15)	Pavement Life / Mean
Cell	B44	K8	B31	C31
Minimum =	1.46963E+07	1.241904E+07	12.81439	3.19558
Maximum =	3.03952E+07	2.232156E+07	27.15098	6.785005
Mean =	2.260479E+07	1.749998E+07	19.99999	4.999998
Std Deviation =	1918868	1249975	2854116	0.7385281
Variance =	3.682053E+12	1.562436E+12	8.726799	0.5454237
Skewness =	0.1592061	-9.529207E-04	-4.731112E-05	-2.706401E-05
Kurtosis =	2.931615	2.999367	2.480014	2.39999
Errors Calculated =	0	0	0	0
Mode =	2.027611E+07	1.745298E+07	20.03623	5.009075
5% Perc =	1.953114E+07	1.544371E+07	15.05029	3.76302

Current Variables: 1 output 24 inputs Settings: Simulations=1 Iterations=10000

48



49



50

Agency Cost

- Probability
- Sensitivity

51

Simulation Summary of Results

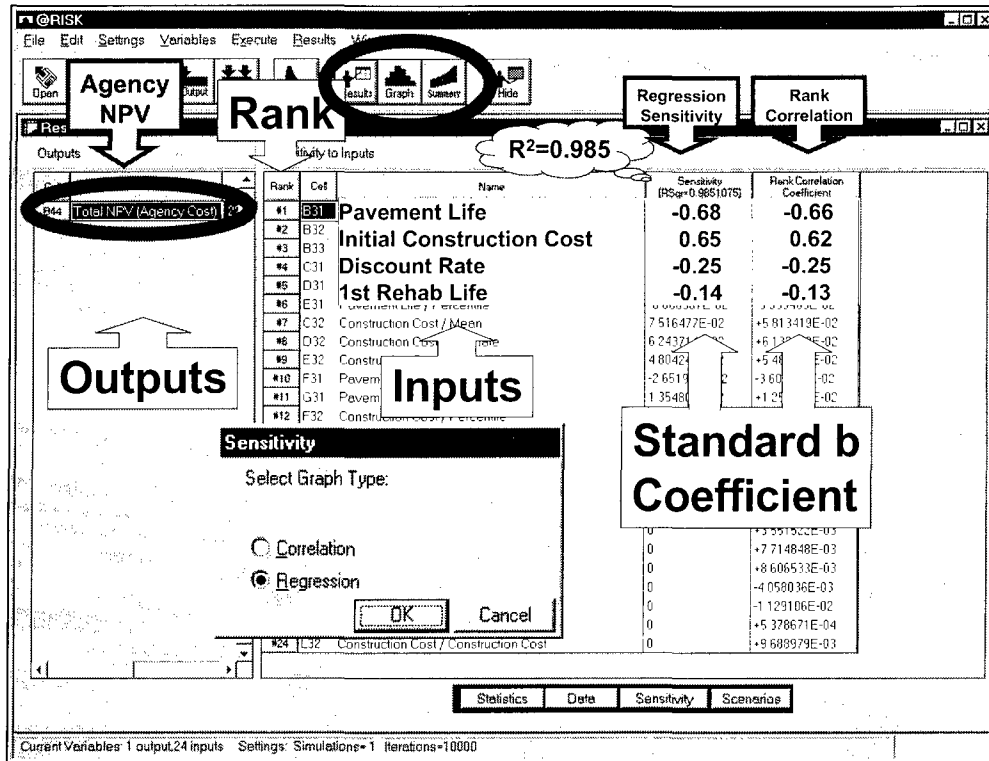
Cell	Name	Minimum	Mean	Maximum
B44	Total NPV (Agency Cost) / Initial	1.64696E+07	2.50479E+07	3.03862E+07
K8	(Input) Distribution	1.241904E+07	1.745298E+07	2.232156E+07
B31	(Input) Pavement Life / Alternative A Sample	12.81439	19.99999	27.15099
C31	(Input) Pavement Life / Mean	3.19558	4.999999	6.785005
D31	(Input) Pavement Life / Estimate	3.215575	5	6.790531
E31	(Input) Pavement Life / Percentile	3.216117	5	6.796125
F31	(Input) Pavement Life / Percentile	3.210124	4.999999	6.802984
G31	(Input) Pavement Life / Deviation	3.209837	4.999999	6.795606
H31	(Input) Pavement Life / Tigen	3.213648	5	6.798399
B1	(Input) Pavement Life	3.207759	5	6.799276
B31	(Input) Pavement Life	3.208104	5	6.799376

Simulation Statistics

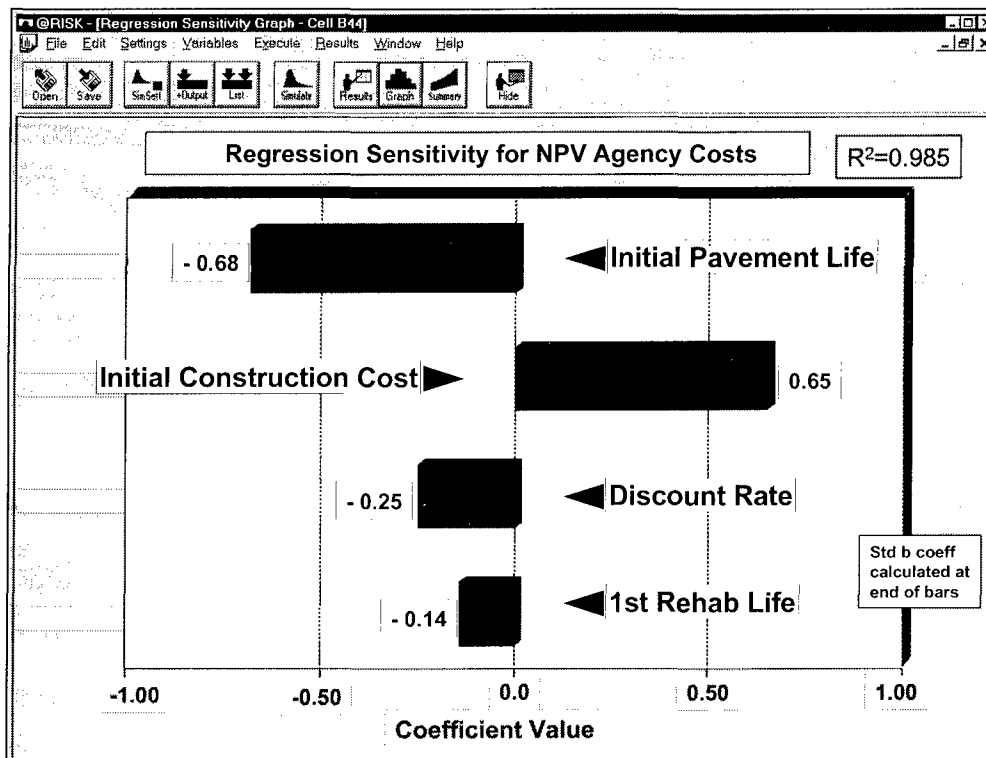
Name	Total NPV (Agency Cost) / Initial	Distribution	Pavement Life / Alternative A Sample	Pavement Life / Mean	Pavement Life / Tigen
Description	Output	Normal(17500000,1250000)	Tigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15,\$G\$15,\$H\$15)	Tigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15,\$G\$15,\$H\$15)	Tigen(\$B\$15,\$C\$15,\$D\$15,\$E\$15,\$F\$15,\$G\$15,\$H\$15)
Cell	B44	K8	B31	C31	D31
Minimum =	1.64696E+07	1.241904E+07	12.81439	3.19558	3.215575
Maximum =	3.03862E+07	2.232156E+07	27.15099	6.785005	6.790531
Mean =	2.50479E+07	1.745298E+07	19.99999	4.999999	5
Std Deviation =	1918868	1249975	2.954116	0.7385281	0.7385269
Variance =	3.68205E+12	1.562438E+12	8.726799	0.5454237	0.5454221
Skewness =	0.1592061	-9.529207E-04	-4.731112E-05	-2.706401E-05	-2.434887E-05
Kurtosis =	2.938115	2.999367	2.400014	2.38999	2.399977
Errors Calculated =	0	0	0	0	0
Mode =	2.027611E+07	1.745298E+07	20.03629	5.009075	5.009085
5% Perc =	1.953114E+07	1.544371E+07	15.05029	3.76302	3.762943
10% Perc =	2.019897E+07	1.589777E+07	15.99839	3.999524	3.99972
15% Perc =	2.061688E+07	1.620418E+07	16.72641	4.181541	4.181695
20% Perc =	2.098862E+07	1.644786E+07	17.34015	4.334829	4.334908
25% Perc =	2.13501E+07	1.665688E+07	17.8805	4.469366	4.470016

Current Variables: 1 output, 24 inputs Settings: Simulations=1 Iterations=10000

52



53



54

Sensitivity Analysis Observations
<ul style="list-style-type: none">■ Initial Pavement Life and Initial Construction Cost have the greatest influence on Agency NPV ■ Discount Rate and 1st Rehab life have minor influence on Agency NPV

55

Agency Cost
<ul style="list-style-type: none">● Probability● Sensitivity● Scenario Analysis

56

Simulation
 Simulation #1 of
 SoftwareDemo3.xls
 Iterations= 10000 Simulations= 1
 # Input Variables= 24
 # Output Variables= 1
 Sampling Type= Latin Hypercube
 Runtime= 00:01:15
 Run on 8/11/98 at 8:51:54 AM

Summary of Results

Cell	Name	Minimum	Mean	Maximum
B44	Total NPV (Agency Cost) / Initial	1.64836E+0	2.260479E+0	3.039522E+0
K9	Input Distribution	1.241904E+0	1.749398E+0	2.232156E+0
B31	Input Pavement Life / Alternative A Samples	12.81439	19.99398	27.15098
C31	Input Pavement Life / Mean	3.19558	4.999398	6.795005
D31	Input Pavement Life / Estimate	3.215575	5	6.790531
E31	Input Pavement Life / Percentile	3.216117	5	6.796125
F31	Input Pavement Life / Percentile	3.210124	4.999399	6.802994
G31	Input Pavement Life / Deviation	3.209837	4.999398	6.795606
H31	Input Pavement Life / Trigen	3.207759	4.999397	6.793399
I31	Input Pavement Life	3.206789	5.000000	6.795205

Simulation Statistics

Name	Total NPV (Agency Cost) / Initial	Distribution	Pavement Life / Alternative A Samples	Pavement Life / Mean	Pavement Life
Description	Output	Normal(17500000,1250000)	Trigen(\$B15,\$C\$15,\$D\$15,\$E\$15,\$F\$15,\$G\$15,\$H\$15,\$I\$15)	Trigen(\$B16,\$C\$16,\$D\$16,\$E\$16,\$F\$16,\$G\$16,\$H\$16,\$I\$16)	Trigen(\$B16,\$C\$16,\$D\$16,\$E\$16,\$F\$16,\$G\$16,\$H\$16,\$I\$16)
Cell	B44	K9	D31	C31	D31
Minimum =	1.64836E+07	1.241904E+07	12.81439	3.19558	3.215575
Maximum =	3.039522E+07	2.232156E+07	27.15098	6.795005	6.790531
Mean =	2.260479E+07	1.749398E+07	19.99398	4.999398	5
Std Deviation =	1.918368	1249375	2.954116	0.7385281	0.7385289
Variance =	3.682053E+13	1.562436E+12	8.726799	0.5454237	0.5454221
Skewness =	0.1592061	9.529207E-04	-4.731112E-05	-2.706401E-05	-2.434887E-05
Kurtosis =	2.931615	2.993367	2.400014	2.39999	2.399977
Errors Calculated =	0	0	0	0	0
Mode =	2.027611E+07	1.745298E+07	20.03629	5.009075	5.009086
5% Perc =	1.953114E+07	1.544371E+07	15.05029	3.76302	3.762943
10% Perc =	2.019937E+07	1.589777E+07	15.99339	3.999634	3.999972
15% Perc =	2.061638E+07	1.620418E+07	16.72641	4.181541	4.181695
20% Perc =	2.096822E+07	1.644786E+07	17.34015	4.344829	4.344908
25% Perc =	2.135001E+07	1.665688E+07	17.8805	4.463966	4.470016

Current Variables: 1 output, 24 inputs Settings: Simulations=1 Iterations=10000

57

Significant Inputs

Agency NPV

By Target

Median of Samples in Iterations Meeting Target

Target#1: Inputs when Target #1 is Achieved (Total NPV (Agency Cost) / Initial >75%)
 B31=17.30627 B32=1.84487E+07

Target#2: Inputs when Target #2 is Achieved (Total NPV (Agency Cost) / Initial <25%)
 B31=22.61025 B32=1.646424E+07

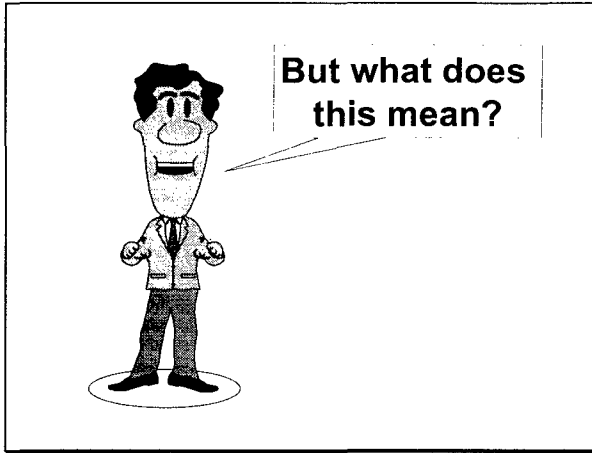
Target#3: Inputs when Target #3 is Achieved (Total NPV (Agency Cost) / Initial >90%)
 B31=16.26066 B32=1.880667E+07

Significant Input Distributions for this Scenario

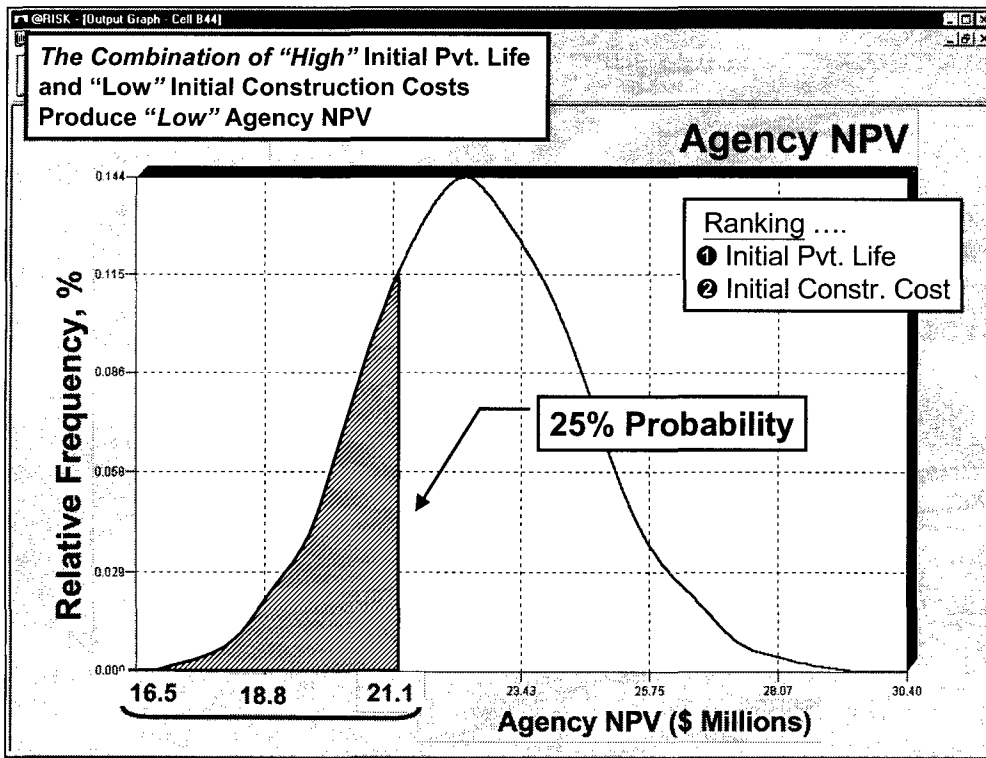
Cell	Name	Percentile	Actual	Ratio Median to Std Deviation	E
B31	Initial Pavement Life	79.6 %	22.6 yrs	+0.88	
B32	Initial Constr. Cost	20.4 %	\$16.5 M	-0.83	

Current Variables: 1 output, 24 inputs Settings: Simulations=1 Iterations=10000

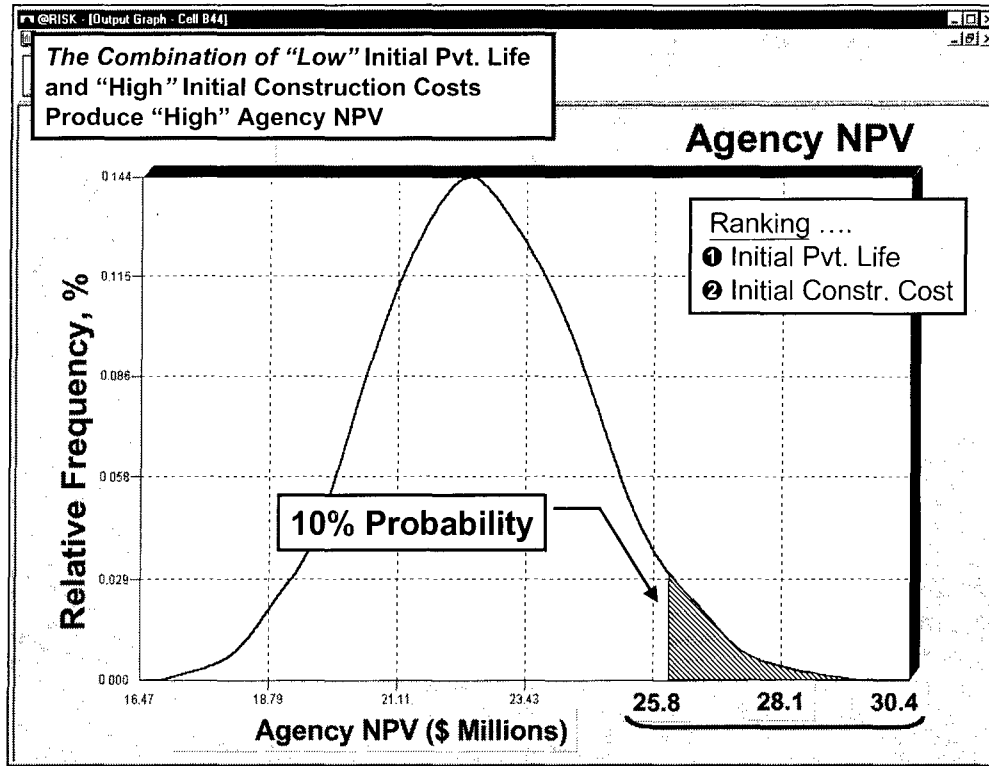
58



59



60



61

**Scenario Analysis
Observations**

The Combination of

- "High" Initial Pvt. Life and "Low" Initial Construction Costs Produce "Low" [$< 25\%$] Agency NPV
- "Low" Initial Pvt. Life and "High" Initial Construction Costs Produce "High" [$> 90\%$] Agency NPV

62

The screenshot shows the @RISK software interface. The top menu bar includes File, Edit, Settings, Variables, Execute, Results, Window, and Help. The main window is divided into two panes. The left pane, titled 'Simulation', shows settings for Simulation #1 of 'SoftwareDemo3.XLS', with 10000 iterations and 24 input variables. The right pane, titled 'Summary of Results', displays a table of simulation results for various cells and variables.

Cell	Name	Minimum	Mean	Maximum
B44	Total NPV (Agency Cost) / Initial	1.64696E+07	2.260479E+07	3.039523E+07
K9	(Input) Distribution	1.241904E+07	1.749998E+07	2.232156E+07
B31	(Input) Pavement Life / Alternative A Samples	12.81439	19.99999	27.15098
C31	(Input) Pavement Life / Mean	3.19558	4.999998	6.785005
D31	(Input) Pavement Life / Estimate	3.215575	5	6.790531
E31	(Input) Pavement Life / Percentile	3.216117	5	6.796125
F31	(Input) Pavement Life / Percentile	3.210124	4.999999	6.802994
G31	(Input) Pavement Life / Deviation	3.209837	4.999998	6.795606
H31	(Input) Pavement Life / Tigen	3.213548	5	6.798399
I31	(Input) Pavement Life	3.207759	4.999997	6.789776
J31	(Input) Pavement Life	3.208789	5.000004	6.795205

Below the summary table is a 'Simulation Statistics' pane, which provides a detailed breakdown of the simulation results for the 'Total NPV (Agency Cost) / Initial' cell, including distribution parameters like mean, standard deviation, and percentiles.

63

The screenshot shows a Microsoft Excel spreadsheet titled 'SoftwareDemo3.xls'. The spreadsheet is set up to calculate the Net Present Value (NPV) for 'Alternative A Samples'. The data is organized into columns for 'Initial', 'Rehab', and 'Rehab' at different years (20, 25, 30). The NPV is calculated based on agency costs, construction costs, and a discount rate of 4%.

		Initial	Rehab	Rehab	Rehab
31	Pavement Life	20.0	5.0	5.0	5.0
32	Construction Cost	\$ 17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
33	Discount Rate, (%)	4			
40	Year >>>	0	20	25	30
41	Agency Costs (Constant \$)	\$ 17,500,000	\$ 4,375,000	\$ 4,375,000	\$ 4,375,000
42	Present Worth Factor	1.0000	0.4564	0.3751	0.3083
43	Agency Cost (Present Worth)	\$ 17,500,000	\$ 1,996,693	\$ 1,641,136	\$ 1,348,894
44	Total NPV (Agency Cost)	\$ 22,486,723			

64

Risk Modeling Review

- Build the traditional NPV model
- Identify and describe uncertain variables using risk functions
- Define simulation parameters and output variables
- Run the simulation
- Analyze Results

65

Parting Caveat ...

- *@Risk* and *BestFit* have been used for illustration purposes.
- Other software packages are available to do risk analysis.

