LOADING SPECTRUM EXPERIENCED BY BRIDGE STRUCTURES IN THE UNITED STATES

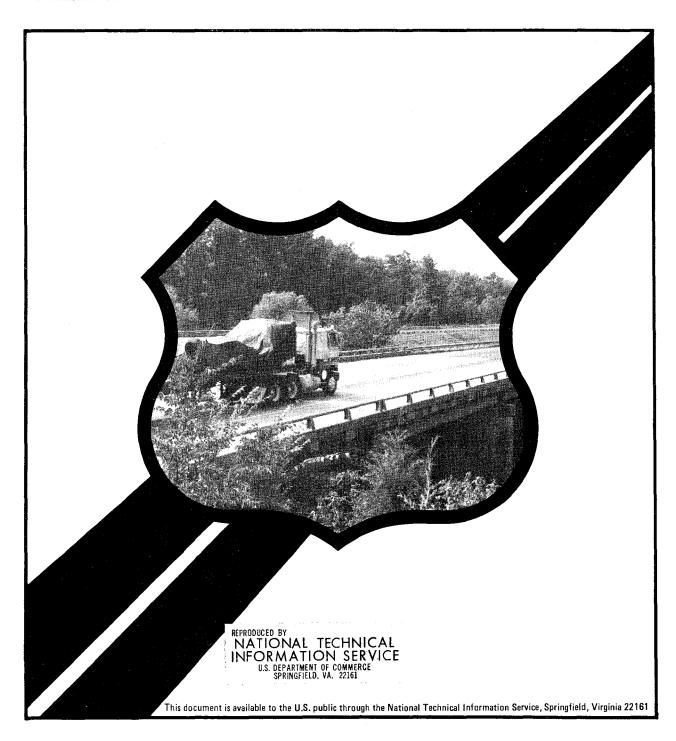
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FOREWORD

In the last decade, highway engineers and planners have devoted considerable attention to the collection and evaluation of detailed information on truck weights and characteristics. This information is vital for evaluating structural performance and determining maintenance requirements of the Nation's bridges and pavements.

Extensive research has been conducted to develop new techniques for collecting the required data in a more efficient and cost effective manner. A number of automated instrumentation systems which are capable of weighing and classifying heavy vehicles as they travel down the highway have resulted from this research. These weigh-in-motion (WIM) systems can collect large volumes of information which accurately reflects the loading spectrum in the traffic stream.

The purpose of this study was to use bridge weigh-in-motion technology to collect truck weight and classification data at a representative sample of bridge sites in the United States and to develop a loading spectrum from the information obtained. In the process of conducting this work, a number of significant improvements were made to bridge WIM instrumentation.

Richard E. Hay, Director
Office of Engineering
and Highway Operations
Research and Development

Federal Highway Administration

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More than 27,000 trucks were weighed in seven states using a Bridge Weigh-in-Motion system. The system used instrumented highway bridge girders to act as equivalent static scales to obtain truck gross and axle weights, dimensions and speed. Improvements were made in the system software and hardware such that weighing can now be performed automatically with no traffic observer necessary. The weighing operation is undetectable by truck drivers hence the results are not biased due to heavy trucks avoiding the scale. Night weighing has been performed with no danger to crew personnel or highway traffic. The accuracy of the inmotion weighing has been established by repeatability tests which show standard deviations of 1.1 kips and by comparison with static weigh stations which show standard deviations of 2.3 kips on gross weight for random traffic. Three systems have been designed, developed, tested and delivered to FHWA for operation by two crew members at the technician level with part time supervision by an engineer. Weighing rates in excess of 200 trucks per hour have been realized. The Loading Spectrum obtained for all trucks weighed at 33 sites is similar to previous studies with the exception of vehicles weighing more than 80,000 pounds for which a frequency of approximately 50 per 1000 was observed. Sixty nine percent of all trucks weighed in this study had velocities greater than 55 mph and seven percent had velocities greater than 64 mph.				
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CHAPTER ONE

Introduction

1.1 General Remarks

The acquisition of truck axle and gross weight information has received considerable attention from highway engineers and planners. These data are essential for determining the structural and maintenance requirements of bridges and pavements. In addition, accurate truck weights are important in planning, economic and enforcement surveys.

In the last two years the need for vast amounts of accurate truck weight data has become further intensified. Section 16 of the Surface Transportation Assistance Act of 1978 called for a study of uniform weight limits throughout the United States. Simultaneously, Section 506 of the same act provided for a Cost Allocation Study to analyze costs and impacts on roadway construction and maintenance associated with different vehicle classes. With this increased interest in the effects of heavy truck loads, the Federal Highway Administration (FHWA) issued a Notice of Proposed Rulemaking setting forth the requirements for administering a program of vehicle weight enforcement. The proposed regulation requires each state to formulate a plan that provides for an evaluation of its enforcement accomplishments.

The General Accounting Office (GAO) has also issued its report "Excessive Truck Weight: An Expensive Burden We Can No Longer Support" and proposed the need for (a) a model weight enforcement program and (b) assurances that complete geographic coverage is achieved (1).* In addition, the Transportation Research Board has completed a study compiling data on weight regulation, enforcement and permit operation and has completed a new study entitled "Criteria for Evaluation of Truck Weight Enforcement" (2,3).

^{*} Numbers in parentheses refer to references.

In the context of all these activities, the discussion of truck weights and the effectiveness of enforcement operations is still being carried out with a lack of adequate information. There have been only a few systematic and unbiased field studies conducted of current weight statistics. Both loadometer surveys and weigh stations can easily be avoided by drivers with the aid of CB radios and considerable bias is added to the massive data now reported. In addition, the cost of acquiring truck survey data has increased dramatically to the point where many states have reduced weighing operations and the number of survey sites.

1.2 Truck Weight Data Requirements

"The characteristic requirements for truck weight data has been divided into several categories. Each user group has unique applications which affect the quantity, accuracy and type of truck data needed."(4)

PLANNING - Highway planners utilize truck data for economic studies, forecasts and freight modelling. Statistical data is needed on hourly, daily and seasonal variations, number of trucks, weights and type. Thus, certain information on hauling categories is needed in addition to the weight and truck axle configuration. These categories can only be visually identified such as tankers, open steel flatbeds, equipment haulers, etc. Gross weight rather than axle weight is generally more important to planners.

ENFORCEMENT - As an aid to weight enforcement, truck weight data can be used to determine routes with significant numbers of overweight trucks, the presence of by-pass routes around fixed weigh stations and the magnitude and percentage of overweights among various types of haulers, e.g., by hauling category or axle configuration. Visual information may not be necessary, but accurate axle spacings are needed for a check on legal limits. In order to directly help enforcement officials the data gathering must be totally undetected.

PAVEMENT DESIGN - Pavement life is sensitive to the number and statistical distribution of equivalent number of axle loads. Data for planning the maintenance, rehabilitation and reconstruction of pavements uses such axle load distribution (tandems and other closely spaced groups must be distinguished because of their different axle equivalent factors). Since only the distributions are important, the accuracy of any individual weighing is not so critical as long as no systematic bias is introduced into the data.

BRIDGE DESIGN - It has been recognized that a major obstacle to a probabilistic based design for bridges is the limited knowledge of the loading spectrum. A complete bridge loading model requires individual truck weight distributions, and axle loads and the spacing or headway between trucks to establish maximum lifetime loading. For an application such as fatigue design of steel bridges, the statistical distribution of individual truck weights is sufficient. Gross weight accuracy is more important than individual axle loads since bridges are usually long relative to the axle spacings.

1.3 Weigh-In-Motion

It is thus well recognized that large volumes of inexpensively gathered truck data is needed which survey the total truck population in an undetected weighing operation. In attempting to achieve these goals a number of organizations have participated in the development of pavement scales for weigh-in-motion operations. The difficulty with these systems is that the relatively light scale "bounces" during the truck passage, leads to high scatter and possibly bias of results at normal speeds (5). Pavement scales are effective, however, when used on special low speed ramps adjacent to fixed scales for sorting the heavy truck population. Attention must be given to the maintenance of such scales especially in cold climates and frequent pavement resurfacing near the scale is needed.

In recognition of some of the limitations with pavement systems the FHWA launched series of feasibility studies to recommend alternative weigh-in-motion systems. One feasibility study by Moses and Goble (5) has led to the system described herein. The passage of vehicles over existing bridges is used to carry out the weighing operation. The system combines strain gage response on main longitudinal bridge members carrying the traffic stream with tapeswitches to provide vehicle dimensions and velocity. bridge becomes a massive scale and provides the equivalent static weight of the vehicle. The approach described herein offers several advantages over payement scales. The dynamic oscillations in tire contact force are filtered and reduced by the relatively massive inertia of the bridge. Further, they are almost totally eliminated by a statistical smoothing algorithm that has been derived for processing the bridge girder strains and calculating the truck gross and axle weights (6). Utilizing existing highway bridges as the scales provide numerous potential scale locations and permits an easily These "scales" are not easily detected by drivers since portable operation. the instrumentation is primarily located under the bridge leaving no visible indications of a scale. Inexpensive reusable strain transducers are quickly clamped to the bridge and the portable monitoring instrumentation is rapidly moved to different sites for heavy vehicle surveys.

1.4 Background

A series of projects to investigate strains in bridge girders was begun in 1971 at Case Western Reserve University (CRWU) (7),(8). The investigators noticed that given the truck axle configuration there was good correlation between gross vehicle weight and peak girder strain.

The initial field testing of the bridge weigh-in-motion concept was done at CWRU with a research prototype system delivered to FHWA in 1978 (9). The system used strain gages attached to steel bridge girders and the field

testing was patterned after an earlier CWRU study for Ohio Department Of Transportation (ODOT) to obtain bridge strain histories (10). The concept was further broadened by developing a truck weight loadometer system for ODOT and is presently being used by their Bureau of Transportation Services. This system was utilized to provide seasonal variation on truck weights at four Ohio sites as part of the Cost Allocation data program. Their results are not described herein. The ODOT system is currently being used to assist loadometer operations and also to provide enforcement assistance.

1.5 Objectives

The study objectives were to:

- (a) Design, fabricate, and test three mobile weigh-in-motion (WIM) systems using the latest bridge weighing technology;
 - (b) Advance Bridge WIM technology;
- (c) Use the Bridge WIM systems to acquire representative U.S. truck data in seven states and transmit the data to the DOT Cost Allocation team assessing the effect of current vehicle loadings on the life of the highway systems.

Data was to be collected for periods of 20 to 28 hours at approximately 40 sites along corridors chosen by FHWA and state officials. In some cases data collection at specific sites was required. The first system was to be designed, fabricated, tested, and data collection begun after three months from the start of the project. The second system was to be fabricated, tested, and delivered to FHWA within five months from the beginning of the project for use by FHWA in collecting data for the Cost Allocation Study.

Field personnel were to be trained to operate the system without an engineer present. Two teams of two men each were assigned to field positions. The first team consisted of a student and an engineering technician, both with

no previous experience with computers or electronics. In the second team were a fourth year civil engineering student and an electronics technician. Civil engineers assigned to the project were occasionally in the field for demonstrations, correlations or to relieve the field technicians.

1.6 Report Outline

The weighing operation and the equipment used is described in Chapter 2. A more detailed account is presented in the operation and maintenance manuals prepared for FHWA. Previous reports on projects performed at CWRU provide historical insight to the development of the current system (9,4).

Several modifications were made to the state-of-the-art system developed at CWRU including the change in software enabling higher acquisition rates, automatic operation, real time in-field weight processing and improved signal conditioning. These modifications are discussed in Chapter 3.

The survey results, including weight data from more than 27,000 trucks obtained at 32 sites, are presented and discussed in Chapter 4. Other WIM data acquired by FHWA and ODOT using the same type of operation are not discussed in this report. An evaluation of the Bridge Weigh-In-Motion system and an extensive section on its accuracy is presented in Chapter 5.

Conclusions and recommendations are discussed in Chapter 6. Improvements are suggested to broaden the routine use of the system in other areas. The system in its present state performs well and is designed for routine use by state agencies. The suggested improvements would be primarily in software and would be adaptable to present hardware systems. The further application of WIM output to bridge load models is not discussed herein but is the subject of several research reports and proposals (4,14).

CHAPTER TWO

Weighing Operation

The Bridge Weigh-In-Motion method uses strain transducer response measured during a vehicle passage over an instrumented span to calculate a vehicle's weights. The position of each axle as a function of time during the crossing is also required and is obtained from tapeswitches. The weight processing procedure is described in Section 2.3.

2.1 Equipment Set-up

Figure 1 shows the field equipment setup for a typical weighing operation. Reusable strain transducers, which measure bridge structural response for vehicle weight determination, are attached to the girders. Tape switches used to determine vehicle velocity and axle spacing are placed on the roadway surface. An optional keypad may be used to record visual hauling information. The signals from the tape switches, strain transducers and keypad are carried by cables to an instrumentation van located below the bridge out of view of drivers.

The instrumentation van houses the entire weigh-in-motion system (Figure 2) including a portable power supply. At the heart of the system is a MINC minicomputer (PDP 11) manufactured by Digital Equipment Corporation (DEC). MINC laboratory modules required for this application include two hardware clocks, an analog to digital converter, and a digital input device. The MINC is supplied with a dual floppy disk drive for software and data storage. A signal conditioning center constructed by the project is used to collect, condition and amplify the strain signals and to condition the keypad and

tapeswitch signals through a debounce circuit. As part of this project an autobalancing signal conditioning system was developed and tested that will maintain a constant zero on the strain channels. This constant zeroing aids in detecting other vehicles that may be on the instrumented span during a weighing. Strain responses from individual girders are summed to provide the overall bridge response.

During operation, an operator stationed in the instrumentation van monitors the strain and tapeswitch signals on an oscilloscope and the computer output on a CRT display. An intercom system is used between the keypad operator and the van.

2.2 Data Acquisition

As a vehicle approaches the instrumented span, the appropriate hauling category (box, flat, tank, auto carrier, open hauler, log carrier, etc) may be input by the optional keypad observer. The system then operates automatically as if the observer were not present. When the steering axle arrives at the first tapeswitch which is located approximately four feet before the bridge the computer begins acquiring strain data from the strain transducers. The crossing of each axle on each tape switch is timestamped to provide vehicle velocity and axle spacing. All axles of the vehicle have been received when one of two constraints has been met. The first constraint is the limitation on axle spacing of 37 feet (11.3 meters). The second constraint is the limitation on the vehicle length from the first axle to the last axle which for this study was set to 65 feet (19.8 meters). Hence, data acquisition continues until no axle pulse has been received for the time equivalent to 37 feet (11.3 meters) (37 divided by the vehicle velocity) or the time equivalent to 65 feet (19.8 meters) from the time the first axle was received. distance was based on the maximum expected axle length and could be modified if necessary.

Once either of the constraints have been met, the computer program classifies the vehicle, i.e., as a truck or a car. A car has been arbitrarily defined as any two axle vehicle with an axle spacing less than 12.1 feet (3.7 meters) or any vehicle causing a girder peak strain that is less than a preset level. A vehicle is classified as a truck if the number of axles is greater than two, or in the case of a two axle vehicle the axle spacing is at least 12.1 feet (3.7 meters) and the peak strain measured during the vehicle crossing on any girder is greater than a preset level. This preset strain level is site dependent and on the order of 10 microstrain. The purpose of this constraint is to prevent a car pulling a trailer from being classified as a truck.

If the vehicle is determined by the program to be a truck, strain acquisition is continued until the last axle has reached the end of the instrumented span. Velocity and axle spacing information is displayed on the CRT and the strain data, tape switch activation times, and site information are recorded on a floppy disk. The recording process has been programmed to allow the computer to perform other tasks such as processing the data to obtain the vehicle gross and axle weights simultaneously. Data acquisition times range from 1.2 to 2.0 seconds without processing and between 1.7-to 2.5 seconds with processing for each truck. Note that these times are dependent on the span length, vehicle length and vehicle velocity. For example, a 60 foot (18.3 meter) truck traveling 80 feet per second, 55 mph (24.6 m/s, 34 km/h) and crossing a span 38 feet (11.6 meters) in length would require 1.23 seconds for a crossing of the entire truck over the span.

Once the vehicle is determined by the program to be a car, strain sampling is discontinued. The car velocity is classified, however, and this data is stored in a separate file and can be used for velocity statistics. The program then prepares for the next vehicle.

In order to process the vehicle in real time during data acquisition, the bridge moment influence line must have been previously determined, stored on disk, and read into the computer memory. The influence line can be determined by an engineer in the office with a structural analysis program that is available for this purpose. The influence line is discussed further in Section 2.4.

2.3 Weight Processing

The concept of correlating truck weight with girder strain record has been suggested by several investigators since the bridge responds primarily by longitudinal beam action. The correlation has been found accurate when the class of vehicles being compared includes the number and spacing of axles(5). The conclusion was that accurate weigh-in-motion predictions of static axle and gross weights must also contain vehicle axle configuration including number and spacing of axles in addition to strain records.

The Bridge Weigh-In-Motion analysis is an "inverse" type problem in that the structural response (bending moment) is measured, but the live loads causing this moment must be calculated. In theory, the number of unknowns for each vehicle equals the number of axles, N, and these could be determined by N different bending moments, i.e. strains, recorded for N different positions of the truck along the bridge. Since data are recorded continuously during truck passage this "redundant" data effectively increases the number of separate "weighings" of the vehicle so the results could be averaged to reduce any errors. The principle of data redundancy was extended to its logical conclusion in the statistical smoothing algorithm derived to remove the effects of bridge vibration. In effect, the axle weights are found which minimize the least square difference between the measured strain and the value calculated from the vehicle dimensions and bridge influence line.

Figure 3 shows a typical beam slab bridge suitable for use as a scale. Generally, such bridges must be long and stiff in the longitudinal (traffic) direction relative to the transverse direction. During the passage of a truck, the bridge will oscillate about a static displacement position as illustrated in Figure 4. Assume each girder has a strain transducer in the longitudinal direction at midspan. The gross bending moment in the beam slab bridge can be found by summing the individual girder moments. Each girder stress is related to girder moment from the relationship (as presented by Moses (6)):

$$\sigma_{i} = \frac{M_{i}}{S_{i}} \tag{1}$$

where σ_i - stress in the i-th girder

 M_{i} - bending moment in the i-th girder

 $\mathbf{S}_{\mathbf{i}}$ - section modulus of the i-th girder with respect to gage location

i = 1, ...N (number of girders)

The moment can be expressed in terms of the strain as

$$M_{i} = S_{i}\sigma_{i} = ES_{i}\varepsilon_{i}$$
 (2)

where E - modulus of elasticity

 ϵ_{i} - strain measured in the i-th girder

The sum of the individual girder moments equals the gross bending \mbox{moment} (M) or

$$M = \sum M_{i} = \sum ES_{i} \varepsilon_{i}$$
 (3)

Taking E and S as constant for each girder gives

$$M = E S \Sigma \varepsilon_{i}$$
 (4)

Thus, the sum of the girder strains is proportional to the gross bending moment.

To set up the necessary equations requires the bending moment influence line, I(x) for the transducer location. This is illustrated in Figure 6 for a simple span bridge. The value of the ordinate is the bending moment for a unit axle load located at this point along the bridge. Consider a vehicle illustrated in Figure 5 with axle weights A(1), A(2), ... A(N) and axle spacings L(1), L(2), ... L(N-1). The bending moment can be expressed for any location, X, of the first axle measured from the support as follows

$$M(x) = A_{1}I(X) + A_{2}I(X-L_{1}) + A_{3}(X-(L_{1}+L_{2})) + ... + A_{N}I(X-(L_{1}=L_{2}+...L_{N-1}))(5)$$

$$= \sum_{j=1}^{N} A_{j} I(X - \sum_{j=1}^{j-1} L_{j})$$

By using Equation 5 and picking N location X(1), X(2), ... X(N), there would be N equations to solve for the N unkown values A(1), A(2), ... A(N). Instead of this calculation, a statistical smoothing procedure has been derived. Using a high rate of sampling of the strain record, the gross bending moment can be expressed as a function of time. In a similar way,

using the velocity and axle spacing, the influence line for each axle can be written as a function of time. Summing the effects of each axle, the expected static bending moment can be written as

$$M(t) = \sum_{i=1}^{N} A_{i} I_{i}(t)$$

$$(6)$$

where M(t)- induced bending moment at gage location

t - time

 $I_{i}(t)$ - gross bending moment influence value at gage location for i-th axle

M(t) in equation 6 is the predicted static moment in terms of the axle weights. The dynamic bridge response can be filtered out by defining an error function (e) which is the difference between the predicted record M(t) and the measured record denoted as $M^*(t)$. Thus

$$e = \sum_{t=1}^{T} (M(t) - M*(t))^{2}$$
 (7)

or substituting for the predicted moment, M(t) gives

where T is the number of time increments used in the smoothing process.

The difference between M(t) and $M^*(t)$ is squared in Equation 7 so that a least square fit to axle weights is found. This minimizes the influence of the dynamic oscillation which occurs about the static response. Minimizing the scalar error, e, leads to the following equation for finding A(i).

To put in matrix form let

$$F_{ij} = \sum_{t=1}^{T} I_i(t)I_j(t)$$
 (10)

$$M_{j} = \sum_{t=1}^{T} M^{*}(t) I_{j}(t)$$
(11)

In matrix form, the axle weights are found by solving the equation

$$FA = M \tag{12}$$

where the square matrix
$$F = (F_{i,j})$$

column matrix
$$M = \{M_{\hat{j}}\}$$

and column matrix
$$A = \{A_{j}\}$$

The solutions for axle weights can be expressed in the matrix form as

$$A = F^{-1}M \tag{13}$$

The gross vehicle weight (GVW) is found by summing axle weights or

$$GVW = \sum_{i=1}^{N} A_{i}$$
 (14)

In general, due to dynamic oscillations, a moving vehicle will cause individual axle loads within a group to continually change. Thus, it is not possible to accurately measure or distinguish individual axle weights within a tandem group so the output of the processing program automatically combines axle weights of a tandem group.

The square matrix(F) is a function only of the influence line and axle spacings while the column matrix $\{M\}$ depends on the measured record as well as the influence line. For a single span bridge, the influence line is a simple triangle as illustrated in Figure 6. For continuous spans as in Figure 7 showing a three span example, the influence line must be calculated by

indeterminate structural analysis. The results are dependent on the section properties which affect the stiffness. However, the influence line is not itself very sensitive to approximations in section moment properties as shown by Kriss (11).

The above algorithm is basically an averaging procedure which smooths the strain record by a least square prediction method. It is thus biased towards the accuracy of the heavier axles. The gross weight predicted has been shown (11) to be insensitive to small errors in velocity, axle spacing and arrival time at the start of the record. The front axle, however, because it is usually light relative to the other axle combinations is more sensitive to small errors in the timing. Front axle weights are discussed in more detail in Section 3.2.2.

2.4 Calibration Procedure

The equations to solve for axle weight use the bending moments that are linearly related to strains. In principle, the elastic modulus and section modulus can be calculated from the section drawings and material properties. In practice, especially for concrete bridges, this strain-moment relationship is more accurately found by using a calibration vehicle with known weights.

In calibrating a bridge, a vehicle of known weight (including each axle weight), such as a three axle dump, a loaded five axle flat bed, or an open hauler loaded with gravel may be used to make several (4 to 6) passes in each instrumented lane. This is accomplished during normal data acquisition. During the first one or two passes the calibration factor is established. Subsequent passes are for confirmation and to determine the variation at the site. An engineer need not be present. This procedure need only be done once for each bridge unless, of course, the bridge is completely resurfaced or has undergone other structural changes.

2.5 Influence Line Calculation

The bending moment influence line can be calculated from the geometrical properties of the bridge. Two Fortran IV (RT-11) programs were written for this purpose; BILINE for continuous bridges and SIMPLE for simply supported BILINE uses the structural analysis program STAN while SIMPLE calculates a linear function with a maximum at the transducer location. calculated, the influence line is normalized to a peak of 1.0. Each program allows the position of the transducers to be specified. calculated influence line is then stored in a file with a name that identifies the bridge location and the length of the influence line in feet. An example influence line for a continuous 3 span bridge with span lengths of 44, 63 and 44 feet (13.4, 19.2 and 13.4 meters) is presented in Figure 7. These programs are very easy to use and require only the section moduli of the girders and the span lengths. The time needed to input the information is one to two minutes while the analysis done by the computer requires three to seven minutes depending on the length of the bridge. This is done only once at each site and may be stored for later testing.