66

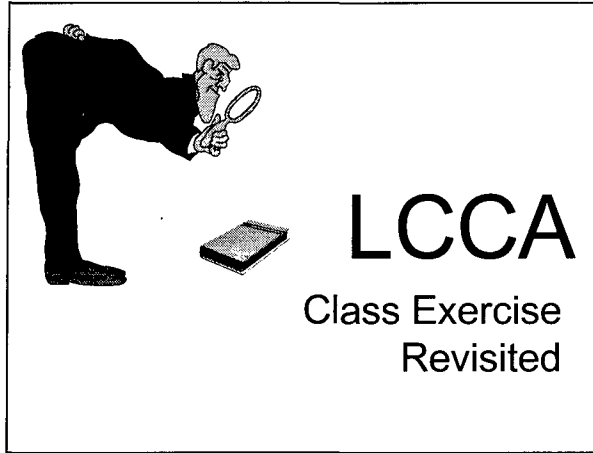
For More Information:

- *@Risk* and *BestFit*
 - Palisade Corporation
 - <http://www.palisade.com>
 - Phone: 800-432-7475
- *Crystal Ball*
 - Decisioneering
 - <http://www.decisioneering.com>
 - Phone: 800-289-2550

67

End Session

1



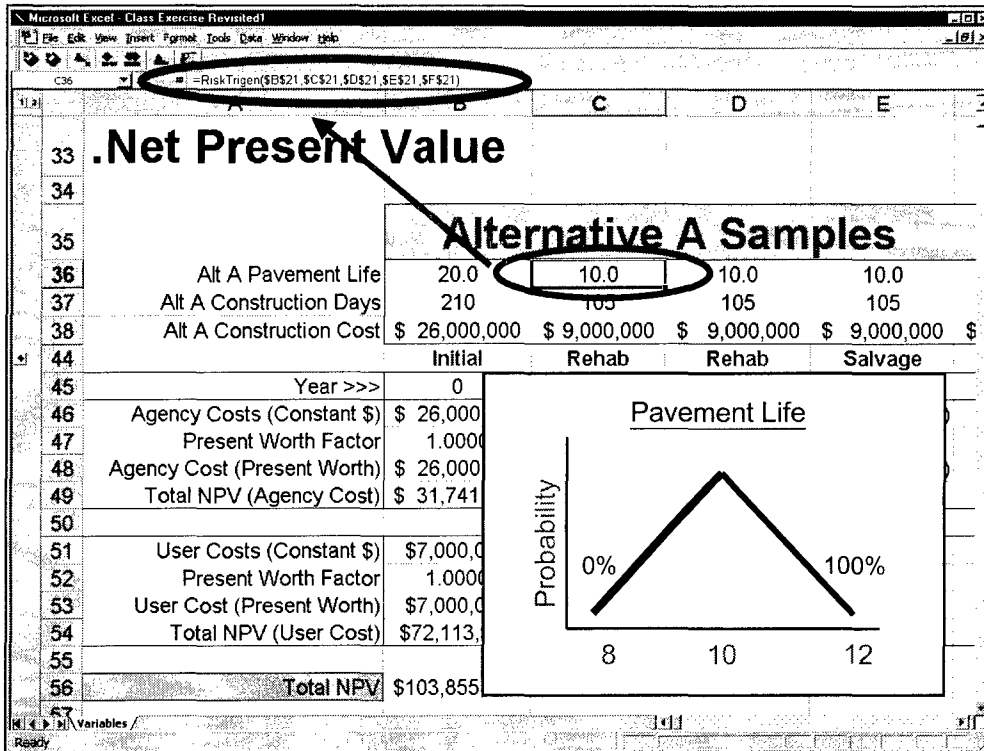
2

Microsoft Excel - Class Exercise Revisited1.xls

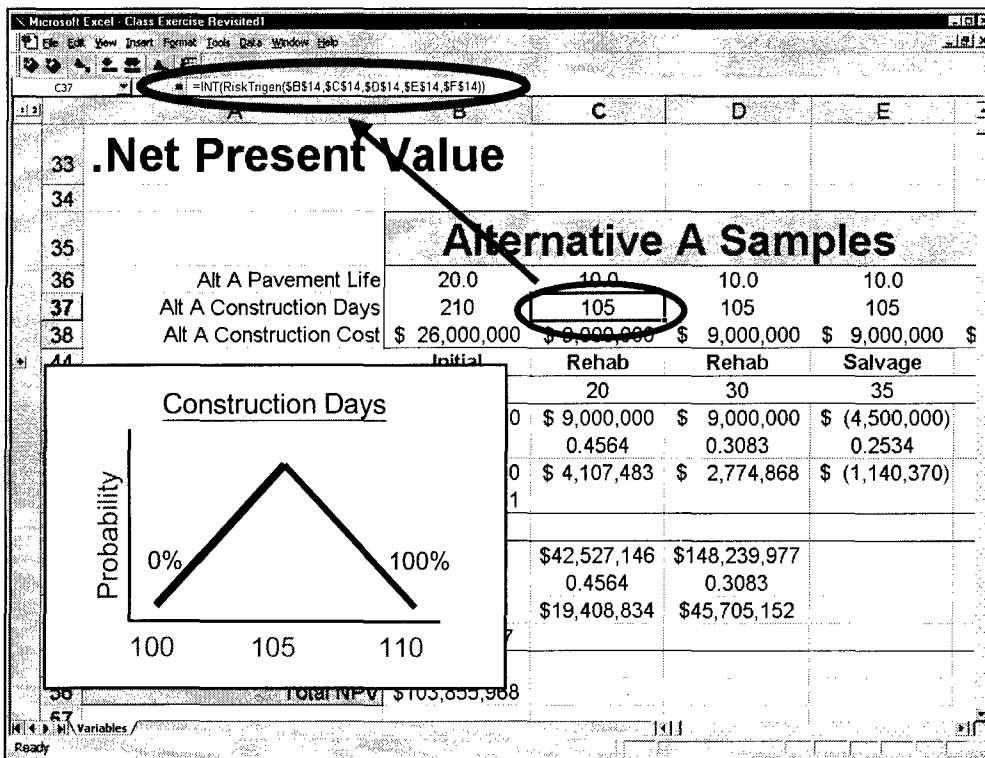
File Edit View Insert Format Tools Data Window Help

Class Exercise Revisited						
Analysis Period	35	years				
Discount Rate	4.0%					
.Risk Analysis Input Parameters						
Variable	Lower Estimate	Most Likely	Upper Estimate	Lower Percentile	Upper Percentile	Distribution Type
Construction Days						
Alternative A						
Initial	200	210	220	0	100	Trigen
Rehab	100	105	110	0	100	Trigen
Alternative B						
Initial	150	165	180	0	100	Trigen
Rehab	70	85	100	0	100	Trigen
Performance Estimates						
Alternative A						
Initial	16	20	24	0.00	100.00	Trigen
Rehab	8	10	12	0.00	100.00	Trigen
Alternative B						
Initial	10	13	16	0.00	100.00	Trigen
Rehab	6	7	8	0.00	100.00	Trigen

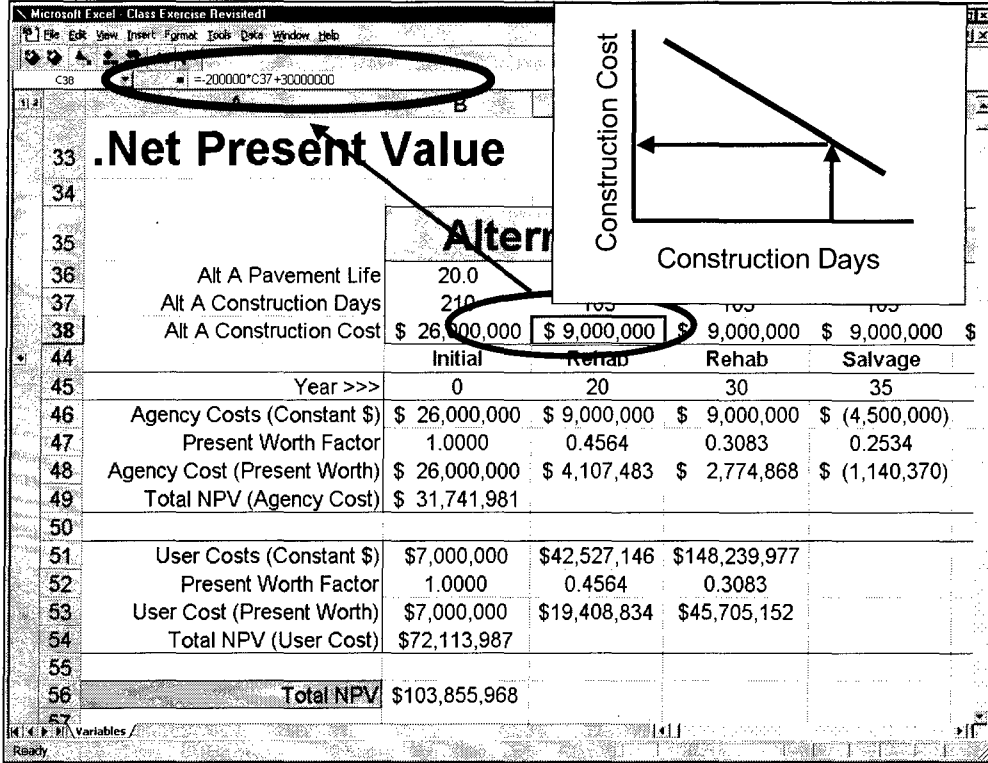
3



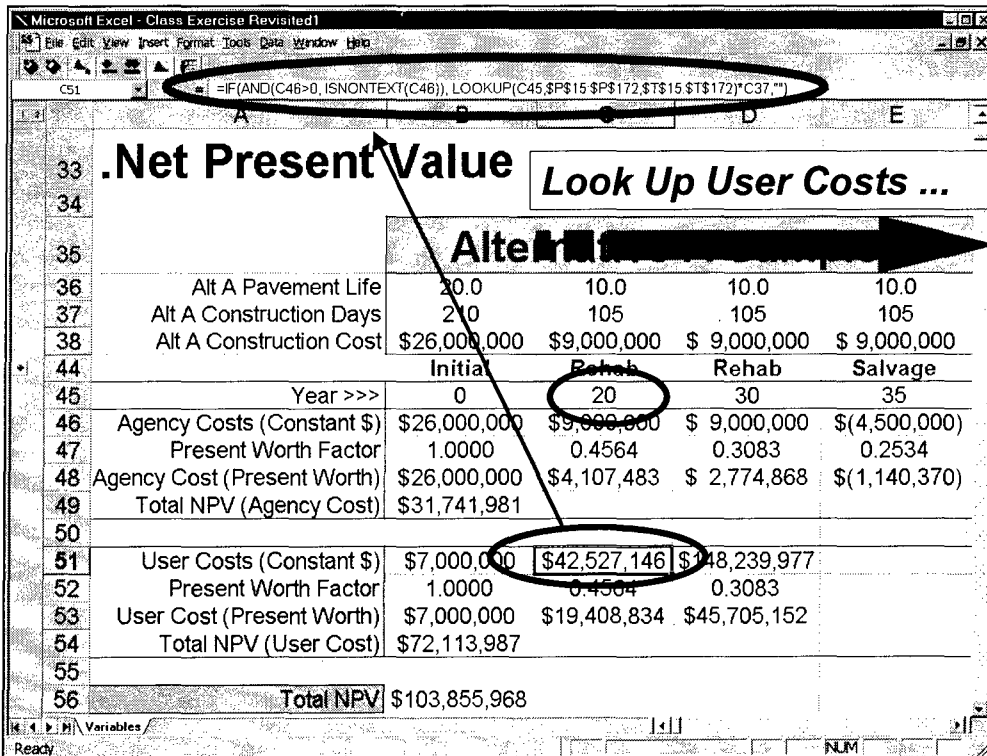
4



5



6



7

Microsoft Excel - Class Exercise Revisited

Year	AAADT	Delay/Veh. min	Delay hrs	Daily Cost
0	40000	5.0	3333	\$ 33,333
1	41200	5.5	3777	\$ 37,767
2	42436	6.1	4279	\$ 42,790
3	43709	6.7	4848	\$ 48,481
4	45020	7.3	5493	\$ 54,929
5	46371	8.1	6223	\$ 62,234
6	47762	8.9	7051	\$ 70,511
7	49195	9.7	7989	\$ 79,889
8	50671	10.7	9051	\$ 90,514
9	52191	11.8	10255	\$ 102,553
10	53757	13.0	11619	\$ 116,192
11	55369	14.3	13165	\$ 131,646
12	57030	15.7	14915	\$ 149,155
13	58741	17.3	16899	\$ 168,993
14	60504	19.0	19147	\$ 191,469
15	62319	20.9	21693	\$ 216,934
16	64188	23.0	24579	\$ 245,786
17	66114	25.3	27848	\$ 278,476
18	68097	27.8	31551	\$ 315,513
19	70140	30.6	35748	\$ 357,476
20	72344	33.7	40502	\$ 405,020
21	74412	37.0	45889	\$ 458,889

Value Time \$ 10.00 per hour
 Directional AADT (initial) 40000 vpd
 Traffic Growth Rate 3 percent
 Delay per veh. Growth Rate 10 percent

Variables/

Daily Cost of Delay in Year 20

8

Microsoft Excel - Class Exercise Revisited

=IF(AND(C46>0, ISNONTEXT(C46)), LOOKUP(C45,\$P\$15:\$P\$172,\$T\$15:\$T\$172)*C37,"")

	Initial	Rehab	Rehab	Salvage
Alt A Pavement Life	20.0	10.0	10.0	10.0
Alt A Construction Days	210	105	105	105
Alt A Construction Cost	\$26,000,000	\$9,000,000	\$ 9,000,000	\$ 9,000,000
Year >>>	0	20	30	35
Agency Costs (Constant \$)	\$26,000,000	\$9,000,000	\$ 9,000,000	\$(4,500,000)
Present Worth Factor	1.0000	0.4564	0.3083	0.2534
Agency Cost (Present Worth)	\$26,000,000	\$4,107,483	\$ 2,774,868	\$(1,140,370)
Total NPV (Agency Cost)	\$31,741,981			
User Costs (Constant \$)	\$7,000,000	\$42,527,146	\$18,239,977	
Present Worth Factor	1.0000	0.4564	0.3083	
User Cost (Present Worth)	\$7,000,000	\$19,402,924	\$5,705,152	
Total NPV (User Cost)	\$72,113,981	(Daily Costs)(Construction Days)		
Total NPV	\$103,855,968			

Variables/

9

Microsoft Excel - Class Exercise Revisited

File Edit View Insert Format Tools Data Window Help

=IF(G66>=(\$B\$4-0.5),IF(G71="Salvage",-G65/G63*G68,IF(G66>=\$B\$4-0.5,"",G65)),G65)

	A	B	C	D	E	F	G
59							
60							
61							
62	Alternative B Samples						
63	Alt B Pavement Life	13.0	7.0	7.0	7.0	7.0	7.0
64	Alt B Construction Days	165	85	85	85	85	85
65	Alt B Construction Cost	\$ 21,000,000	\$ 5,000,000	\$ 5,000,000	\$ 5,000,000	\$ 5,000,000	\$ 5,000,000
71		Initial	Rehab	Rehab	Rehab	Rehab	Salvage
72	Year >>>	0	13	20	27	34	35
73	Agency Costs (Constant \$)	\$ 21,000,000	\$ 5,000,000	\$ 5,000,000	\$ 5,000,000	\$ 5,000,000	\$ (4,285,714)
74	Present Worth Factor	1.0000	0.6006	0.4564	0.3468	0.2636	0.2534
75	Agency Cost (Present Worth)	\$ 21,000,000	\$ 3,002,870	\$ 2,281,935	\$ 1,734,083	\$ 1,317,760	\$ (1,086,066)
76	Total NPV (Agency Cost)	\$ 28,250,582					
77							
78	User Costs (Constant \$)	\$ 5,500,000	\$ 14,364,367	\$ 34,426,738	\$ 82,509,744	\$ 197,749,141	
79	Present Worth Factor	1.0000	0.6006	0.4564	0.3468	0.2636	
80	User Cost (Present Worth)	\$ 5,500,000	\$ 8,628,867	\$ 15,711,914	\$ 28,615,746	\$ 52,117,199	
81	Total NPV (User Cost)	\$ 110,571,726					
82							
83	Total NPV	\$ 138,822,308					
84							
85							
86							
87							
88							
89							
90							

Ready

10

Microsoft Excel - Class Exercise Revisited

File Edit View Insert Format Tools Data Window Help

	A	B	C
91			
92			
93	.Deterministic Results		
94	Alt A Agency NPV	\$ 31,741,981	
95	Alt B Agency NPV	\$ 28,250,582	
96	Alt A User NPV	\$ 72,113,987	
97	Alt B User NPV	\$ 110,571,726	
98	Alt A Total NPV	\$ 103,855,968	
99	Alt B Total NPV	\$ 138,822,308	
100			
101			
102			
103			

Ready

11

Simulation Processing
<ul style="list-style-type: none">■ Latin Hypercube■ 44 Input Variables■ 6 Output Variables■ 10,000 Iterations■ Run Time = 3 min 51 sec

12

Simulation Results

13

Microsoft Excel - Class Exercise Revisited1.xls

File Edit View Insert Format Tools Data Window Help

.Risk Analysis Summary Results						
Net Present Value (\$Millions)						
	Agency		User		Total	
	Alt A	Alt B	Alt A	Alt B	Alt A	Alt B
Minimum	\$28.9	\$24.1	\$30.9	\$51.5	\$61.0	\$78.5
Maximum	\$35.8	\$33.2	\$119.2	\$128.5	\$153.5	\$155.3
Mean	\$31.9	\$28.4	\$72.4	\$88.8	\$104.3	\$117.3
Std Deviation	\$1.0	\$1.4	\$10.0	\$20.0	\$9.2	\$20.3
Mode	\$29.1	\$27.0	\$71.5	\$61.8	\$63.6	\$88.7
Percentile - 10	\$30.6	\$26.5	\$60.2	\$61.5	\$93.3	\$89.3
25	\$31.2	\$27.4	\$65.4	\$66.4	\$98.0	\$94.0
50	\$31.8	\$28.4	\$71.8	\$95.9	\$103.6	\$125.0
75	\$32.6	\$29.4	\$78.8	\$105.9	\$110.0	\$134.5
90	\$33.2	\$30.3	\$85.8	\$111.7	\$116.6	\$140.0

Variables /

14

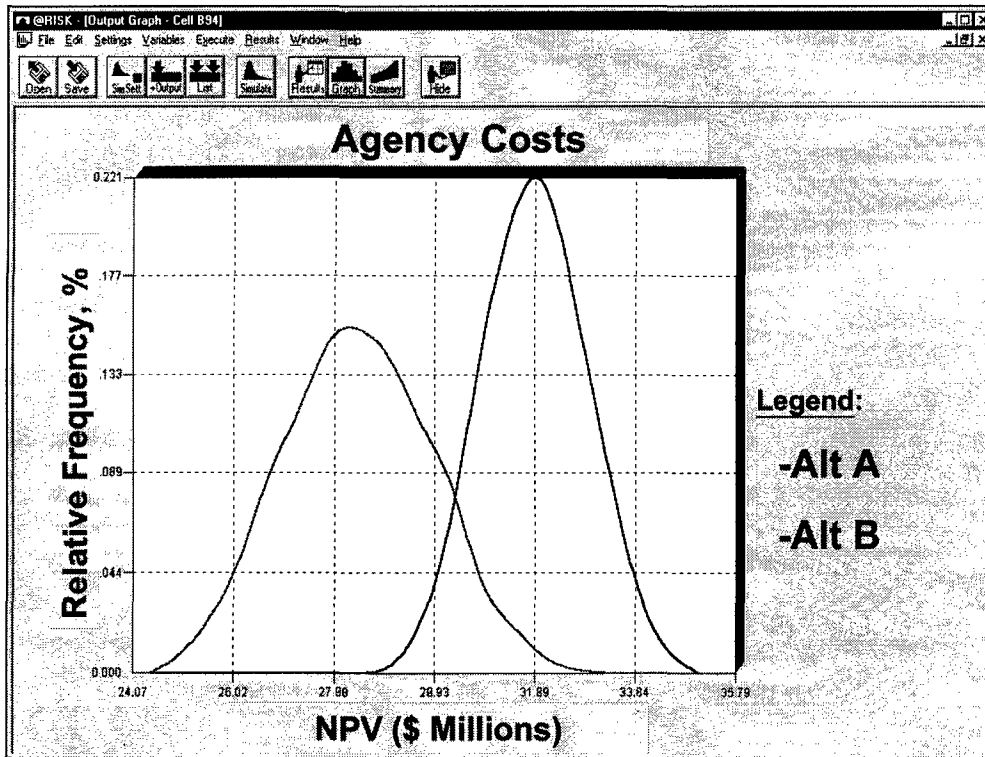
Agency Costs

15

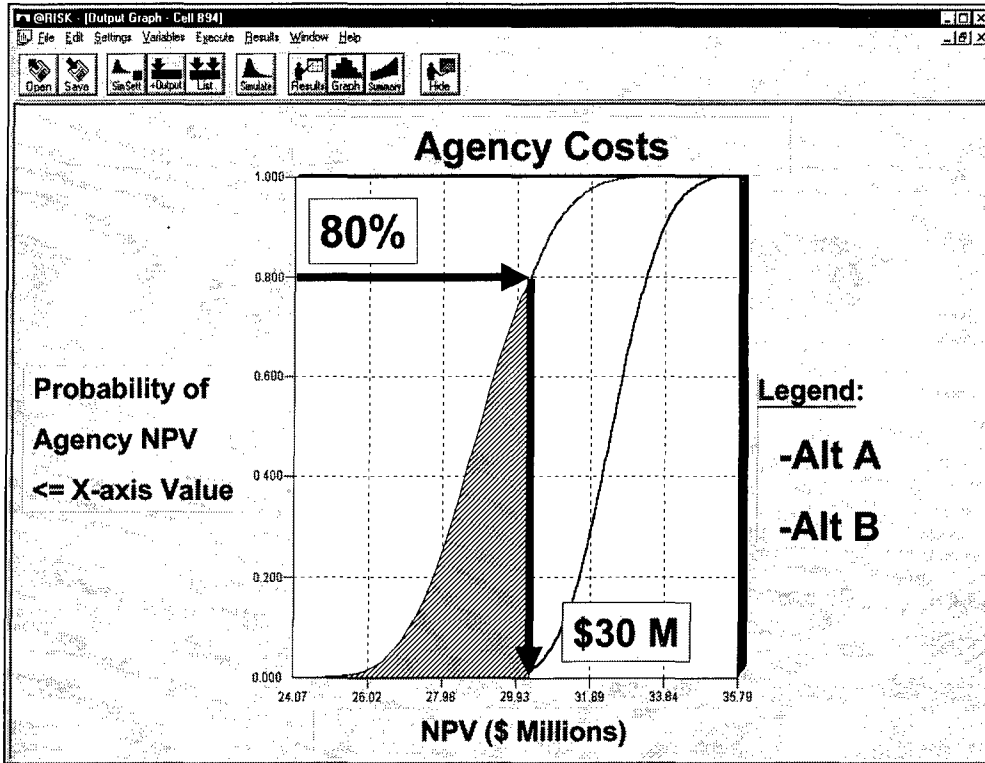
Agency Cost

- Probability
- Sensitivity
- Scenario Analysis

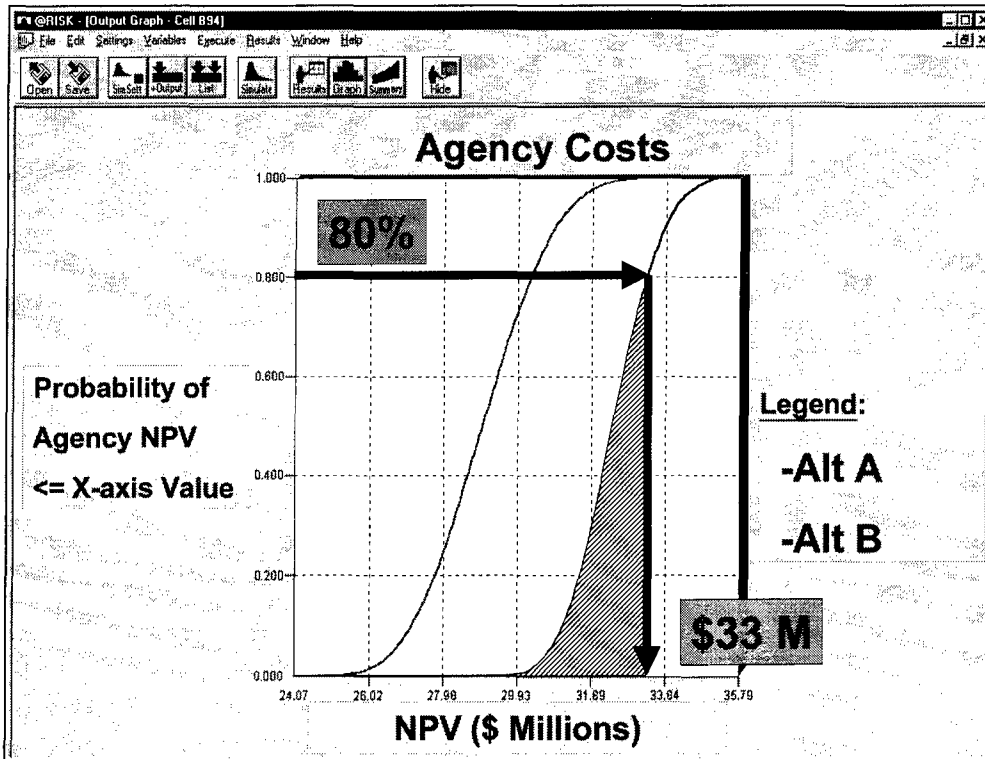
16



17



18



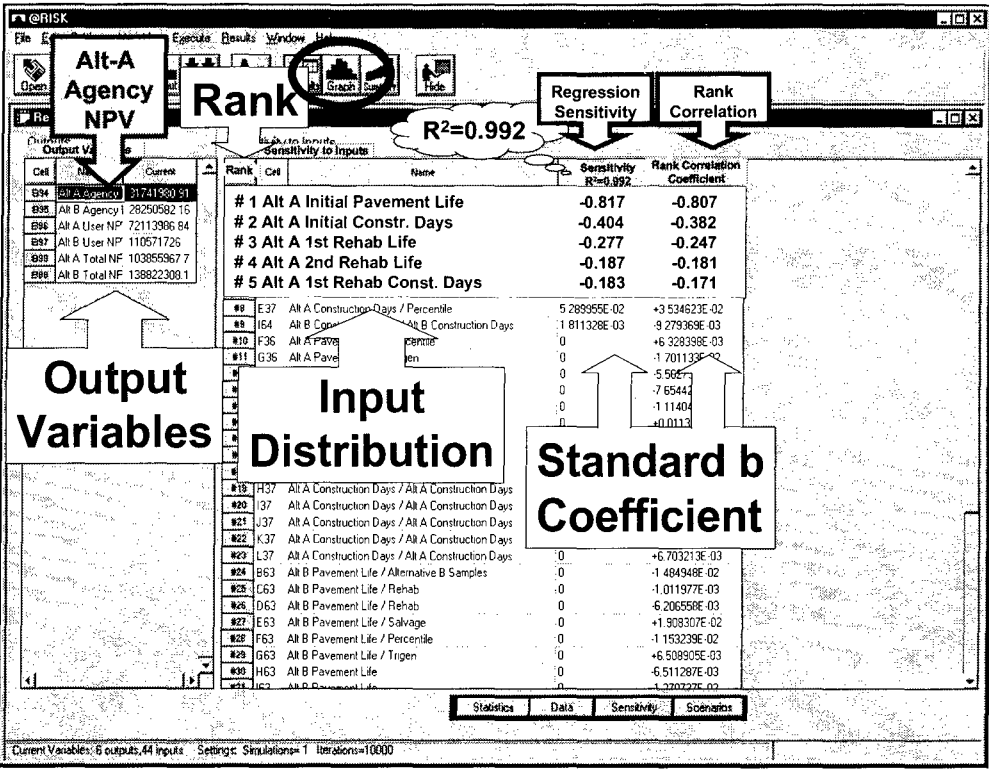
19

Probability Observations
Agency Costs ... <ul style="list-style-type: none">■ Alt-B is 42% <i>more</i> variable than Alt-A■ At <i>any</i> given level of reliability Alt-B is <i>less</i> expensive than Alt-A.