The program PLINFL displays a plot of the influence line on the terminal screen along with the numerical values. This serves as a check on the input properties as well as the calculations.

CHAPTER THREE

System Modifications

One major objective of the study was to advance bridge weigh-in-motion technology. Research efforts were directed toward three areas:

- 1) the system's operational efficiency and capabilities,
- 2) measurement and analysis techniques and
- 3) equipment and instrumentation improvements.

3.1 System Operation - Data Acquisition

Significant changes were made in the software enabling the system to operate automatically, process the weights in the field, and to eliminate the input of the number of axles from an observer via the keypad box. The greatest improvement resulting from completely rewriting the software is the decrease in time needed to acquire data for each truck. The initial BASIC software required approximately 30 seconds per vehicle whereas the new FORTRAN software requires less than 2 seconds per vehicle for most bridges. The program permits simultaneous analog (strain) and digital (tapeswitch) data acquisition, hence the first tapeswitch now need be only a few feet from the beginning of the bridge as compared to the 60 feet (18.2 meters) required with the previous programs. The second tapeswitch can be placed on the bridge. This change reduced the errors associated with determining the truck's position on the bridge with time.

The duration of strain acquisition is flexible and is a function of vehicle velocity, vehicle length and the instrumented span length. The result is the acquisition of more closely spaced trucks; in fact, the headway limitation is now practically equal to the instrumented span length (when the system is not in the processing mode).

Acquisition no longer requires the number of axles to be input. Axles are counted and the vehicles are classified automatically with no observer needed when taking data in one lane. Night weighing is accomplished without sacrificing the safety of the highway traffic or that of the weighing crew. The system is, for the most part, undetectable by traffic when there is no observer. An optional keypad allows visual hauling category data to be input. This information classifies the vehicle into one of sixteen categories (i.e. box, flat, dump, tank, etc. discussed in Section 3.3.2) for later statistical analysis. When an observer was used, the truckers' reactions, as monitored via CB radio, were that of curiosity and at no time did they suspect that a weighing operation was in progress. The observer can also be used to avoid the recording of bad weighings and observe if there are tapeswitch problems.

Other key advantages realized in the software change include better file organization which decreased time and increased flexibility of data management for processing. Also added were the capability of weighing traffic in either one of two lanes, the ability to process the data and obtain gross and axle weights in real time (this requires approximately .5 seconds), maintaining an automobile velocity histogram, and a feature allowing the program start-up data to be input from a file. A special disk writing routine was assembled to allow simultaneous data acquisition and data storage or data processing and data storage. The standard Fortran writing routines previously used required exclusive use of the processor so that no other activity such as calculations or hardware initializations could take place. This feature reduces the headway limitation by approximately .7 seconds thus adding to weighing efficiency in dense traffic streams.

3.2 Measurement and Analysis Techniques

3.2.1 Transducer Location

During the planning phase of the project the effect of transducer location was examined. It was deemed undesireable to be constrained to placing the transducers at midspan since at some sites this installation would be difficult due to limited access to the bridge girders. The software used to calculate influence lines was modified to accommodate any transducer location. This reduced setup time as the crew could place the transducers at the most easily accessed location. This has not resulted in a loss of accuracy and in the case of long spans actually improved the accuracy of the individual axle weights.

Prior to this project, data had been acquired on only a few carefully selected bridges. Many of the bridge structures used in the data acquisition phase of this project were less than ideal and, as such, provided the opportunity to test the limits of the concept and equipment. In fact, it was intended by the FHWA research manager to test the extreme limits of the WIM capability. The system proved capable of accurately weighing vehicles on every structure utilizing longitudinal beams to carry the loads. Included were skewed bridges (up to 50 degrees), two-girder (plategirder) bridges, prestressed and reinforced concrete structures and a concrete pan formed bridge.

The instrumentation cable system to transfer data to the computer was designed such that only two types of cables were used. The system proved to be very flexible and easily adapted to all bridges used in this study.

3.2.2 Front Axle Weight Determination

The accuracy of the gross vehicle weight (GVW) was very good on all of the bridges tested during this project. With a few of these structures, axle weight distributions, in particular the front or steering axle, were not as accurate as desired. One minor cause is the least-square prediction procedure which is biased towards the accuracy of the heavier axles (tandems). The front axle is usually light relative to the other axle combinations and is therefore sensitive to small errors in timing. This is especially true for bridges with large skews as one wheel of an axle can be up to 9 feet (2.7 meters) onto the bridge when the other wheel first reaches the span.

The weight of the front axle A(1) is calculated by the algorithm derived in section 2.3. Front axle weights were also computed by two other techniques:

- a) calculate the front axle weight, A(1)i, using just the initial portion of the record when only the front axle is on the instrumented span.
- b) calculate a typical front axle weight, A(1)*, based on axle configuration and gross vehicle weight.

The weight from the processing algorithm, A(1), is compared to the typical weight, $A(1)^*$. If A(1) is within -2 to +4 kips (910 to 1810 kg) of $A(1)^*$, A(1) is accepted as being correct. If A(1) is not within this tolerance, $A(1)^*$ is compared with A(1), the value calculated while the front axle is alone on the span. If this is within the same tolerance, the value A(1) is accepted as correct. Otherwise the value of $A(1)^*$ is assigned and the drive A(1) are adjusted so that the gross vehicle weight is maintained. The empirical equations for $A(1)^*$ for each type of axle configuration are presented in Table 1. In most cases the front axle computed from the strain record and influence line were within the tolerance and accepted. The typical values $A(1)^*$ computed as a function of GVW and axle configuration were not often used.

3.2.3 Data Inspection and Output

Several programs were developed to inspect the data. One program, PLDATA, reads the strain data for each truck and graphically displays the records on the CRT for each instrumented girder. Inspection of these records allows the engineer to decide if the data for any truck should be discarded from the data set. Inspecting general data quality, spotting data with other vehicles on the bridge, and selection of appropriate girder weighting factors can be more easily accomplished. Girder weighting factors are further discussed in Section 5.5.

A program called INDTRK was developed to sort processed truck data by weight, axle configuration, and hauling category. Output consists of a table with one line per truck which includes axle weights and spacings, gross weight, arrival time, speed, hauling category, and axle configuration. Typical output is presented in Table 2.

The program SUMARY statistically combines the truck data. Vehicles are sorted by speed, length, gross weight, axle configuration, and visual hauling code. Results are presented in a single page summary, examples of which are shown in Tables 5, 6 and 12 for the three formats available.

3.3 Equipment and Instrumentation Improvements

3.3.1 Tapeswitches

A tapeswitch is made of two copper strips so that as an axle crosses the switch, the insulation layer compresses allowing the copper strips come into contact and close the switch. A typical axle crossing will produce an electrical pulse 15 milliseconds in length.

Three methods of installation have been used: duct tape, silicon caulking, and aluminum channels.

The duct tape is used with only the tapeswitch insert (the actual switch) which is approximately one half inch wide and one tenth inch high. One piece of duct tape four inches wide and slightly longer than the switch will hold the tapeswitch in place for several days. The tape will also hold once the pavement becomes wet but must be applied to a dry pavement. Due to the color of the duct tape and the extremely low profile, the tapeswitches applied in this manner were not easily noticed by traffic. The duct tape will not adhere to the pavement in temperatures below 20 degrees F.

Silicon caulking is used with a rubber boot in which the tapeswitch is inserted. The caulking does require more time for application. Afterwards, the hardened caulking must be cleaned from the boot. The caulking holds well and was successfully applied even when the pavement was wet.

The aluminum channels hold the rubber boot in which the tapeswitch is inserted and are held in place by anchor bolts that have been shot into the road surface with the use of a Ramset gun. Problems encountered with this method are the time required to install the anchor bolts, the varying pavement types and strengths, and the time required to bolt the channels to the pavement. In addition, the crossing of an axle over a channel produces a loud sharp noise that is clearly heard by the driver. Otherwise the channels do work well and can be applied when the pavement is wet.

In general the tapeswitch system worked well. Installation of four tapeswitches was usually completed within 5 to 10 minutes by a two man crew. Duct tape was the preferred and most used method of attachment. The switches were essentially undetectable when attached with the duct tape. Once installed the switches rarely came off and provided accurate axle pulses. They were applied to every bridge site, a particularly good feature since each location tested for this project was a "new" site. They are also applicable to data acquistion on bridges with bidirectional traffic.

Tapeswitches usually last through five to twenty days of data acquisition depending on weather, traffic, proper installation, etc. Since they sometimes fail and since they are not always easily installed, several other methods of axle detection were investigated. The problem is not that of velocity determination, which could be accomplished with loop detectors or radar, but that of axle detection so that axle spacing can be accurately determined.

Another method investigated was the use of the shear wave generated in the girder web due to the wheel impact as the vehicle's tires first reach the bridge. Two instruments were used to measure this phenomena, an accelerometer and a strain transducer. The energy of the wave has two components, potential, which is stored as elastic strain energy, and kinetic, which is the movement of the mass of the girder. The strain measurement worked best when the transducer was attached vertically at the leading edge of the bridge near the top of the girder. The accelerometers performed best when attached either at the same location as the strain transducer or on the support bearing. The best location for accelerometers varied from bridge to bridge. Both instruments worked well in determining the presence of the first axle but gave unreliable results for subsequent axles due to the forward axles statically and dynamically loading the structure. A sample record of a strain transducer used in this manner is presented in Figure 8.

Other axle detection systems were considered. In an earlier girder strain/fatigue life study at Case Western Reserve University, a photocell arrangement was used with a source (infrared) on one side of the roadway and a receiver on the other. The passage of an axle broke the beam and triggered the circuitry. The advantage of this system is that it requires no attachments to the road surface and it will work in most weather conditions. However, for multiple lane highways, it is difficult to distinguish in which lane the vehicle is traveling. Another problem occurs when traffic is along side the truck being weighed since additional axles would be detected. This method would therefore require mandatory use of the keypad or the development

of additional software logic to reliably sort axles of simultaneous vehicle crossings. A minimum of two sets of photocells would be needed to obtain velocity and axle spacings.

Another variation of the photocell arrangement would incorporate loop detectors in each lane to determine vehicle velocity, length, and the lane in which the vehicle is traveling. One photocell set would be needed to obtain axle spacings. The advantage of this system is that the operation could be fully automated; the visual observer would not be needed to sort multiple vehicle crossings. The loop detectors would necessarily be installed permanently. This would reduce setup time on return visits. Since loop detectors are not portable, each bridge for which data is desired would require installation.

A slightly different off roadway photocell system would use narrow beam proximity detectors. This type of detector has both sender and receiver in a single unit. Current technology of these devices limits their range to about 7 to 10 feet (2.1 to 3.0 meters); therefore they must be used on the outside lanes of traffic. These detectors are also more sensitive to dirt, etc., coating the lens than are the cross roadway sender/receiver paired detectors.

This project used tape switches exclusively for acquiring weight data. The short time constraint of this study and the large number of structures tested prohibited the use of another unproven system. It is likely that future systems could successfully employ some combination of photocells and/or loop detectors to avoid placing tapeswitches on the road surface.

3.3.2 Keypad

In previous Bridge WIM systems, a keypad or button box was used to indicate truck arrival and the number of axles to be taken. With improvement in the acquisition program, this axle counting task is now performed

automatically, freeing the keypad for use in higher level tasks. For this project the keypad was assigned the tasks of providing information on the lane (up to two) and the truck type. The fifteen categories assigned are given in Table 3. These categories allowed some breakdown by hauling category or body type for later statistical analysis.

The system could be operated in three modes with respect to the keypad. First, the keypad could be omitted entirely, requiring the program to decide which vehicle crossings were trucks. In the second mode the keypad entry was optional and could be used to supply information on lane and hauling category while the system decided on which vehicles data would be acquired as above. In the third mode, the keypad mandatory mode, the keypad was used to signal vehicle arrival, lane and hauling category. The system would then consider only those vehicles whose arrival was signaled by the receipt of a hauling category. The system still counts the axles and decides which vehicles to accept based on the user input constraint values on maximum strain and axle spacing. The third mode should be used at sites with high traffic density.

An audio headphone set was included with the keypad to facilitate communications between keypad operator and the instrumentation van.

3.3.3 Signal Conditioning Center

Two different units were assembled and tested. A commercial strain conditioning unit manufactured by Vishay was used in Systems 1 and 2. These units were installed in a rack that also housed the input/output connectors and the digital debouncing circuitry. System 3 used an autobalancing strain conditioning unit designed and built which is similar to that used in the previous FHWA sponsored bridge WIM project at CWRU. The autobalancing feature provided a constant zero thereby eliminating drift. The zero also allows for

easy detection of other vehicles that may be on the instrumented span during the crossing of a vehicle being weighed. This is important when the system is operating in the automatic mode and no observer is present. Although all three systems were more compact than previous systems, the signal conditioning center for System 3 was also modularized for greater flexibility. Circuitry was included to accommodate accelerometers or photocells as substitutes for the tapeswitches.

3.3.4 Strain Transducers

Earlier bridge WIM systems used foil strain gages. Reuseable transducers which can be installed with clamps or bolts are much more desireable in that they can be quickly installed on any bridge in any weather condition. Clamping strain transducers that are small and lightweight with reproducible sensitivities were designed and used in this project with great success. They were originally developed for use in the rugged environment of pile driving to measure the pile impact. These transducers are used world wide with some transducers still in use after 10 years.

3.3.5 Other Considerations

The system will operate in most any weather environment. The temperature extremes encountered in this project were -5 degrees F in Ohio to 110 degrees F in California. A temperature of 45 degrees F to 100 degrees F must be maintained in the the instrumentation van. In adverse weather conditions it may be necessary to place the instrumentation van or an unmarked car along the roadway so that the observer (if necessary) is sheltered. Alternatively, a camera/monitor system could be used so that the weighing operation is undetected.

The floppy disk drives are sensitive to the frequency of the AC power. The manufacturers' specifications indicate a tolerance of 2 Hz. The field personnel for this project reported a slightly greater range, about 3 Hz., depending on the van temperature and humidity.

CHAPTER FOUR

Survey Results

The data presented herein was collected as part of the FHWA Cost Allocation Study. FHWA and ODOT also collected truck weight data with Bridge WIM systems as part of the same study, however their results are reported elsewhere. The truck weight data collected was turned over to FHWA. Conclusions and implications drawn from the data are to be made by FHWA.

4.1 Vehicle Weight Data

Data has been acquired at 33 sites in the States of Arkansas, California, Georgia, Illinois, New York, Ohio and Texas. The results presented in the statistical summary for a total of 27,513 vehicles (which does not include the I-880 California site) include gross vehicle weights, vehicle velocities, vehicle length, lane, and arrival times. In addition, each vehicle was classified by axle configuration and visual hauling information was acquired on approximately 70 percent. Two hundred fifteen vehicles from the I-880 site near Sacremento, California were not included in any summary tables as discussed in Section 5.3.2. A summary of the number of trucks weighed by state, weekday/weekend, and type of route is presented in Table 4. Axle and tandem weights and arrival times were acquired for each vehicle and are stored on the floppy disks that have been submitted to FHWA. Printing individual data for each of the 27,513 vehicles is beyond the scope of this report as the data would require 500 to 600 additional pages. An example is presented in Table 2.

A summary of all trucks weighed (except I-880) is presented in two formats in Tables 5 and 6. The table formats, the routes tested and the

testing procedure were all approved by FHWA. Histograms are presented for: all trucks, Figure 9; all tractor trailers, Figure 10; all 3S-2 vehicles, Figure 11; all single units, Figure 12; and all truck velocities, Figure 13. In addition, the data has been classified by weekday or weekend and is given in Tables 7 and 8 with the corresponding histograms presented in Figures 14 and 15 for all trucks, all tractor trailers and all single units. The entire population has also been classified by route type with histograms presented for all trucks, tractor trailers, and single units for each type: Interstate, Table 9 and Figure 16; U.S. routes, Table 10 and Figure 17; and State routes, Table 11 and Figure 18.

The data for each of the seven states is summarized in Tables 12 through 18 with histograms for all trucks, all tractor trailers, and all single units for each state presented in Figures 19 through 25. Each site is summarized in Tables 19 through 51.

Girder weighting factors (see Section 5.5) were used in all cases to de-emphasize any car traffic that may be on the bridge in another lane. These weighting factors vary depending on the bridge and girder configuration and are listed on the processing log submitted with the site logs and floppy disks. Most vehicle data presented herein were weighed while alone on the bridge.

A total of 73 days were spent acquiring data of which 10 were weekend days. Typically the data was collected during one 12 hour shift and one 8 hour shift at each site. A third 8 hour shift was added for each site in the state of Texas. As Table 4 shows, there is clearly a difference in the volume of traffic as an average of 403 trucks per day were weighed on weekdays whereas the average was 209 trucks per day on weekend days. The weight histograms presented for all weekdays and all weekends, Figures 14 and 15, indicate the vehicle weights on the weekdays were slightly heavier. This is also apparent with the loadometer survey factor, Lo, discussed below, as the value was 0.414 for the weekday traffic and 0.385 for the weekend traffic.

4.2 Discussion of Results

4.2.1 Trends

The model and corridors were chosen by FHWA personnel who were assisted by personnel from the individual states. In at least two cases, sites were chosen that were believed to have a large number of heavy vehicles. sites were SR 36 near Caldwell, Texas, and SR 51 near Peru, Illinois. An FHWA report entitled "1975 National Truck Characteristics Report" published April. 1978 (12), includes data to which the data acquired in this project can be compared. The comparison is made between the Interstate Rural Roads data by FHWA and All Sites and Interstate Sites by BWS. The two sites mentioned above as having a large proportion of heavy vehicles is included in the All Sites category, but is not included in the Interstate Sites category. The comparison of the cumulative distribution of these data is presented in Table 52. The agreement with the FHWA results is excellent for both BWS categories. major point of difference is in the 80 kip (36,360 kg) or more range where the BWS results predict 41 to 58 trucks per thousand and the FHWA results indicate 17 trucks per thousand. This could be due to the fact that the Bridge Weigh-In-Motion system is undectable by truck drivers and thus is not avoided by heavier vehicles.

The field personnel were instructed to be as unbiased as possible in acquiring data. At most sites this was not a problem since headways were large enough to permit the acquisition of most vehicles. The crews were instructed not to weigh pickup trucks, recreational vehicles, house trailers in tow, or cars pulling trailers. In addition, any two axle vehicle with an axle spacing less than 12.1 feet (3.7 meters) was not weighed. A typical large pickup truck has an axle spacing close to 12 feet (3.65 meters).

4.2.2 Loadometer Survey Value

The loadometer survey value, Lo, is a damage spectrum factor applicable to bridge fatigue and pavement distress. Damage is proportional to loading cubed. The equation is (10):

$$L_{o} = \sum_{i=1}^{N} \frac{(W_{i})^{3}}{72} f(W_{i})$$

$$W_{i} = \text{Weight Category i}$$

$$f(W_{i}) = \text{Frequency of } W_{i}$$

where

A previous study performed at CWRU calculated Lo values from strain measurements on bridge girders (10). The "typical" value from both the strain measurements and loadometer surveys was determined to be 0.4. A similar value was used in preparing the AASHTO bridge fatigue requirement. Lo can be considered a measure of the shape of the loading spectrum and the predicted

"damage" for which a bridge must be designed. A higher Lo corresponds to a more severe loading environment. A value of 0.41 was obtained from the all sites data. The range of values for the seven states was 0.26 in Georgia to

0.51 in Illinois. The data are presented in Table 53.

4.3 Velocities

Velocity is needed in order to weigh a vehicle and to calculate axle spacings needed to classify a vehicle as a truck or a car. The velocities from all crossings can be retained and a histogram presented for all truck traffic, all car traffic, and all car and truck traffic. In addition, the truck velocity data can be presented in hourly intervals. Each summary table provides a breakdown in increments of 3 mph for the vehicles summarized in the

table. A few of the sites tested had maximum speed limits that were less than 55 mph. The summary of all sites, presented in Table 5 and Figure 13, shows 69 percent of the trucks weighed were traveling in excess of 55 mph. However, only 18 percent were going faster than 61 mph and 7 percent greater than 64 mph.

4.4 Strain Data

The weight calculation algorithm requires strain as a function of time for each vehicle. This strain data for each channel has been recorded on the data disks. Using the shunt voltage and transducer calibration, both recorded on the site logs, the actual strain at the transducer location can be determined. An example of the strain record is presented in Figure 8.

In addition, the peak strain for each vehicle weighed can be determined for each instrumented girder and presented in a peak strain histogram. An example is presented for the Caldwell site on Highway 36, Texas. This data is presented in Figures 26 through 29.

The strain data could be used to develop a load-strain model or to examine the dynamic response of the structure.

4.5 Histogram Comparison: Weigh Station and Bridge WIM

A correlation was performed on January 22, 1982 with a weigh station on I-90 in northeast Ohio (Section 5.1.3). A total of 293 trucks were weighed at the weigh station while 330 were weighed at the bridge site located 17 miles (27 kilometers) downstream, both weighing east bound traffic. The histograms for the weigh station and the bridge site are presented in Figure 30 and superimposed in Figure 31. Tables 54 and 55 contain the statistical weigh station and Bridge WIM data for this comparison.

Three exits are located between the weigh station and bridge site. It is possible that these exits are used by a higher percentage of lighter trucks which could affect a frequency presentation. The number of trucks over 50 kips (22,730 kg) was still nearly the same at both the weigh station (107 out of 293 total trucks) and at the bridge (100 of 218 trucks). In addition, 5 steel haulers were recorded at the bridge site while none were observed at the weigh station. The processed results from the bridge data indicate that four of the five had rear tandems loaded close to the allowable limit if not greater.

While acquiring the data at the weigh station, 5 vehicles were issued citations for overweight axles, none for exceeding the gross weight limit. Each of the five drivers made the unprompted statement that he would bypass the weigh station in the future. This weigh station issued citations to 0.3% of the vehicles weighed in 1980.

CHAPTER FIVE

Evaluation Of Bridge Weigh-In-Motion System

5.1 System Accuracy

During the course of the project several opportunities were available to assess the accuracy of the WIM system and to compare it with other weighing methods. It should be noted that the values to which the bridge WIM weights are compared are not in themselves exact but do have some variability. This is shown in repeatability measures performed on these "reference" scales with the calibration vehicle used to calibrate the bridge. It has been noted that two different platform scales have weighed the same truck and found a difference of 2.5 to 3 percent. The following measures are presented:

- A. Repeatability: Data from 74 crossings of calibration vehicles ranging in weight from 36.2 to 58.5 kips (16,450 to 26,590 kg) at 11 sites. Section 5.1.1.
- B. Repeatability: Data from 28 crossings of the same calibration vehicle at 6 sites in Arkansas. Section 5.1.2.
- C. Static Scale Comparison: Two correlations performed at the same site in Northeast Ohio. Repeatability is presented for both systems. Section 5.1.3.
- D. Portable Scale Comparison: Correlation with a two-wheel portable scale. Repeatability is presented for both systems. Section 5.1.4.
- E. Radian WIM Comparison: Section 5.1.5.

The accuracy measures reported for cases A and B and for all repeatability data are the standard deviation and the coefficient of variation (c.o.v.). For the correlations in cases C, D, and E two comparisons are made, the ratio of the weight predicted by the WIM system to that of the other method in which case the c.o.v. is given, and the difference between the WIM weight and the other weight in which case the standard deviation is given. The ratio method is biased toward the lower magnitude weights. For example consider an error of 2 kips (910 kg) in a weighing. For a truck weighing 8 kips (3,640 kg) this would give a ratio of 8/6 or 1.33. The same error with a vehicle weight of 65 kips (29,550 kg) would have a ratio of 65/63 or 1.03. The difference method is unbiased to the magnitude of the weight and thus is a better measure.