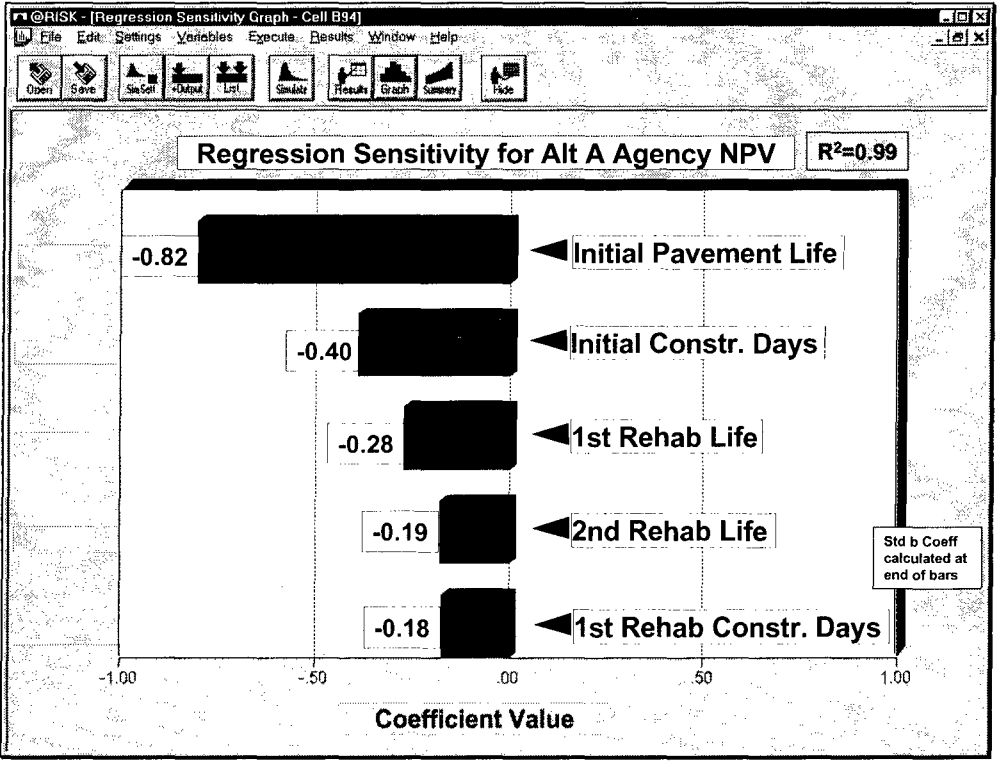
20

Agency Cost
<ul style="list-style-type: none">■ Probability■ Sensitivity

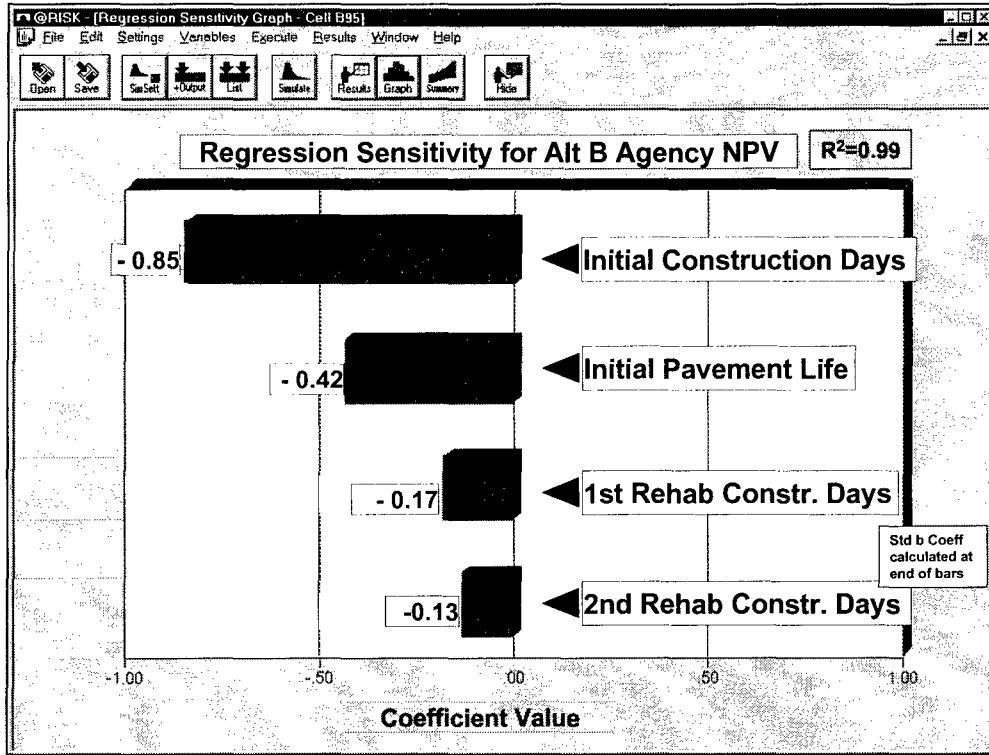
21



22



23



24

Sensitivity Observations

Agency Costs ...

- Alternative A
 - Initial Pavement Life has the greatest influence on Agency NPV
- Alternative B
 - Initial Construction Days has the greatest influence on Agency NPV

25

Agency Cost

- Probability
- Sensitivity
- Scenario Analysis

26

Significant Inputs

Inputs When Agency NPV < 25%

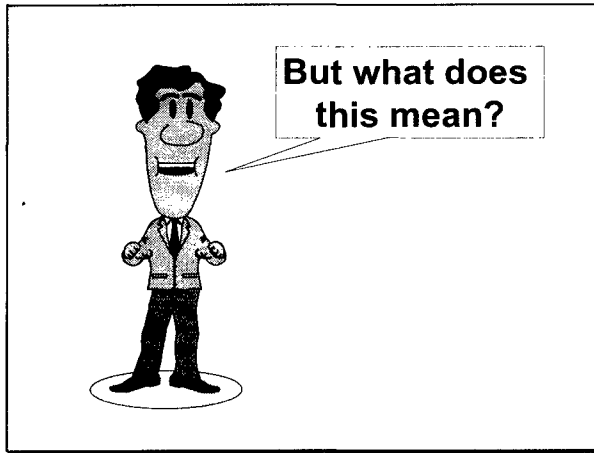
Output Variables

Ratio Δ Med. to Std

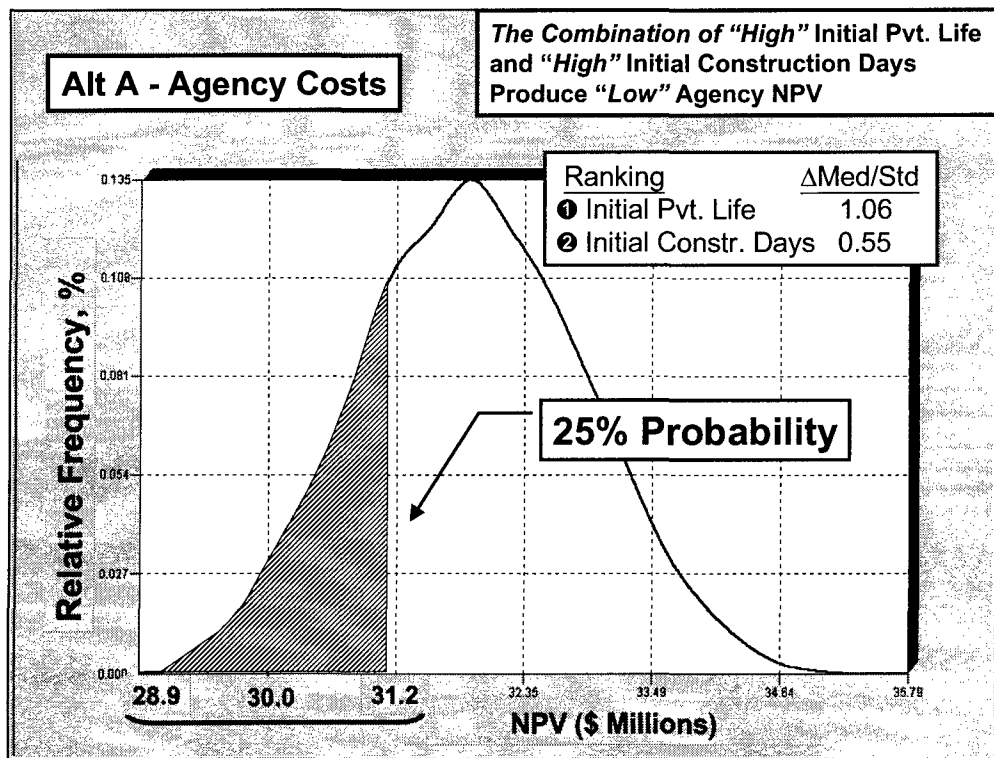
Significant Input Distributions for this Scenario			
	Percentile	Median	
Alt A Initial Pavement Life	83.8%	21.7 Yrs	+ 1.06
Alt A Initial Constr. Days	69.8%	212 Days	+ 0.55

Current Variables: 6 outputs, 44 inputs Settings: Simulations= 1 Iterations=10000

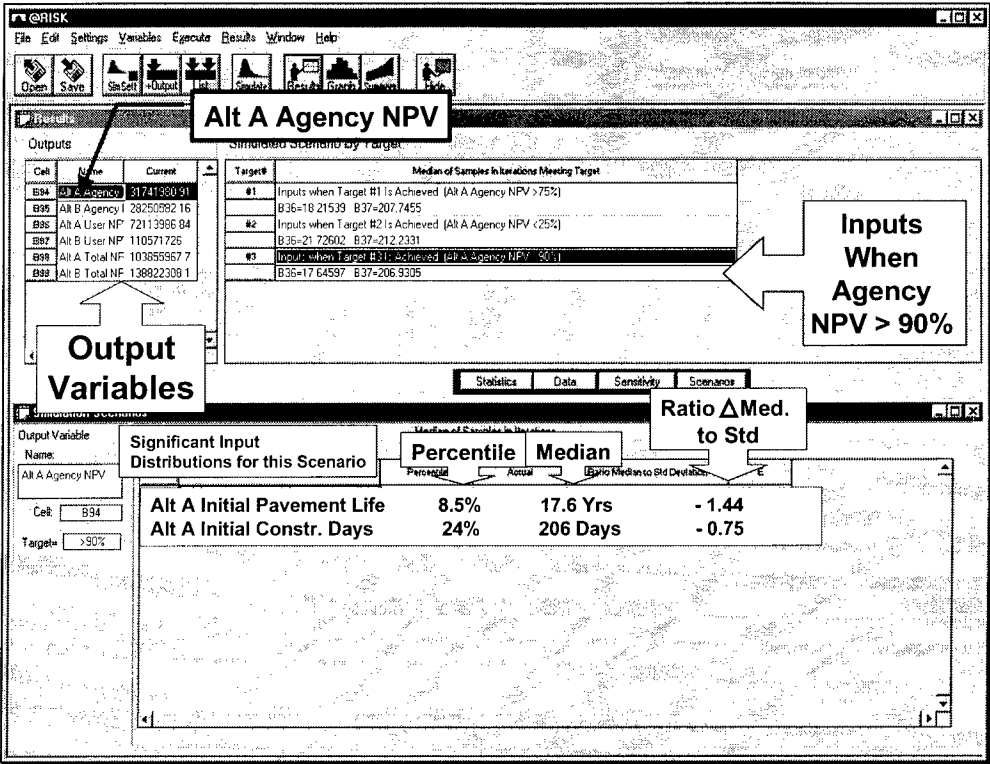
27



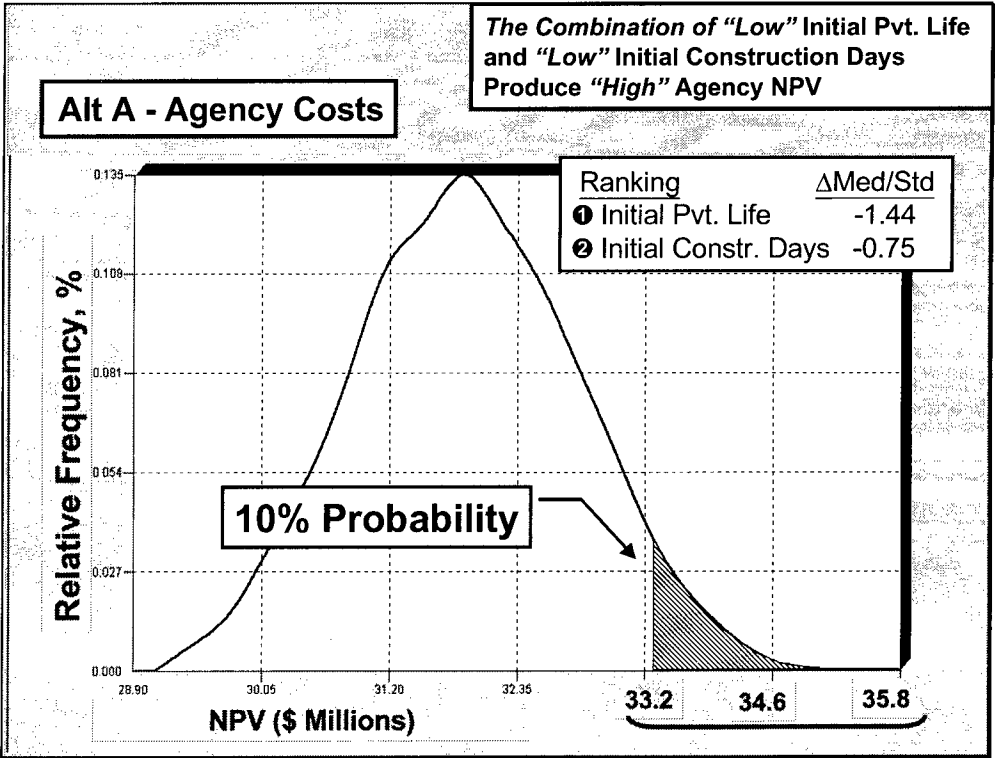
28



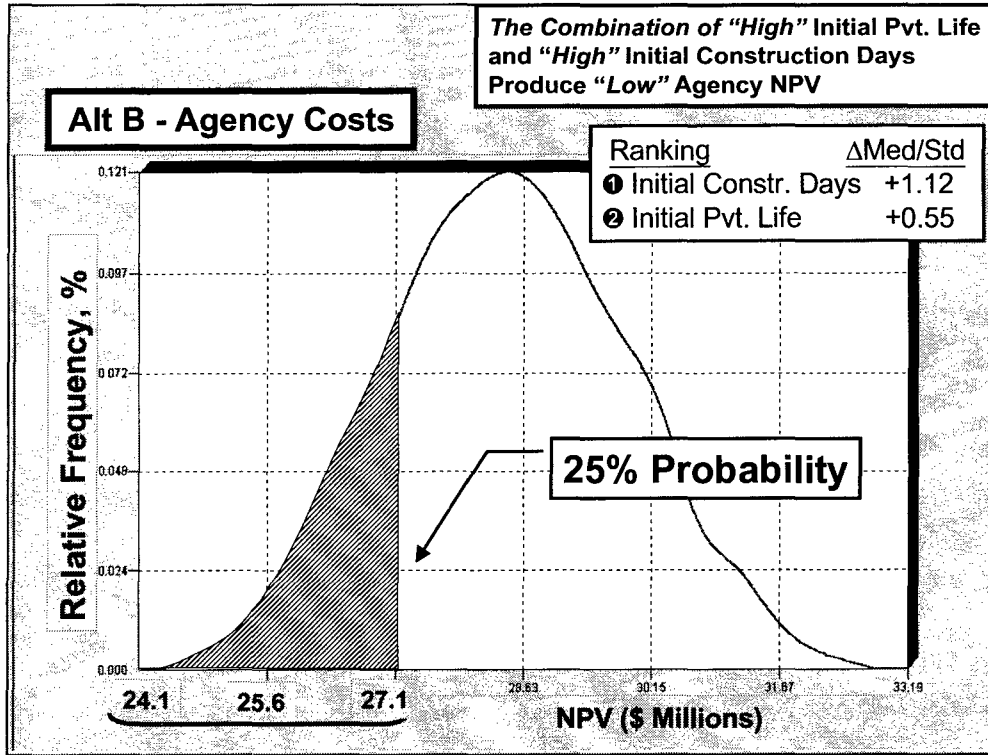
29



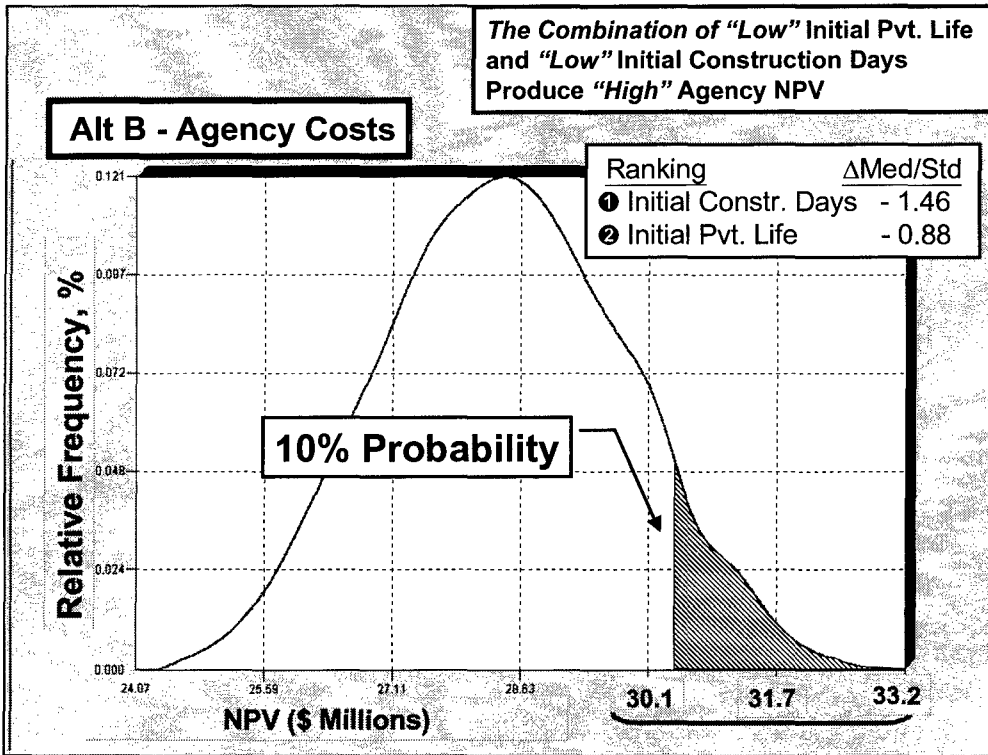
30



31



32



33

Scenario Analysis
Observations

For Alt A & B The Combination of

- "High" Initial Pvt. Life and "High" Initial Construction Days Produce "Low" [$< 25\%$] Agency Costs
- "Low" Initial Pvt. Life and "Low" Initial Construction Costs Produce "High" [$> 90\%$] Agency Costs

34

Scenario Analysis
Observations Cont'd.

Alternative A ...

- Initial Pavement Life is more significant than Initial Constr. days in both scenarios.

35

Scenario Analysis
Observations Cont'd.

Alternative B ...

- Initial Constr. Days is more significant than Initial Pavement Life in both scenarios.

36

User Costs

37

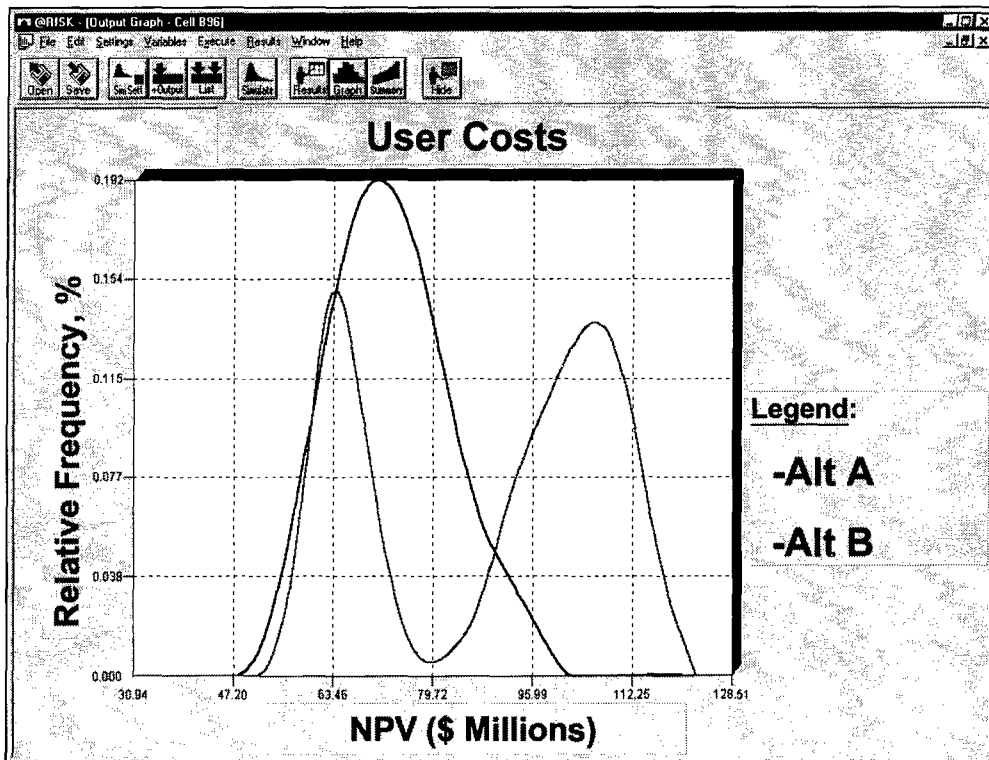
.Risk Analysis Summary Results						
Net Present Value (\$Millions)						
	Agency		User		Total	
	Alt A	Alt B	Alt A	Alt B	Alt A	Alt B
Minimum	\$28.9	\$24.1	\$30.9	\$51.5	\$61.0	\$78.5
Maximum	\$35.8	\$33.2	\$119.2	\$128.5	\$153.5	\$155.3
Mean	\$31.9	\$28.4	\$72.4	\$88.8	\$104.3	\$117.3
Std Deviation	\$1.0	\$1.4	\$10.0	\$20.0	\$9.2	\$20.3
Mode	\$29.1	\$27.0	\$71.5	\$61.8	\$63.6	\$88.7
Percentile - 10	\$30.6	\$26.5	\$60.2	\$61.5	\$93.3	\$89.3
25	\$31.2	\$27.4	\$65.4	\$66.4	\$98.0	\$94.0
50	\$31.8	\$28.4	\$71.8	\$95.9	\$103.6	\$125.0
75	\$32.6	\$29.4	\$78.8	\$105.9	\$110.0	\$134.5
90	\$33.2	\$30.3	\$85.8	\$111.7	\$116.6	\$140.0