The system accuracy as a result of these studies is a standard deviation of $1.1\,\mathrm{kips}$ (500 kg) on gross weight for repeatability and a standard deviation of $2.3\,\mathrm{kips}$ ($1,045\,\mathrm{kg}$) on gross weight for random traffic when compared to a static scale.

5.1.1 Repeatability: General

As a typical illustration, the data from 74 calibration truck crossings at the initial 11 sites in the states of Georgia, Arkansas, and Texas are presented in Table 56. The standard deviation of the gross weight is 1.12 kips (510 kg) which corresponds to a c.o.v. of 2.3 percent. The standard deviation on the rear tandem weight is 1.34 kips (610 kg) and the c.o.v. is 7.2%. The c.o.v. of the ratio is greater for the rear tandem comparison. The rear tandem magnitudes are lower than the gross weight magnitudes but the standard deviation of the difference comparison is only slightly greater. The results are presented in Table 57.

5.1.2 Repeatability: Arkansas

The same five axle truck was used to calibrate all six sites in Arkansas over a period of 24 days. The vehicle was loaded at the beginning of the project and left undistrubed for the 24 day period. Each bridge had steel girders with span lengths ranging from 39 to 84 feet (12 to 25.6 meters). Five were simply supported and one was continuous over the piers. Two bridges had skews of 45 degrees and one had a skew of 50 degrees. The data from 28 crossings is presented in Table 56. The c.o.v. and the standard deviation are 1.74% and 1.02 kips (460 kg).

5.1.3 Static Scale Comparison: Ohio

Two correlations were performed at the same site located on I-90 over Paine Road in Northeast Ohio. The same weigh station which is located approximately 17 miles (27 km) upstream from the bridge was used in both studies (see Section 4.5). The scale was 12 feet (3.7 meters) in length and could weigh single axles or tandems. The first correlation was performed by CWRU and ODOT personnel on June 3 and 4, 1980. A total of 149 vehicles could be matched using visual descriptions and times of arrival.

Repeatability tests were done on the static scale and the Bridge system. Due to the weighing procedure at the weigh station the standard deviation of 1.74 kips (790 kg) was actually greater than the standard deviation of the Bridge system which was 1.10 kips (500 kg). The data for both repeatability tests are presented in Tables 58 and 59. The static scale requires a short period of time for the free vibrations of the scale to dampen out. This time is usually 5 to 10 seconds per axle or tandem except for tank trucks with liquid cargo which could require several minutes per axle or tandem. The ODOT personnel felt this time requirement was too long as the truck traffic would back up considerably, hence each axle was weighed as the scale was still

moving. In a few cases the weights were obtained as the vehicle was moving slowly across the scale. In addition, tandems were sometimes partially off the scale. Not surprisingly, the standard deviation of the difference was 3.08 kips (1,400 kg) with a c.o.v. of 7.12%. Of course, the standard deviation of 3.08 kips (1,400 kg) includes the variation inherent in the static scale weights. The accuracy of the Bridge system could be estimated from the relationship:

$${}^{2}_{\text{BWIM}} = {}^{2}_{\text{Sample}} - {}^{2}_{\text{Static}}$$
 (16)

The second correlation was performed by project personnel on January 22, 1982. This time the weights were not taken on the static scale until the scale had stopped moving. This procedure resulted in a continuous backup of truck traffic. If the backup became too severe, the trucks were passed through and no accurate weighings were recorded. With this procedure, the highest rate of trucks which could be measured was 52 per hour. Tank trucks with liquid cargo were not weighed with the same accuracy. A few weighings were obtained with the scale moving slightly. The resulting correlation of 87 vehicles had a standard deviation of 2.32 kips (1050 kg). The data is plotted in Figure 32.

A calibration vehicle was used on February 11, 1982 to measure the repeatability of the static scale. The standard deviation was .15 kips (68 kg) on gross weight for 21 repeat weighings. Since each axle or tandem is a separate and independent weighing, the number of independent gross weights obtained would be n cubed, the number of weighings for each axle raised to the third power (it takes three weights to make up a gross weight, steering axle, drive tandem and rear tandem). Thus there are close to 7,000 independent estimates of the gross weight with a range from 80.05 to 81.85 kips (36,390 to

37,210 kg). That same day the vehicle was used to calibrate a bridge on I-80 in Eastern Ohio. The standard deviation was .80 kips (364 kg) with a range of 79.7 to 81.5 kips (36,390 to 37,050 kg). A summary of the repeatability of the static scale is presented in Table 60.

5.1.4 Portable Scale Comparison: Arkansas

A correlation study was performed in Arkansas with a portable scale that was also collecting data for the HPMS study. A total of 56 trucks were correlated for a standard deviation of 3.01 kips (1,370 kg) of the difference between gross weight obtained by BWS and the portable scale. An attempt was made to measure the repeatability of the portable scale, however, of approximately 15 weighings only 5 weighings were considered good by the scale personnel and reported to BWS. These weights are presented in Table 61. should be noted that these weighings were performed with greater care than weighings of normal traffic. The driver reported that the vehicle was moved while on the scale so that the "best" weight could be obtained. Presumably, the "best" weight was the already known axle weights. Even so, the standard deviations on gross and rear tandem weights were .58 kips and .93 kips (260 and 430 kg). The average of the gross weights is 2.5% higher than the gross weight as measured on a platform scale, however the average of the rear tandem weight was 30.7 kips (13,950 kg) which compares very well with the platform scale weight of 30.6 kips (13,910 kg). The conclusion is drawn that this study does not measure the true accuracy of either the Bridge WIM system or the portable scale, although the correlation is good since the one to one comparison agrees well.

5.1.5 Radian WIM Comparison: Texas

A comparison study was performed with a Radian in-pavement weigh-in-motion system near Iowa Park, Texas on Route 287. The Radian system uses two pavement scales approximately 18 inches (.46 meters) long, one for

each wheel of an axle. Texas has several Radian WIM sites around the state. The scale transducers must be installed each time a site is visited, a process that takes about one and one-half hours during which the lane must be closed to traffic. Each axle is weighed separately and the axle weights are summed to obtain the gross weight. The pavement appeared to be in very good condition, a must with the Radian system.

The sample is 99 trucks that were weighed by both systems. The ratio of the gross weights has a mean of .89 and a c.o.v. of 10.0%. The standard deviation of the difference in gross weight is 5.3 kips (2,410 kg). The Radian c.o.v. can be estimated by equation 16 using 10.0% for the combined c.o.v. and 2.3% for the BWS c.o.v. The result is a c.o.v. of 9.7% for the Radian system alone. This corresponds very well to their published c.o.v. of 10.0% for gross vehicle weights of traffic traveling faster than 40 mph (64 km/h). It should be noted that the Bridge WIM system accuracy should be independent of vehicle velocity because of the smoothing algorithm described in Chapter 2.

The calibration truck was weighed by both systems a number of times for a repeatability measure. The data is presented in Table 62. The actual weight is 67.34 kips (30,610 kg). For gross weights the Bridge WIM standard deviation and c.o.v. are 1.08 kips (490 kg) and 1.60% while the Radian standard deviation and c.o.v. are 2.11 kips (960 kg) and 3.17%. It should be noted that the random traffic was tested at normal highway speeds near 55 mph (88 km/h). The test truck speed was consistently lower, about 45 mph (72 km/h). The Radian system performs better with slower traffic, less than 35 mph (57 km/h).

5.2 Velocity Accuracy - Effect on Gross Weight

Analytical parameter studies by Kriss (11) have shown that small errors in velocity measurements can produce small errors in gross vehicle weight. This relationship is presented in Figure 33. The tapeswitches are sampled at

least 50 times per millisecond. Once an axle is detected, a clock is read to provide a "timestamp" for that axle. This hardware clock is running at 1 kilohertz (1000 counts/second) which means that the timestamp, an integer, is given in milliseconds. The clockrate does introduce some error in the velocity calculation since the timestamp may be off by at most 1 millisecond. For example, if 125.3 milliseconds is the time required for the steering axle of the truck to travel the distance between the first and second tapeswitches, the timestamp will be 125. The resulting error in velocity will depend on the vehicle velocity and the tapeswitch spacing which along with the time will be used to compute the velocity.

The maximum velocity error for various tapeswitch spacings are given in Figure 34. A vehicle traveling 50 mph (82 km/h) crossing tapeswitches spaced at 7 feet (2.1 meter) will have an error that is at most 1.05%. Referring to Kriss's work and Figure 33, one can see that this small error will produce gross weight errors that are also very small (slightly less than the velocity error).

During the initial projects at CWRU the velocities measured by the tapeswitches were compared with radar and found to be in excellent agreement. As discussed above, the error in velocity is random and of low magnitudes. There is no loss of accuracy in the Bridge Weigh-In-Motion method as the velocity of the vehicle being weighed increases.

5.3 Bridge Selection

Prior to this project, only bridges in Ohio and Virginia had been used to collect truck weight data and those sites that were used were more or less ideal. One of the project's objectives was to determine the limits of the bridge weigh-in-motion concept. The bridge structures tested during this

project were chosen by FHWA or the State involved, with some input from BWS, after the routes were fixed. A list of the structures tested is presented in Table 63.

Although several of the structures tested proved to be difficult, good agreement for gross weights were obtained from all bridges. The axle weights from only one bridge, I-880, California, were thought to be questionable. This conclusion was made since for unknown reasons the axle weights on two (of four) of the calibration vehicle crossings were not good, although the gross weights were excellent. The axle weights of the random traffic on this bridge appear to be normal in almost every case.

5.3.1 Skewed Bridges

Six of the thirty-three structures had large skews defined here to be 30 degrees or greater. Four had skews of 45 degrees or greater. The difficulty with these structures is the shift in influence line for each side of the truck. For example, a truck with a wheelbase of 9 feet (2.8 meters) on a bridge with a 45 degree skew, one wheel of the front axle is on the bridge when the other wheel is 9 feet (2.8 meters) 'behind'. The best approach in processing this data is to redefine the beginning of the record, the start of the vehicle on the bridge, by shifting the strain record in time. This is easily done with several calibration truck crossings.

The gross weights from the skewed bridges were excellent as Table 56 shows. Four of the six Arkansas bridges had large skews. The standard deviation on gross weights for these four bridges is 1.01 kips (460 kg).

Axle or tandem weights were not quite as good. Three of the skewed bridges in Arkansas gave very good rear tandem weights as shown in Table 56. The standard deviation is 1.68 kips (760 kg) compared to 1.34 kips (610 kg) for all of the bridges presented in the table. The fourth Arkansas bridge,

I-30 at Route 67, had a 50 degree skew and a span of 84 feet (25.6 meters). The calibration vehicle data for this bridge is presented in Table 64. Although the gross weights are excellent, the variation in the rear tandem weights is significant. The standard deviation on the rear tandem weights is 3.28 kips (1,490 kg) for these four crossings. This variation is believed to be due to the combination of the large skew and the long length of the span. In general, axle or tandem weights are not as well defined on long spans as compared to shorter spans.

Skewed bridges presented no special difficulty in obtaining good agreement for gross vehicle weights. Most skewed bridges (including skews of 45 degrees) also provided excellent axle weights. If a structure with a skew greater then 30 degrees must be used to collect axle weights, a greater number of calibration vehicle crossings (say 10 to 15) should initially be made.

5.3.2 Difficult Bridges

A few structures required special effort to assure good vehicle weights. The primary reason for the difficulty is other traffic on the bridge contributing to the measured strain of the vehicle being weighed. This is due to either extremely dense traffic (57,000 average daily traffic (ADT)) or very long bridges. In all cases, the gross weights obtained when the vehicle was alone on the bridge were in good agreement. Axle weights were considered to be good in all but one case, the box girder bridge on I-880 in California. Each structure is discussed below.

US 101 - Ventura

This site had a high ADT and a relatively long span of 84 feet (26 meters). Data from this site was further complicated by the fact that the bridge had four traffic lanes that were carried by these plate girders, making separation of cars and the vehicle being weighed difficult.

I-5 - California - Mokelumne River

This was a reinforced concrete continuous bridge. Due to restricted access to the girders for transducer installation, the last span of the bridge was instrumented. The processing program had to be modified to analyze the data. At the time of vehicle arrival at the instrumented span, a strain was already induced at the transducer location since the vehicle was entirely on the preceeding span. A processing program in use at CRWU was used for this instance. The analysis routine was changed so that the truck appears to be driven backwards over the bridge. The analysis is begun when the last axle is at the end of the span and continues until the front axle reaches the beginning of the span. The results from this bridge with this analysis technique are considered to be excellent.

I-880 - California - Sacramento

This prestressed concrete bridge was a box girder. Theoretically, the strain due to a vehicle at any lateral position would produce the same strain at any point along the cross-section of the bottom member. The box girder was continuous across the supports with the instrumented span length being 83 feet (25.3 meters). To complicate matters further, the ADT of this bridge is 57,000 vehicles which was extremely dense.

Due to the relatively low strains encountered, the transducers were placed such that the transducers spanned cracks thereby magnifying the apparent strain. Assuming the cracks are minute the transducer response should be linear.

The calibration vehicle used had a gross weight of 60.3 kips (27,410 kg). Of the four good crossings, the predicted gross weight ranged from 60.0 to 60.7 kips (27,270 to 27,590 kg) which is excellent repeatability. However, the variation in axle weights was found to be unacceptable.

Due to the

questionable reliability of the calibration truck axle weights, this data was not included in the final tables combining all trucks, California trucks, etc. Examination of the axle weights of the random traffic recorded at this site would indicate that most axle weight distributions for the random traffic appear to be reasonable.

I-80 Illinois

The traffic over this two lane bridge was carried by two deep plate girders. The instrumented span length was 90 feet (27.4 meters) and the overall bridge length was greater than 400 feet (122.0 meters). The girders were continuous over the supports. This made the isolation of vehicles difficult while acquiring data. During processing, however, it was quite easy to determine when another vehicle was affecting the weighing. Only those vehicles that were alone on the bridge are included in the statistical summaries.

5.4 Processing Results Using Shorter Span Lengths

During this project, strain data was acquired from the time the first axle of a vehicle arrived at the first tapeswitch until the last axle had reached the end of the instrumented span. Thus, the length of time of strain acquisition is flexible; the acquistion time is determined for each vehicle and is a function of the vehicle velocity, the vehicle length (first axle to last axle) and the distance from the first tapeswitch to the end of the instrumented span. The maximum number of samples that can be acquired is a function of the sampling rate and the number of girders instrumented.

Longer span lengths require a greater headway limitation in order to have only one vehicle on the span at a time. By reducing the length of the span over which the data is processed, the likelihood of a trailing vehicle

influencing the results is reduced. This is accomplished during processing by entering a reduced span length in place of the real span length. The data is then processed until the last axle reaches the modified span length.

This technique was tried on only two bridges with span lengths of 53.5 and 37.0 feet (16.3 and 11.3 meters). The results indicate that small reductions (25%) in span length had little effect on the gross weight or weight distributions of heavier trucks.

5.5 Girder Weighting Factors

As a vehicle crosses an instrumented bridge span. the lateral distribution of each axle load has been assumed to be constant with axle position along the bridge. Consider, for example, a bridge with six girders carrying two lanes of an interstate highway. The peak strain for a vehicle in the right lane (girders 2 and 3) would be distributed as (normalized) .20, .90, 1.00, .80, .25, .15 for girders 1 through 6. The strain on girders 1 and 6 is due primarily to the dynamic response of the structure and therefore girders 1 and 6 were usually not recorded during this project. A vehicle in the left lane might have a distribution such as .10, .20, .60, 1.00, .80, .40. By weighting the strain record for each girder the effect of another vehicle can be lessened. Note that the calibration factor must be calculated for each set of weighting factors used. Typical weighting factors used in the data analysis would be:

The effect of using weighting factors was investigated using the January 22, 1982 correlation data from the Ohio I-90 site (Section 5.1.3). The standard deviation of the prediction was calculated for each set of weighting

factors. The results are presented in Table 65. The use of weighting factors does increase the variability. In general, the more the factors are reduced the higher the variability. The best approach to minimize the effects of additional traffic on the span would be to monitor the tapeswitches in both lanes whenever a vehicle is being weighed. Thus when an additional vehicle is detected in the other lane, severe weighting factors would be used. The majority of weighings which would have the vehicle alone on the span would use uniform weighing factors and thus have less variability.

One possible reason for the increased variability with reduced weighting factors could be the effect of lateral position of the vehicle while on the span. When using all of the girder strain records evenly (weighting factors of 1.0), a 5 foot (1.5 meter) difference in lateral position would not be noticeable. However, this would not be the case if the weighting factors of the 3 girders directly beneath the lane were nonuniformly reduced.

Another possible reason is that the lateral distribution is not constant with position along the span, but rather the load appears to be more evenly distributed at the transducer location as the load is further along the span. This is even more apparent for continuous bridges with the load on the second span.

For the general traffic recorded, auto traffic in adjacent lanes sometimes caused minor shifts in the apparent zero strain level in the girders associated with the auto traffic lane. The use of girder weighting factors in this project greatly reduced this influence in most cases.

5.6 Weigh-in-Motion Economics

The need for weigh-in-motion operations has been documented. There are the benefits of added personnel safety with no stopping of traffic when comparing portable weighing with the Bridge WIM operation. There is also the

advantage of unbiased results of an undetected operation. In addition, there is the question of economics. Because fixed weigh stations can now cost several million dollars each, the comparison should be made with portable scales. The main advantage to weigh-in-motion in this regard is crew size and weighing rate. A portable system will utilize between 10 and 15 personnel for flagging, positioning, weighing and recording if any meaningful weighing rate is to be maintained. This Bridge Weigh-In-Motion system can be operated by two field personnel and is capable of weighing rates well above 100 trucks per hour, depending on the site. This project saw a maximum weighing rate of 219 trucks per hour.

The Bridge Weigh-In-Motion system is portable and mobile, i.e. the same system can support weighing operations at several different sites. No special construction (ramps, extra lanes, etc.) is required as the system adapts to existing highway bridges.

Setup time at a new site is about 1.5 to 2 hours although the field personnel for this project were able to reduce setup times to near 1 hour due to experience and organization. Repeat visits require even less time. If several consecutive days of weighing are to be done, the transducers and tapeswitches can be left in place, thereby reducing the set-up time to less than one-half hour. In addition to the two field personnel, a calibration vehicle is needed (once per site) and effective supervisory control from an engineer for site selection and weight processing. Although the system can process in the field within one second of a truck crossing, general processing, including the preparation of statistical weight table and summaries, should be done after a test is completed. Allow about one day of processing for each 3 to 5 days of field testing.

Considering initial setup, processing time, travel and routine maintenance, one can expect about 7 weighing shifts for a two man crew during each two week period. Assuming an average traffic of 70 trucks per hour

(limited only by the traffic on the route as the Bridge WIM system is capable of weighing over two hundred trucks per hour), this should mean about 3000 trucks every two weeks per test crew, i.e. 70 trucks/hour x 6 hours x 7 shifts/2-week period. This rate would vary with traffic, number of shifts per site, travel time and maintenance responsibilities. The best estimates obtained for a portable scale operation were 42 trucks per hour. Reference 3 gives typical weighing capacities for semi-portable scales at 25 vehicles/hour and for portable scales at 3 vehicles/hour.

As a weighing operation, the system causes no delay to truckers, thus reducing fuel and time costs and hence freight costs. The system was designed to monitor traffic in the normal traffic flow at normal operating speeds. The predictions do not appear to suffer from any loss of accuracy at high speeds. The current system could be modified to include photodetectors and/or loop detectors with additional software logic so that the system could be permanently installed and operate completely unattended. These permanent installations could provide continuous data collection.

In addition to test personnel, supervisory engineer and calibration truck, there are equipment maintenance costs. Of the five systems in use today for a combined period of six years, the total maintenance and replacement costs are less than \$10,000. If desired, the major system items could be placed under a service contract for approximately \$4,000/year. The other major item is the instrument vehicle; ODOT uses a standard van and FHWA uses a Winnebago type RV designed to facilitate demonstration of the system to others. The large vehicle is more comfortable for heavy use and houses its own generator.

CHAPTER SIX

Conclusions and Recommendations

6.1 Conclusions

- 1. Three complete Bridge Weigh-In-Motion systems have been designed, assembled, utilized and delivered for operation to FHWA. The systems have been designed to collect unbiased truck weight data in an undetected manner at normal highway speeds. One of these systems uses an autobalancing signal conditioning system designed during this project.
- 2. More than 27,000 trucks have been weighed during the study in seven states. This weight data is summarized in this report and has been turned over to FHWA. The data appears to be unbiased. The greater percentage of heavy trucks obtained in this study compared to other reports seems to indicate that the weighing operation was probably not bypassed. This may have been the case with other studies using weighing methods that are more apparent to truckers. The data acquired by FHWA and ODOT with Bridge Weigh-In-Motion systems are not discussed here.
- 3. The state-of-the-art of weighing trucks in motion utilizing bridges as scales has been greatly advanced:
 - -the time required to acquire each truck has been reduced by more than a factor of ten:
 - -the weighing operation has been automated such that data in one lane can be acquired without an observer except under heavy traffic conditions;

- -the data can be processed in real time in the field to obtain axle and gross weights in less than one second after the vehicle has crossed the instrumented span;
- -the system can acquire data in two lanes with an observer operating the keypad;
- -the number of axles are automatically counted allowing optional visual hauling information to be input via the keypad and observer;
- -the hardware system has been modified to reduce size and increase flexibility and performance.
- 4. Night weighing has been done easily with no danger to crew personnel or to the traffic.
- 5. The system was used on a routine basis to acquire large amounts of data with only two crew personnel and part time supervision by an engineer.
 - 6. The weighing operation is undetected by drivers.
- 7. Accurate and repeatable weighings may be carried out. On a statistical basis the results compare quite favorably with both fixed and portable weighing techniques. The accuracy of the gross weights is better than axle weights. Its application is believed acceptable for unbiased planning studies, pavement loads, economic studies, and undetected enforcement screening.
- 8. The economics of this in-motion weighing technique indicate high weighing rates with two man crews at low cost. The hardware equipment is primarily off-the-shelf and can be placed under the manufacturer's maintenance contract.

- 9. Sites can be monitored quickly and routinely. Initially, at each site a calibration vehicle is needed. Usual setup is less than two hours with two personnel. Setup during repeat visits is shorter.
- 10. The system was demonstrated and/or presentations were made to state and federal officials in 12 states. An estimated 150 officials viewed the system in action.
- 11. The range of acceptable bridge types has been expanded. Site selection was generally not a problem and weighing sites were found to meet the FHWA data requirements.
- 12. Crew training required only a minimal effort. A total of 4 people were involved in data acquisition at any one time with two systems operating. FHWA personnel were also instructed in the operation of the systems and have performed their own independent weight studies.
- 13. Velocity measurements are another indirect benefit of the Bridge WIM operation.
- 14. Average truck weighings per weekday were double that of weekend days. However, the gross weight histrograms were similar.
- 15. The study data agrees very well with the interstate rural roads data reported in the "1975 Bational Truck Characteristics Report" by FHWA, April 1978, except in the greater than 80 kip (36,360 kg) category. The 1975 report gives a frequency of 17 per 1,000 whereas this study has found 41 per 1,000 (all sites) and 58 per 1,000 (all interstates).
- 16. The loadometer survey value from all sites of .41 compares very well to the typical value of .40 as determined by previous studies. The value for each of the seven states ranged from .26 in Georgia to .51 in Illinois. The higher the value, the more severe the loading.

17. A summary of truck velocities for all sites indicate that 69 percent were in excess of 55 mph (88 km/h), 18 percent were greater than 61 mph (98 km/h) and 7 percent were greater than 64 mph (102 km/h).

6.2 Limitations

- 1. Field crews must be adequately trained and supervised. An engineer must be in charge of the weighing operations to make decisions on test sites and data evaluation.
- 2. Although there are many potential weighing sites, not all bridges will be satisfactory scales. Bridges with large skew (greater than 45 degrees), or excessive vibration should be used only after careful review of calibration test data. Reinforced concrete bridges with significant cracking should be used with caution. Prestressed concrete beams work well. Trusses, timber bridges and older type structures have not yet been studied.
- 3. The accuracy of individual axle weights is reduced when trying to sort weights of closely spaced axles (tandems). Gross vehicle weights are not affected. Tandem axles have been treated as one unit in the weighing, as reported by many static weigh stations. Problems have occured on a few bridges with the relatively light front axle weights, but since these axles are generally known within narrow limits, methods of adjustment were employed.
- 4. The current processing algorithm assumes a constant vehicle velocity over the span, although modifications could be made to also account for vehicles having constant acceleration.