38

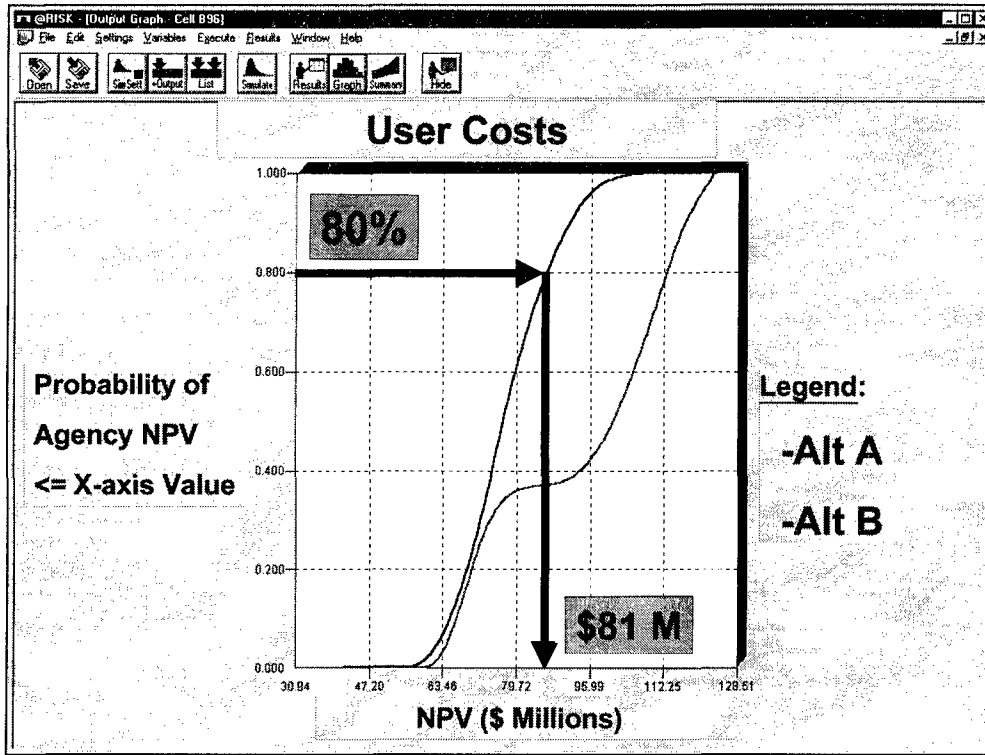
User Costs

- Probability
- Sensitivity
- Scenario Analysis

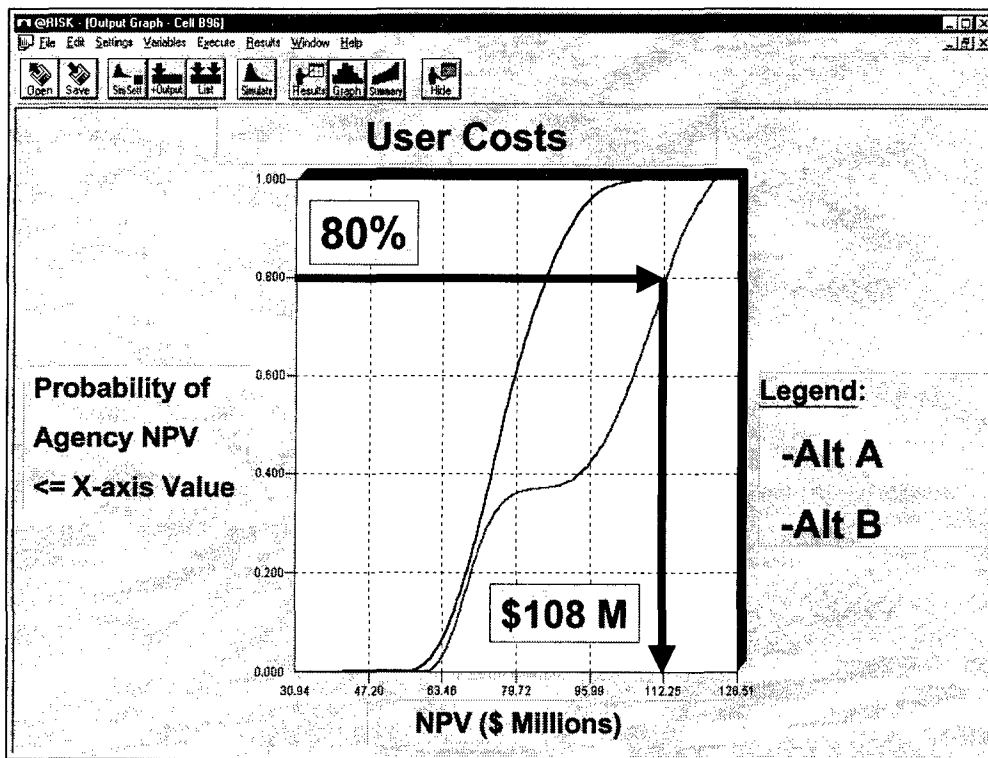
39



40



41



42

Probability Observations

User Costs ...

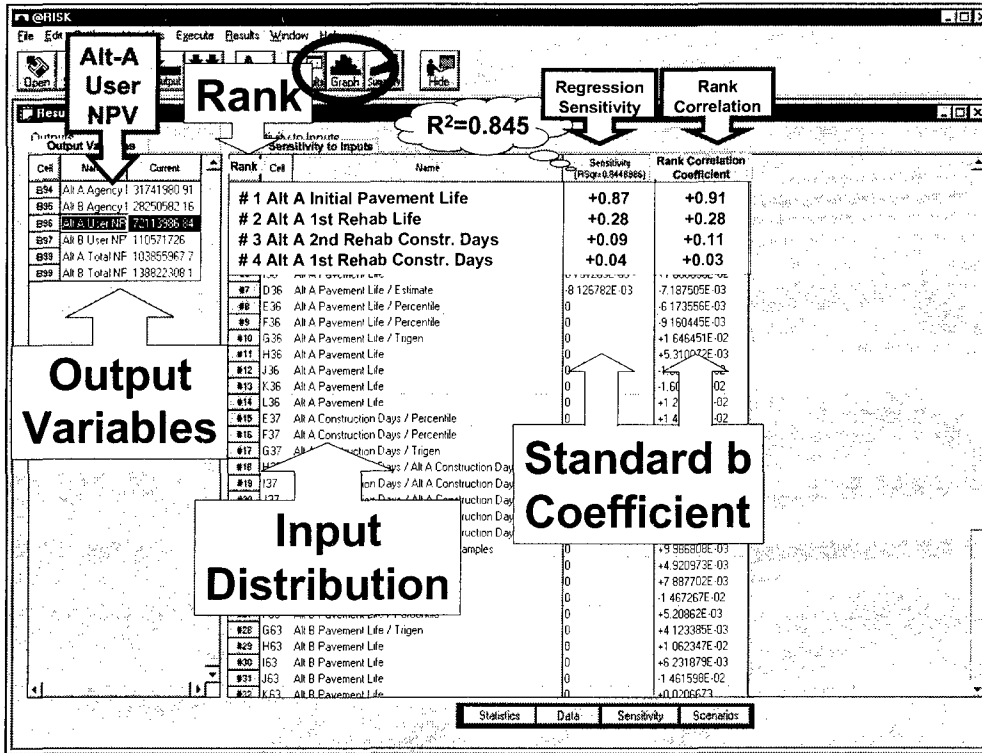
- At *any* given level of reliability Alt-A is *less* expensive than Alt-B.
- Alt-B is a *Bi-modal* Distribution
- Alt-B is *twice* as variable as Alt-A

43

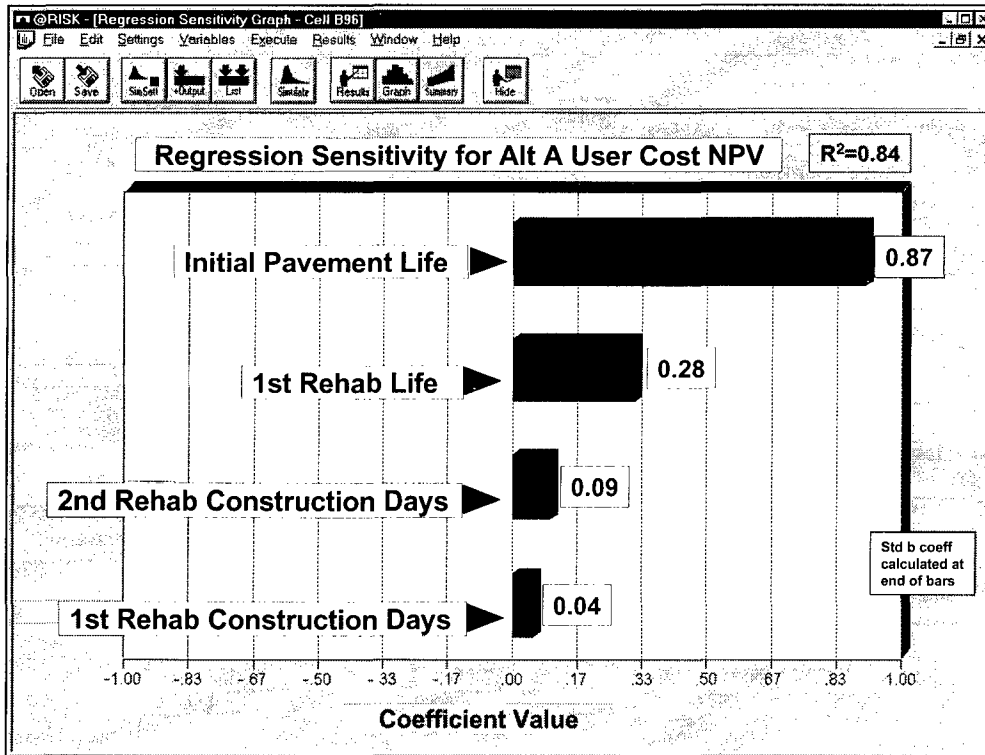
User Costs

- Probability
- Sensitivity

44



45



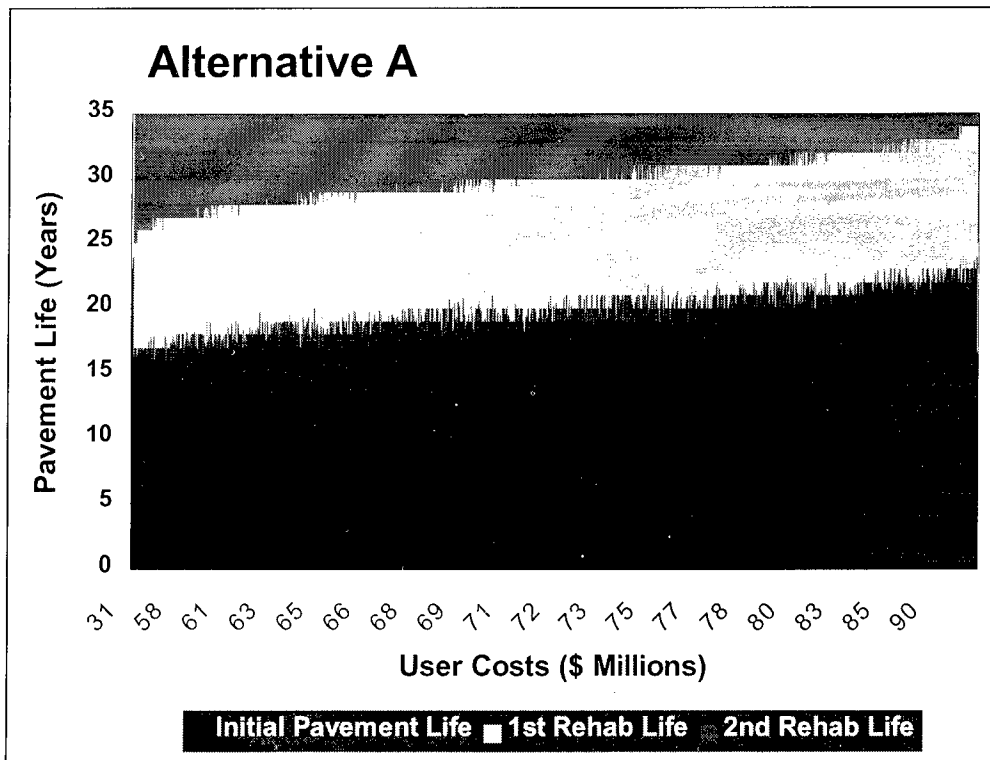
46

The Graph says ...
When Initial Pavement
Life Samples High ...

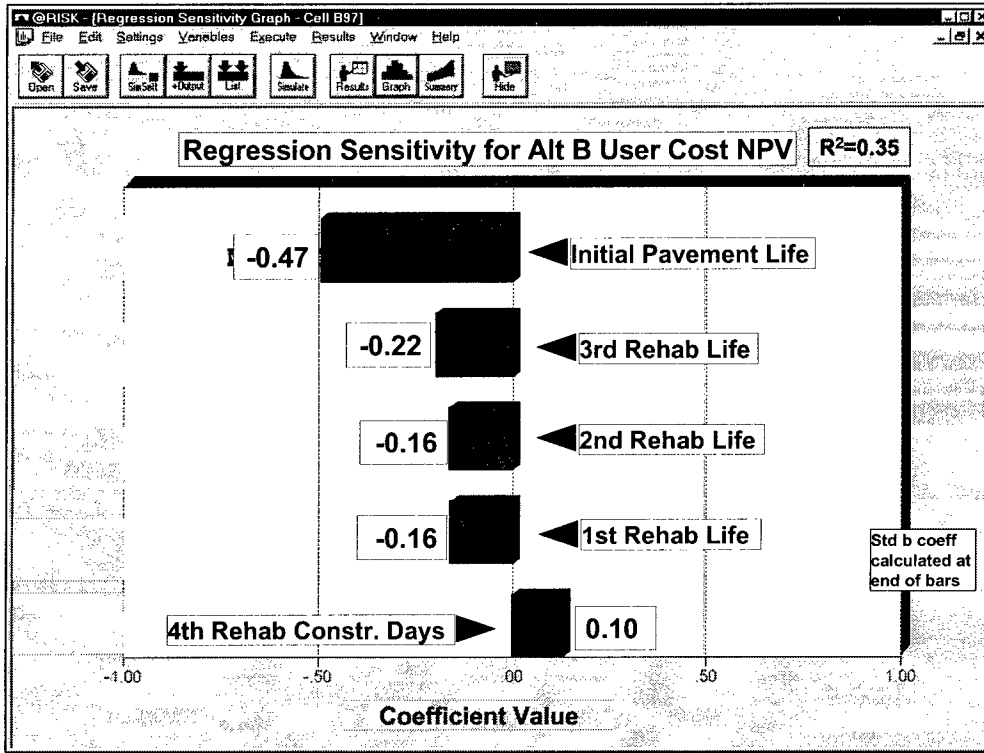
High User Costs are
Produced.

Does this make sense?

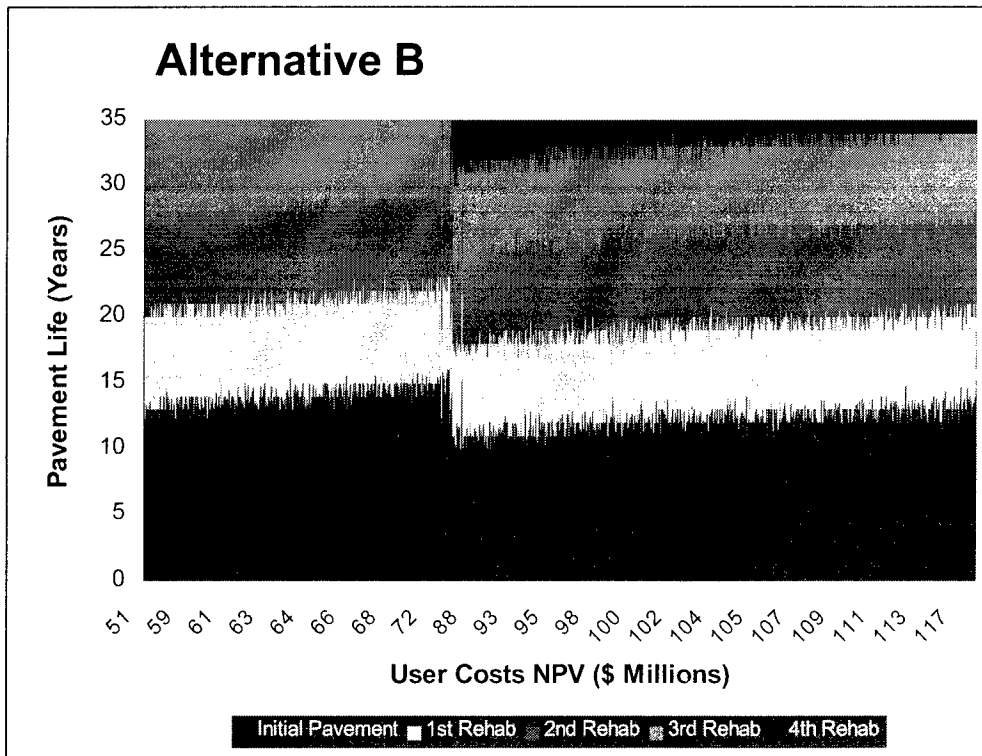
47



48



49



50

Sensitivity Observations
<i>For both Alt A & B User Costs ...</i> <ul style="list-style-type: none">■ Initial Pavement Life has the greatest influence

51

Sensitivity Observations Cont'd
Alternative A <ul style="list-style-type: none">■ The variability of pavement life was such that two rehabs <i>a/ways</i> occurred during analysis period■ As a result lower pavement lives produced lower user costs.

52

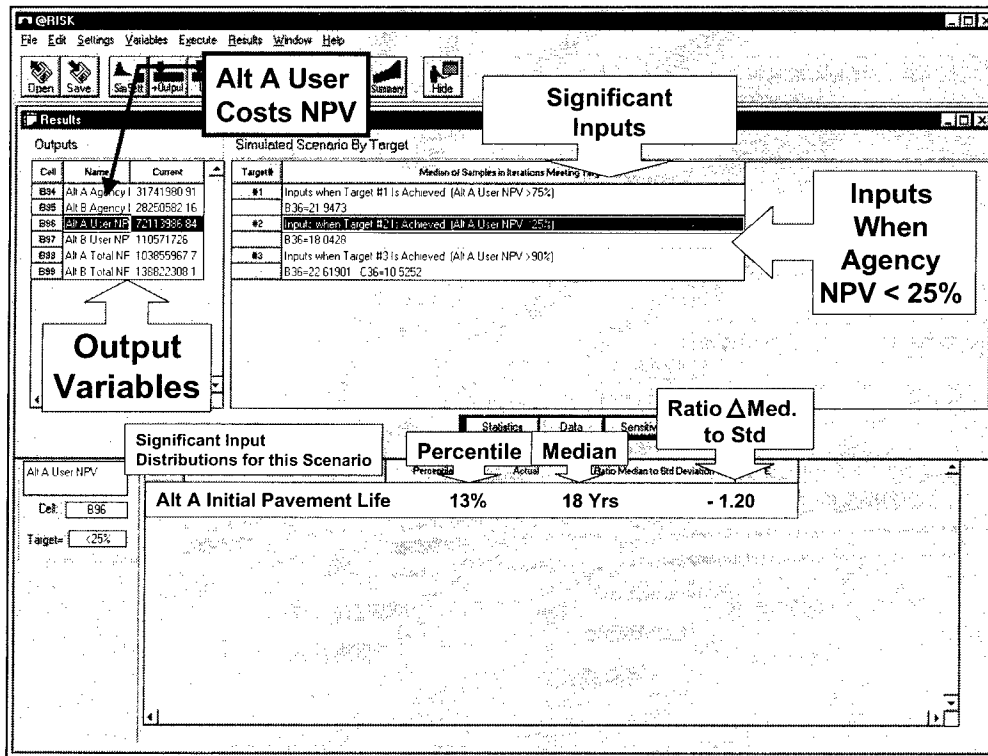
Sensitivity Observations Cont'd
Alternative B <ul style="list-style-type: none">■ The variability of pavement life was such that 3 and 4 rehabs occurred during analysis period■ This caused a Bi-modal Distribution■ As a result lower pavement lives produced higher user costs.