6.3 Future Work

The system in its present state is versatile and capable of handling most weighing situations. Further improvement depends on the needs and uses for truck data and the cost incentives to acquire large volumes of data. Some of the changes that should be considered include:

- 1. Further automating the system so that two or more lanes of traffic can be obtained with no observer necessary. This would also include a further check for other traffic on the bridge during a weighing. This is being undertaken by Bridge Weighing Systems, Inc.
- 2. Develop a technique to weigh trucks that are side by side and/or closely spaced such that large amounts of truck headway data can be acquired. This is important in developing bridge design loading models. CWRU has been awarded an HPR study from ODOT for this work.
- 3. Some sites may be considered for permanent instrumentation as a substitute for the more expensive fixed static scale weighing. Strain gages mounted on some Ohio bridges were successfully performing two years later without any protection from the weather. For permanent installation, strain gages or transducers should be protected from the elements and vandals.
- 4. Permanent weighing sites may also warrant greater degrees of automation. For example, a stand alone system could be developed and data could be telemetered or sent over phone wire to a central installation.
- 5. If in the future, trucks are required to carry airplane-type transponders for identification for tolls, etc., this could be tied to weigh-in-motion.
- 6. The range of sites could be extended, especially for monitoring rural locations, using older bridges to check conformance with the posted weight

limits. The limits of other types of bridges should be investigated such as truss, arch, timber and frame structures.

7. Testing is needed to evaluate enforcement effectiveness by simultaneously weighing on bypass roads. The seasonal variation in the enforcement effectiveness should also be investigated. Some of these operations are now being conducted by ODOT, also using the WIM method.

Table 1: Front Axle Weight Equations

Type Code	Equation
1 5,8,9,13,14,15 12 2,4 3,6,7,10,11	0.35(GVW) 7.1 + 0.04(GVW) 7.4 + 0.03(GVW) 3.3 + 0.21(GVW) 3.2 + 0.15(GVW)
Acceptable Variation codes are given in	

1 kip = 454 kilograms

ID# TIME SPEED	CONFIG K	AXLE WEIGHTS	× AXLE	SPACINGS	> LENGTH
168 88046 37 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10000000000000000000000000000000000000	00000000000000000000000000000000000000	\$\\\ \frac{4}{4} \\ \frac{4} \\ \frac{4}{4} \\ \frac{4} \\ \frac{4}{4} \\ \frac{4} \\ \frac{4} \\ \frac{4}{4} \\ \frac{4}{4} \\ \frac{4}{4} \\ \frac{4}{4} \	43401009007101918989280200800110099119901099119393517 43444443443333334344443 444 333444 33344 33344383434 72222222222222222222222222222222222	00000004000000000000000000000000000000

Table 2 - Trucks over 60 Kips GVW - Caldwell, Texas

ID#	TIME SPEE			AXLE WEIGHTS		× :	AXLE	SPACI	NGS		> LENGTU
812434504589030356789137899079245 900011122222333444445555568889999 9000000000000000000000000000000	LANE 121 1 592 1223 1 592 1223 1 566 1233 1 666 1233 1 666 1303 1 666 1306 1 657 1311 1 1 566 1307 1 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 566 1311 1 1 1 566 1311 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	88888888888888888888888888888888888888	0113003213091411291289608926110390 V774.661791111111111111111111111111111111111	13 13 14 14 14 14 12 17 17 18 18 18 18 19 19 11 18 18 18 19 19 11 18 18 18 19 19 11 18 18 18 18 18 18 18 18 18 18 18 18	000000000000000000000000000000000000000	0 1427 0 1310 0 1336 0 1255 0 1448 0 1211 1 1 1 1	1310635021122421921321114291323223 444444444434484443444444 2 2 2	1222222221123222322 1 21232322 112322232221123222322 1 22222182 222222182 222222182 222222182 22222222	3343444 343434 344344 4 4333 4343444 8909000819190080170020089990392720	000000000000000000000000000000000000000	L0000000000000000000000000000000000000

Table 2 - Trucks over 60 Kips GVW - Caldwell, Texas (Continued)

Type - See Table 3
Configuration - See Table 3
Gross Axle Weights in kips (1 kip = 454 kilograms)
Axle spacing in 0.1 feet (1 ft = .305m)

Table 3: Vehicle Category and Configuration Key

Code (Keypad)	Category (Body Type)	Code (Universal)	Classification (Configuration)
17 18 20 24 33 34 36 40 65 66 68 72 129 130 132	IGNORE/LANE 2 SPECIAL OTHER TEST CHEMICAL FUEL CONCRETE TANK OPEN HAULER MACHINERY STEEL FLAT DUMP BUS CAR CARRY BOX	1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16	2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 2S-2 SPLIT 3S-2 3S-2 SPLIT 2S-3 2S-3 SPLIT 2S-1-2 3S-3 3S-3 SPLIT 3S-1-2 OTHER/BAD SPACING
0	UNCLASSIFIED*		

^{*}For data acquired automatically without optional keypad input

Table 4: Survey Summary

Vehicle Type	Number Weighed
All Trucks	27513
Singles	5013
Tractor Trailers	22002
3S-2	15802

	No. of	No. of	Average
	Trucks	Days	Trucks/Day
State Routes	6957	18	222
U.S. Routes	2665	12	387
Interstate Routes	17891	43	416
All Routes	27513	73	377
Weekdays	25420	63	403
Weekends	2093	10	209
Arkansas	4442	13	342
Texas	7354	22	334
Georgia	3688	8	461
California	4996	10	500
Illinois	2584	9	287
New York	2960	8	370
Ohio	1489	3	496

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TABLE 7 ALL SITES, ALL STATES, WEEKDAYS ONLY (SI) 20 25 30 35 40 50 70 65 GVW IN KIPS-0 45 TO 15 TOTAL 25420 927 ALL TRUCKS SPECIAL 39 79 2 83 67 54 49 47 41 42 38 39 28 12 OTHER Ō Ü LIVESTOCK Ø FUEL CONCRETE S 1018 61 44 37 20 TANK 21 13 76 33 7 ġš B ġΪ OPEN 22 23 40 45 1<u>1</u> 7 MACHINERY STEEL FLAT BED 24 O 27 18Ğ 70 DUMP SUS 53 722 661 2Š 480 CAR CARRY S 473 443 398 481 509 680 504 335 248 136 BOX TRUCK 710 1001 îŝ **3** UNCLASSIFY 2 SINGLE 3 SINGLE 131 373 **46** 1582 25-1 5,4 O 4 SINGLE -0 42 2Ŝ 12<u>0</u> 25 1Ž 7 35-1 $2\overline{1}$ 4Ē ŽŻ """SPLIT ŠŽ S 7 21 1015 1211 1538 1218 37 45 68 66 1 3<u>9</u> 3<u>3</u> 5 15 " " SPLIT 115 28-3 """SPLIT Ō B Ö S 25-1-2 35-3 2<u>1</u> 5 19 18 " " SPLIT Ō 35-1-2 G BAD OR NO 5<u>2</u> 4Š LANE #1 2567 1355 1919 2853 1919 1364 1180 1075 1052 1223 1387 1764 1318 136 140 277 331 168 131 107 74 77 76 78 87 115 27 ġ LANE #2 0-31 31-34 34-37 37-40 40-43 43-46 46-49 49-52 SPEED-MPH 61-64 ัร18 73+ NO. TRUCKS SPEED-MPH 52-55 55-58 58-61 64-67 67-70 70-73 NO. TRUCKS 1-2 1 77 3-4 76 4-5 2-3 5-6 6-7 7-8 8-9 8-10 10-11 11-12 NO. AM 42 77 48 76 92 116 198 308 NO. PM 2787 2767 2698 2570 1935 1375 1157 1002 950 1451 2147 607 135 LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 60+ 533 740 1036 2888 9292 4692 1461

TABLE 8 ALL SITES, ALL STATES, WEEKEND DAYS ONLY (10) GVW IN KIPS-0 20 30 TO 15 <u>9</u>5 ALL TRUCKS 13<u>1</u> SPECIAL 6Ž Δ Ο OTHER Ò LIVESTOCK ā FUEL CONCRETE à ō Ü Ö TANK OPEN G MACHINERY Ū 17 STEEL Ω \$ 7 Ç 82 57 22 794 7 6 FLAT BED 7 7 Ō Ğ Ō 16 DUMP S ō 2 56 BUS ĕ CAR CARRY 52 35 Õ 18 3<u>8</u> **BOX TRUCK** 5 Ž Ô UNCLASSIFY 132 42 18 3 2 SINGLE C 7 3 SINGLE ŧΞ Ō Ō Ó. 25-1 4 SINGLE Ũ <u>0</u>7 35-1 3<u>3</u> 2 2S-2 Ō Ó Ò 72 3 62 3 7<u>9</u> 7<u>9</u> 3 " " SPLIT 35-2 7<u>2</u> 7Š 5 Ž """SPLIT 25-3 "SPLIT ¥Ž 7 Ō Ö Ô ō Ō ō ō Õ Ó Ó Õ Ō Ź ž ž Û Ē, Ö ō Ô Ō Ō Ö Ŏ 25-1-2 35-3 **3** Ĝ В Ō """SPLIT Ó Ô Ó Ó Ō Ō Ō Ō Ō 35-1-2 Ō Ž Õ Ü BAD OR NO ALL SINGLE 1641 94 128 2²1 135 94 17 Ž Š Q. **S**Š 17 5 LANE #1 3 LANE #2 ō SPEED-MPH 34-37 37-40 40-43 43-46 46-45 49-52 64<u>-</u>67 NO. TRUCKS 73+ A.A. ā. SPEED-MPH 52-55 55-58 61-64 57-70 Ĩ5 ARRIVAL 12-1 1-2 2-3 3-4 4-5 5-6 6-7 7-5 8-9 9-10 10-11 11-12 23 ō ō Ó LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 80÷

TABLE 9

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TABLE 10 ALL US ROUTES, ALL STATES 20 GVW IN KIPS-0 TO 15 Z0 25 30 35 40 **5**0 55 45 85 70 75 80 85 55 100 105 TOTAL ALL TRUCKS SPECIAL OTHER LIVESTOCK 3 2 7<u>1</u> 2 Ö ß, FUEL 13 76 Ľ, C CONCRETE ? 7 Ō TANK S S Ž OPEN S Ž Ž MACHINERY STEEL £, 17 2 5Ž 9 8 8 1<u>4</u> 2 1 3 7 FLAT BED 2 31 Õ Ō Ô CAR CARRY BOX TRUCK UNCLASSIFY 222 35 20 1151 2ŏ 75 76 51 38 59 3i 78 59 <u>ិ</u> 4Š 75 $2\overline{4}$ Ġ 2 SINGLE G. 10 5 3 SINGLE 12 Ō Ω 2S-1 Ō Ō C Ō Ö 4 SINGLE Ü ្ 35-1 ā š Δ 25-2 "SPLIT 22 1561 2<u>3</u> 3<u>1</u> 5 ō Ō ō ō ŏ Ö Ō Ō Õ Ō 2 1 8Ö 2 Z, """SPLIT 43 25-3 å """SPLIT Ó Ō Ō Ō Ō Ō Õ Ö Ō Ğ ē Õ Ō Ō 25-1-2 35-3 Š Ō Ō ż Ž " " SPLIT 35-1-2 BAD OR NC ЗÕ ALL SINGLE ALL COMBIN LANE #1 LANE #2 Û SPEED-MPH NO.TRUCKS 46-49 129 70-73 49-52 284 73+ 43-46 0-31 34-37 37-40 40-43 58-61 61-64 64-67 55-58 67-70 SPEED-MPH NO. TRUCKS 8-9 9-10 10-11 11-12 112 107 203 287 ARRIVAL 12-1 4-5 7-8 2-3 3-4 5-6 1-2 S-7 NO. AM NO. PM LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 ND. 218 282 90 53 65 123 206 841 612 153 +

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TABLE 11 ALL STATE ROUTES, ALL STATES GUW IN KIPS-0 30 40 75 TO 15 žö ŠÕ. ŘŠ. 8ŏ ĀŠ TOTAL 30 21 14 59 0 17 20 45 24 24 30 29 2 22 ALL TRUCKS 10 SPECIAL ŽŠ. ĠĬ OTHER LIVESTOCK Û 3 FUEL Ō 33 15 16 27 CONCRETE Ò Ō Ω 22 16 7 12 74 TANK 25 29 Ž3 OPEN MACHINERY 735 Ó ō Ō Ē. STEEL 18 23 35 23 Õ FLAT BED 2 5 20 DUMP Ō Зō Õ Ō Ó BUS Ō Ô Ō Ω Ō CAR CARRY 84 1716 **4** 302 176 62 53 59 77 7<u>8</u> **BOX TRUCK** UNCLASSIFY 20<u>9</u> ĠŽ 6Ē S Ą. 2 SINGLE 252 71 42 41 2 5Ŝ 3 SINGLE 7Ē Ō 25-1 4 SINGLE 83 39 35-1 3 73 2 Ō """SPLIT Ė Ō S 15 12 8 2 35-2 25 6 ŽÒ """SPLIT Ó ō ŏ Ō Õ 25-3 88 Ō 23 75 Ò """SPLIT S 25-1-2 35-3 2ġ 2 2 2<u>9</u> 3 8<u>8</u> 3 Ω îż Īά ō " " SPLIT Δ BAD OR NO ß 174 ALL SINGLE 5004 67 678 188 166 ō 115 LANE #1 LANE #2 1337 110 68 57 65 SPEED-MPH 0-31 31-34 34-37 37-40 40-43 43-46 46-49 49-52 NO. TRUCKS SPEED-MPH 52-55 55-58 58-61 51-64 54-67 67-70 70-73 73÷ NO.TRUCKS 4-5 5-6 B-9 S-10 10-11 11-12 312 507 612 775 3-4 5-7 7-8

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TABLE 12 ARKANSAS

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TABLE 14 CALIFORNIA ALL SITES																					
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TABLE 19 ARK. I-30 @ US 87 BENTON

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TABLE 20 ARK. I-40 @ US 161 PROTHO JUNCTION

			IPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	80 90	90 150
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TABLE 21 ARK. US 67 JACKSONVILLE

LENGTH -15 15 NO. 58	ARRIVAL 12-1 NO. AM O NO. PM 49	SPEED-MPH SPEED-MPH NO. TRUCKS	Z SINGLE 3 SINGLE 28-11 4 SINGLE 28-12 28-12 8PLIT 28-13 8PLIT 28-13 8PLIT 38-14-2 ALL COMBINGLE	ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL ELAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY
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TABLE 22 ARK. I-40 @ SR 25 W.BOUND CONWAY

G		IPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	80 80	90 1 5 0
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	TOTAL 1002 125 105 2205 225 364 225 253	49 07 00 1 00 03 4 00 95	80 0 8 0 0 0 1 0 1 0 2 6 4 0 5 3	177 09 00 53 00 156 20 75	124010001312650195	10100032150320016 35	122 00 00 01 2 2 3 0 5 1 0 1 32 75	288 10 0 1 0 1 10 21 7 0 55 171	48 60 00 00 81 20 21 00 35	40000000000004	4 0 0 0 0 0 0 0 1 0 0 0 0
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LENGTH -15 NO. 41								5-50 5 0 44 2			

TABLE 23 ARK. US-65 @ RT 256 PINE BLUFF

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TABLE 24 ARK. I-30 @ RT 25 HOPE

			10 20 20 30		40 50	50 60	60 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK TOPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	2 10 4 0 0 7 5 2 2 5 4 2 7 4	0300000000	54 114 0 0 4 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 1 0 0 0 0 Z 0 0 0 1 3 3 1 0	16001000101790149 129	173 00 00 00 00 10 14 00 15 140	254 1 0 1 0 0 1 0 1 6 16 0 18 209	122 00000030038000162	7 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	300000000000000
2 SINGLE 3 SINGLE 28-1 4 SINGLE 35-1 2S-2 " " SPLIT 35-2 " " SPLIT 25-3 " " SPLIT 25-1-2 35-3 " " SPLIT 35-1-2 BAD OR NO ALL SINGLE ALL COMBIN		000000000000000000000000000000000000000	25 3 0 14 3 0 1 3 0 1 13 15 2 55 13 2 5 5 13 2 0 0 0 0 0 0 0 0 0 0 0 25 92	13 07 22 86 00 10 00 03 13	0520450011010751 151	050011301 13012007000454 16	0 1 0 0 0 2 0 0 0 0 1 0 1 3 2 5 3 2 5 3 5 3 5 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000000000000000000000000000000000000000	000000000000000000000000000000000000000
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	52-5	2 5 55 -5	2 3 58-6	i 1 61-	0 -64 6	5 4-67	17 67-70	7 > 70 -	22 -73	52
ARRIVAL 12 NO. AM NO. PM	0 0	0 (0 0	0	O	0	50	83	93	114
LENGTH -18										

TABLE 26 TEX	AS HWY-36	N.BOUND	CALDWELL		
	IN KIPS-0 TO 10	10 20 20 30	30 40 40 50	50 60 60 70	70 80 90 80 90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	STAL 1511 70 8 0 35 5 5 0 148 0 57 0 145 0 130 1 226 3 4 2 7 0 51 4 842 54	298 328 0 1 13 4 0 1 3 16 0 0 1 8 2 72 1 2 0 2 13 33 114 26 1 0 0 0 5 29 142 134	142 133 1 3 0 2 1 3 3 0 2 2 1 3 3 0 2 2 1 3 3 0 2 2 1 3 3 0 2 2 1 3 3 0 2 2 1 3 3 0 2 2 1 3 0 2 1 3 0 2 1 3 0 2 1 3 0 3 0 2 1 3 0 3 0 3 0 2 1 3 0 3 0 3 0 3 0 3 0 3 0 3 0 3 0 3 0 3	82 80 1 0 0 3 2 2 17 5 1 8 3 12 0 9 1 1 3 3 35	128 182 72 0 0 0 0 0 2 0 1 2 0 24 47 28 0 1 0 5 27 6 19 15 0 5 2 4 8 1 0 13 11 4 3 2 0 0 0 0 0 0 0 0 4 3 50 68 27
Z SINGLE 3 SINGLE 2S-1 4 SINGLE 2S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 2S-1-2 BAD OR NC ALL SINGLE ALL COMBIN					0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	9 52-55 55	0 4 -59 58-61	9 61-64 64-	17 15 -67 67-70	3 46-49 48-52 5 45 119 0 70-73 73+ 0 55 22
ARRIVAL 12-1 NO. AM O NO. PM 151	0 0	0 0	0 0	7 64 1	
LENGTH -15 15 NO. 121	-20 20-25 2 337 38	5-30 30-35 15 91	35-40 40-45 74 242	5 45-50 50 2 305)-55 55-60 60+ 221 55 12

TABLE 27 TEXAS I-45 മ PARK. 73D. 4 HUNTSVILLE

NO ST.	ARRIVAL NO. AM	SPEED-XPI NO.TRUCKI SPEED-MPI	2 S S S S S S S S S S S S S S S S S S S	SPECIAL OTHER OTHER CONCRETE C	; ; ;
15 15- 76	# N 00 H	umum		T スペ マ	S .
20 Zo	\$ 0 N	ସ 22 ଅସ୍ଥାଧ	200 000 000 000 000 000 000 000 000 000	1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	日 1 2 大
-25 25 44	∺ N ຫ : ຫ : ພ :		00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ហ	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
80 80 80 11	# A	တ် ကို ထိ ဇီဝထီဇီ	@ 4 @ F O O O O O O O D O O O O D O O O O D O		N F O O
0 0 0 0 0	No or	4 m 			N 0
35-40 45	က်လက် အလက်	37 - 4 3 - 1 - 6 3 - 7	0 K 4 4 K 4 C C C C C C C C C C C C C C C	и И В Синтивониония В Синтивониония	004
4 	5707	7 % 0 % 0		00 N N O H O H H W O 0 0	4 [
0 U 0 4 0 I	B 0 4	N 9 A 0 7 0 0	ои по со		ກ ປ ວ ວ
জ ০ জ এ	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	43-48 3 67-70 122	000004000040000		7 0
345 55 55	0 % 0 10	46-4 70-7	WO WO O O O O O O O O O O O O O O O O O	NBNOHVHORBOOHOON (70
() () () () ()	000	τι ο το τ	й шоооооооооооооооо		9 0
7 ÷	on in	9-92 22 73+	$\begin{matrix} \omega \\ N \circ \circ \circ \circ \circ \omega \circ \circ \circ \circ \circ \circ \circ \circ \\ \end{matrix}$	· For	л <u>ө</u> э о

TABLE 28 TEXAS I-10 @ US 77 W. BOUND SCHULENBERG

		KIPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	30 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	2: 2: 3: 6: 6: 7:	42 04 0 0 0 0 0 1 0 0 0 1 1 0 8 1 1 1 1 1 1 1	103 07 00 71 00 21 10 13 61	295 12 11 10 33 48 00 01 150	1 4 5 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0	9 9 24 24 24 24	780000003404001 179	18 10 00 00 78 78 14 10 11 378	149 10 00 14 7 4 19 8 20 024 70	30000000214420011	14 00 00 00 14 30 00 00 6
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NO ALL SINGLE ALL COMBIN	2: 7: 80: 2: 2: 1:	7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	29 26 30 1 27 13 00 01 00 00 35 45	5 6 4 1 5 6 1 4 0 2 0 5 1 0 0 1 1 2 2 8 2 1 4 0 2 0 5 1 0 0 1 2 2 8 2 1 4 0 0 1 2 2 2 8 2 1 4 0 0 1 2 2 2 8 2 1 4 0 0 1 2 2 2 8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	024046292001400122 132	030052001000401033	0000130800011004074	0 0 0 0 5 0 0 0 7 0 0 0 1 5 0 0 0 2 0 9 1 5 9	0 0 0 0 4 0 0 0 0 0 0 1 0 0 0 1 4 8 0 0 1 4 8	00000050000100005	000000000000000000000000000000000000000
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	5		0 5-58	1 58-61	61-	L,	15 4-67	2 67-7	:6 70 7 0	45 -73	
ARRIVAL 1: NO. AM NO. PM	O	3 0	0	5	O	0	O	46	92	SO	106
LENGTH -1:		20 -2 5 19							50-55 394	55-60 78	3 10

TABLE 29 TEXAS SR 21 E.BOUND CALDWELL I_{r} SO GVW IN KIPS-0 TO 10 4.0 TOTAL ALL TRUCKS Q Ç SPECIAL Ö OTHER \circ Ö O LIVESTOCK ď, C Ö FUEL Ö Ö Ö CONCRETE \mathbb{Z} 4 ÷ TANK OPEN O ្ Ą, Ξ MACHINERY Ō ្ Ö STEEL \circ FLAT BED O Ö DUMP . 0 Ľ, O Ľ, Ö Ö BUS CAR CARRY Ö Ö BOX TRUCK UNCLASSIFY Ö 2 SINGLE 3 SINGLE O Ö ្ Ö 25-1 Ö 4 SINGLE Õ 35-1 4.7 Ō 28-2 " " SPLIT Ō \circ \circ Ö 14. \overline{z} 35-2 " "SPLIT O 1 ្ ္ O Õ ō Ō Ö " " SPLIT Ö 25-1-2 Ö O ರ 35-3 Ŏ i " " SPLIT Õ Ö Ö 35-1-2 -Ō Ö BAD OR NO Ö Ō ALL SINGLE ALL COMBIN Ö 0-31 31-34 34-37 37-40 40-43 43-46 SPEED-MPH 46-49 49-52 NO. TRUCKS \mathbf{S} 61-64 64-67 70-73 52-55 55-58 57-70 SPEED-MPH 58-G1 73+ NO.TRUCKS Ö 1-2 2-3 3-4 5-6 7-8 8-9 9-10 10-11 11-12 ARRIVAL 12-1 4-5 6-7 NO. AM O Ö NO. PM LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 E0÷

NO.

TABLE 30	TEXA	35	SR 114	E.80	מאטנ	DALL	95					
	GVW		IPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY		ALB140120463439055	43 2 0 0 0 0 0 0 1 1 2 7 1 0 3 5	67 0 10 0 0 1 0 3 1 0 4 13 7 0 18 10	64 02 013 511 0110 004 52	22000011020101033	400000000000000000000000000000000000000	9000023010130054	33100044001150043	50 0 0 0 1 1 1 1 2 0 0 1 1 1 0 0 0 0 0 0	14000007000230011	200000000000000000000000000000000000000
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN			42 1000000000000000000000000000000000000		2 5 9 0 2 7 0 3 1 0 0 1 0 0 0 2 7 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	0 N 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	010001020000000013	0 27 0 0 1 2 1 8 0 0 0 0 0 0 0 0 0 7 1 2 1	0 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	010000000000013	0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS			0	-34 0 -58 51	34-37 0 58-61 72	61-0	1	0	67-70	70	13 13 -73 17	49-52 22 73+ 2
ARRIVAL 1: NO. AM NO. PM	О		=	3-4 0 33	0	5-6 (0 15	0	7-8 0 0		10 1 35 0	0-11 50 0	11-12 44 0
LENGTH -10		-20 2 99	20-25 2: 22					45 45 3 5		-55 56	·55-60 S	

TABLE 31 TEX	AS I-4	40 @ FM	2381 E	.BGUND	BUS	HLANI)			
GVW	IN KIPS	3-0 10 10 20		30 40	40 50	50 60	60 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	0TAL 7524 22 0 1 1 1 6 0 0 0 0 1 1 1 6 1 0 0 0 0 0 0	47 89 0 2 8 13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 12 38 57	8400464408	75200002030314587	5 5 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	101 50 00 11 13 00 00 05 55	11800001000300104 64	12 8 10 20 10 10 10 10 10 10 10 10 10 10 10 10 10	51 10000082012001 28	7100000010000023
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN			11	11		0 1 0 0 1 2 0 7 1 0 0 4 0 0 2 3 1 7 9		000021088008103107 1088008103107		0000006000000007
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	0 52-55	31-34 0 55-38 176	0 59-61) 61-6	2 4 64	3 -67	15 67-70	5 70-	12 ·	48 73+
ARRIVAL 12-1 NO. AM C NO. PM 75	٥	0 0	0	G	O	S	52	60	96	102
LENGTH -15 15- NO. 83										

TABLE 32 TEXAS US 287 N. BOUND IOMA PARK

i E S E S E S E S	NO. AM	SPEED-XP NO. TRUCK NO. TRUCK			
ន ទី ទី		អា ដេ ហេ M	HCC 4 4 4 4 ZM		H B
20 02 02	m i	СТ N	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	AND AGHDAN ON	3 Z
ស ស ស	00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ថា ម លា ស	N ************************************	N WOWOOOOOOOOO HOO	0 0 0
्ड (१) (१) (१)	o i Nos	700 A	4 N O O O O N H O N O O O O N H O N I	© ↓ ©ΩΝΟΟΟΚΟΥΟΝΟ4ΟΚΚ ΟΘ	N == 0 0
0- 30- 35	i i	26 - 07 100 - 07	\$ P	ma +ao++aooavoo+aa	ម ១០
35-40 23	9 o s	5 W Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	14	MARIODEROLARIO OA	0 0 0 0
\$0-45 84-04	-7 7- 0 31 2	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	и ииооонооо р оиоои	ооиооичовч ава о в	4 P
2 IV 1 C3	фой ш ш ш	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00	CO 000040 # 00000 # 000	ល
50 50- 71 2	7 10 0 - 1	3-46 7-70 23	A 0 - 0 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-0 0000 + √ N 00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	60 70
2 2 2 2 3	ONO	#6-# 20-70-7	1 9 0 0 0 0 0 0 0 0 0 0 0 0 0 0	p.a.	70 80
-60 (72	4 C	00 00 de de de	[™] оомо⊬моош <mark>й</mark> оошоо,		000
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TABLE 33 CAL. SR 99 N.BOUND STOCKTON

	GVW IN KIF			20 30 30 40		50 60	60 70	70 80	80 80	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	35 7 1 5 107 125 3 0 219 17 28 3 487	43 1 6 1 6 0 4 3 0 0 4 4 0 13 3	11 4 0 0 33 6 4 15	39 147 37 8 1 2 0 1 1 19 0 0 1 1	9 0 0 0 0 0 5 1 0 2 2 1 2 4 5 4	74 12 0 0 0 2 2 7 1 0 12 1 0 0 37 0	108 12 00 01 34 00 20 00 58 8	182 133 0 1 0 19 121 0 28 1 0 101 5	331100620080000 1100800000	800000000000000011
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN	122 53 50 31 86 34 456 53 14 234 1 11 6 175 986	0 0 0 1 42	19 2 0 2 10 2 10 1 10 7 1 1 1 2 9 0 0	1 0 1 0 4 3 0 0 1 10	5 5 0 4 4 5 7 3 0 3 7 0 0 0 2 6	031028157016000031 71	0 1 0 0 8 2 5 2 20 3 5 0 0 1 1 1 6 10 10 10 10 10 10 10 10 10 10 10 10 10 1	0 0 1 0 8 1 1 0 4 0 0 8 0 0 3 0 0 2 1 8 2	000000015003000003 8	001000110003000006
LANE #1 LANE #2	1008 159	39 1: 4	26 25 32 3	3 128 6 19	72 25			178 4	63 0	6 0
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS		55-5	2 3 58-	4 61 61	-40 4 6 -64 6 148	17 64-67	34 67- 7 0	ļ.	-49 91 -73 2	49-52 125 73+ 0
ARRIVAL 12 NO. AM NO. PM	-1 1-2 2- 0 0 46 177 1-	0 (0 0	5-6 7 94	57	7-8 46 72	28	-10 10 71 0	67	11-12 57 0
LENGTH -15 NO. 56							-50 50 187)-55 5 290	55-60 241	

TABLE 34 CAL. I-5 N.BOUND MOKELUMNE R.

			KIPS TO	10	10 20	20 30	30 40	40 50	50 60			80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIF	6 1	7AL 1077 232 002 762 1197 238 102 525		13 0 1 0 0 0 0 0 0 0 0 0 7 2	882 100012 130043 143008 5	278 6 0 0 1 16 38 3 1 71 5 7 0 119 3	1092300014640203014 203014	102 100 00 00 367 192 00 80 3	95 0 0 0 4 0 1 4 19 0 1 55 1	9 0 0 0 9 7 1 1 41	2:000 23:1000 3:10000 60	27 20 00 00 5 90 07 00 04 0	9010000020200040
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN		77 24 35 08 33 53 53 55 95 22 10 10 10 19 10 19		12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 2 0 0 0 0	53 4 G O 2 4 E 1 3 O O 8 O O 1 O 7 1	10 12 21 0 2 14 120 34 0 56 0 12 254	0 5 5 0 2 7 1 5 5 5 0 1 2 3 0 0 0 3 6 0 1 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0	02002 11133600500022 9	0 0 3 0 0 2 0 6 8 3 0 1 1 1 1 0 0 7 0 8 8	0 0 0 0 143 14 0 32 0 0 2	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 5 2 0 0 18 1 0 0 0 2 7	2 0 0 0 0 0 0 1 2 3 0 0 0 0 0 2 7
LANE #1 LANE #2					76 12	262 18			89 6		155 10		6 3
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS			-31 0 -55 70	55	-	58-63) i 61	0	2 64-67	67-	Ą	6 0-73	
ARRIVAL 12 NO. AM NO. PM	0	O		0		0	0	Q	7-8 0 51	0	19	10-11 76 0	11-12 98 0
LENGTH -15		-20 52				30-35 1:				5-50 229	50-55 345	55-60 252) 60+ ? 8

TABLE 35 CAL. SR 17 S.BOUND FRUITVALE AVE.

Œ	VW IN KIP	5-0 10 10 20		30 40	40 50	50 60	60 70	70 8 0	8 0 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	TOTAL 1719 34 52 1 27 108 111 20 182 23 26 825 266	4 30 0 0 0 0 0 0 2 0 0 0 7 19 0 1	17777110000000000000000000000000000000	267 5400387815416144 144	127 430 0137 42 100 769	102 23 00 55 22 14 20 67 13	113100115187000B0	18200074308100150 50	20000040002400081	7 0 0 0 0 0 0 0 0 0 0 0 2 1
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN	249 93 200 38 114 11 712 82 37 156 1 12 96 3321		19 19 10 10 10 10 10 10 10 10 10 10 10 10 10	95405314618603214 3 3 2 2 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 2 4 2 4 2 4 2 2 4 2 4 2 2 4 2 4 2 2 4 2 4 2 2 4 2	0 10 4 0 3 2 1 2 1 0 7 2 1 2 1 0 0 1 7 1 1 7	020015108015031020	0 5 0 0 5 5 0 0 0 0 1 0 0 0 1 0 0 1 0 0 1 0 0 0 0	000079084076001000 93076001000	000002137004010108	0 0 0 0 0 1 3 00 0 3 0 0 0 0 7
LANE #1 LANE #2	699 1020	25 150 20 12:			52 75		33 81		18	<i>4</i> , 3
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	0-31 18 52-55 437	17 55-58	26 58-6	7 37- 3 . 61-	39	62	8: 677:	3	139	49-52 312 73+ 0
ARRIVAL 12- NO. AM NO. PM 20	0 0	-3 3-4 0 0 58 136	٥	5-6 0 131	0	7-8 19 94	8-S S 141 19			11-12 219 0
LENGTH -15 NO. 98		25 25-30 32 110			10 40- 30 1					

TABLE 36 CAL. US101 VENTURA-OXNARD -JUNE 19,20 GVW IN KIPS-O TO 10 TOTAL ALL TRUCKS SPECIAL Ö Ö \circ GTHER \circ O O LIVESTOCK O Ö FUEL O O Ö Ö O CONCRETE TANK \circ OPEN Z. · · Ó O O MACHINERY O O Ō STEEL FLAT BED İ Ą. DUMP Ö Ō Ö Ō BUS O CAR CARRY O BOX TRUCK Ö UNCLASSIFY Z SINGLE O Ō Ö O 3 SINGLE Ö O 25-1 4 SINGLE Ö O Ö 35-1 \mathbf{O} Õ Ö O Ö " " SPLIT Ō O 35-2 " " SPLIT 4.2 Ö Ö O Ö Ō O 28-3 Õ Ö Ω O " " SPLIT Ö Ö Ō Ō 28-1-2 E 35-3 Ö Ö O Ö Ö Ö Ö " " SPLIT Ö Ö 35-1-2 Ö O Õ BAD OR NO \circ ALL SINGLE 14. ្ Ö ALL COMBIN O 31-34 34-37 37-40 40-43 43-46 46-49 49-52 SPEED-MPH 0-31 NO. TRUCKS 52-55 61-64 67-70 SPEED-MPH 55-58 58-51 64-67 70-73 73+ NO. TRUCKS 1-2 2-3 3-4 4-5 5-5 5-7 7-8 8-9 9-10 10-11 ARRIVAL 12-1 NO. AM Ö S S NO. PM 3: \circ

NO.

LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60

60+

TABLE 37 I-10 W.BOUND CAL. COLTON GVW IN KIPS-0 TO 10 TOTAL ALL TRUCKS \mathbb{S} SPECIAL \mathbb{Z} OTHER O Ö O Õ O O LIVESTOCK () Z. Ō FUEL Ö Ó Ö CONCRETE Ö TANK B, Z OPEN MACHINERY O O Û STEEL Ö O Ó Ó \circ () Ö FLAT BED Ź. Ö DUMP O Ō **(**) Ō ٠<u>;</u> Ö Ö BUS O CAR CARRY O O O Ō Ö BOX TRUCK UNCLASSIFY 0.4 G 2 SINGLE Ö 3 SINGLE Ö Ö Ö 28-1 Ö Õ O Ō Ö Ö O Ō 4 SINGLE 38-1 C, Ö Ō Ö Ö 25-2 4.4. Ö " " SPLIT Ö Ö O Ō Ö 35-2 " " SPLIT Ö 4. Q. 25-3 ŏ ŏ Ò Ō Ó () " " SPLIT Ö Ö O Ü Ō Ō 25-1-2 Ö Ō 1 4 35-3 Ö Ö Ö Ö O " " SPLIT Ō Ō 35-1-2 'n \circ Ö O Ö BAD OR NO Ö ALL SINGLE Ö Ö Ö Ō ALL COMBIN O LANE #1 LANE #2 Ą. Ō

31-34 34-37 37-40 43-46 48-49 SPEED-MPH 0 - 3140-43 49-52 NO.TRUCKS SPEED-MPH 52-55 55-58 58-61 61-54 64-67 67-70 70-73 73+ NO. TRUCKS Õ

3-4 4-5 5-6 6-7 7-8 8-8 9-10 10-11 ARRIVAL 12-1 1-2 2-3 11-12 NO. AM NO. PM O

LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 60+ NO. 92 93 36 39 26 27 35 122 103 187 16

TABLE 38 ILL. SR 89 SPRING VALLEY

	GVW IN	TO 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX UNCLASSIFY	244 0 0 0 5 2 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	191000000000000000000000000000000000000	5800001000010081 2	49400000100050054 24	090000000000000	45 23 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3500000000010005	970000000000005	86700000000001 28	128 88 00 00 0 1 1 0 0 0 0 0 0 0 0 8	33 27 0 0 0 0 1 0 0 0 0 0 0 0
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL COMBIN			300001000000000000000000000000000000000	3530005051000000081	461 01000 144000000000000000000000000000	0 8 0 0 0 N 0 4 10 0 0 N 0 0 0 8 9	05000000000000000000000000000000000000	0 N O O O O O O O O O O O O N D	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 122 4 0 0 0 0 0 0	0000003
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	52	-31 31 51 -55 55		119	61-	12	75	17	70-	8	3
ARRIVAL 12 NO. AM NO. PM		0	0	4-5 0 43	5-8 0 18	6-7 0 0	7-8 23 0	8-9 9-1 25 2	10 10 29 0)-11 55 0	11-12 61 0
LENGTH -15 NO. 49				30-35 27				-50 50- 144	-55 5 15	55-60 1	

TABLE 39 F-1 <u>က</u> Ü ILLINDIS なてく言え 爾 DEM URBU

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TABLE 40 ILL. I-80 W.BOUND VERMILLON RIVER BVW IN KIPS-0 TO 10 TOTAL ALL TRUCKS SPECIAL í DTHER Ö Ŏ Ö ់ O LIVESTOCK FUEL O Ö Ö \circ CONCRETE G Ö Ö S TANK OPEN Ö : MACHINERY Ö O Õ STEEL Ö FLAT BED Ö DUMP O \circ Ö Õ Ö 7.4 Ö Ö <u>ب</u> CAR CARRY Ō ž Ţ Š BOX TRUCK UNCLASSIFY O 2 SINGLE O O 3 SINGLE 28-1 Ö 4 SINGLE O O Ö Ö 35-1 Ö 25-2 " " SPLIT Ö O 39-2 " " SPLIT 7 ្ ្ Î Z, 28-3 Ō Õ Ó ō Ö " " SPLIT i O Ō ្ 28-1-2 35-3 Ö " " SPLIT Ö Ō Ō Õ Ö O Ō Ö 35-1-2 BAD OR NO Ö Ą. ALL SINGLE ALL COMBIN LANE #1 • LANE #2 Ö 31-34 37-40 43-45 SPEED-MPH 0-31 34-37 40-43 48-49 49-52 NO. TRUCKS SPEED-MPH 52-55 55-56 58-81 61-64 S4-G7 **67-70** 70-73 73+ NO.TRUCKS **G7** ARRIVAL 12-1 1-2 2-3 3-4 4-5 5-8 5-7 7-8 8-9 9-10 10-11 11-12 NO. AM Ō O Ö NO. PM E4 * Ò

NO.

LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60

S0+

NO. 9 LENGIH -15 I	ARRIVAL 12-1 NO. AM 5 NO. PM 15	SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	LANE #1	28	ALL TRUCKS SPECIAL OTHER CONCRETE CONCRETE TANK FUEL FUEL FUEL FUEL FUEL FUEL FUEL FUEL	00 VX
5-20 20 23	1444	55 - 42 25 - 43 25 - 43 25 - 43	₽ H ⊩ O	7 7 8 8 8 8 7 7 8 8 8 8 8 8 8 8 8 8 8 8	D	! ~
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30-35 22	ហ្លុំ	34-37 0 58-81 147	00 63 44	00 + 00 0 N 0 + N 0 0 0 0 0 0 0 0 0 0 0	80 000 00 00 00 00 00 00 00 00 00 00 00	ω N ○ O
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55-60 43	0 	-70 22 22	0 14	m0000000000000000000000000000000000000		0 O
6) O O +	60 to 10 20 N N	48-52 18 73+	013	N0000000000000000000000000000000000000		0 0

TABLE 42 GEORGIA I-75 @ PLEASANT VALLEY

		KIPS-0 TO 10	10 20	20 30	30 40	40 50	50 80	50 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	2143122173051153	105 00 00 10 00 41 20 25	158 0 1 1 0 0 1 0 2 0 3 2 0 1 20 1 27	357 1 0 0 0 1 9 1 4 0 12 2 4 2 80 241	26400100128180319 128180319 482	1400011000221310602 302	139 0 0 0 0 1 1 1 2 0 33 87	157 00 00 00 20 24 22 14 100	157 0 0 1 2 0 6 3 0 1 4 0 0 1 32 107	27 0 0 0 0 0 3 1 1 0 0 0 0 0 3 1 8	5000001000004
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN		103 10000000000000000000000000000000000	522008910100000981 3	10 15 15 21 21 21 21 21 21 21 21 21 21 21 21 21	0 11 22 17 19 40 00 24 10 13 241	0 5 1 0 2 7 0 7 2 1 0 0 0 3 0 1 1 5 3 1 3 3	0 3 0 0 7 0 120 0 0 0 0 3 0 6 3	0 2 0 2 0 0 0 0 1 4 0 0 0 1 4 0 2 4 1 5 1	0 0 0 1 0 1 51 1 0 0 1 2 0 1 56	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000100020000200014
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	52	-31 31 1 -55 55 81	4 5-58	6 58-61	61-	16 64 6	4 4-67	67-7	1 0 70	10 -73	49-52 20 73+ 54
ARRIVAL 12 NO. AM NO. PM 1	0 0	Q	0	0	0	0	0	0	64	110	181
LENGTH -15 15-20 20-25 25-30 30-35 35-40 40-45 45-50 50-55 55-60 60+NO. 92 103 35 48 56 62 259 784 95 1 2											

TABLE 43 GEORGIA, SR 365 at EXIT 3

		KIPS TO	1-0 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	8 0	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	738701570241058	99 1 121 001 200 050 052 42	156 100003800380040 560	200440005900350091 951	41 02 00 00 00 14 10 23 16	57200043500320341 21	100 13 0 1 0 14 8 4 0 1 7 0 0 0 3 3 3	48 10 50 33 40 04 10 01 15	4 0 0 0 0 0 2 1 0 0 0 1 0 0 0 0	00000000000000000	200000000000000
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN	1 37	1062514400040019	9000000000000090	78 18 10 0 11 40 00 00 00 00 94 82	8 19 8 0 2 2 14 2 0 0 0 0 0 1 0 0 2 5 1 7 7 7 7	1 11 10 7 20 00 00 00 10 12 12	030103081000000043	0 16 0 3 0 2 0 78 0 0 0 0 0 0 0 1 9 8 0 8 0	0 4 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000000040000000004	0000000000000000000	00000000000000000
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	5	1	55-5	0 8	1 58-61	61-6	1	0 4-87	Ą.	70-	7 73	21 73+
ARRIVAL 12 NO. AM NO. PM 1	٥	o	0	0	0	0	0	0		7	82	11-12 82 0
LENGTH -15 NO. 55				-30 3	30-35 5		0 40-4 7 1(-50 50- 232	-55 5: 67	5-60 1	

TABLE 44 GEORGIA, I-75 and SR 83

	GUW IN	KIPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	60 7 0	70 80	80 90	90 1 5 0
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK	52 45 45 45 45 45 45 95	138 130 14 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0	157 15 25 0 0 1 1 3 0 0 11 15 0 0	425 43 1 0 1 24 3 0 72 11 0 1283	19311001832143263 143263	146 1 0 0 0 0 0 0 3 1 0 8 1 0 2 1 1 2 1	218 0 1 0 0 0 4 5 1 0 30 1 0 2 167	185 1 0 0 0 0 8 0 0 0 19 0 0 0	17 0 1 0 0 0 0 0 1 0 0 0 1 0 0 0 0 1	100000000000000000000000000000000000000	3 0 0 0 0 0 0 1 1 0 0 0 0 0 0
UNCLASSIFY	' 9 <u>5</u>	16 (24	12	8	8	5	8	į	1	o
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-1-2 BAD OR NO ALL SINGLE			90 11 80 13 18 02 00 00 04 11 42	11 15 10 82 27 11 00 10 00 29 39	0 21 20 8 47 0 110 0 0 0 0 2 21 170	0 10 10 10 10 10 10 10 10 10 10 10 10 10	0 1 0 1 4 1 0 3 5 0 0 0 1 1 0 1 2 1 2 1 2 1 2 1 2 1 2 1 2	0 0 0 1 1 0 1 1 0 0 0 0 0 1 0 0 0 0 1 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0000001000010102
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	52	2	1 5-58	0 58-61	61-	3 ·64 6	8 4-67	18 67-70	70-	29 -73	79 73÷
ARRIVAL 12 NO. AM NO. PM 1	0 0	0	0	٥	O	0	0		71	107	96
LENGTH -15 NO. 116			25-30 21					708 :			

TABLE 45 N.Y	. THRU	AAY @ 2	03 3	ERKSHI	RE SPL	JR			
		-0 10 lo 20	20 30	30 40	40 50	50 60	S0 70	70 80 80 90	
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	21 12 15 0 64 20 0 65 17 35 13	7 111 10 0 4 0 0 8 3 0 0 0 0 0 2 0 8 4 15	257728005900840658 2240658 121	114 2000 1100 100 100 100 100 100 100 100 1	5 300003 4005 4 4 2 2 4	5 1100011004:0105 5 5	4100000007521	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	S 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NC ALL SINGLE ALL COMBIN	29 11 0 8 74 3 475 13 4 13 5 1 91 92 21 95	27 34 0 5 0 7 0 0 1 0 20 0 20 0 39 0 0 0 0 0 0 0 0 0 0 0 7 2 0 0 0	7 9 24 20 20 70 0 0 0 0 23 15 238	20 30 1 8 1 2 0 1 2 2 1 0 0 5 2 1 1 1 0 0 5 2 1 1	0200170013102125	100 40 12 20	10 + 2 0 0 + 4 3		
LANE #1 LANE #2		7 102	213 44	96 22			54 12	16 S	
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	O	31-34 0 55-58 88	34-37 0 58-61 112	61-64	4 64-	O	3-45 3 7-70 12:	46-49 3 70-73 25	49-52 11 73+ 27
ARRIVAL 12-1 NG. AM 0 NO. PM 83	1-2 2-3 0 0 78 62	0	4-5 ; 0 82	0		0 0	0 22		
LENGTH -15 15- NO. 19	-20 20-25 43 15		30-35 5		40-45 117				0 60+ 4 31

NO. THE SE	ARRIVAL 12- NO. AM 11 NO. PM 7	SPEED-MPH SPEED-MPH NO.TRUCKS	LANE #1	BUN H H H H M M M M M M M M M M M M M M M	SPECIAL SPECIAL GTHER GTHER CONCRETE TANK CORCHINERY STEEL .	
118 20-	- 0 C C C C C C C C C	57 0 - 3 1 1 1 2 1 3 1 1 1 1 1 1 1 1 1 1 1 1 1	# 6 # 4 0 #	2 4 6 7 8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		> 2
71 25	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	or or	မပ		₩ ₩0004000600μ600μ0	် ဂ ဂ ဂ
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0 1 1 0 4	0 4 0 4	6 G 7	ស	M M M M M M M M M M M M M M M M M M M		ω 0 0
10 40	6-7 33	4 B	* 00	ω μ « ω ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο	7 700ччочюмчю4ч	4 B
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5-50 51 254	104 104 1	43-46 58 67-70	ភ ជំ	<u>\$</u> Ñ ຫ ⊬໐໐໐໐໐⊹໐໐໐໐໐	р 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 ®
0-55 64	0001	46 70	57 3	m ○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○○	о оноошнишиноорн	80 80
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© 00 + +	11-12 45	49-52 84 73+ 0	24. 44 Q.			й 0 0