53

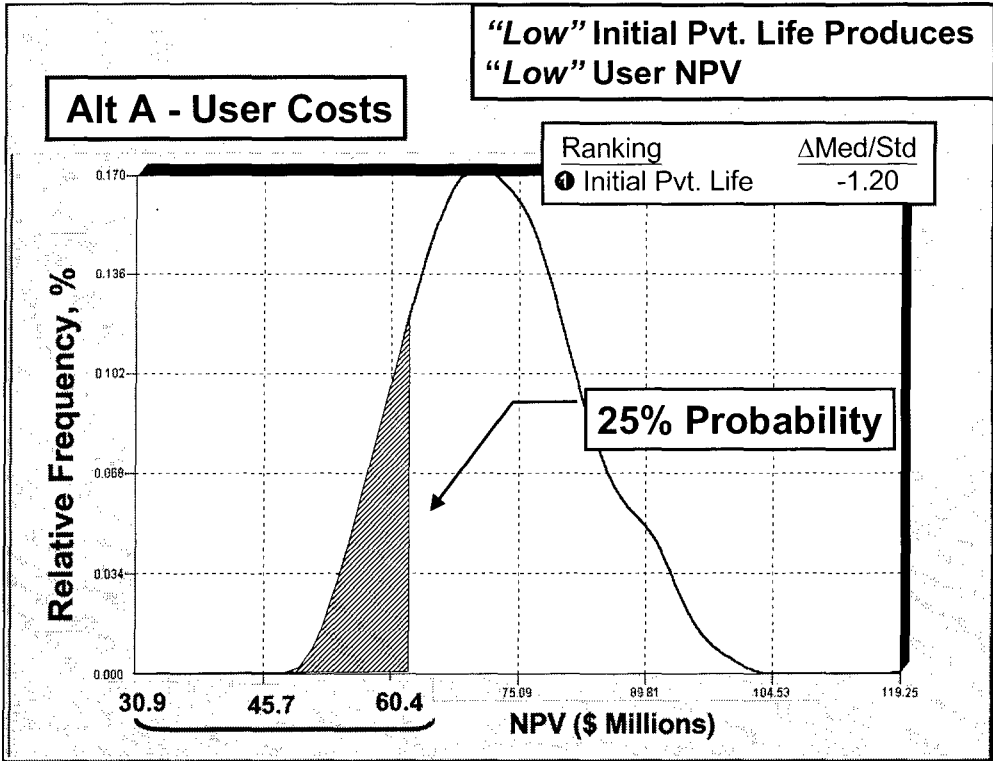
User Costs

- Probability
- Sensitivity
- Scenario Analysis

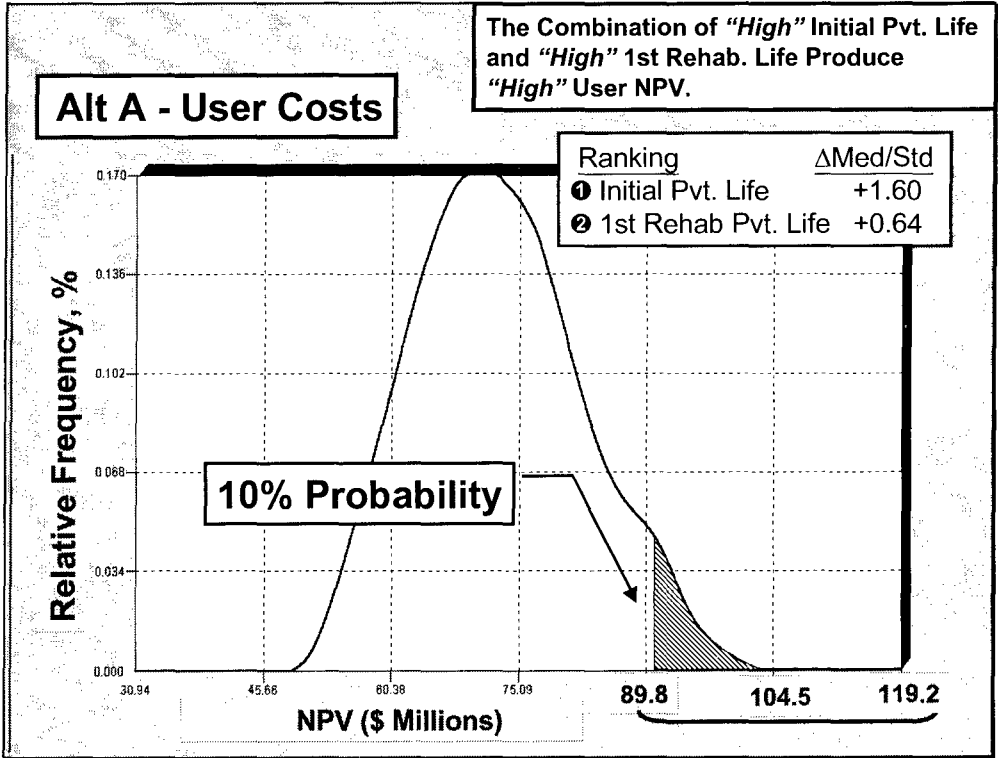
54



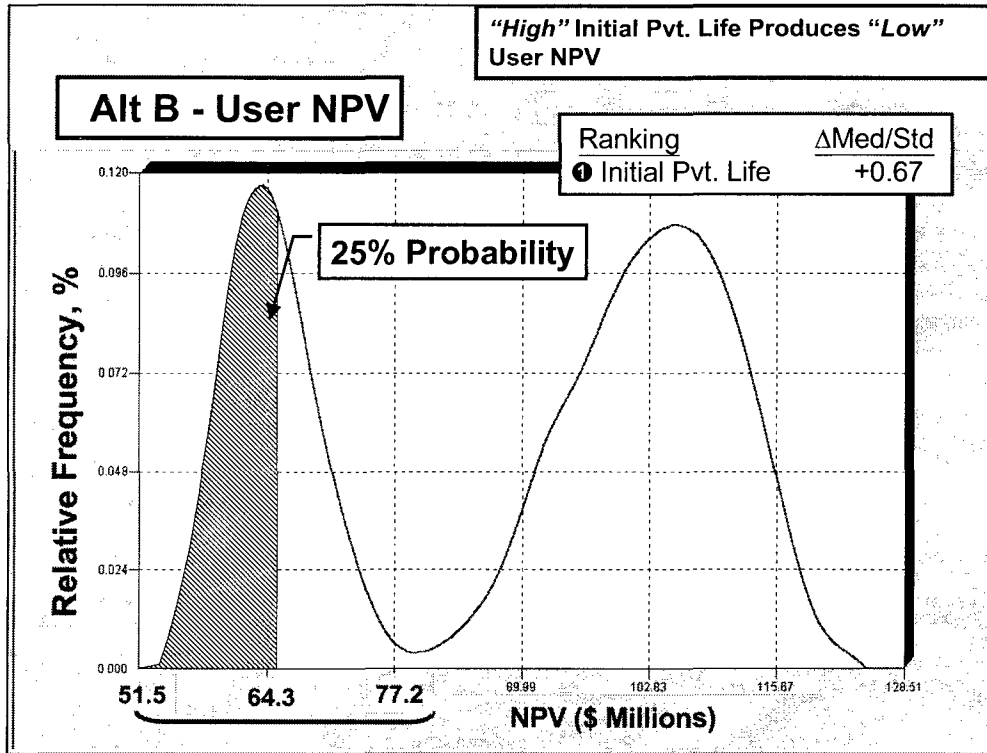
55



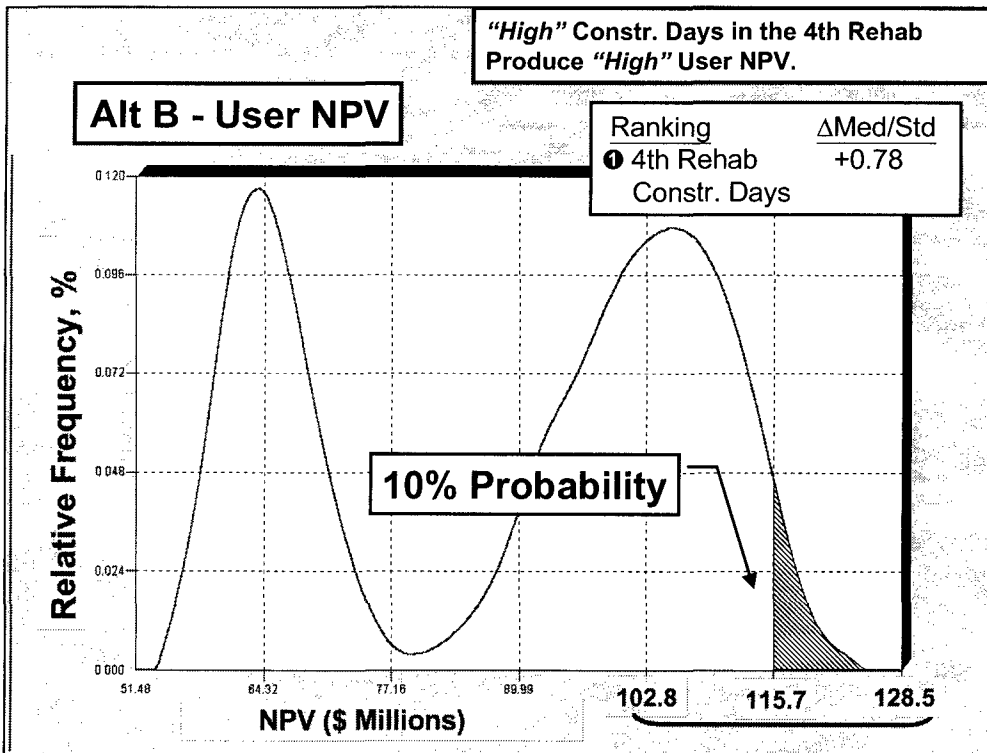
56



57



58



59

**Scenario Analysis
Observations**

User Costs Alternative A

- “Low” Initial Pavement Life produces Low User NPV
- The Combination of “Low” Initial Pvt. Life and “High” 1st Rehab. Life Produce Low User NPV

60

**Scenario Analysis
Observations**

User Costs Alternative B

- “High” Initial Pavement Life produces Low User NPV
- “High” Constr. Days in the 4th Rehab produce “High” User Costs.

61

Which Alternative would you select?

Must define Agency’s tolerance for risk.

62


**If User Costs Dominate ...
Reevaluate Alternatives**

- Decrease construction time
(accelerate contractor production)
- Lane Rental (A+B Bidding)
- Temporary bypass
- Increase shoulder strength
- Other?

63

End Session

1



Presentation Techniques

2

Know Your Audience

- Does your audience understand ...
 - LCCA?
 - Discounting?
 - User costs?
 - Value of time?

3

You need to know ...

- Do they need a Risk Primer?
- Do they buy into the risk analysis approach?
- Do they buy-in to your analysis

4

Here's Some Advice ...

- Don't bury them in statistics
- List significant inputs
 - Identify what's driving the tails of the distribution ...
 - Can you control it?
- Show results graphically

5

Report

- One page summary (???)
- Supporting documentation

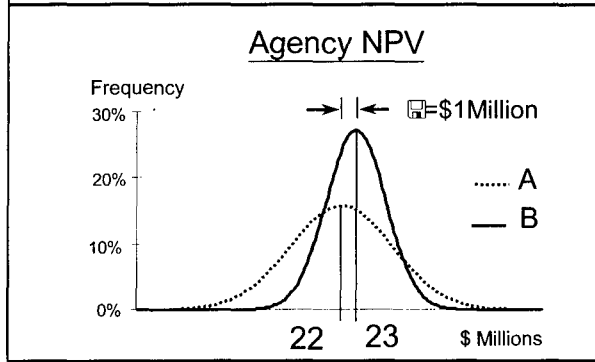
6

One Page Summary

- Inputs: List uncertain variables
- Outputs:
 - Histogram & Cumulative
 - Mean, Std. Dev., Percentiles
- Analysis of results
 - Tornado graphs, Scenario analysis
- Recommendations
 - Include level of risk

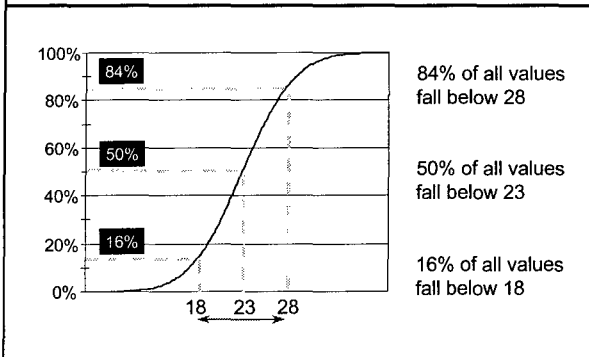
7

Histogram -



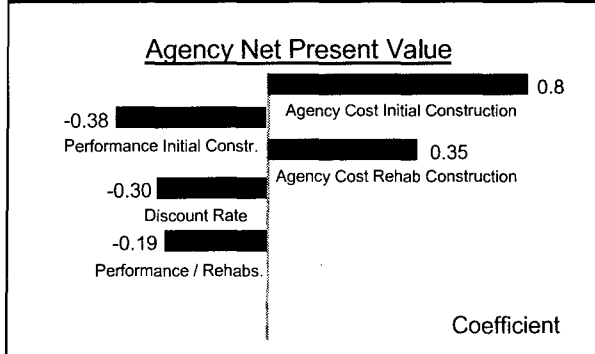
8

Cumulative -



9

Regression Sensitivity (Tornado Graphs)



10

Supporting Documentation

- Distributions with supporting justifications
- Structure and layout of model
- Sensitivity analysis of proposed distributions
- Analysis of uncertain events

11

End Session

1

Benefits
and
Implementation

A cartoon character of a man in a dark suit and white shirt, with his arms outstretched as if presenting or explaining something.

2

Benefits of LCCA

- Informed decisions
- Sensitivity to user costs
- More effective use of resources
- Support funding requests

3

Benefits of LCCA Con't

- Objective basis for resource allocation
 - Network, project, & design
- Assess funding consequences

4

Benefits of Risk Analysis

- Better design strategies
- Improved design procedures
- Effective engineering input to policy decisions

5

Benefits of RA Cont'd

- Expose areas of uncertainty
- Quantify risk
- Opportunity for mitigating action
- Improved credibility
- Assess impact of risk on investment decisions
- Avoid disasters

6

Benefits of RA Cont'd

- Determine significance of difference between alternatives
- Examine influence of underlying variables on final results
- Evaluate all possible outcomes

7

Caveats

- New concept
- Requires statistical background
- Computer intensive
 - Proprietary software
 - Complex models
- Requires risk management
“buy in” by senior executives



8

Implementation

9

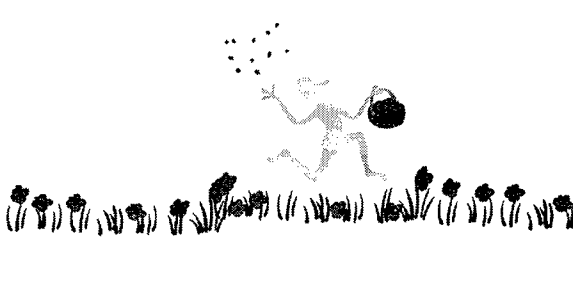
Obstacles

- Lack of awareness
- Resistance to change
- Time pressures
- Lack of communication
- Unavailability of resources

10

Implementation Steps
<ul style="list-style-type: none">■ Awareness■ Change■ Communication■ Resources

11

<h2 style="text-align: center;">Plant the Seed</h2> 
--

12

Four Stages of Learning
<ul style="list-style-type: none">① Unconscious Incompetence② Conscious Incompetence③ Conscious Competence④ Unconscious Competence

13

Implementation Steps
<ul style="list-style-type: none">■ Identify a champion■ Understand classical LCCA■ Assess current procedures■ Determine data availability■ Tap expert opinion

14

Probabilistic Champion
<ul style="list-style-type: none">■ Believer■ Well founded in LCCA■ Spreadsheet literate■ Time available

15

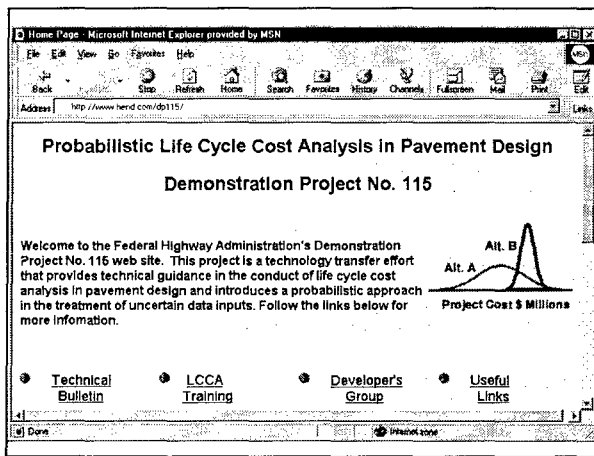
Resources
<ul style="list-style-type: none">■ Equipment■ Software■ Personnel■ Training■ User groups

16

Resources Con't

- DP 115 Case Study States
 - Two contacts per state
 - List of names & address available upon request
- DP 115 Web Site
<http://www.hend.com/dp115>

17



18

Top Management Support

- Establish objectives
- Provide policy input
- Provide resources
- Provide "Bureaucratic Clout"



19

Steering Committee
<ul style="list-style-type: none">■ LCCA procedure■ Probabilistic approach

20

Documentation
<ul style="list-style-type: none">■ Standardize SHA's approach to LCCA■ Document SHA LCCA procedures■ Apply consistently

21

End Session



1



Workshop Summary



2

Key Areas Covered


- Traditional LCCA
- User Cost
- Risk Analysis Approach



3

Things to Remember

- LCCA decision support tool
- NHS LCCA requirements
- Document procedures
- Document inputs
- Dispose of all issues




4

Recommendations

LCCA ...

- Long analysis periods
- Constant dollars
- Real discount rates (3-5%)
- NPV




5

Recommendations

■ Value of time

- Passenger \$10 - \$13
- Single Unit Trk \$17 - \$20
- Combo Trk \$21 - \$24




6

Recommendations

Agency Costs ...

- Include agency overhead
- Ignore sunk cost
- Don't sweat reactive maintenance and salvage value



7

Recommendations

User Costs ...

- Traffic grows
- Queuing cost dominate
- Hourly distributions key
- \$ Value of time major influence
- Circuitry can be major



8

Recommend a risk analysis approach in the treatment of uncertainty.



9

The End





Class Exercise No. 1

Net Present Value

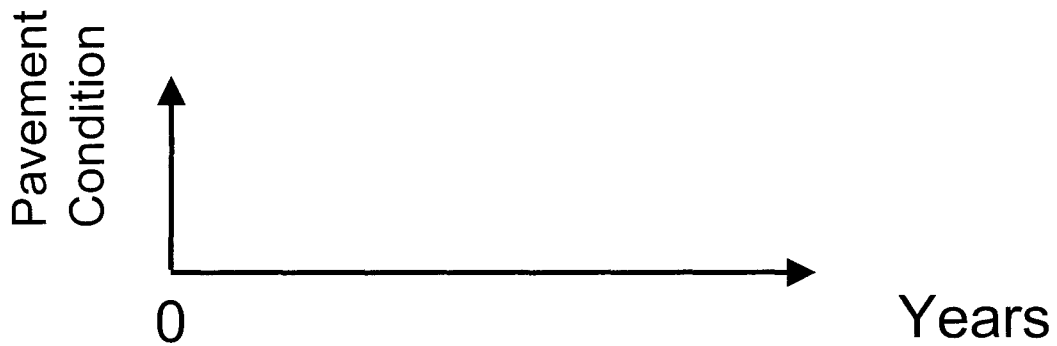
Compute the Net Present Value (NPV) for the following Alternative.

	Initial Constr.	Rehab.
Design Period, (yrs)	20	10
Agency Cost (\$ Millions)	26	9
Construction Period (days)	210	105

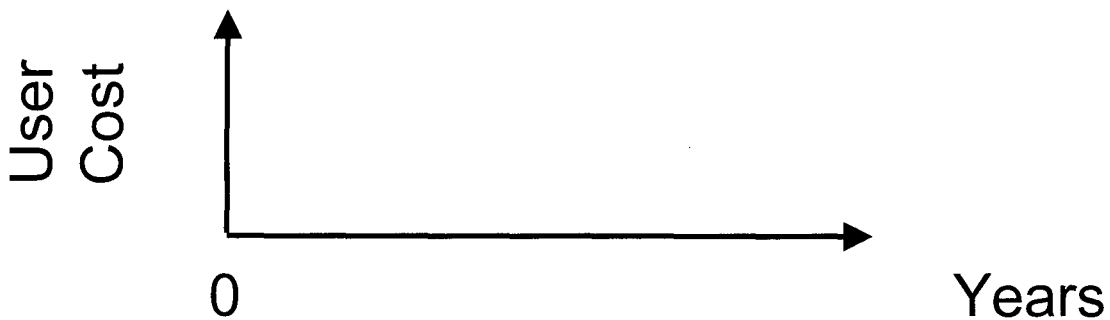
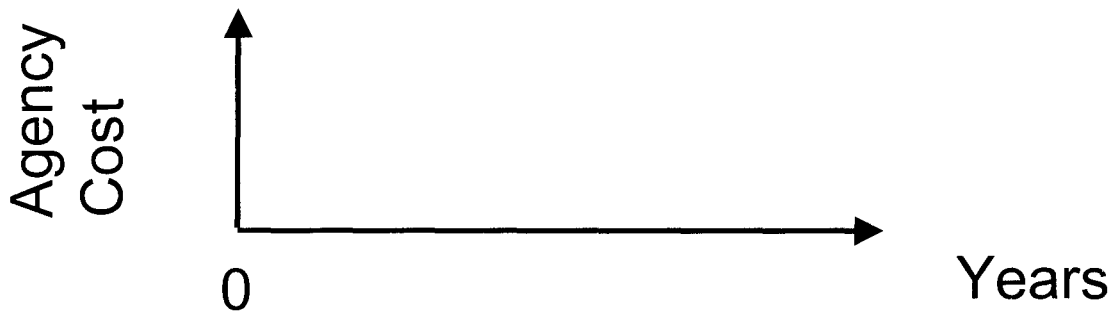
Assume the following:

Analysis Period (yrs)	30
Daily Delay (Hours)	3000
Routine Maintenance	Insignificant
Discount Rate, %	4
Value of Time (\$/hr)	10

Performance Curve



Expenditure Streams



User Cost Calculations

$$\text{User Cost} = \text{Construction Days} \times \text{Daily Delay} \times \text{Value of Time}$$

Initial Construction

$$\begin{aligned} \text{User Cost} &= \text{_____ days} \times \text{_____ hrs/day} \times \text{_____ \$/ hr.} \\ &= \$ \text{_____} \end{aligned}$$

Rehabilitation

$$\begin{aligned} \text{User Cost} &= \text{_____ days} \times \text{_____ hrs/day} \times \text{_____ \$/ hr.} \\ &= \$ \text{_____} \end{aligned}$$

$$NPV = InitialCost + \sum_{k=1}^N FutureCost \left[\frac{1}{(1+i)^{n_k}} \right]$$

Note: Quantity in brackets is present value factor from page 1.5.

Agency Cost

NPV =

User Cost

NPV =

Present Value Factors

Year n	Discount Rate, (i)				
	4.0%	4.5%	5.0%	5.5%	6.0%
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4388	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301



Class Exercise No. 2

Net Present Value

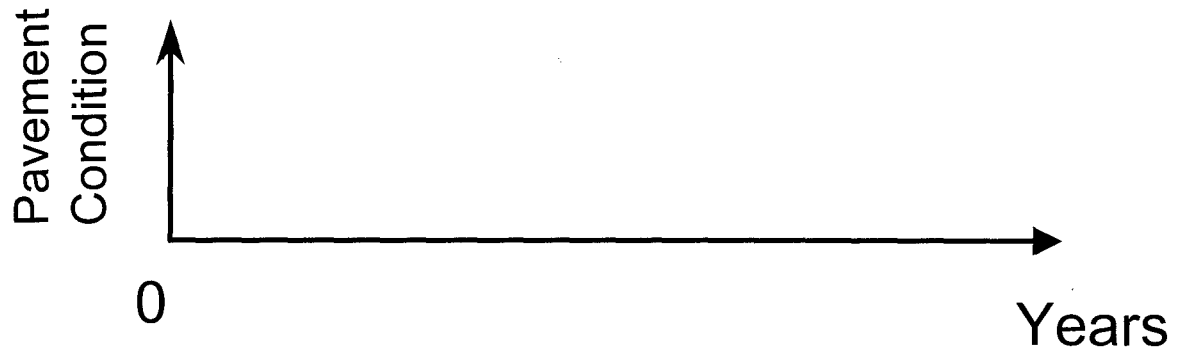
Compute the Net Present Value (NPV) for the following Alternative.

	Initial Constr.	Rehab.
Design Period, (yrs)	20	8
Agency Cost (\$ Millions)	30	9
Construction Period (days)	200	80

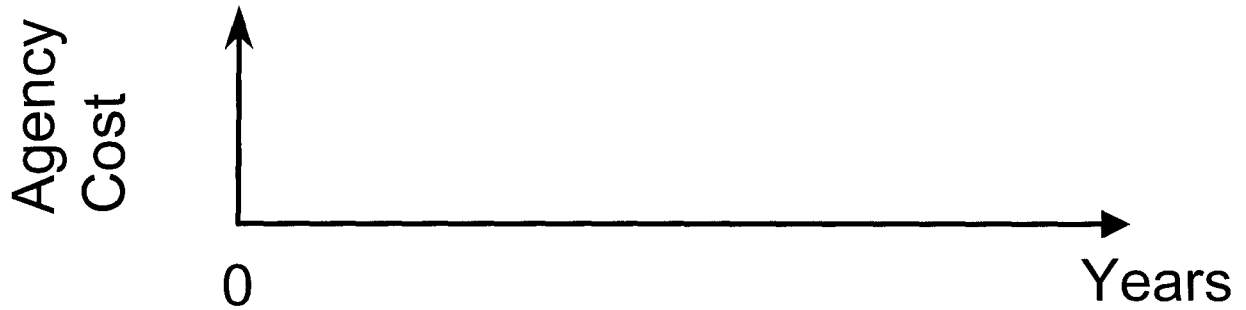
Assume the following:

Analysis Period (yrs)	30
Daily Delay (Hours)	4000
Routine Maintenance	Insignificant
Discount Rate, %	4
Value of Time (\$/hr)	15

Performance Curve



Expenditure Streams



$$NPV = InitialCost + \sum_{k=1}^N FutureCost \left[\frac{1}{(1+i)^{n_k}} \right]$$

Note: Quantity in brackets is present value factor from page 2.5.

Agency Cost

NPV =

User Cost

NPV =

Present Value Factors

Year n	Discount Rate, (i)				
	4.0%	4.5%	5.0%	5.5%	6.0%
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4388	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301



Class Exercise No. 3

Work Zone User Cost

The eastbound lanes of a six-lane facility are undergoing rehabilitation. Figures 1 and 2 provide a layout of the work zone and the associated user cost components. The facility carries 95,000 vehicles per day of which 90% are passenger cars, 6% single unit trucks, and 4% combination unit trucks. The directional factor is 54% for the eastbound direction. A 7 mile work zone closing one lane will be in place 24 hours each day until construction is complete. It is estimated to take 75 days to complete construction. The upstream approach speed is posted at 55 mph and the speed through the work zone will be posted at 35 mph. The free flow capacity of the roadway is estimated at 2100 vehicles per hour per lane (vphpl) while the work zone capacity is estimated at 1400 vphpl. A capacity analysis of the work zone is shown in Table 1. This class exercise includes three separate problems identified below.

Problem ①

Determine the quantity of traffic associated with each work zone user cost component.

Problem ②

Determine the reduced speed delay to traverse the work zone and queue.

Problem ③

Calculate the user costs associated with the work zone.

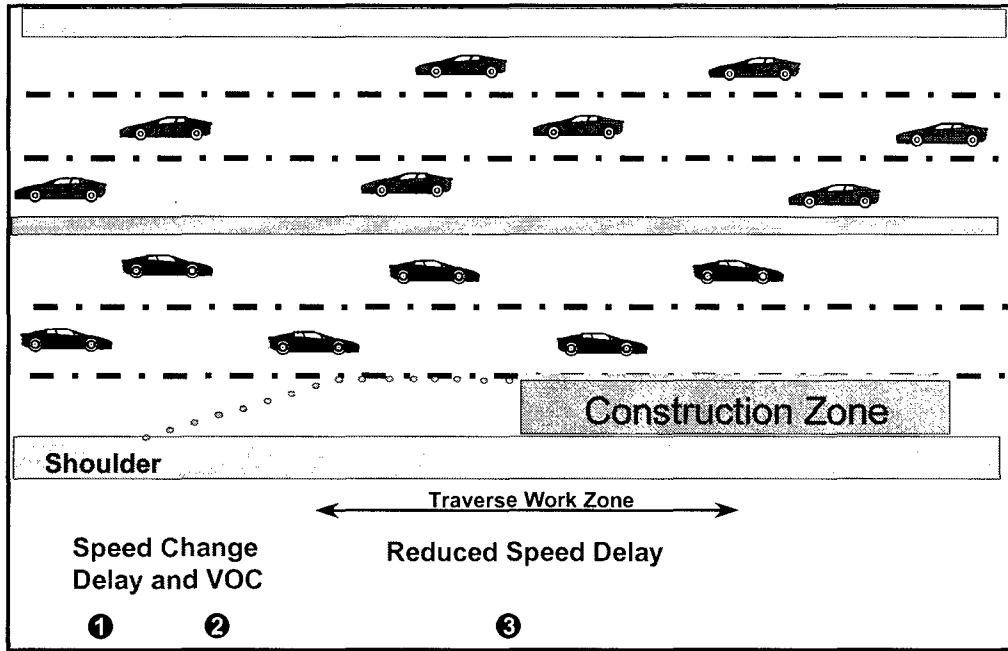


Figure 1. Eastbound Work Zone User Cost Components at Free Flow Conditions.