TABLE 47 N.Y. I-87 @ SR 146 S.BOUND

G.	N IN KIP OT		10 20 20 30		40 50	50 60	60 70	70 80	90 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	TOTAL 577 21 24 310 55 12 201 19 101 120	44 21 2 0 0 1 1 0 0 5 1 0 0 5 1 0 0 1 0 0 1 5 1 0 0 1 0	50 30 30 30 30 30 30 30 30 30 30 30 30 30	84 31 10 00 52 10 70 27 32	51 21 50 01 21 07 00 18 12	17 00 00 0 11 00 00 0 11 2	0 3000100N11850070	37 100002001500089	9 300000100050005N	200000000000000000000000000000000000000
2 SINGLE 3 SINGLE 25-1 4 SINGLE 35-1 25-2 " " SPLIT 35-2 " " SPLIT 25-3 " " SPLIT 25-1-2 35-3 " " SPLIT 35-1-2 BAD OR NO ALL SINGLE ALL COMBIN	121 25 10 11 8 8 28 17 02 10 8 14 14	44 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 9 13 4 5 13 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	16300936440040037	0 1 0 0 0 3 1 8 NO N 0 1 0 0 5 1	000000000000000000000000000000000000000	00002114301001000	000000000000000000000000000000000000000	000001011000000000	00000011000000000
LANE #1 LANE #2	408 169		70 102 25 89				57 8	27 10		2
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	0 52-55	55-58	; 58- 8	0 1 61	1 -64 6	1 84-67	7	70-7	5 '3	31 73+
ARRIVAL 12-1 NO. AM O NO. PM 62	0	0 0	O	28	56	68		54	88	11-12 31 0
LENGTH -15 1 NO. 25			0 30-3 3 i						-60 2	

TABLE 48 N.Y. I-80 W.BOUND & CHAUTAUGUA CREEK

		KIPS-0 TO 10	10 20	20 30	30 40	40 50	50 60	60 70	70 80	80 90	90 150
ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIEN	5 6 4 11 21 28 48	6 16 0 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	47 22 1000 300 81 107 2	208 11 1 2 0 28 11 0 0 35 3 4 0 4 0 5	134 14 31 00 80 24 80 7 2	73 8 0 0 1 0 1 0 0 1 3 0 0 7 5 0	52301000020500082 42	110 40 00 24 20 12 30 28 2	181 4 0 0 10 18 14 0 3 29 6 1 10 13	23 1 0 0 1 4 1 0 0 8 0 0 7 1	12 00 00 00 4 10 00 13 00 30
2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 29-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 BAD OR NO ALL SINGLE ALL COMBIN	10 57 5 1	4703166:2634216B	8 4 4 0 0 N 11 11 00 0 0 0 0 0 0 0 0 0 0 0 0	1	4 14 2 2 3 0 10 10 10 18 13 135	0 0 1 0 1 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0000040510000101010101010101010101010101	0300001 881102700003107	0000129 150124101001 8	0 0 0 0 0 2 1 2 0 1 0 0 0 0 2 3	00000082000000200
LANE #1 LANE #2	73 1 <i>4</i>)	B 14 B 2	45 2	138 42	125 29	63 10	53 9	96 14	152 29	16 7	2 4
SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	5	০ 2-55 চ	0 5-58	0 58-61	61-	1 -84 6	3 34-37	6 7 -7	4 70 70	9 73	12 73+
ARRIVAL 12 NO. AM NO. PM 1	0	0 0	. 0	O	6	39	80	85	128	126	103
LENGTH -15											

TABLE 49 CHIC ISO OVER PAINE

NO. 22	ARRIVAL 12- NO. AM NO. PM 6	SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	A C C C C C C C C C C C C C C C C C C C	SPECIAL TRUCKS SPECIAL OTHER CONCRETE TANK C
15-20 20-25 34 13	1 1-2 2-3 0 0 0 5 101 104	8 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	В 44 В 60 В В В В В В В В В В В В В В В В В	GVW IN KIRS- TOTAL TO 1 738 8 8 8 100 8 23 7 404 1
25-30 3	μ μ 4 Ο Ω 4 Ο Ω	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ЙООООООООООООЙ М Ш№400№ш400900	00 004000440000400400400400400400400400040000
30-35 20-35 20-35	4-5 5- 0 5- 97 6	34-37 58-61 130	# N N N N N N N N N N N N N N N N N N N	ми и мионовичои мионови
5-40 40 44	6 6 7 6 7 8 8 8 9	87-0 81-0 80-0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
104 001 104 001	7-e 0	40-43 64-87 57	α п N ω O O O O H N H P O P O	00 P P P P P P P P P P P P P P P P P P
99 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000	63-46 67-70	\$ 000000000000000000000000000000000000	# 4
0 U U U U	0 0 1 0 1 0 1 0 1	48-48 40 70-73 2	м -	700 80000000000000000000000000000000000
4 0 0 0 0 0 4	0 U N	40-52 70-52 +	₩ ೧೦ ೦ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

当記 m 50 CHIC IBO DVER RT 43 DISKS 1,2 TEB 11,1982

NO. 15 15	ARRIVAL 12-1 NO. AM 0 NO. PM 130	SPEED-MPH SPEED-MPH NO. TRUCKS	LANE #1	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	SPECIAL TRUCKS SPECIAL CTHER TOCK CONCRETE TANK CARCHINERY STEEL BUS FLAT BED DUNP SOX TRUCK CNOCLASSIFY	4 × ×
-20 27 20 20	e e con	0 0 0 0 0 0 0 0 0	ចា ហ	о о о о о о о о о о о о о о о о о о о		기 당
) 0 0 0 0 0	1		O P	0 N 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	М чесосомосмососьно	7 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
ω ω Ο ω	H 0 5		ស ខា ធ	N M 7 0 0 0 0 0 0 0 0 0 + 0 10 10 11 11	0000HH000HH0000NNN	N :: 0 0
3 0 1 24 43 44 51	E S S S S S S S S S S S S S S S S S S S	34-37 0 58-61 175	8 0 &	<mark>7</mark> ш 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	84040004040000000000000000000000000000	3 0 0
35-40 20	0000 0000	37-4 61-6 10	н () н () н	# # W W W W W W W W W W W W W W W W W W	177 77 177 177 177 177 177 177 177 177 1	\$ G
4 0 - 4 0	007	400 400 400 400	71 7	7 + 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	**************************************	0 0 0 0
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-50 51 402	000 0-0	43-4 67-7	72 4	noooon + oou m o + u oooo	ж ти ти ти ти ти ти ти ти ти ти ти ти ти	80 70
143	000	0 4 0 70	137	# 000000000000000000000000000000000000	14 7400000000000000000000000000000000000	800
55-G0 7	0-11-	149 -73 -73	10 0 4	00000000000000000000000000000000000000	N ≪√000000004400004	0 0 0 0
0 0 +	4 H H H H H W O	49-52 47 73+ 20	ч 0 4	000000044000000000	и и с о о о о с с ч ч и и и о о и о	150

Table 51: Metric Conversions for Tables 5 through 50

1 foot = .305 meters

1 mph = 1.6 kph

Table 52: Comparison of Study Data with 1975 National Truck Characteristic Report

Frequency per 1000 Vehicles

Weight Group	FHWA* Interstate Sites	BWS All Sites	BWS Interstate Sites
kips			***********
30 or more	607	622	656
40 or more	466	482	509
50 or more	374	386	404
60 or more	285	290	296
70 or more	150	160	143
80 or more	17	58	41

^{*1975} National Truck Characteristics Report, US DOT, April 1978 $\,$

¹ kip - 454 kilograms

Table 53: Damage Spectrum Factor (Lo)

Category	Damage Factor
All Sites Georgia Ohio New York Illinois Texas Arkansas California	.41 .26 .50 .35 .51 .51 .39
Weekdays	.41
Weekends	.39
Interstate	.40
US Routes	.48
State Routes	.40

Table 54: I-90 Static Scale Results - January 22, 1982

GVW IN KIPS	FROM TO	0 15	15 20	20 25	25 30	30 35	35 40	40 45	45 50	50 55	55 60	60 65	65 70	70 75	75 80	80 85	85 90
	TOTAL																
ALL TRUCKS	293	10	13	11	39	60	17	19	17	9	8	17	22	24	25	2	0
2 SINGLE	29	10	13	6	0	0	0	0	0	0	0	0	0	0	0	0	0
3 SINGLE	8	0	0	3	1	2	2	0	0	0	0	0	. 0	0	0	0	0
2S - 1	6	0	0	1	2	1	0	1	1	0	0	0	0	0	0	0	0
2S - 2	20	0	0	1	5	3	3	5	2	0	0	0	0	0	0	0	0
3S - 2	206	0	. 0	0	28	50	12	13	13	7	7	16	18	24	18	0	0
3S-2 SPLIT	16	0	0	0	3	3	0	1	0	0	0	0	1	0	6	2	0
3S - 3	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
2S-1 - 2	7	0	0	0	0	0	0	0	0	2	1	0	3	0	1	0	0

NOT CLASSIFIED BY WEIGHT

BY HAULING	CATEGORY
BOX	194
FLAT	48
TANK	33
DUMP	9
OTHER	9

BY ARRIVAL TIME

11:00 - 12:00 47 12:00 - 1:00 42 1:00 - 2:00 52 2:00 - 3:00 51 3:00 - 4:00 50 4:00 - 5:00 31 5:00 - 5:45 20

1 kip = 454 kilograms

	TABLE 55 OH	0 190	OVER PA	INE ALL	JAN	22,198	32												
				15 20 20 25	25 30	30 35	35 40	40 45	45 50	50 55	5 5 6 0	60 65	65 70	70 75	75 80	80 85	85 90	90 150	
	ALL TRUCKS SPECIAL OTHER LIVESTOCK FUEL CONCRETE TANK OPEN MACHINERY STEEL FLAT BED DUMP BUS CAR CARRY BOX TRUCK UNCLASSIFY	TOTAL 218 5 2 0 0 1 22 15 1 4 29 11 3 2 123 0	11 12 00 00 00 00 00 00 00 00 00 00 00 00 00	2 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 1 0	15 00 00 00 00 00 02 20 00 80	32 10 00 01 14 44 00 00 00 16 00	14000001200500060	21 0 0 0 0 0 0 0 0 0 1 1 1 1 7 0	13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 0 0 0 0 0 0 1 0 0 0 0 1 0 0 0 4 0	7000 0000 0001 1005 0	13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	29 0 0 0 0 0 1 3 0 1 1 1 0 0 2 2 0	20200003202200090	130000005100300120	200000000000000000000000000000000000000	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
	2 SINGLE 3 SINGLE 2S-1 4 SINGLE 3S-1 2S-2 " " SPLIT 3S-2 " " SPLIT 2S-3 " " SPLIT 2S-1-2 3S-3 " " SPLIT 3S-1-2 3S-1-2 ALL SINGLE ALL COMBIN	12 3 4 2 8 33 2 123 22 0 0 0 2 17 199	11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 1 1 1 0 0 0 2 0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0000340620000000000000000000000000000000	012035063600100011	00000000000000000000000000000000000000	1 0 0 0 0 0 0 1 1 0 0 0 0 0 0 0 0 0 0 0	000000000000000000000000000000000000000	0 0 0 1 0 0 0 1 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 0 0 1 0	0000010310001000	0010000051000000007	0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 1 1 3 1 2 2 0 0 0 0 0 0 2 8	0 0 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0	00000000000000000000000000000000000000	00000000000000000000000000000000000000	000000000000000000000000000000000000000	
;	SPEED-MPH NO.TRUCKS SPEED-MPH NO.TRUCKS	0-31 (5 2-55 45) 5 55-5	0 1 B 58-61	61-	0	0-43 7 9-67 4	43-46 8 67-70 0	70-	22	9-52 50 73+ 0								
	ARRIVAL 12-1 NG. Ar O NO. PM 42	1-2 2 0 32	2-3 3- 0 21 3	0 0	5-6 0 0	6-7 7 0 0	7-6 6 0 0	3-9 9 - 0 0	10 10 0 0	-11 1 0 0	1-12 58 0								
	LENGTH -15 15 NO. 2	5-20 20- 9	-25 25- 6	30 30-35 1 2	35-4 1	0 40-4 6 2		-50 50 122	-55 5 27	5-60 5	60+ 3								

Table 56: Calibration Vehicle Crossings

Table 56A - Calibration Vehicle Weighings - Gross Weights

Date	Location	Static Wt. kips		WI	M Weigh	nt (kips	s)	
2/10/81 2/17/81 2/20/81	I-30 & 67 - ARK I-40 & 161 - ARK I-40 & 25 - ARK	58.50 58.50 58.50	58.4 58.5 58.6	57.9 58.3 58.6	58.8 58.2 58.7	58.7 59.1 58.9	58.7	
2/23/81 3/02/81	US 67 - ARK US 65 & 256 -ARK	58.50 58.50	58.8 59.0	60.7 58.2	59.7 56.2	57.2 58.1	56.9	
3/05/81 3/19/81	I-30 & 29 - ARK I-75 - GA	58.50 36.18	58.1 33.4	58.7 34.7	58.0 35.5	59.0 34.1	61.4	56.6
3/19/81	US 59 - TEX	46.10	48.1 47.0	48.0 43.8	47.2 45.3	45.4 44.6	46.5	46.7
3/24/81	SR 36 - TEX	45.68	46.5 46.1	44.7 45.5	47.0 46.7	45.0 45.2	44.7 46.1	44.7
3/28/81 4/13/81	I-45 - TEX I-10 & 77 - TEX	45.82 46.03	45.8 47.9	46.1 44.7	44.1 45.1	45.7 46.2	45.9 46.6	
	Table 56B - Ca	alibration Veh	icle We	eighings	- Rear	• Tanden	n	
2/17/81 2/20/81	I-40 & 161 - ARK I-40 & 25 - ARK	30.60 30.60	29.4 30.2	31.4 30.0	26.4 30.0	31.6 29.8		
2/23/81 3/02/81	US 67 - ARK US 65 & 256 - ARK	30.60 30.60	31.8 33.0	31.6 31.8	31.2 31.6	30.4 31.8	30.2	
3/05/81 3/19/81	I-30 & 29 - ARK US 59 - TEX	30.60 14.60	29.2 14.0 13.4	29.6 13.8 9.8	29.2 13.8 14.0	30.2 13.8 13.4	31.4 12.8	28.6 14.2
3/24/81	SR 36 - TEX	16.06	17.4 17.1	16.8 16.8	17.4 17.1	16.8 16.8	17.1 16.8	16.5
3/28/81 4/13/81	I-45 - TEX I-10 & 77 - TEX	16.06 16.60	15.6 16.0	16.4 14.4	15.4 14.2	16.8 15.8	16.2 15.8	

	No. of	Ratio: Coeff. of	Difference: Standard	Me	an:
Data		Variation	Deviation	Ratio	Differ.
Gross Wt	74	2.3	1.12	.9981	0431
Rear Tandem	Wt 54	7.2	1.34	.9854	2437

Table 57 - Summary of Calibration Vehicle Crossinss

Table 58: Weigh-In-Motion Repeatability Test June 4, 1980 I-90 & Paine Road - Span 1

Crossing	Front Axle kips	Rear Tandem kips	Gross Weight kips
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23	14.10 14.10 14.20 10.70 14.20 14.00 12.60 14.40 12.90 13.10 12.70 14.00 11.90 14.40 12.50 14.30 14.00 14.10 15.10 11.70 14.30 12.30 14.40	33.20 36.40 33.20 33.90 32.70 33.10 32.60 32.80 33.70 34.10 33.90 32.60 35.00 33.40 33.00 32.80 33.20 32.70 34.50 33.00	47.30 50.50 47.40 44.60 46.90 47.10 45.10 47.10 46.60 47.10 46.50 46.50 47.80 47.20 47.80 47.20 47.80 47.40 46.50 47.10
Mean (kips)	13.47	33.50	46.96
Standard Deviation (k	cips) 1.09	.89	1.10
Coefficient of Variaton (%)		2.67	2.35
1 kip = 454 ki	ilograms		

Table 59: Weigh Station Repeatability Test June 4, 1980 I-90, Ohio

Crossing	Front Axle kips	Rear Tandem kips	Gross Weight kips
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15	14.95 14.35 15.20 14.65 15.75 13.60 15.05 15.55 14.65 15.15 15.55 14.90 15.20 13.75 13.35	32.05 31.85 33.65 32.35 33.25 28.60 33.20 32.60 31.75 33.60 29.20 33.25 32.45 32.65 32.30	47.00 46.20 48.85 47.00 49.00 42.20 48.25 48.15 46.40 48.75 44.75 48.15 47.65 46.40 45.65
Mean (kips)	14.70	32.18	46.06
Standard	14.70	32.10	46.96
Deviation (ki	ips) .70	1.40	1.74
Coefficent of Variation (%)	4.73	4.35	3.71
1 KiloNewton =	4.45 kips		

Table 60: Weigh Station Repeatability Test February 11, 1982

	Mean kips	Standard Deviation kips
Steering Axle	9.070	.039
Drive Tandem	28.850	.097
Rear Tandem	42.470	.082
Gross Weight	80.380	.115

Table 61: Repeatability of Portable Scales

Weighing	Gross Weight kips	Rear Tandem Weights kips
1	59.5	30.7
2	60.7	31.4
3	59.1	29.0
4	60.0	31.5
5	60.4	31.3

Table 62: Bridge WIM and Pavement WIM Repeatability May 7 ,1981

BWS Weights	Radian Weights*
kips	kips
68.5 65.0 68.1 67.3 68.1 69.2 68.5 66.8 67.8 65.9 67.5 67.8	65.152 71.568 65.592 63.784 65.384 68.400 65.816 67.920 65.976 67.912 72.522**
N = 13	N = 10
x = 67.56	x = 66.75
o = 1.08 kips	\sigma = 2.11 kips
c.o.v. = 1.60%	c.o.v. = 3.17%

*High speed data

**Axle bounced - not used in calculations.

Actual Calibration Truck Weight = 67.34 kips

Table 63: Bridge Structures used for Data Acquisition

State	Bridge Location	Traffic Direction	Span Type	Instr. Span Length ft	Girder Type	Comments
Arkansas	I-30 & US 67 I-40 & SR 161 US 67 & Mp 10.9 I-40 & SR 25	West West South West	\$ \$ \$ \$ \$	84 59 42 55	\$ \$ \$ \$ \$	50 degree skew 45 degree skew
	US 65 & SR 256 I-30 & CR 29	South West	S S	48 39	S S	30 degree skew 45 degree skew
Georgia	I-75 & Pleasant Valley	North	С	42	S	20 degree skew
	SR 365 (Exit 3) I-75 & 83	North North	S S	31 35	S S	
Texas	US 59 & FM 444 Hwy 36 - Caldwell	South North	S S	49 33	PC S	
	I-45 & Park Road Huntsville	South	Č	44	S S	45 degree skew
	I-10 & US 77	West	С	59	S	
	SR 21 - Caldwell	East	S	40	RC	Panformed Reinforced Concrete
	SR 114 - Dallas	East	S S	49	PC	
	I-40 & FM 2381	East	S	49	PC	
	US 287 - Iowa Par	k North	S	52	PC	
California	I-10 - Colton	West	S	47	S	
	US 101 - Ventura	South	S	84	S	<pre>4 lanes, 3 deep plate girders</pre>
	SR 17 & Fruitvale Ave Oakland	South	S	50	S	8 lanes, 18 girders, dense traffic, hinge in first span
	SR 99 - Stockton	North	С	34	S	30 degree skew
	I-5 - Mokelumne River	North	S	42	RC	End span instrumented
	I-880 - Sacrament	o East	С	83	PC	Box girder, dense

Table 63: Bridge Structures used for Data Acquisition (cont.)

State	Bridge Location	Traffic Direction	Span Type	Instr. Span Length ft	Girder Type	Comments
Illinois	I-70 - Vandalia	West	S	33	S	
	SR 55 over Ill. River - Peru	North & South	S S	73	S S	Data acquired simultaneously
	SR 89 - Spring Valley	South	С	50	S	v
	I-80 over Little Vermillion Rive	West r	С	90	S	Traffic carried by 2 deep plate girders
New York	I-90 & 43	East	S	87	S	
	I-90 & 203	West	S	33	S	
	I-87 & SR 146	South	\$ \$ \$ \$	34	S S S	
	I-90 over Chauta- qua Creek	West	S	68	S	
Ohio	I-90 & Paine Road I-80 & SR 43	East West	C C	37 36	S S	

Span Type: S - Simple, C - Continuous Girder Type: S - Steel, PC - Prestressed Concrete, RC - Reinforced Concrete

¹ foot = .305 meters
1 kip = 454 kilograms.

Table 64: Calibration Crossings for Arkansas I-30 over Route 67 Skew of 50 Degrees

Crossing	Gross Weight kips	Rear Tandem Weight kips
1 2 3 4	58.4 57.9 58.8 58.7	28.8 35.8 39.8 39.8
Actua1	58.5	30.6

Table 65: Variability for Various Girder Weighting Factors

		Weighting	Factors	3	Standard Deviation
Girder	2	3	4	5	kips
	1.00 1.00 1.00 0.60 1.00 0.80 1.00 1.00 0.40 0.40	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	1.00 0.50 0.60 0.00 0.60 1.00 0.60 1.00 0.80 1.00 1.00	1.00 0.00 0.20 0.00 0.20 0.00 0.20 0.60 0.20 1.00 0.25 0.40	2.32 2.71 2.60 3.12 2.51 2.50 2.65 2.55 2.38 2.52 2.28 2.39 2.39
	0.25 0.25	0.50 1.00	1.00 1.00 0.75	0.50 0.25	2.45 2.43

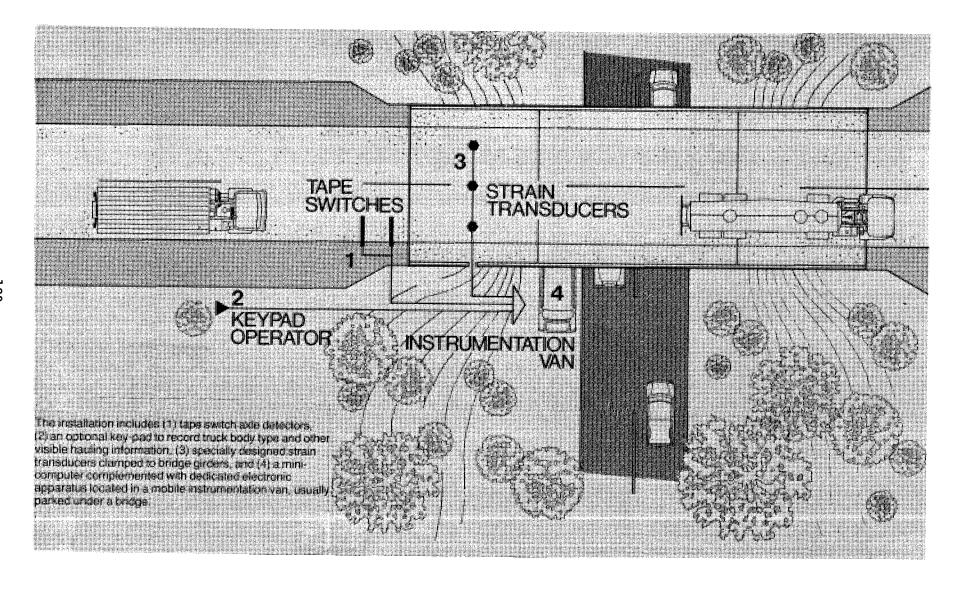


Figure 1: Typical System Setup

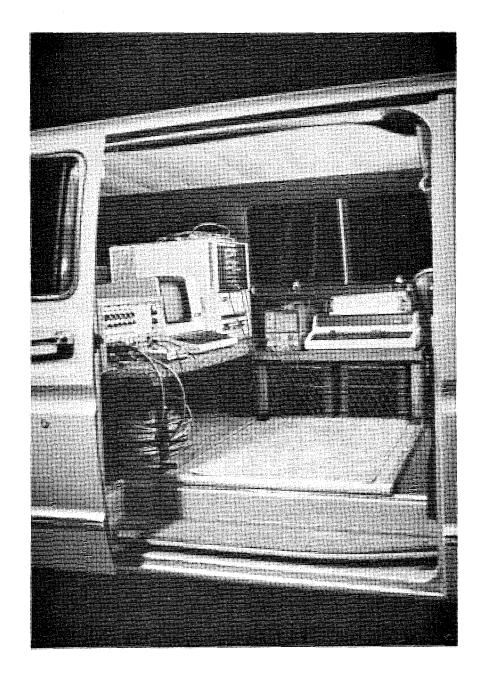


Figure 2 - Bridge WIM System



Figure 3 - Typical Beam Slab Bridge

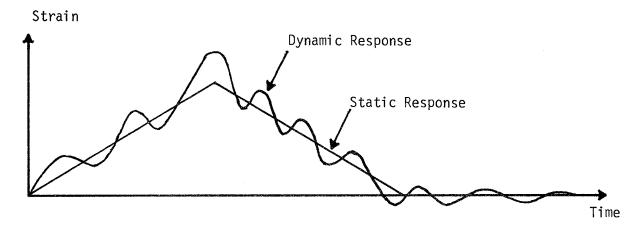


Figure 4: Strain Response due to Truck Axle Crossing

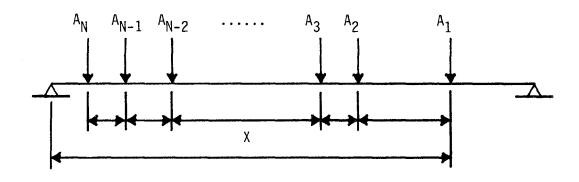


Figure 5: Location of N Axle Truck on Bridge

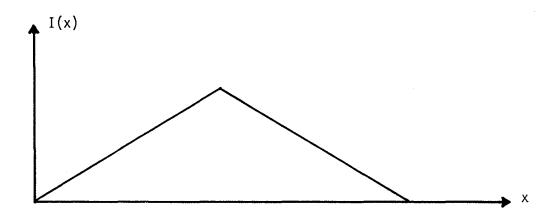


Figure 6: Influence Line, I(x), of Center Span Moment for Simple Span Bridge

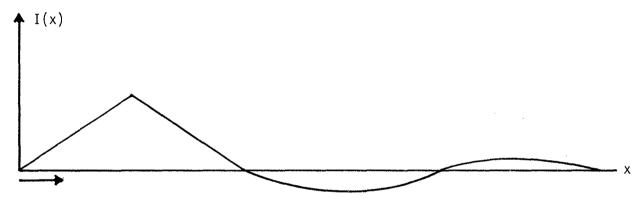
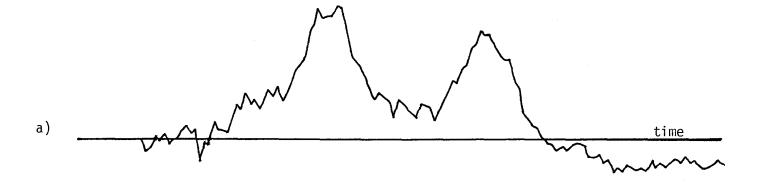


Figure 7: Influence Line, I(x), of 1st Span Center Moment for Three Span Continuous Bridge



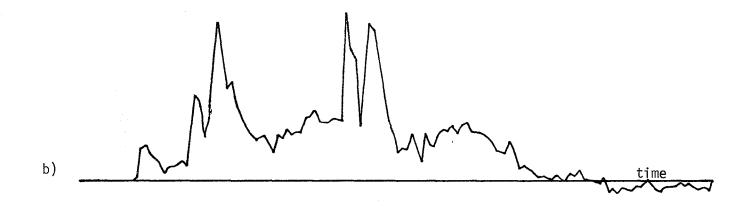


Figure 8 - Strain Records of a 3S-2 vehicle a) at Center of Span b) at Reaction

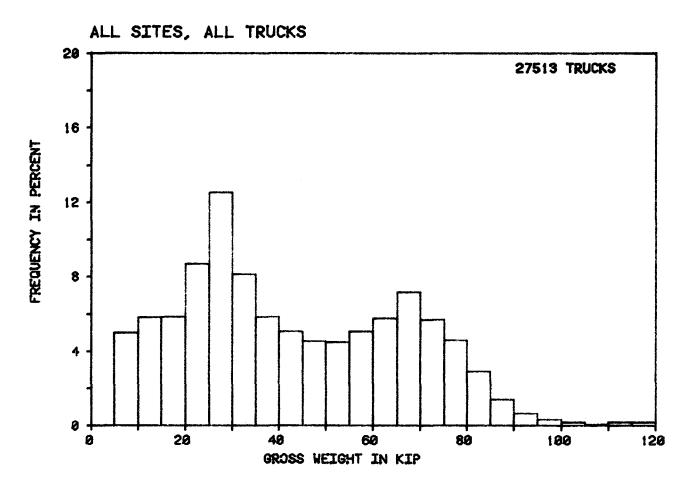


Figure 9: Gross Weight Histogram - all sites, all trucks

l kip = 454 kilograms

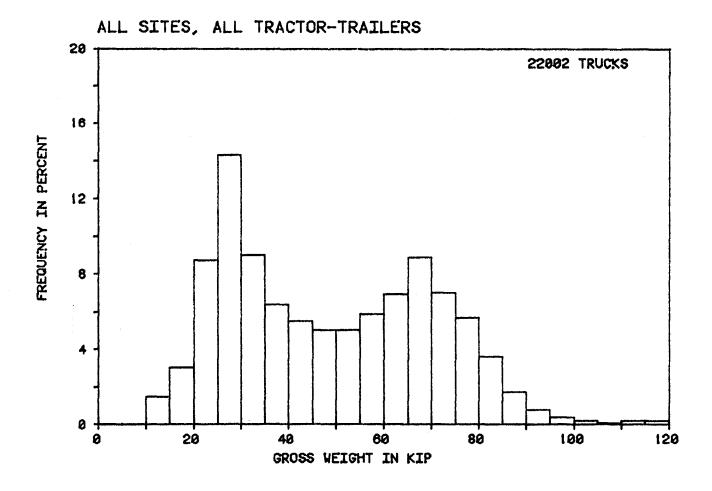


Figure 10: Gross Weight Histogram - All sites, all tractor-trailers 1 kip = 454 kilograms

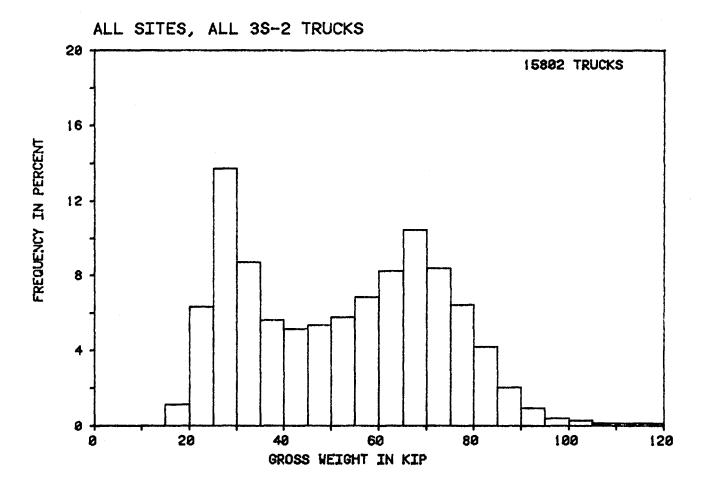


Figure 11: Gross Weight Histogram - All sites, all 3S-2 trucks l kip = 454 kilograms

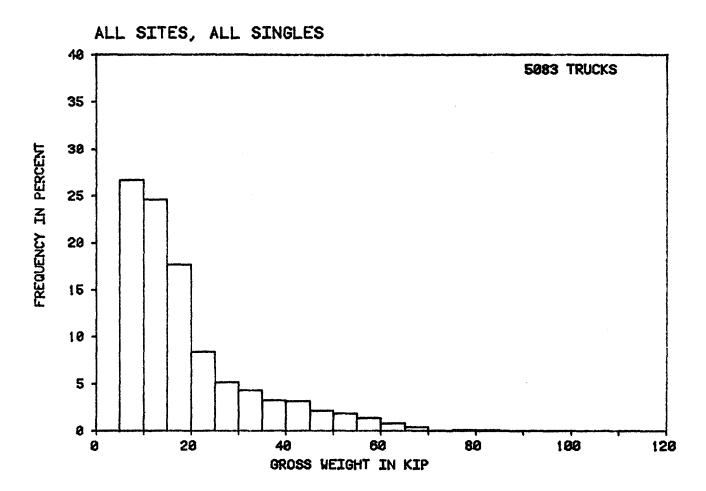


Figure 12: Gross Weight Histogram - All sites, all single trucks l kip = 454 kilograms

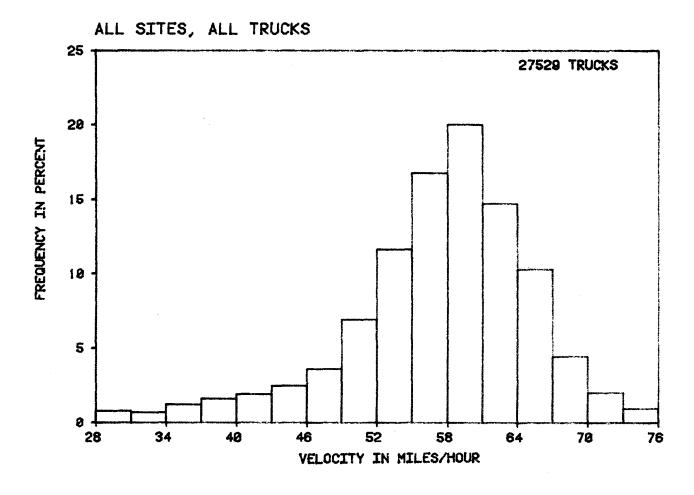
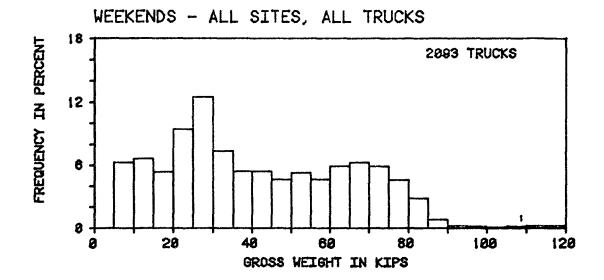
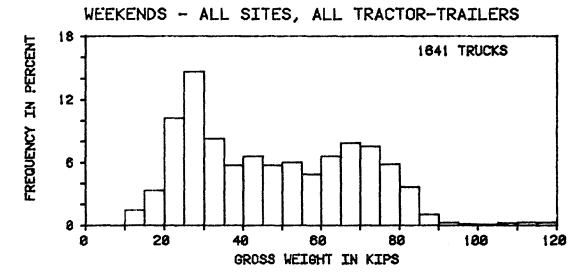


Figure 13: Velocity Histogram - All sites, all trucks | 1 mph = 1.6 kph





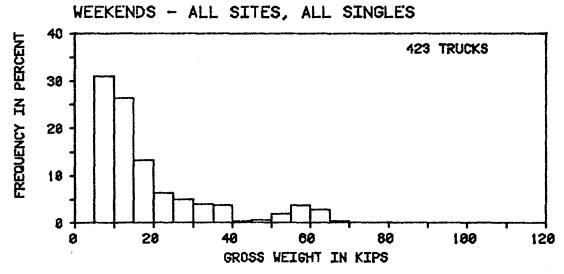
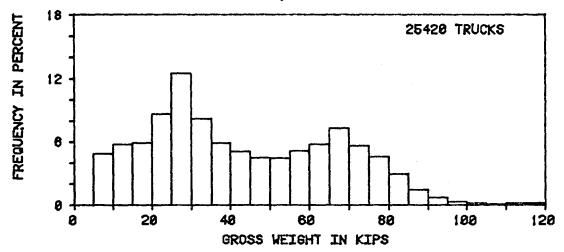
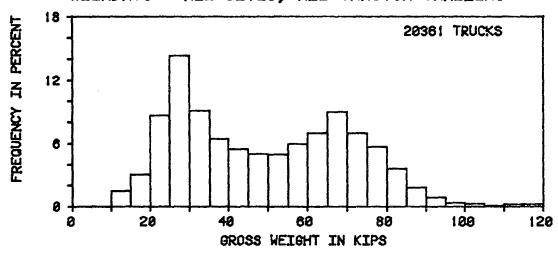


Figure 14: Gross Weight Histogram - Weekends: all sites 1 kip = 454 kilograms





WEEKDAYS - ALL SITES, ALL TRACTOR-TRAILERS



WEEKDAYS - ALL SITES, ALL SINGLES

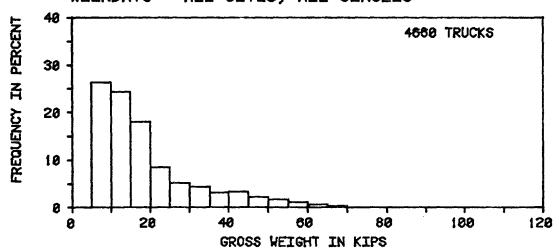
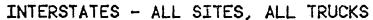
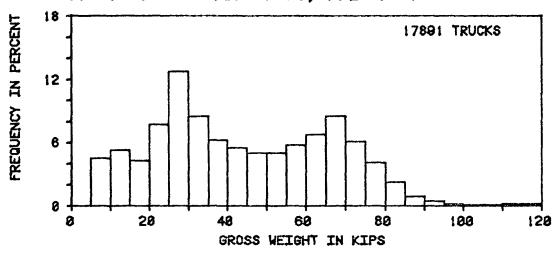
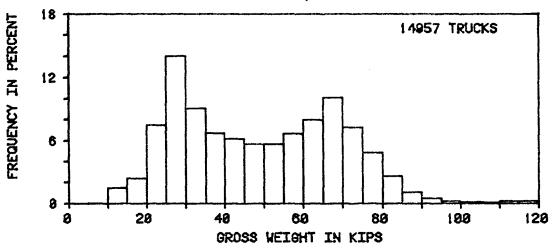


Figure 15: Gross Weight Histogram - Weekdays, all sites l kip = 454 kilograms





INTERSTATES - ALL SITES, ALL TRACTOR-TRAILERS



INTERSTATES - ALL SITES, ALL SINGLES

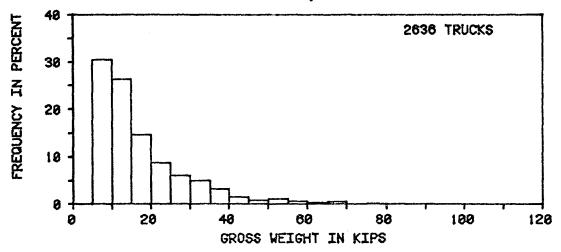
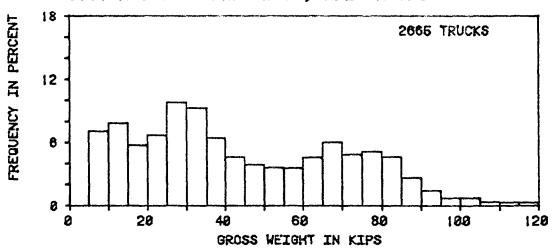
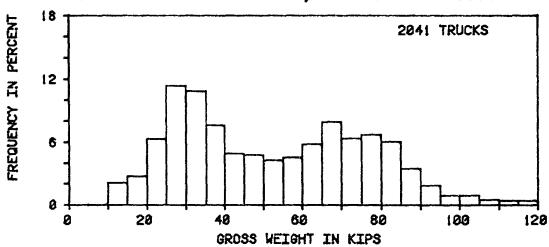


Figure 16: Gross Weight Histogram - Interstates, all sites l kip = 454 kilograms

U.S. ROUTES - ALL SITES, ALL TRUCKS



U.S. ROUTES - ALL SITES, ALL TRACTOR-TRAILERS



U.S. ROUTES - ALL SITES, ALL SINGLES

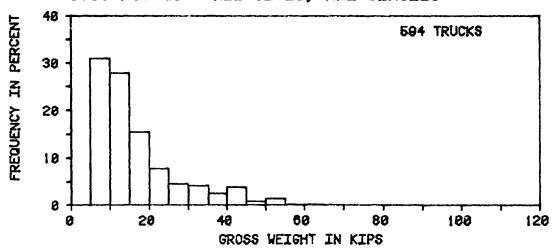
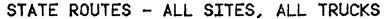
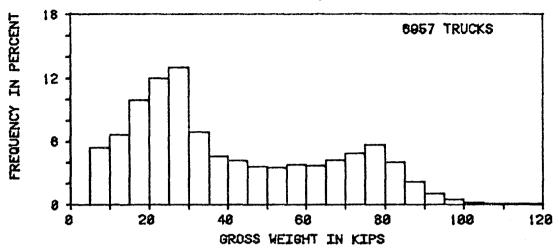
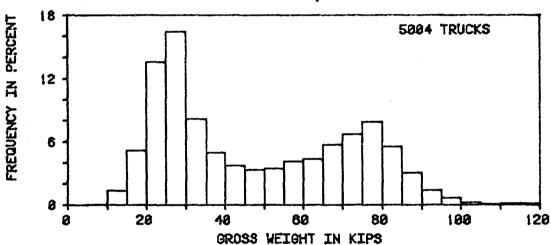


Figure 17: Gross Weight Histogram - U.S. Routes, all sites, all trucks l kip = 454 kilograms





STATE ROUTES - ALL SITES, ALL TRACTOR-TRAILERS



STATE ROUTES - ALL SITES, ALL SINGLES

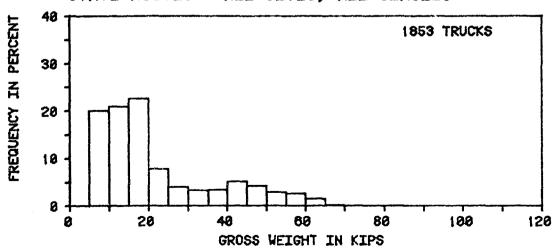
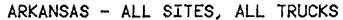
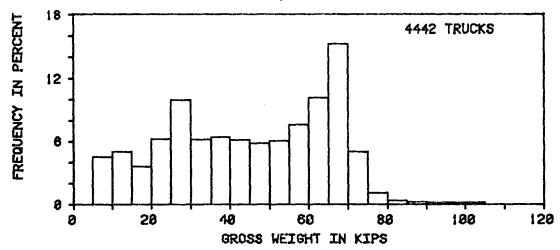
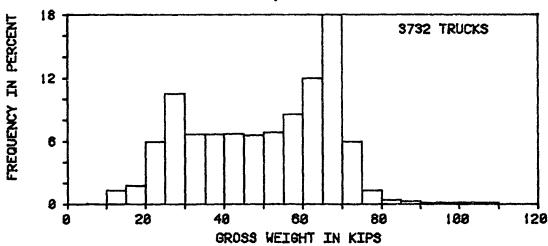


Figure 18: Gross Weight Histogram - State Routes, all sites, all trucks 1 kip = 454 kilograms -140-





ARKANSAS - ALL SITES, ALL TRACTOR-TRAILERS



ARKANSAS - ALL SITES, ALL SINGLES

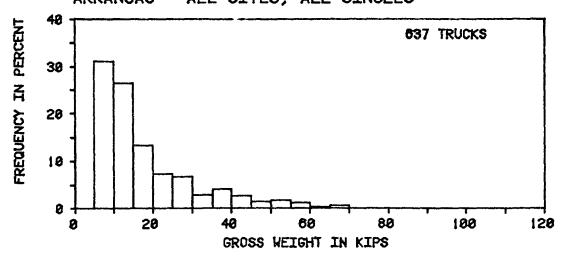
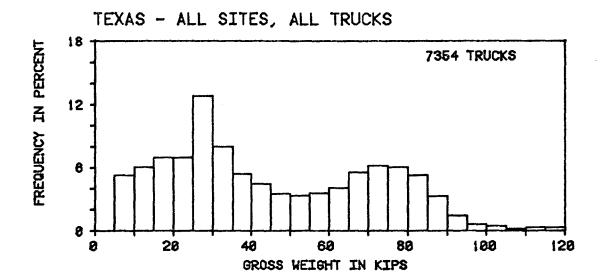
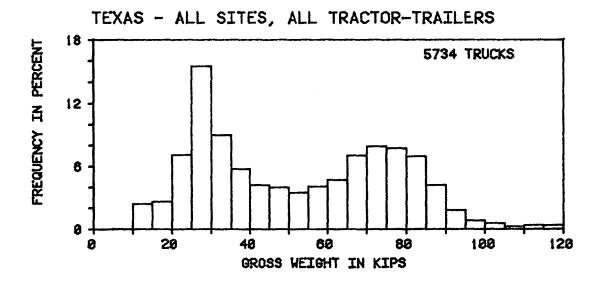


Figure 19: Gross Weight Histogram - Arkansas, all sites, all trucks l kip = 454 kilograms





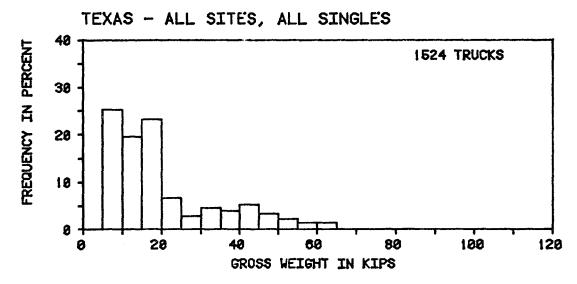
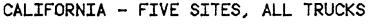
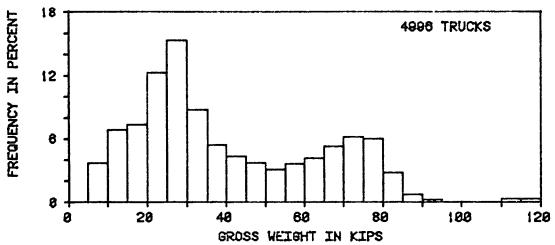
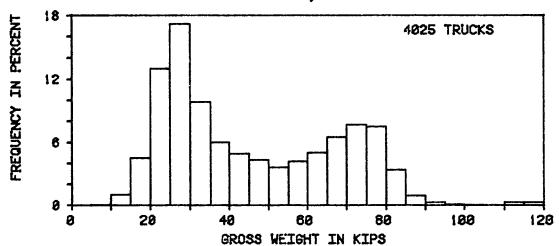


Figure 20: Gross Weight Histogram - Texas, all sites, all trucks l kip = 454 kilograms





CALIFORNIA - FIVE SITES, ALL TRACTOR-TRAILERS



CALIFORNIA - FIVE SITES, ALL SINGLES

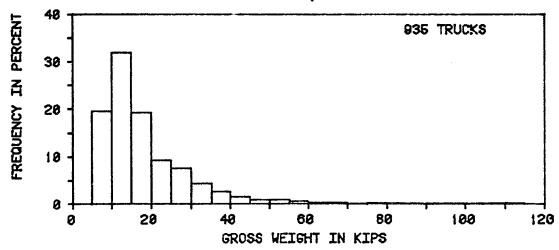
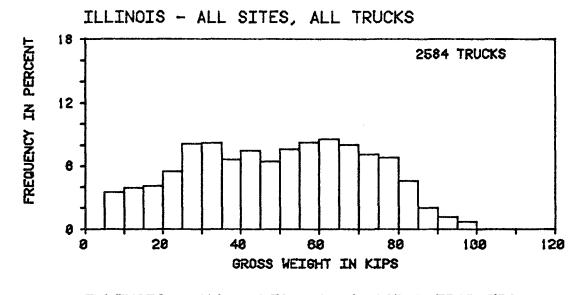
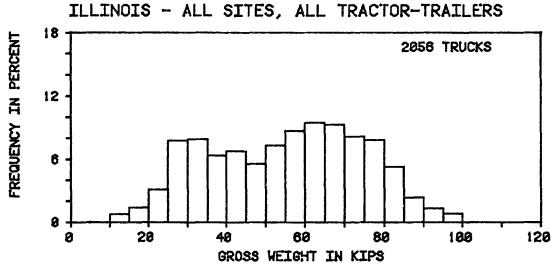