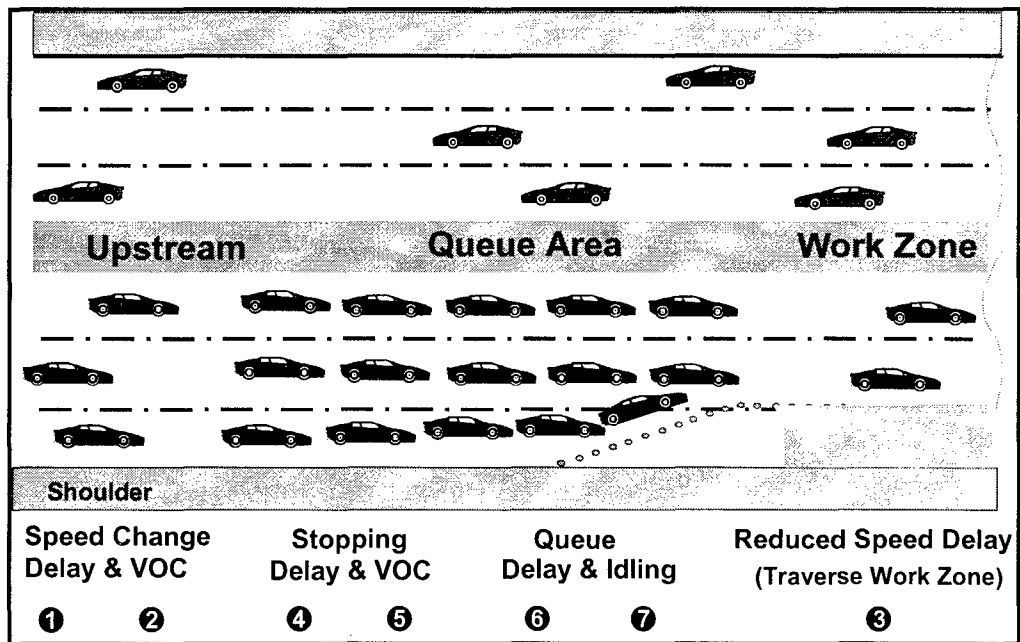


Figure 2. Eastbound Work Zone User Cost Components at Forced Flow Conditions.

Table 1. Capacity Analysis of Work Zone Operation.

			Eastbound				AADT		95,000	
Directional Factor			54.0%		Directional AADT		51,300			
			Hrly	Vehicle		Queuing	Culm.	Vehicles that ...		
			Distri.	Demand	Capacity	Rate	Que Veh.	Stop	Traverse WZ	SlowDown
Hour			%	vph	vph	vph		55-0-55	at 35 mph	55-35-55
0	-	1	0.9	462	2,800	(2,338)	0	0	462	462
1	-	2	0.5	257	2,800	(2,544)	0	0	257	257
2	-	3	0.4	205	2,800	(2,595)	0	0	205	205
3	-	4	0.4	205	2,800	(2,595)	0	0	205	205
4	-	5	0.6	308	2,800	(2,492)	0	0	308	308
5	-	6	1.8	923	2,800	(1,877)	0	0	923	923
6	-	7	4.4	2,257	2,800	(543)	0	0	2,257	2257
7	-	8	6.2	3,181	2,800	381	381	3,181	3,181	0
8	-	9	5.7	2,924	2,800	124	505	2,924	2,924	0
9	-	10	5.1	2,616	2,800	(184)	321	2,616	2,616	0
10	-	11	5.2	2,668	2,800	(132)	189	2,668	2,668	0
11	-	12	5.6	2,873	2,800	73	261	2,873	2,873	0
12	-	13	6.0	3,078	2,800	278	539	3,078	3,078	0
13	-	14	5.9	3,027	2,800	227	766	3,027	3,027	0
14	-	15	6.4	3,283	2,800	483	1,249	3,283	3,283	0
15	-	16	7.4	3,796	2,800	996	2,246	3,796	3,796	0
16	-	17	7.8	4,001	2,800	1,201	3,447	4,001	4,001	0
17	-	18	7.5	3,848	2,800	1,048	4,494	3,848	3,848	0
18	-	19	5.9	3,027	2,800	227	4,721	3,027	3,027	0
19	-	20	4.9	2,514	2,800	(286)	4,435	2,514	2,514	0
20	-	21	4.0	2,052	2,800	(748)	3,687	2,052	2,052	0
21	-	22	3.3	1,693	2,800	(1,107)	2,580	1,693	1,693	0
22	-	23	2.4	1,231	2,800	(1,569)	1,011	1,231	1,231	0
23	-	24	1.7	872	2,800	(1,928)	0	457	872	415
Total			100	51,300				46,268	51,300	5,032

Note: The number of vehicles required to stop (55-0-55 mph) during the last hour (23-24) is prorated based on the amount of time required to clear the queue remaining in the previous hour.

Problem No. 1

Using the data from the 24 hour capacity analysis shown in Table 1 quantify the traffic affected in each of the following cost components for the duration of the project.

- ① **WZ Speed Change Delay (Free Flow)**
55 ➔ 35 ➔ 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

- ② **WZ Speed Change VOC (Free Flow)**
55 ➔ 35 ➔ 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

- ③ **Reduced Speed Delay Base Case (Free Flow)**
35 vs 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

Note: Problem No. 1 continued on next page.

Problem No. 1 Continued

- ④ Queue Stopping Delay (Forced Flow)
55 → 0 → 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

- ⑤ Queue Stopping VOC (Forced Flow)
55 → 0 → 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

- ⑥ Queue Added Travel Delay
Queue Speed vs. 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

- ⑦ Queue Idle VOC

Vehicle Class	Eastbound Number of Vehicles
90% Auto	
6% Single Unit	
4% Combination	

Problem No. 2

Compute the reduced speed delay to traverse the work zone.

		Work Zone Length	Time
Work Zone Speed	_____ mph	_____ mi.	_____ hrs.
Upstream Speed	_____ mph	_____ mi.	_____ hrs.
A Increased Time to Traverse the Work Zone:			_____ hrs.

Compute the reduced speed delay to traverse the queue.

Computation Step		Value
❶ Maximum number of queued vehicles		
Queue Volume (V_Q) "Capacity of Work Zone"		
Queue Capacity (C_Q) "Upstream Capacity"		
V_Q/C_Q		
Queue Speed (S_Q) (See Figure 3 Page 3.7)		
❷ Work Zone Density (V_Q/S_Q)		
Upstream Volume (V_U) "Demand at Max. No. Queued Vehicles"		
Upstream Speed (S_U)		
❸ Upstream Density (V_U/S_U)		
Δ Density (❷ - ❸)		
❹ Maximum Queue Length (❶ / Δ Density)		
❺ Average Queue Length (❹ / 2)		
Queue Travel Time	❻ $\text{❺} / \text{Queue Speed } (S_Q)$ (hrs)	
	❼ $\text{❺} / \text{Upstream Speed } (S_U)$ (hrs)	
B Queue Delay (hrs) (❻ - ❼)		

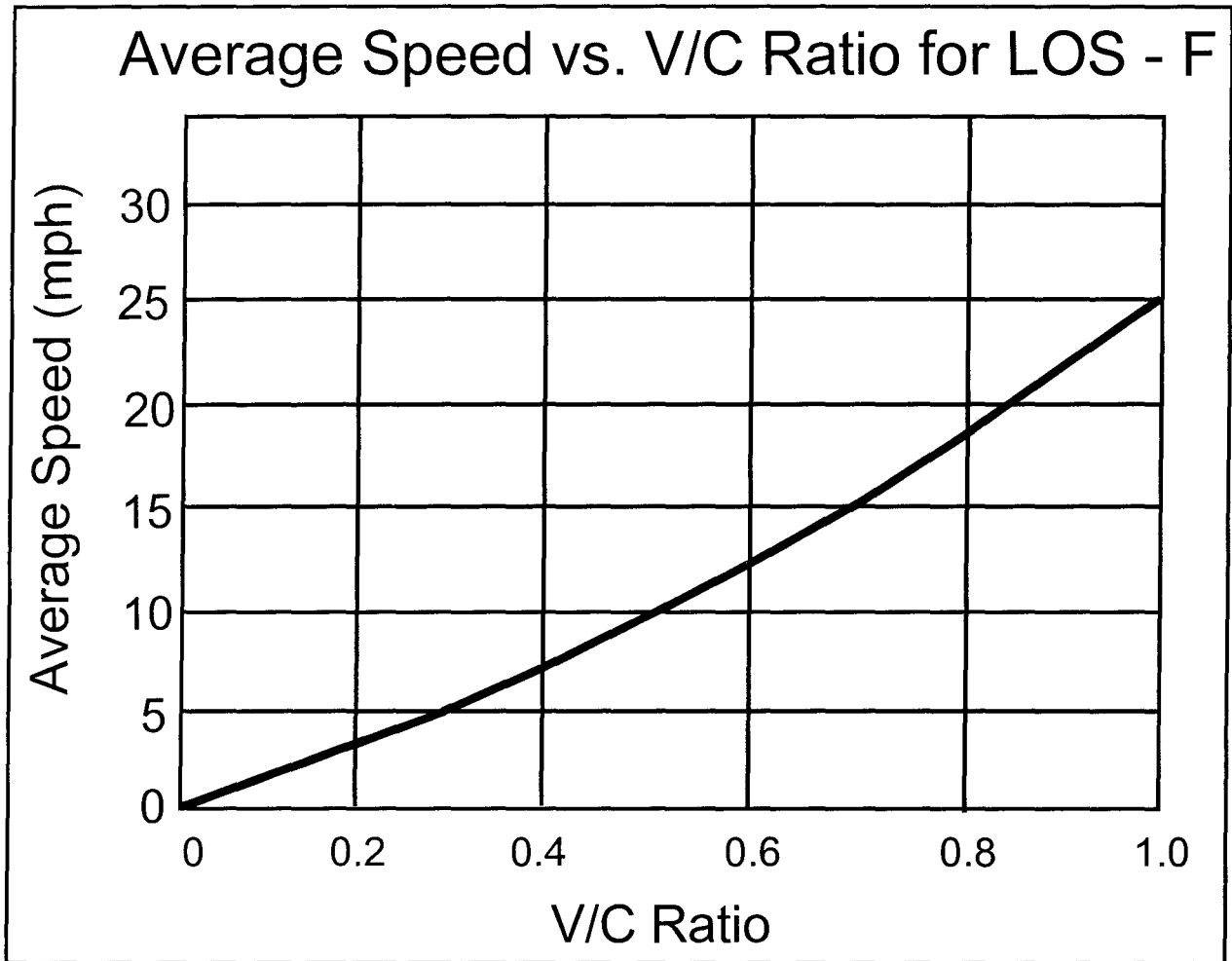


Figure 3. V/C Ratio versus Average Queue Speed (Source: NCHRP 133).

$V = V_Q =$ Capacity in Work Zone

$C = C_Q =$ Upstream Capacity

Problem No. 3

Compute the following user costs associated with the work zone.

Cost Component	Veh. Class	Number Vehicles	Added VOC	Added Time (Hrs/Veh.)	Value of Time ⁽⁴⁾ (\$/Veh. Hr)	Cost (\$)	%
①WZ Speed Change Delay (55-35-55)	Auto			/1000 ^(1,3)			
	SU			/1000 ^(1,3)			
	Combo			/1000 ^(1,3)			
②WZ Speed Change VOC (55-35-55)	Auto		/1000 ^(1,2)				
	SU		/1000 ^(1,2)				
	Combo		/1000 ^(1,2)				
③WZ Reduced Speed Delay (35 vs. 55)	Auto			(5)			
	SU			(5)			
	Combo			(5)			
④Queue Stopping Delay (55-0-55)	Auto			/1000 ^(1,3)			
	SU			/1000 ^(1,3)			
	Combo			/1000 ^(1,3)			
⑤Queue Stopping VOC (55-0-55)	Auto		/1000 ^(1,2)				
	SU		/1000 ^(1,2)				
	Combo		/1000 ^(1,2)				
⑥Queue Added Travel Time Delay (Queue Speed vs 55)	Auto			(6)			
	SU			(6)			
	Combo			(6)			
⑦Queue Idle VOC	Auto		(7)	(6)			
	SU		(7)	(6)			
	Combo		(7)	(6)			
Total Work Zone User Cost >>>>>							

Notes:

1. See Table 2 Page 3.9 for added time and vehicle running cost.
2. Speed change VOC typically given as \$/1000 Veh.
3. Speed change delay typically given as Hr/1000 Veh.
4. See Table 3 Page 3.9 for recommended values of time.
5. See Problem No.2 Page 3.6 Answer **A**
6. See Problem No. 2. Page 3.6 Answer **B**
7. See bottom of Table 2 on Page 3.9 for Idle cost rates. Note Idling cost units \$/Veh.-Hr.

Table 2. Added Time and Vehicle Running Cost / 1000 Stops and Idling Costs (August 1996).

Initial Speed (mph)	Added Time (Hr / 1000 Stops) (Excludes Idling Time)			Added Cost (\$/1000 Stops) (Excludes Idling Time)		
	Pass.	Trucks		Pass.	Trucks	
	Cars	Single	Combination	Car	Single	Combination
5	1.02	0.73	1.10	2.70	9.25	33.62
10	1.51	1.47	2.27	8.83	20.72	77.49
15	2.00	2.20	3.48	15.16	33.89	129.97
20	2.49	2.93	4.76	21.74	48.40	190.06
25	2.98	3.67	6.10	28.67	63.97	256.54
30	3.46	4.40	7.56	36.10	80.23	328.21
35	3.94	5.13	9.19	44.06	96.88	403.84
40	4.42	5.87	11.09	52.70	113.97	482.21
45	4.90	6.60	13.39	62.07	130.08	562.14
50	5.37	7.33	16.37	72.31	145.96	642.41
55	5.84	8.07	20.72	83.47	160.89	721.77
60	6.31	8.80	27.94	95.70	178.98	798.99
65	6.78	9.53	NA	109.02	195.84	NA
70	7.25	NA	NA	123.61	NA	NA
75	7.71	NA	NA	139.53	NA	NA
80	8.17	NA	NA	156.85	NA	NA
Idling Cost (\$ / vehicle-hour)				0.6927	0.7681	0.8248

Table 3. Recommended Value of Time (August 1996).

Vehicles Class	\$ / Vehicle hour	
	Value	Range
Passenger	11.58	10 - 13
Single Unit	18.54	17 - 19
Combination	22.31	21 - 24



Class Exercise No. 4

Life Cycle Cost Analysis

A State highway agency is conducting a Life Cycle Cost Analysis of a 6-lane facility (3 lanes per direction). The current directional AADT is 40,000 vehicles per day. The State is considering two alternatives for the initial construction and rehabilitation strategy for one direction. Planned work zones will be in place 24 hours per day during which time the facility is reduced to 2 lanes of operation. Performance life ranges for the two alternative strategies are shown in Table 1.

Compute the total Net Present Value (NPV) for each alternative. Use a 35 year analysis period. Include in your analysis the effect of salvage value, if applicable. Construction costs are directly related to the number of days allowed for initial construction and rehabilitation activities as shown in Figures 1 - 4. Use Table 2 to summarize your selected input values. Real opportunity cost of money to the State highway agency is 4%. The SHA estimates the value of time to be \$10 per hour. Routine reactive maintenance cost differences between alternatives are insignificant. Use the formula provided to calculate net present value. If needed use Table 3 for the appropriate discount factor. Use Table 4 to determine the daily cost of delay. Use Table 5 as a worksheet.

Table 1. Performance life ranges.

	Alternative – A				Alternative – B			
	Initial		Rehabs.		Initial		Rehabs.	
	Low	High	Low	High	Low	High	Low	High
Performance (years)	16	24	8	12	10	16	6	8

Table 2. Selected input values.

	Alternative – A			Alternative – B				
	Initial	Rehabs.		Initial	Rehabs.			
		1	2		1	2	3	4
Performance (years)								
① Construction Days¹								
② Agency Cost² (\$)								
③ Daily Delay Cost³ (\$)								
④ User Cost \$ (① x ③)								

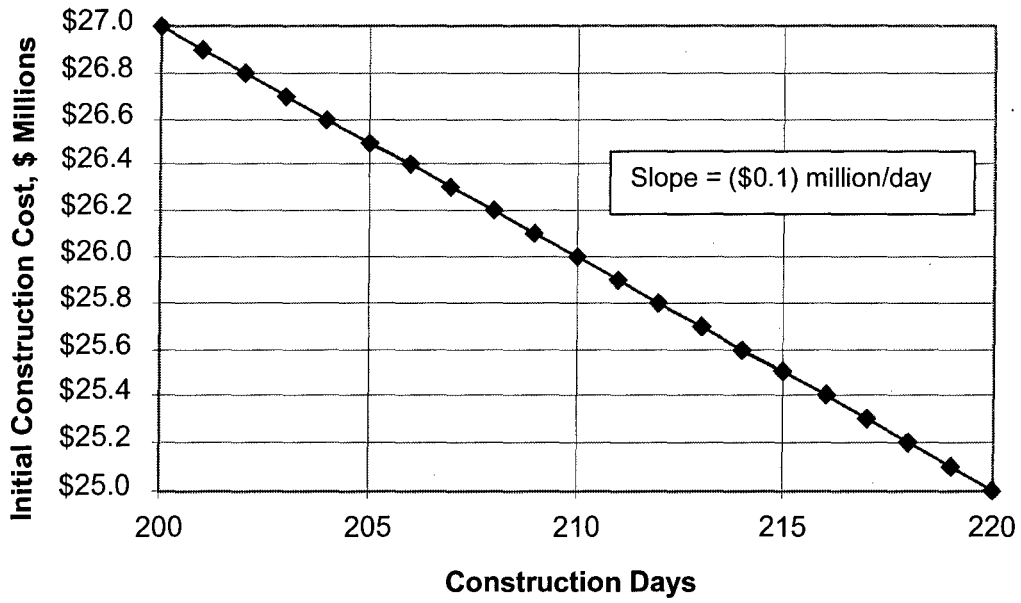
Notes: ^{1,2} See Figures 1.4

³ See Table 4 Page 4.5

Table 3. Discount factors.

Year n	Discount Rate				
	4.0%	4.5%	5.0%	5.5%	6.0%
0	1.0000	1.0000	1.0000	1.0000	1.0000
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4388	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301

**Figure 1. Initial Agency Construction Costs
Alternative A**



**Figure 2. Agency Rehabilitation Costs
Alternative A**

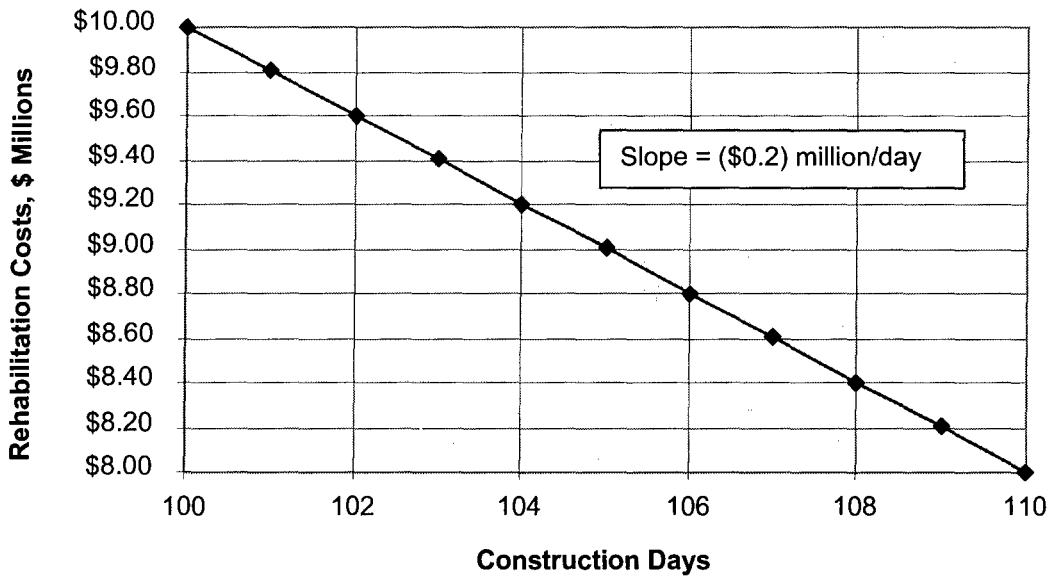


Figure 3. Initial Agency Construction Costs - Alternative B

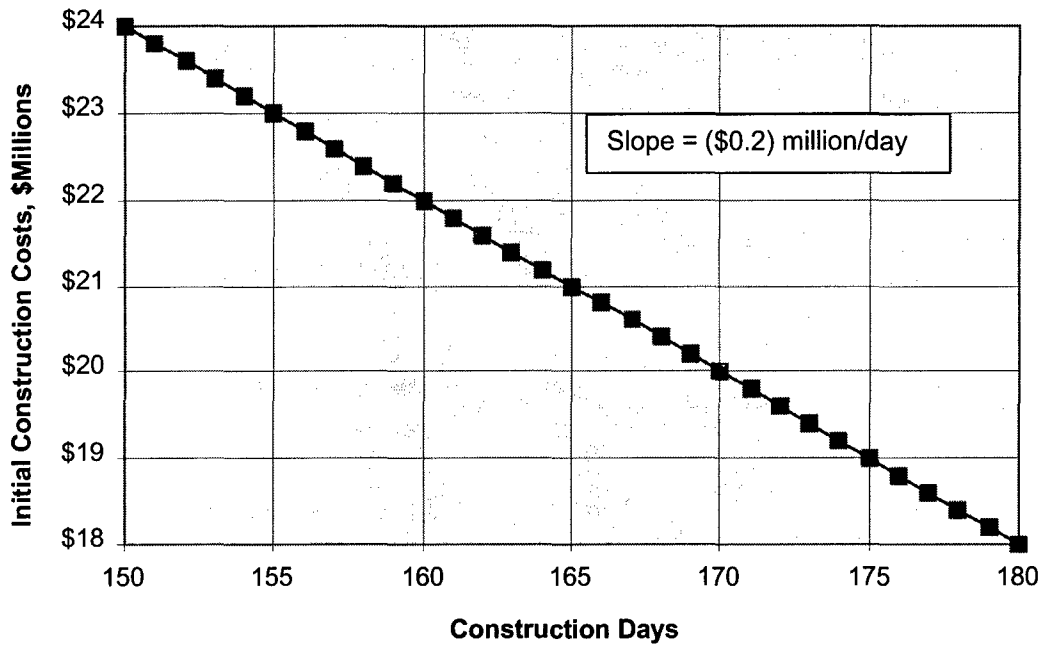


Figure 4. Agency Rehabilitation Costs Alternative - B

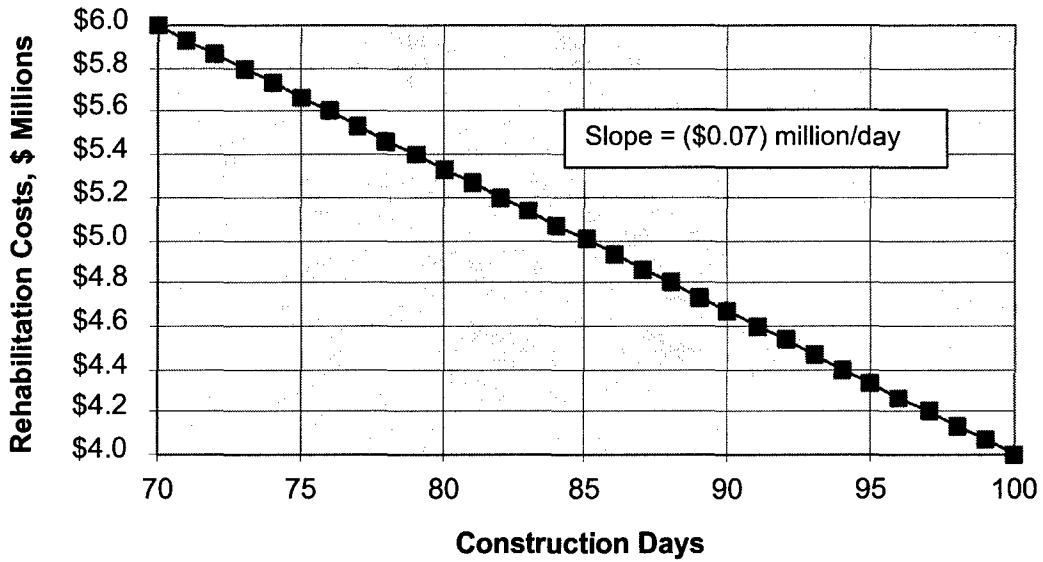


Table 4. Daily cost of delay.

		Value Time \$	10.00 per hour	
		Directional AADT(initial)	40000 vpd	
		Traffic Growth Rate	3 percent	
		Delay Per Veh. Growth Rate	10 percent	
Year	AADT	Delay/Veh. min	Daily Delay hours	Daily Cost
0	40000	5.0	3333	\$ 33,333
1	41200	5.5	3777	\$ 37,767
2	42436	6.1	4279	\$ 42,790
3	43709	6.7	4848	\$ 48,481
4	45020	7.3	5493	\$ 54,929
5	46371	8.1	6223	\$ 62,234
6	47762	8.9	7051	\$ 70,511
7	49195	9.7	7989	\$ 79,889
8	50671	10.7	9051	\$ 90,514
9	52191	11.8	10255	\$ 102,553
10	53757	13.0	11619	\$ 116,192
11	55369	14.3	13165	\$ 131,646
12	57030	15.7	14915	\$ 149,155
13	58741	17.3	16899	\$ 168,993
14	60504	19.0	19147	\$ 191,469
15	62319	20.9	21693	\$ 216,934
16	64188	23.0	24579	\$ 245,786
17	66114	25.3	27848	\$ 278,476
18	68097	27.8	31551	\$ 315,513
19	70140	30.6	35748	\$ 357,476
20	72244	33.6	40502	\$ 405,020
21	74412	37.0	45889	\$ 458,888
22	76644	40.7	51992	\$ 519,920
23	78943	44.8	58907	\$ 589,070
24	81312	49.2	66742	\$ 667,416
25	83751	54.2	75618	\$ 756,182
26	86264	59.6	85675	\$ 856,755
27	88852	65.5	97070	\$ 970,703
28	91517	72.1	109981	\$ 1,099,806
29	94263	79.3	124608	\$ 1,246,081
30	97090	87.2	141181	\$ 1,411,809
31	100003	96.0	159958	\$ 1,599,580
32	103003	105.6	181232	\$ 1,812,324
33	106093	116.1	205336	\$ 2,053,363
34	109276	127.7	232646	\$ 2,326,460
35	112554	140.5	263588	\$ 2,635,880
36	115931	154.6	298645	\$ 2,986,452
37	119409	170.0	338365	\$ 3,383,650
38	122991	187.0	383368	\$ 3,833,675
39	126681	205.7	434355	\$ 4,343,554
40	130482	226.3	492125	\$ 4,921,247

Note:
Values shown are for illustrative purposes only.

Table 5. NPV Worksheet.

	Year				
	0				
Alternative - A					
Agency Cost (Constant \$)					
Present Worth Factor					
Agency Cost (Present Worth)					
Total NPV (Agency Cost)					
User Cost (Constant \$)					
Present Worth Factor					
User Cost (Present Worth)					
Total NPV (User Cost)					
Grand Total NPV (all costs)					

	Year				
	0				
Alternative - B					
Agency Cost (Constant \$)					
Present Worth Factor					
Agency Cost (Present Worth)					
Total NPV (Agency Cost)					
User Cost (Constant \$)					
Present Worth Factor					
User Cost (Present Worth)					
Total NPV (User Cost)					
Grand Total NPV (all costs)					

Class Exercise No. 1

Net Present Value (Solution)

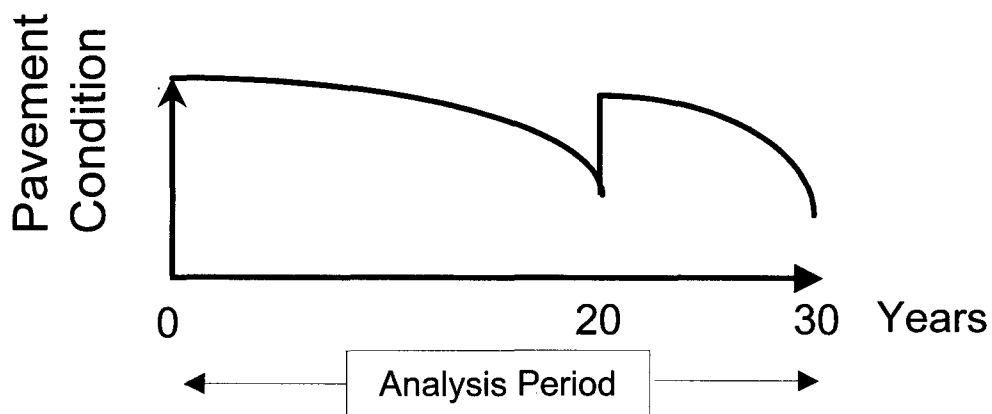
Compute the total Net Present Value (NPV) for the following Alternative.

	Initial Constr.	Rehab.
Design Period, (yrs)	20	10
Agency Cost (\$ Millions)	26	9
Construction Period (days)	210	105

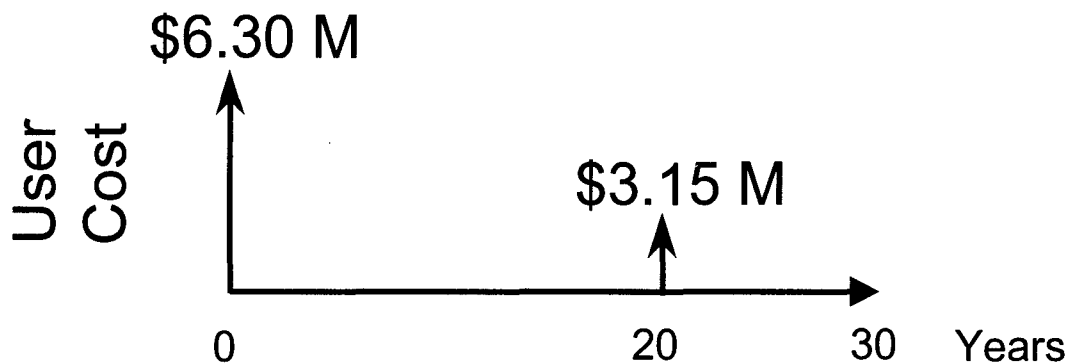
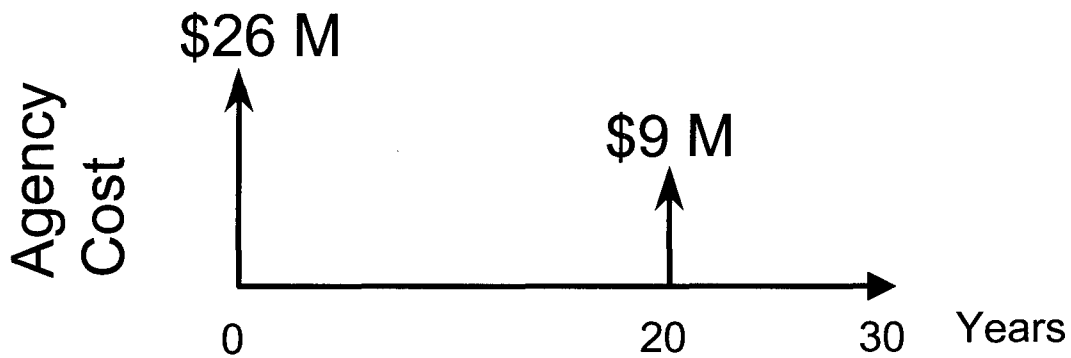
Assume the following:

Analysis Period (yrs)	30
Daily Delay (Hours)	3000
Routine Maintenance	Insignificant
Discount Rate, %	4
Value of Time (\$/hr)	10

Performance Curve



Expenditure Streams



User Cost Calculations

$$\text{User Cost} = \text{Construction Days} \times \text{Daily Delay} \times \text{Value of Time}$$

Initial Construction

$$\begin{aligned}\text{User Cost} &= 210 \text{ days} \times 3000 \text{ hrs/day} \times \$10 / \text{hr.} \\ &= \underline{\$6.30 \text{ Million}}\end{aligned}$$

Rehabilitation

$$\begin{aligned}\text{User Cost} &= 105 \text{ days} \times 3000 \text{ hrs/day} \times \$10 / \text{hr.} \\ &= \underline{\$3.15 \text{ Million}}\end{aligned}$$

$$NPV = InitialCost + \sum_{k=1}^N FutureCost \left[\frac{1}{(1+i)^{n_k}} \right]$$

Note: Quantity in brackets is present value factor from page 1.5.

Agency Cost

$$NPV = \$26 + \$9 (0.4564) = \$30.12 \text{ M}$$

User Cost

$$NPV = \$6.3 + \$3.15 (0.4564) = \$7.74 \text{ M}$$

Present Value Factors

Year n	Discount Rate, (i)				
	4.0%	4.5%	5.0%	5.5%	6.0%
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4389	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301



Class Exercise No. 2

Net Present Value (Solution)

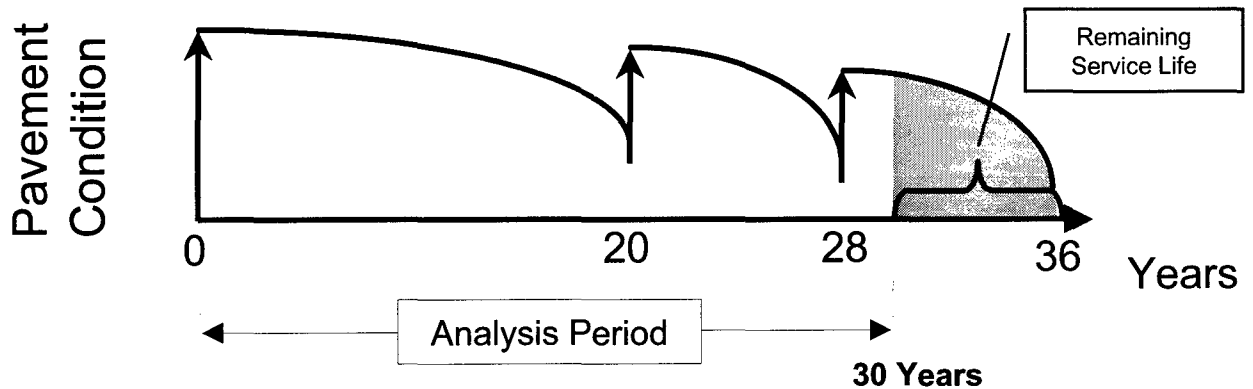
Compute the total Net Present Value (NPV) for the following Alternative.

	Initial Constr.	Rehab.
Design Period, (yrs)	20	8
Agency Cost (\$ Millions)	30	9
Construction Period (days)	200	80

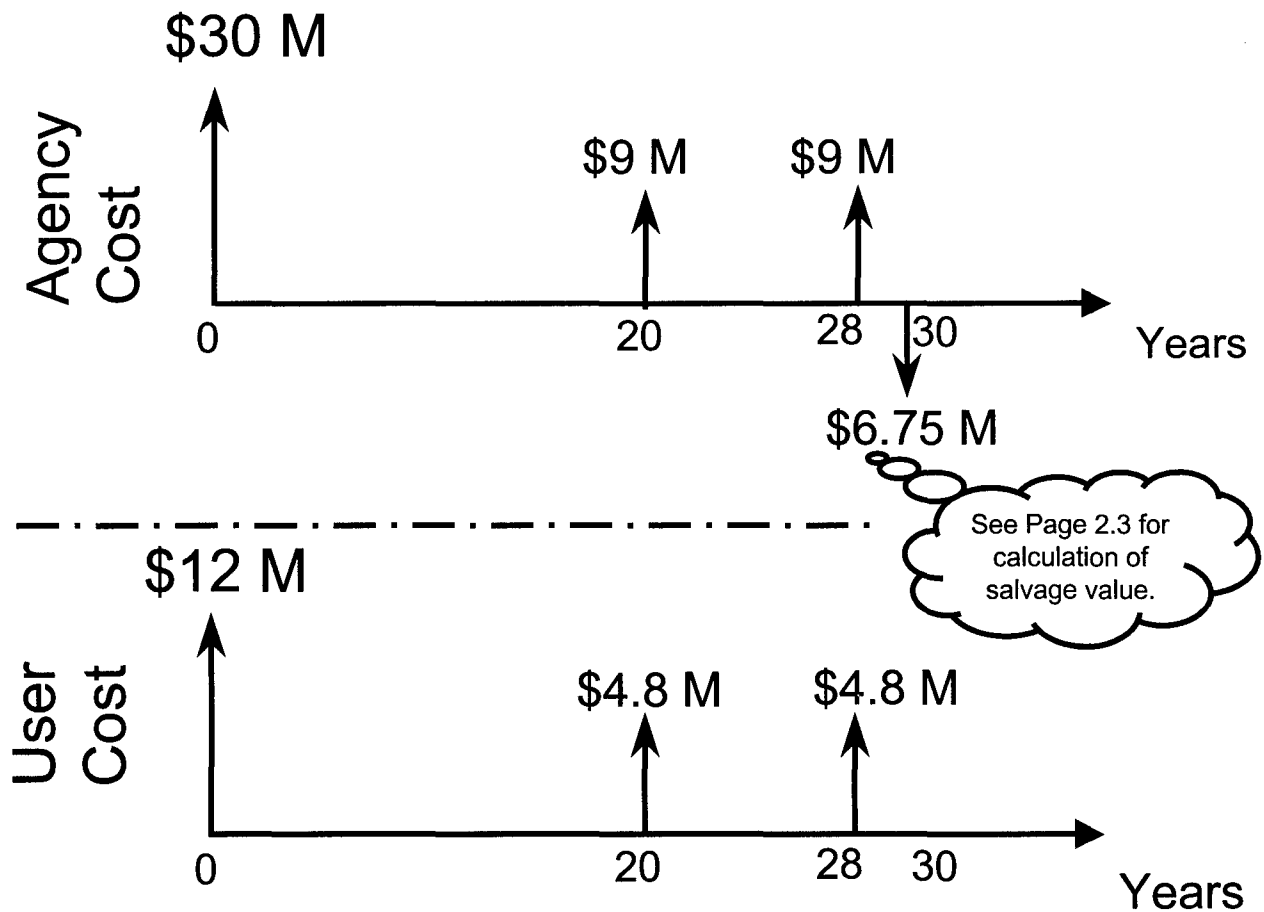
Assume the following:

Analysis Period (yrs)	30
Daily Delay (Hours)	4000
Routine Maintenance	Insignificant
Discount Rate, %	4
Value of Time (\$/hr)	15

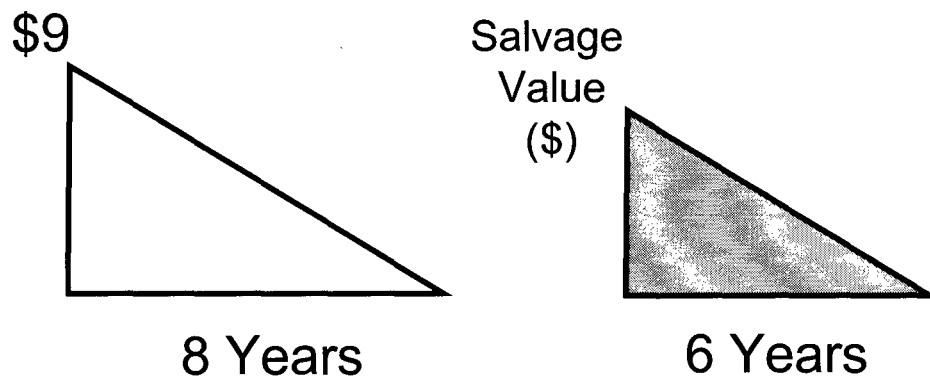
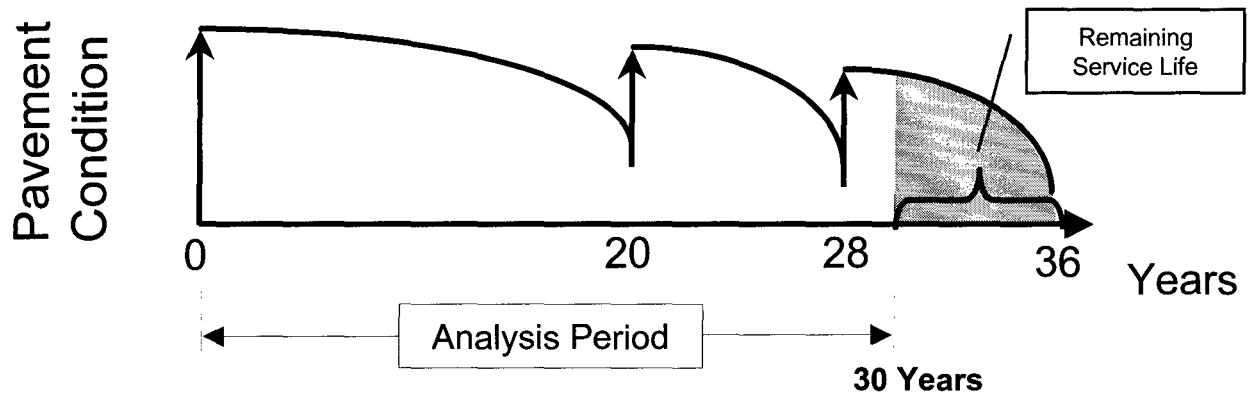
Performance Curve



Expenditure Streams



Salvage Value Calculation



$$\text{Salvage Value} = (9 / 8) * 6 = \$ 6.75$$

User Cost Calculations

$$\text{User Cost} = \text{Construction Days} \times \text{Daily Delay} \times \text{Value of Time}$$

Initial Construction

$$\text{User Cost} = 200 \text{ days} \times 4000 \text{ hrs/day} \times \$15 / \text{hr.}$$

$$= \underline{\$12 \text{ Million}}$$

Rehabilitation

$$\text{User Cost} = 80 \text{ days} \times 4000 \text{ hrs/day} \times \$15 / \text{hr.}$$

$$= \underline{\$4.8 \text{ Million}}$$

$$NPV = InitialCost + \sum_{k=1}^N FutureCost \left[\frac{1}{(1+i)^{n_k}} \right]$$

Note: Quantity in brackets is present value factor from page 2.6.

Agency Cost

$$\begin{aligned} NPV &= \$30 + \$9(0.4564) + \$9 (0.3335) - \$6.75 (0.3083) \\ &= \$35.03 \text{ M} \end{aligned}$$

User Cost

$$\begin{aligned} NPV &= \$12 + \$4.8(0.4564) + \$4.8 (0.3335) \\ &= \$15.79 \text{ M} \end{aligned}$$

Present Value Factors

Year n	Discount Rate, (i)				
	4.0%	4.5%	5.0%	5.5%	6.0%
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4388	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301

Class Exercise No. 3 (Solution)

Work Zone User Cost

Problem No. 1

Using the data from the 24 hour capacity analysis shown in Table 1 quantify the traffic affected for each of the following user cost components for the duration of the project.

❶ **WZ Speed Change Delay (Free Flow)**
55 ➔ 35 ➔ 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	339,647
6% Single Unit	22,643
4% Combination	15,095

❷ **WZ Speed Change VOC (Free Flow)**
55 ➔ 35 ➔ 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	339,647
6% Single Unit	22,643
4% Combination	15,095

❸ **Reduced Speed Delay Base Case (Free Flow)**
35 vs 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	3,462,750
6% Single Unit	230,850
4% Combination	153,900

Note: Problem No. 1 continued on next page.

Problem No. 1 Continued

④ Queue Stopping Delay (Forced Flow)

55 → 0 → 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	3,123,103
6% Single Unit	208,207
4% Combination	138,805

⑤ Queue Stopping VOC (Forced Flow)

55 → 0 → 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	3,123,103
6% Single Unit	208,207
4% Combination	138,805

⑥ Queue Added Travel Delay

Queue Speed vs. 55 mph

Vehicle Class	Eastbound Number of Vehicles
90% Auto	3,123,103
6% Single Unit	208,207
4% Combination	138,805

⑦ Queue Idle VOC

Vehicle Class	Eastbound Number of Vehicles
90% Auto	3,123,103
6% Single Unit	208,207
4% Combination	138,805

Problem No. 2

Compute the reduced speed delay to traverse the work zone.

		Work Zone Length	Time
Work Zone Speed	35 mph	7 mi.	0.20000 hrs.
Upstream Speed	55 mph	7 mi.	0.12727 hrs.
A Increased Time to Traverse the Work Zone:			0.07273 hrs.

Compute the reduced speed delay to traverse the queue.

Computation Step		Value
① Maximum number of queued vehicles		4,721 vehicles
Queue Volume (V_Q) "Capacity of Work Zone"		2,800 vehicles
Queue Capacity (C_Q) "Upstream Capacity"		6,300 vehicles
V_Q/C_Q		2,800/6,300 0.44
Queue Speed (S_Q) (See Figure 3 Page 3.4)		8 mph
② Work Zone Density (V_Q/S_Q)		2,800 / 8 = 350 vpm
Upstream Volume (V_U) "Demand at Max. No. Queued Vehicles"		3,027 vehicles
Upstream Speed (S_U)		55 mph
③ Upstream Density (V_U/S_U)		3,027 / 55 = 55 vpm
Δ Density (② - ③)		295 vpm
④ Maximum Queue Length (① / Δ Density)		4,721 / 295 = 16.0 miles
⑤ Average Queue Length (④ / 2)		16.0 / 2 = 8.0 miles
Queue Travel Time	⑥ ⑤ / Queue Speed (S_Q) (hrs)	8 / 8 = 1 Hour
	⑦ ⑤ / Upstream Speed (S_U) (hrs)	8 / 55 = 0.1455 Hour
B Queue Delay (hrs) (⑥ - ⑦)		0.8548 Hour

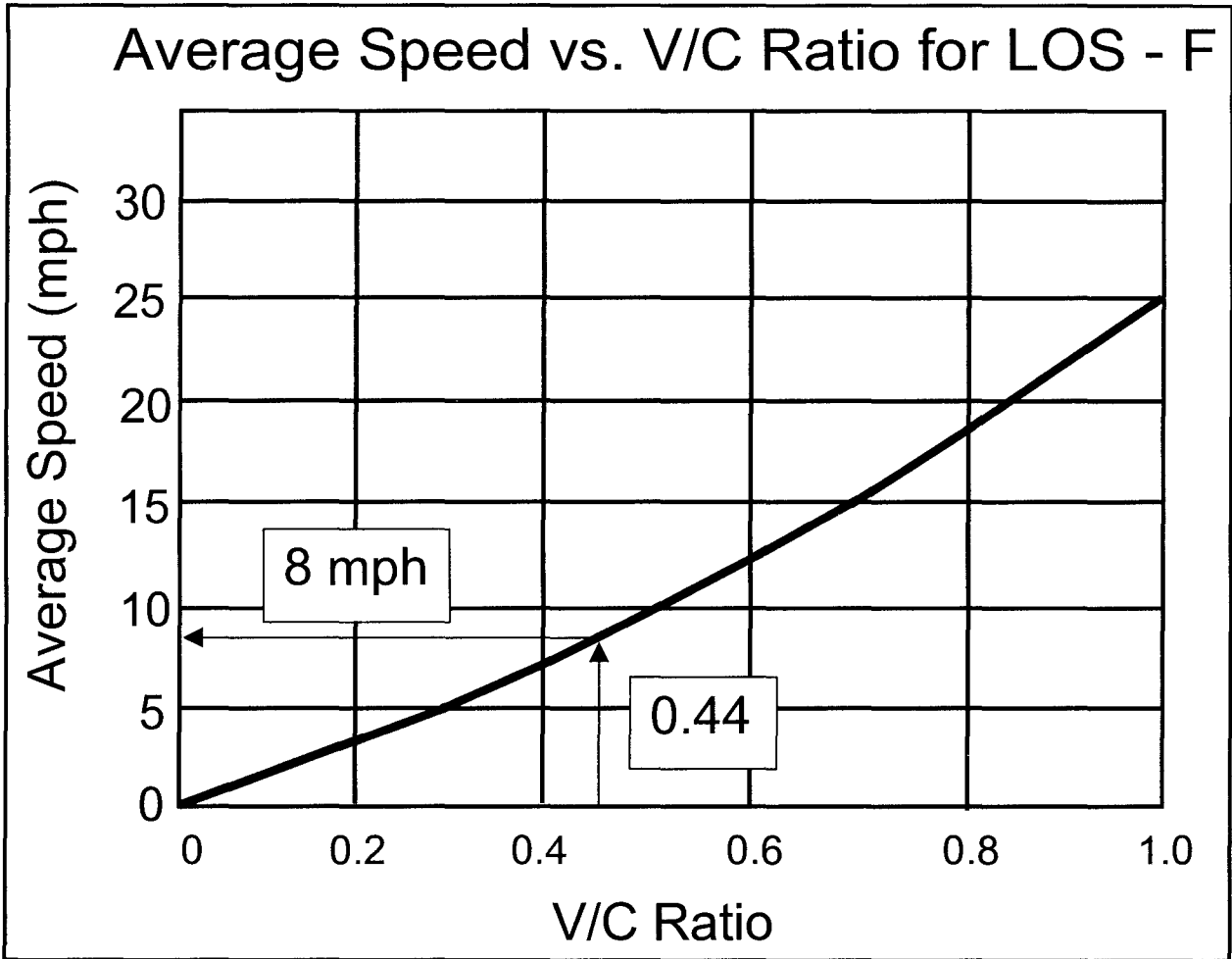


Figure 4. V/C Ratio versus Average Queue Speed (Source: NCHRP 133).