Figure 21: Gross Weight Histogram - California, five sites, all trucks 1 kip = 454 kilograms





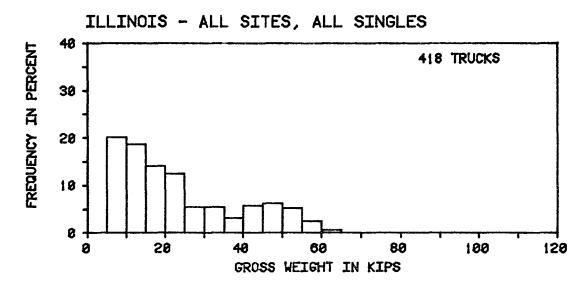
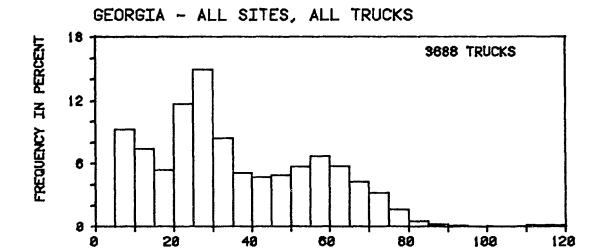
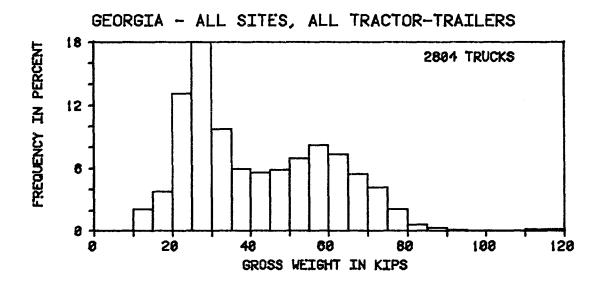


Figure 22: Gross Weight Histogram - Illinois, all sites, all trucks l kip = 454 kilograms



GROSS WEIGHT IN KIPS



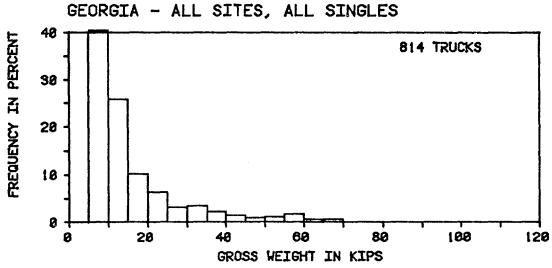
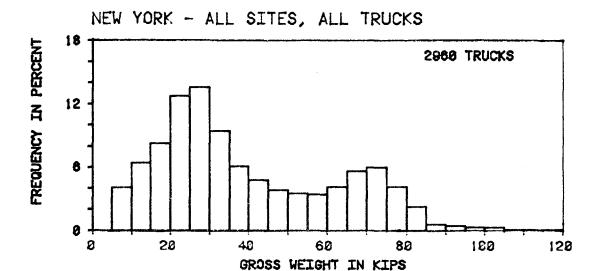
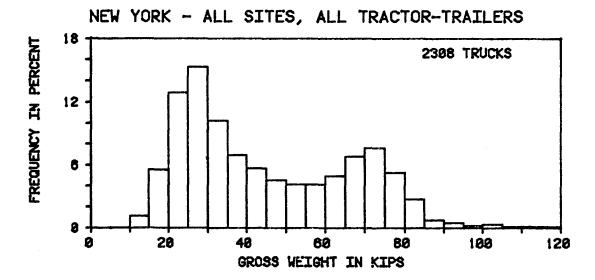


Figure 23: Gross Weight Histogram - Georgia, all sites, all trucks l kip = 454 kilograms





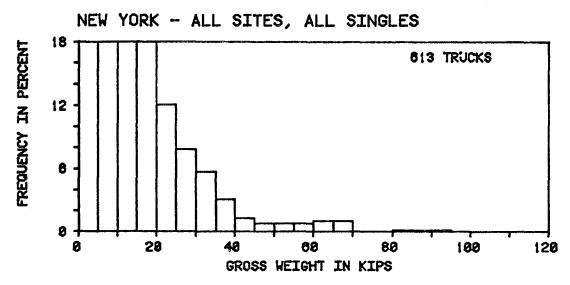
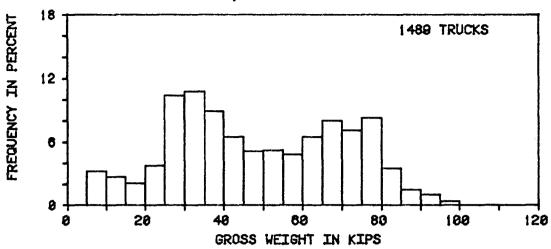
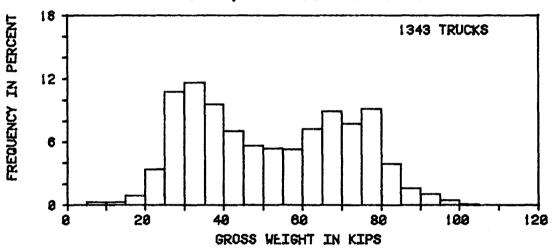


Figure 24: Gross Weight Histogram - New York, all sites, all trucks 1 kip = 454 kilograms





OHIO - ALL SITES, ALL TRACTOR-TRAILERS



OHIO - ALL SITES, ALL SINGLES

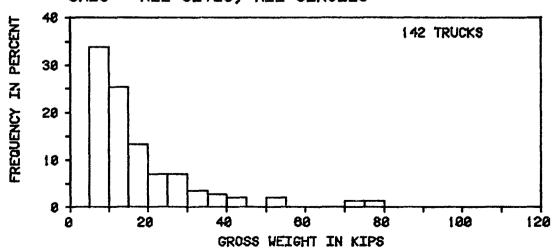
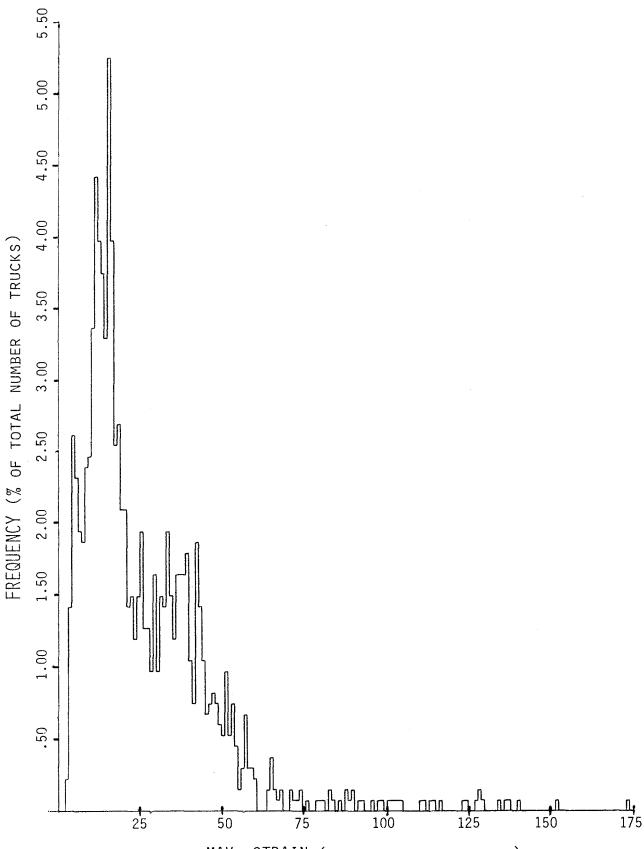
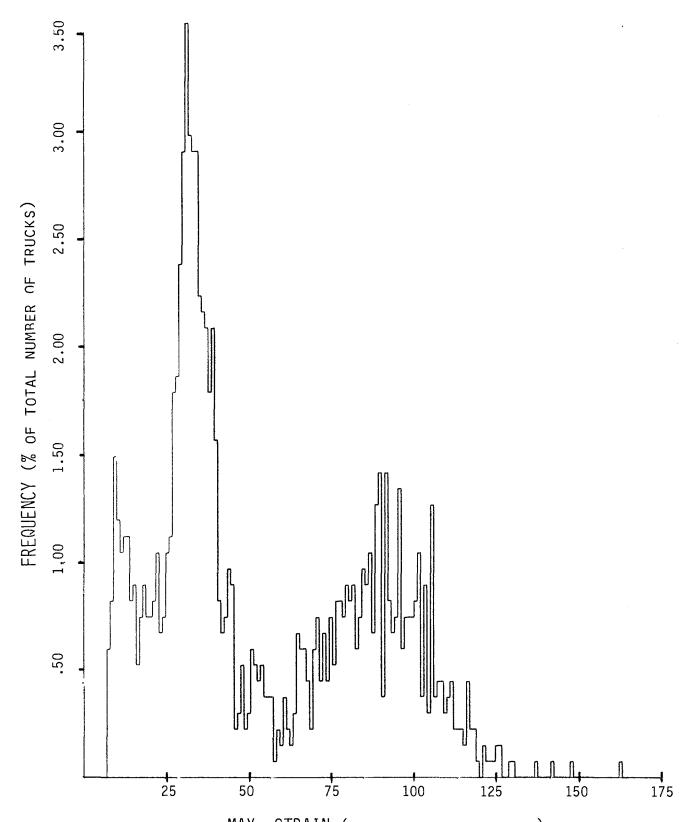


Figure 25: Gross Weight Histogram - Ohio, all sites, all trucks l kip = 454 kilograms

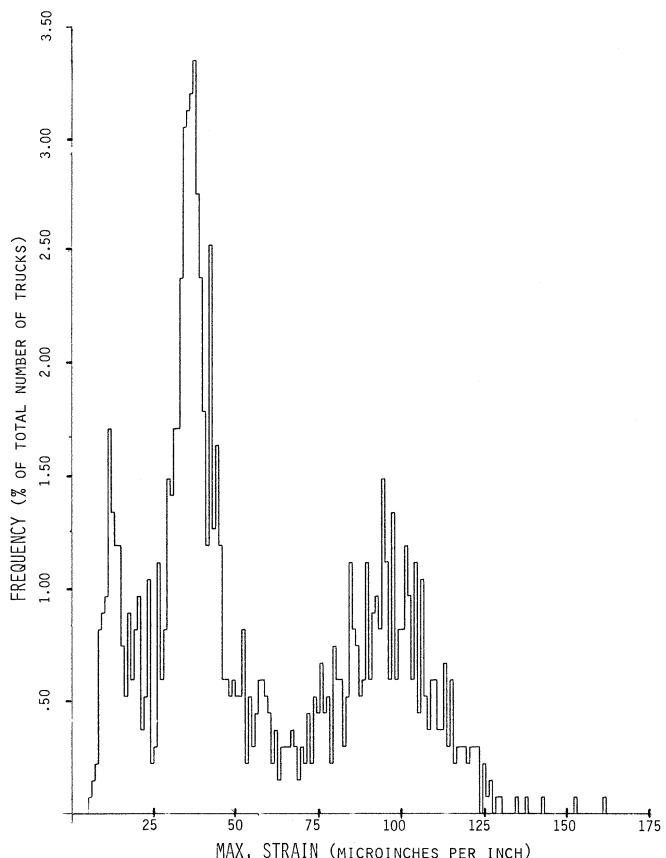


MAX. STRAIN (MICROINCHES PER INCH)

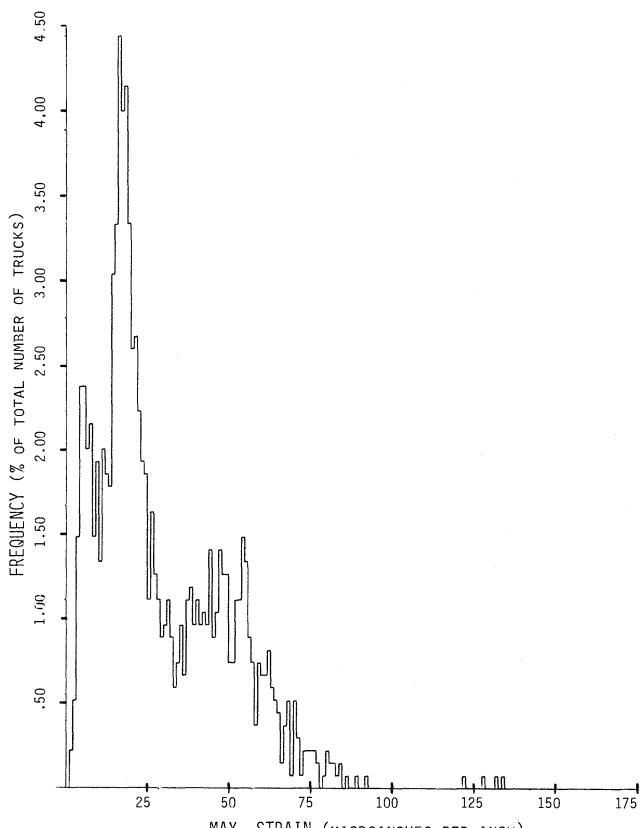
Figure 26: Maximum Strain Histogram, Caldwell-Girder 1



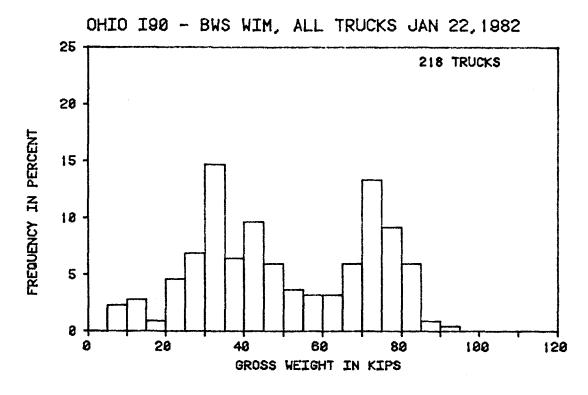
MAX. STRAIN (MICROINCHES PER INCH)
Figure 27: Maximum Strain Histogram, Caldwell-Girder 2



MAX. STRAIN (MICROINCHES PER INCH)
Figure 28: Maximum Strain Histogram, Caldwell-Girder 3



MAX. STRAIN (MICROINCHES PER INCH)
Figure 29: Maximum Strain Histogram, Caldwell-Girder 4



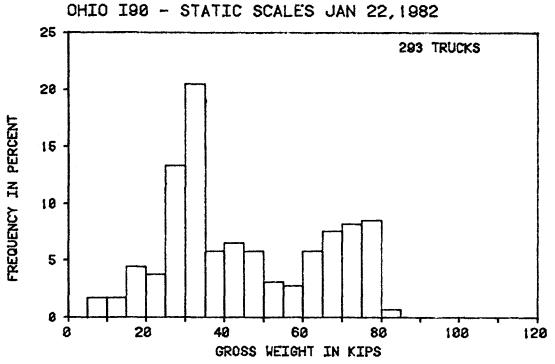
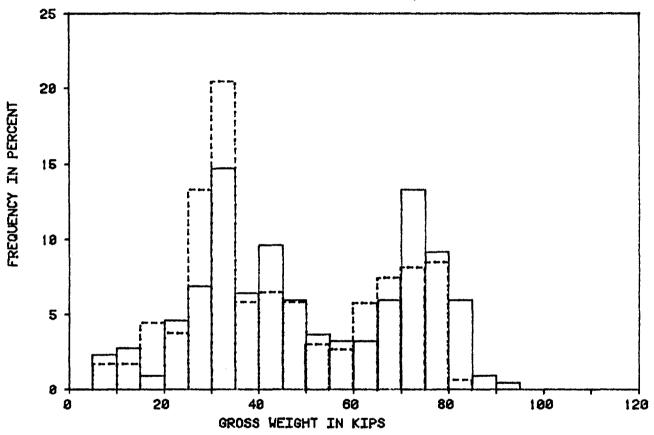


Figure 30: Gross Weight Histogram - Ohio I90, WIM versus Static Scale 1 kip = 454 kilograms

OHIO 190 OVER PAINE ROAD - JAN 22,1982



BWS WIM SYSTEM - 218 TRUCKS
STATIC SCALES - 293 TRUCKS

Figure 31: Gross Weight Histogram - Ohio I90 **ove**r Paine Road Bridge WIM System versus Static Scales - Superimposed l kip = 454 kilograms

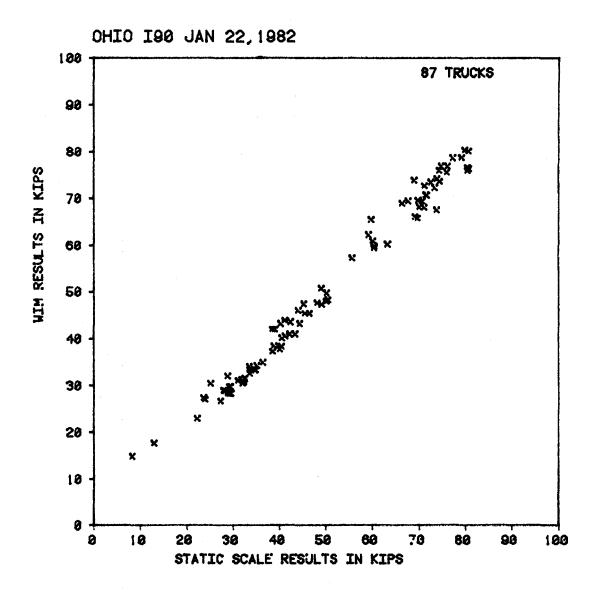


Figure 32: WIM Correlation with Static Scale 1 kip = 454 kilograms

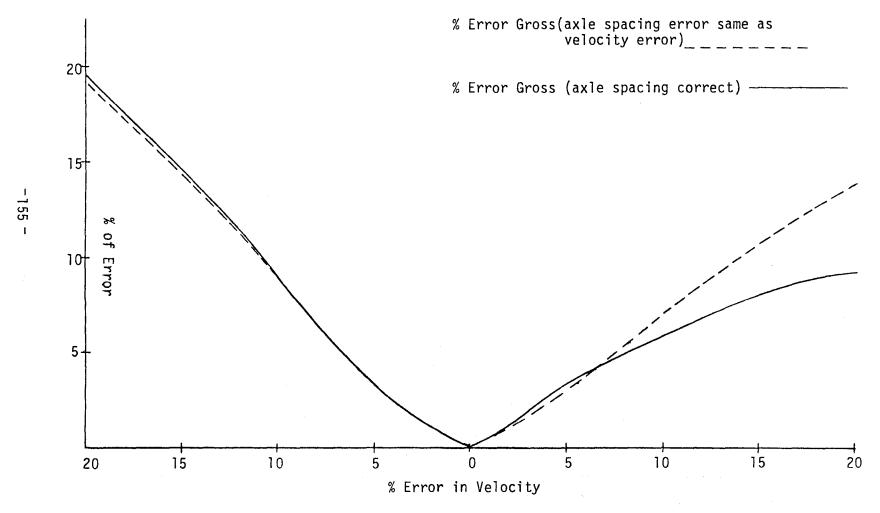


Figure 33: Gross Weight Error Versus Velocity Error

TAPE SWITCH SPACING - FEET

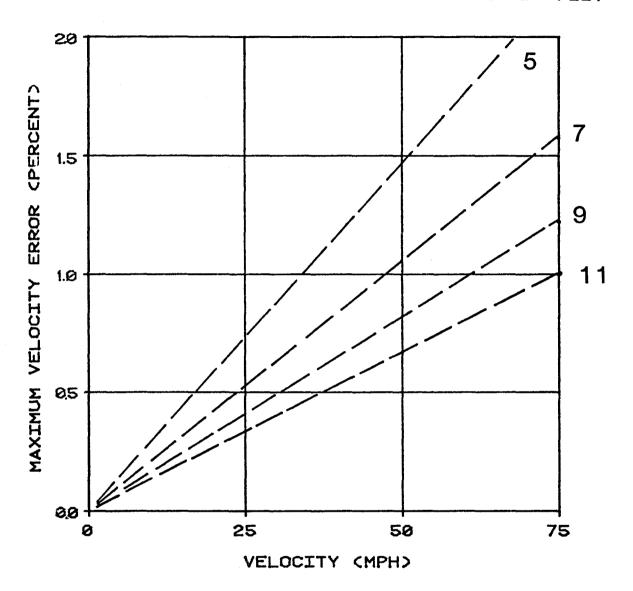


Figure 34: Maximum Velocity Error
Resulting from 1ms Clock Rate
1 mph = 1.6 kph
1 ft = .305m

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FEDERALLY COORDINATED PROGRAM (FCP) OF HIGHWAY RESEARCH AND DEVELOPMENT

The Offices of Research and Development (R&D) of the Federal Highway Administration (FHWA) are responsible for a broad program of staff and contract research and development and a Federal-aid program, conducted by or through the State highway transportation agencies, that includes the Highway Planning and Research (HP&R) program and the National Cooperative Highway Research Program (NCHRP) managed by the Transportation Research Board. The FCP is a carefully selected group of projects that uses research and development resources to obtain timely solutions to urgent national highway engineering problems.*

The diagonal double stripe on the cover of this report represents a highway and is color-coded to identify the FCP category that the report falls under. A red stripe is used for category 1, dark blue for category 2, light blue for category 3, brown for category 4, gray for category 5, green for categories 6 and 7, and an orange stripe identifies category 0.

FCP Category Descriptions

1. Improved Highway Design and Operation for Safety

Safety R&D addresses problems associated with the responsibilities of the FHWA under the Highway Safety Act and includes investigation of appropriate design standards, roadside hardware, signing, and physical and scientific data for the formulation of improved safety regulations.

2. Reduction of Traffic Congestion, and Improved Operational Efficiency

Traffic R&D is concerned with increasing the operational efficiency of existing highways by advancing technology, by improving designs for existing as well as new facilities, and by balancing the demand-capacity relationship through traffic management techniques such as bus and carpool preferential treatment, motorist information, and rerouting of traffic.

3. Environmental Considerations in Highway Design, Location, Construction, and Operation

Environmental R&D is directed toward identifying and evaluating highway elements that affect

• The complete seven-volume official statement of the FCP is available from the National Technical Information Service, Springfield, Va. 22161. Single copies of the introductory volume are available without charge from Program Analysis (HRD-3), Offices of Research and Development, Federal Highway Administration, Washington, D.C. 20590. the quality of the human environment. The goals are reduction of adverse highway and traffic impacts, and protection and enhancement of the environment.

4. Improved Materials Utilization and Durability

Materials R&D is concerned with expanding the knowledge and technology of materials properties, using available natural materials, improving structural foundation materials, recycling highway materials, converting industrial wastes into useful highway products, developing extender or substitute materials for those in short supply, and developing more rapid and reliable testing procedures. The goals are lower highway construction costs and extended maintenance-free operation.

Improved Design to Reduce Costs, Extend Life Expectancy, and Insure Structural Safety

Structural R&D is concerned with furthering the latest technological advances in structural and hydraulic designs, fabrication processes, and construction techniques to provide safe, efficient highways at reasonable costs.

6. Improved Technology for Highway Construction

This category is concerned with the research, development, and implementation of highway construction technology to increase productivity, reduce energy consumption, conserve dwindling resources, and reduce costs while improving the quality and methods of construction.

7. Improved Technology for Highway Maintenance

This category addresses problems in preserving the Nation's highways and includes activities in physical maintenance, traffic services, management, and equipment. The goal is to maximize operational efficiency and safety to the traveling public while conserving resources.

0. Other New Studies

This category, not included in the seven-volume official statement of the FCP, is concerned with HP&R and NCHRP studies not specifically related to FCP projects. These studies involve R&D support of other FHWA program office research.

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