Note:

$V = V_Q =$ Capacity in Work Zone

$C = C_Q =$ Upstream Capacity

Problem No. 3

Compute the following user costs associated with the work zone.

Cost Component	Veh. Class	Number Vehicles	Added VOC	Added Time (Hrs/Veh.)	Value of Time ⁽⁴⁾ (\$/Veh. Hr)	Cost (\$)	%
①WZ Speed Change Delay (55-35-55)	Auto	339,647		1.90 / 1000 ^(1,3)	11.58	7,473	0.02
	SU	22,643		2.94 / 1000 ^(1,3)	18.54	1,234	0.00
	Combo	15,095		11.53 / 1000 ^(1,3)	22.31	3,883	0.01
②WZ Speed Change VOC (55-35-55)	Auto	339,647	39.42 / 1000 ^(1,2)			13,385	0.03
	SU	22,643	64.01 / 1000 ^(1,2)			1,449	0.00
	Combo	15,095	317.93 / 1000 ^(1,2)			4,799	0.01
③WZ Reduced Speed Delay (35 vs. 55)	Auto	3,462,750		0.0727 ⁽⁵⁾	11.58	2,916,265	6.76
	SU	230,850		0.0727 ⁽⁵⁾	18.54	311,270	0.72
	Combo	153,900		0.0727 ⁽⁵⁾	22.31	249,710	0.58
④Queue Stopping Delay (55-0-55)	Auto	3,123,103		5.84 / 1000 ^(1,3)	11.58	211,207	0.49
	SU	208,207		8.07 / 1000 ^(1,3)	18.54	31,151	0.07
	Combo	138,805		20.72 / 1000 ^(1,3)	22.31	64,164	0.15
⑤Queue Stopping VOC (55-0-55)	Auto	3,123,103	83.47 / 1000 ^(1,2)			260,685	0.60
	SU	208,207	160.89 / 1000 ^(1,2)			33,498	0.08
	Combo	138,805	721.77 / 1000 ^(1,2)			100,185	0.23
⑥Queue Added Travel Time Delay (Queue Speed vs 55)	Auto	3,123,103		0.8548 ⁽⁶⁾	11.58	30,915,530	71.64
	SU	208,207		0.8548 ⁽⁶⁾	18.54	3,299,792	7.65
	Combo	138,805		0.8548 ⁽⁶⁾	22.31	2,647,190	6.13
⑦Queue Idle VOC	Auto	3,123,103	0.6927 ⁽⁷⁾	0.8548 ⁽⁶⁾		1,849,325	4.29
	SU	208,207	0.7681 ⁽⁷⁾	0.8548 ⁽⁶⁾		136,708	0.32
	Combo	138,805	0.8248 ⁽⁷⁾	0.8548 ⁽⁶⁾		97,867	0.23
Total Work Zone User Cost >>>>>						43,156,773	

Notes:

1. See Table 2 Page 3.9 for added time and vehicle running cost.
2. Speed change VOC typically given as \$/1000 Veh.
3. Speed change delay typically given as Hr/1000 Veh.
4. See Table 3 Page 3.9 for recommended values of time.
5. See Problem No.2 Page 3.6 Answer **A**
6. See Problem No. 2. Page 3.6 Answer **B**
7. See bottom of Table 2 on Page 3.9 for Idle cost rates. Note Idling cost units \$/Veh.-Hr.

Table 2. Added Time and Vehicle Running Cost / 1000 Stops and Idling Costs (August 1996).

Initial Speed (mph)	Added Time (Hr / 1000 Stops) (Excludes Idling Time)			Added Cost (\$/1000 Stops) (Excludes Idling Time)		
	Pass. Cars	Trucks		Pass. Car	Trucks	
		Single	Combination		Single	Combination
5	1.02	0.73	1.10	2.70	9.25	33.62
10	1.51	1.47	2.27	8.83	20.72	77.49
15	2.00	2.20	3.48	15.16	33.89	129.97
20	2.49	2.93	4.76	21.74	48.40	190.06
25	2.98	3.67	6.10	28.67	63.97	256.54
30	3.46	4.40	7.56	36.10	80.23	328.21
35	3.94	5.13	9.19	44.06	96.88	403.84
40	4.42	5.87	11.09	52.70	113.97	482.21
45	4.90	6.60	13.39	62.07	130.08	562.14
50	5.37	7.33	16.37	72.31	145.96	642.41
55	5.84	8.07	20.72	83.47	160.89	721.77
60	6.31	8.80	27.94	95.70	178.98	798.99
65	6.78	9.53	NA	109.02	195.84	NA
70	7.25	NA	NA	123.61	NA	NA
75	7.71	NA	NA	139.53	NA	NA
80	8.17	NA	NA	156.85	NA	NA
Idling Cost (\$ / vehicle-hour)				0.6927	0.7681	0.8248

Table 3. Recommended Value of Time (August 1996).

Vehicles Class	\$ / Vehicle hour	
	Value	Range
Passenger	11.58	10 – 13
Single Unit	18.54	17 – 19
Combination	22.31	21 – 24

Class Exercise No. 4 (Solution)

Life Cycle Cost Analysis

A State highway agency is conducting a Life Cycle Cost Analysis of a 6-lane facility (3 lanes per direction). The current directional AADT is 40,000 vehicles per day. The State is considering two alternatives for the initial construction and rehabilitation strategy for one direction. Planned work zones will be in place 24 hours per day during which time the facility is reduced to 2 lanes of operation. Performance life ranges for the two alternative strategies are shown in Table 1.

Compute the total Net Present Value (NPV) for each alternative. Use a 35 year analysis period. Include in your analysis the effect of salvage value, if applicable. Construction costs are directly related to the number of days allowed for initial construction and rehabilitation activities as shown in Figures 1 - 4. Use Table 2 to summarize your selected input values. Real opportunity cost of money to the State highway agency is 4%. The SHA estimates the value of time to be \$10 per hour. Routine reactive maintenance cost differences between alternatives are insignificant. Use the formula provided to calculate net present value. If needed use Table 3 for the appropriate discount factor. Use Table 4 to determine the daily cost of delay. Use Table 5 as a worksheet.

Table 1. Performance life ranges.

	Alternative – A				Alternative – B			
	Initial		Rehabs.		Initial		Rehabs.	
	Low	High	Low	High	Low	High	Low	High
Performance (years)	16	24	8	12	10	16	6	8

Table 2. Selected input values.

	Alternative – A			Alternative – B			
	Initial	Rehabs.		Initial	Rehabs.		
		1	2		1	2	3
Performance (years)	20	10	10	15	7	7	7
① Construction Days ¹	220	100	100	180	70	70	70
② Agency Cost ² (\$)	25 M	10 M	10 M	18 M	6 M	6 M	6 M
③ Daily Delay Cost ³ \$	33,333	405,020	1411,809	33,333	216,934	519,920	1,246,080
④ User Cost \$ (① x ③)	7.3 M	40.5 M	141.2 M	6.0 M	15.18 M	36.39 M	87.22 M

Notes: ^{1,2} See Figures 1.4.

³ See Table 4 Page 4.5.

Table 3. Discount Factors.

Year	Discount Rate				
n	4.0%	4.5%	5.0%	5.5%	6.0%
0	1.0000	1.0000	1.0000	1.0000	1.0000
1	0.9615	0.9569	0.9524	0.9479	0.9434
2	0.9246	0.9157	0.9070	0.8985	0.8900
3	0.8890	0.8763	0.8638	0.8516	0.8396
4	0.8548	0.8386	0.8227	0.8072	0.7921
5	0.8219	0.8025	0.7835	0.7651	0.7473
6	0.7903	0.7679	0.7462	0.7252	0.7050
7	0.7599	0.7348	0.7107	0.6874	0.6651
8	0.7307	0.7032	0.6768	0.6516	0.6274
9	0.7026	0.6729	0.6446	0.6176	0.5919
10	0.6756	0.6439	0.6139	0.5854	0.5584
11	0.6496	0.6162	0.5847	0.5549	0.5268
12	0.6246	0.5897	0.5568	0.5260	0.4970
13	0.6006	0.5643	0.5303	0.4986	0.4688
14	0.5775	0.5400	0.5051	0.4726	0.4423
15	0.5553	0.5167	0.4810	0.4479	0.4173
16	0.5339	0.4945	0.4581	0.4246	0.3936
17	0.5134	0.4732	0.4363	0.4024	0.3714
18	0.4936	0.4528	0.4155	0.3815	0.3503
19	0.4746	0.4333	0.3957	0.3616	0.3305
20	0.4564	0.4146	0.3769	0.3427	0.3118
21	0.4388	0.3968	0.3589	0.3249	0.2942
22	0.4220	0.3797	0.3418	0.3079	0.2775
23	0.4057	0.3634	0.3256	0.2919	0.2618
24	0.3901	0.3477	0.3101	0.2767	0.2470
25	0.3751	0.3327	0.2953	0.2622	0.2330
26	0.3607	0.3184	0.2812	0.2486	0.2198
27	0.3468	0.3047	0.2678	0.2356	0.2074
28	0.3335	0.2916	0.2551	0.2233	0.1956
29	0.3207	0.2790	0.2429	0.2117	0.1846
30	0.3083	0.2670	0.2314	0.2006	0.1741
31	0.2965	0.2555	0.2204	0.1902	0.1643
32	0.2851	0.2445	0.2099	0.1803	0.1550
33	0.2741	0.2340	0.1999	0.1709	0.1462
34	0.2636	0.2239	0.1904	0.1620	0.1379
35	0.2534	0.2143	0.1813	0.1535	0.1301

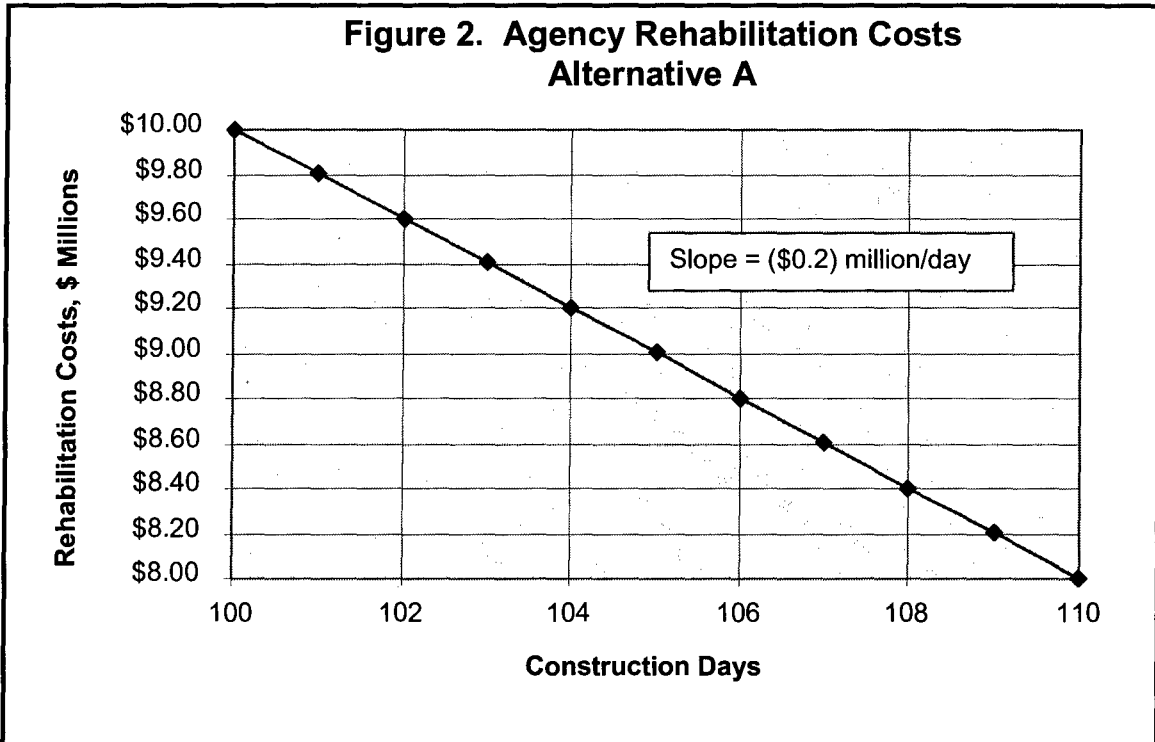
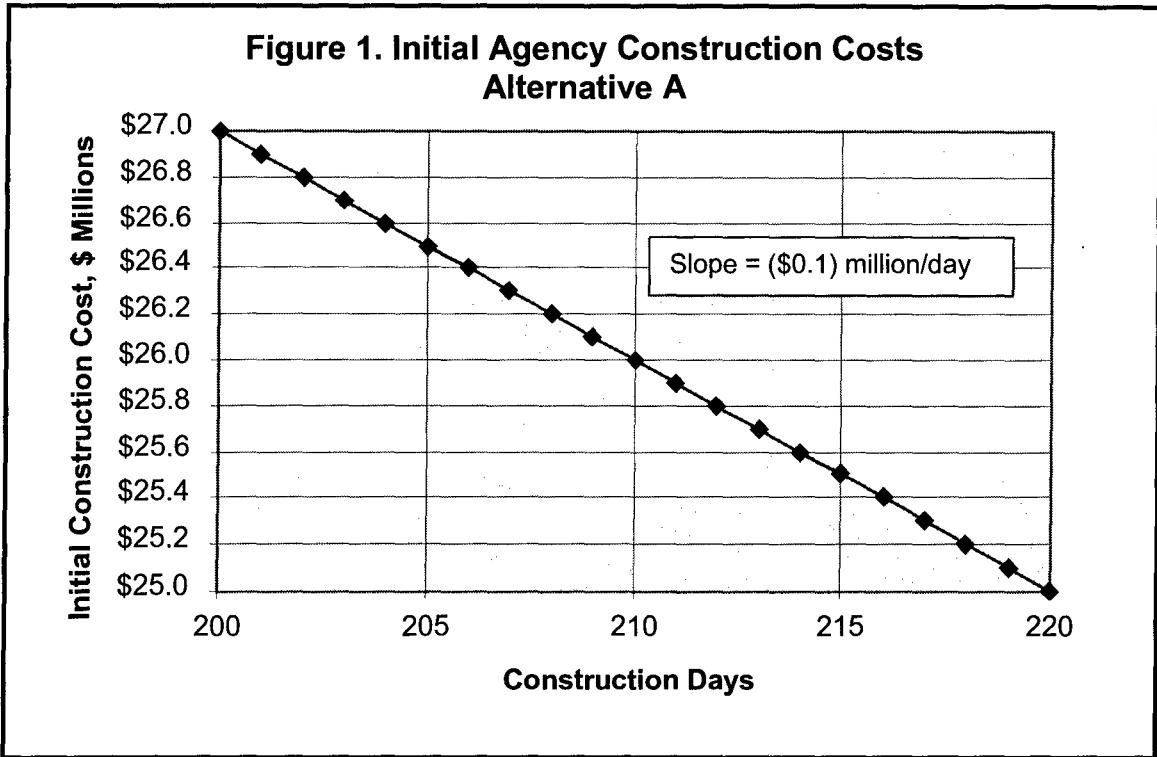


Figure 3. Initial Agency Construction Costs - Alternative B

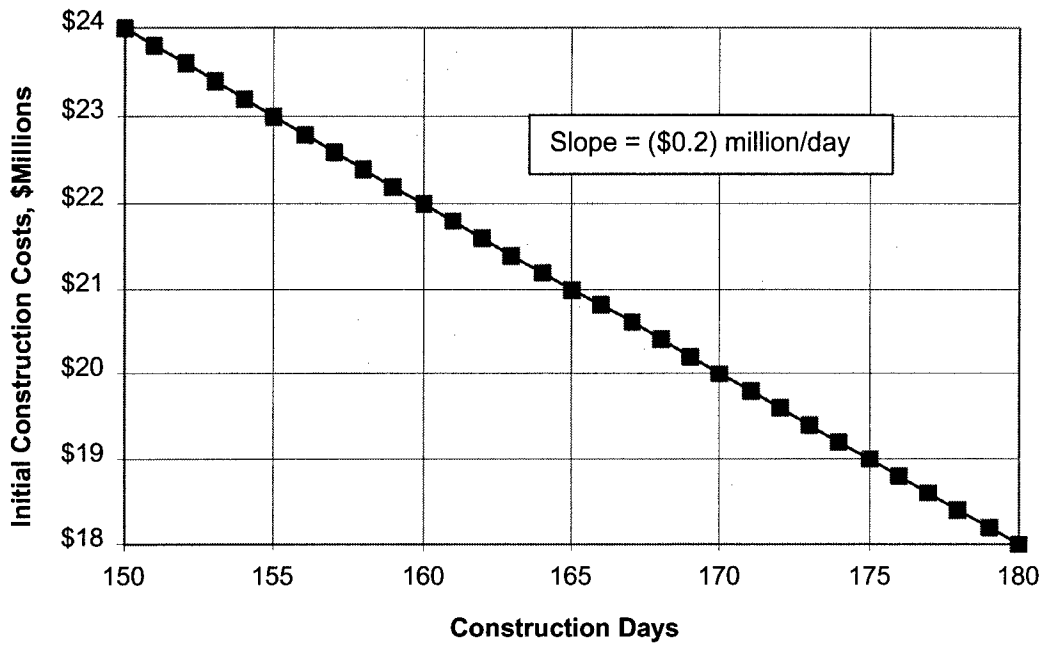


Figure 4. Agency Rehabilitation Costs Alternative - B

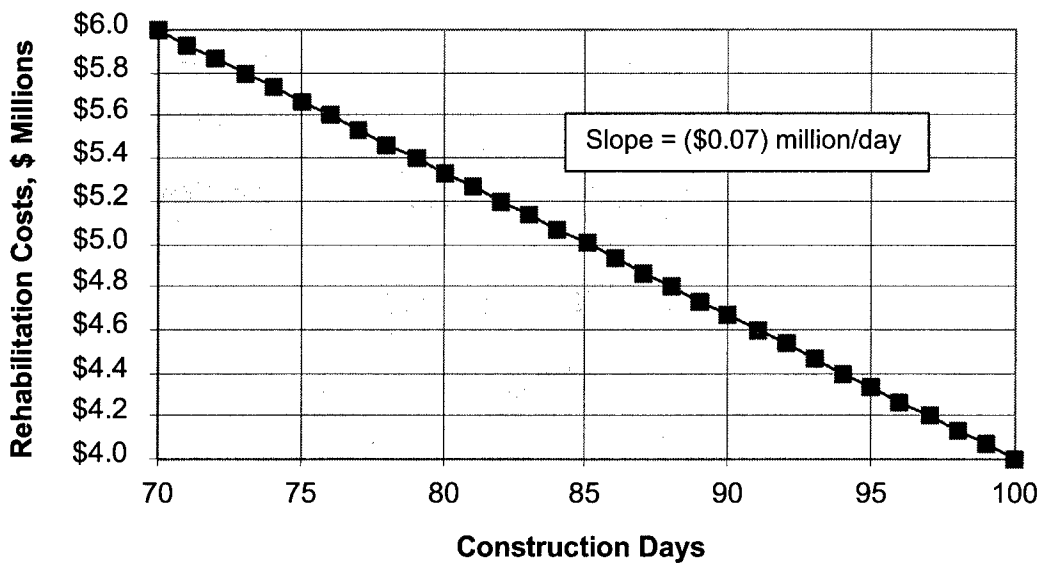


Table 4. Daily cost of delay.

Value Time \$ 10.00 per hour Directional AADT(initial) 40000 vpd Traffic Growth Rate 3 percent Delay Per Veh. Growth Rate 10 percent				
Year	AADT	Delay/Veh. min	Daily Delay hours	Daily Cost
0	40000	5.0	3333	\$ 33,333
1	41200	5.5	3777	\$ 37,767
2	42436	6.1	4279	\$ 42,790
3	43709	6.7	4848	\$ 48,481
4	45020	7.3	5493	\$ 54,929
5	46371	8.1	6223	\$ 62,234
6	47762	8.9	7051	\$ 70,511
7	49195	9.7	7989	\$ 79,889
8	50671	10.7	9051	\$ 90,514
9	52191	11.8	10255	\$ 102,553
10	53757	13.0	11619	\$ 116,192
11	55369	14.3	13165	\$ 131,646
12	57030	15.7	14915	\$ 149,155
13	58741	17.3	16899	\$ 168,993
14	60504	19.0	19147	\$ 191,469
15	62319	20.9	21693	\$ 216,934
16	64188	23.0	24579	\$ 245,786
17	66114	25.3	27848	\$ 278,476
18	68097	27.8	31551	\$ 315,513
19	70140	30.6	35748	\$ 357,476
20	72244	33.6	40502	\$ 405,020
21	74412	37.0	45889	\$ 458,888
22	76644	40.7	51992	\$ 519,920
23	78943	44.8	58907	\$ 589,070
24	81312	49.2	66742	\$ 667,416
25	83751	54.2	75618	\$ 756,182
26	86264	59.6	85675	\$ 856,755
27	88852	65.5	97070	\$ 970,703
28	91517	72.1	109981	\$ 1,099,806
29	94263	79.3	124608	\$ 1,246,081
30	97090	87.2	141181	\$ 1,411,809
31	100003	96.0	159958	\$ 1,599,580
32	103003	105.6	181232	\$ 1,812,324
33	106093	116.1	205336	\$ 2,053,363
34	109276	127.7	232646	\$ 2,326,460
35	112554	140.5	263588	\$ 2,635,880
36	115931	154.6	298645	\$ 2,986,452
37	119409	170.0	338365	\$ 3,383,650
38	122991	187.0	383368	\$ 3,833,675
39	126681	205.7	434355	\$ 4,343,554
40	130482	226.3	492125	\$ 4,921,247

Note:
 Values shown are for illustrative purposes only.

Table 5. Solution

	Year				
	0	20	30	35	
Alternative - A					
Agency Cost (Constant \$)	25.00 M	10.00 M	10.00 M	-5.00 M	
Present Worth Factor	1.000	0.4564	0.3083	0.2534	
Agency Cost (Present Worth)	25.00 M	4.564 M	3.083 M	-1.267 M	
Total NPV (Agency Cost)	31.38				
User Cost (Constant \$)	7.333 M	40.50 M	141.2 M		
Present Worth Factor	1.000	0.4564	0.3083		
User Cost (Present Worth)	7.333 M	18.48 M	43.53 M		
Total NPV (User Cost)	69.34 M				
Grand Total NPV (all costs)	100.7 M				

	Year				
	0	15	22	29	35
Alternative - B					
Agency Cost (Constant \$)	18.00 M	6.000 M	6.000 M	6.000 M	-0.8571 M
Present Worth Factor	1.000	0.5553	0.4220	0.3207	0.2534
Agency Cost (Present Worth)	18.00 M	3.332 M	2.532 M	1.924 M	-0.2172 M
Total NPV (Agency Cost)	25.57 M				
User Cost (Constant \$)	6.000 M	15.18 M	36.39 M	87.22 M	
Present Worth Factor	1.000	0.5553	0.4220	0.3207	
User Cost (Present Worth)	6.000 M	8.429 M	15.54 M	27.97 M	
Total NPV (User Cost)	57.94 M				
Grand Total NPV (all costs)	83.51 M				

Selection of Construction Days.

Alternative	Agency Cost	User Cost	Construction Days (Range)	Construction Days (Selection)
Alt - A Initial	100,000 / day	33,333 / day	200 to 220	220
Alt - A Rehab Year 20	200,000 / day	405,020 / day	100 to 110	100
Alt - B Initial	200,000 / day	33,333 / day	150 to 180	180
Alt - B Rehab Year 15	70,000 / day	216,934 / day	70 to 100	70

