# Interstate and Primary PCC Pavement Overlays Review

Final Report for

MLR-23-01

December 2023
Construction & Materials Bureau



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Ву

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Construction & Materials Bureau

Iowa Department of Transportation

Ames, Iowa 50010

December 2023

#### **TECHNICAL REPORT DOCUMENTATION PAGE**

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#### **DISCLAIMER**

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#### Introduction

lowa has a long history of PCC pavement overlays. As early as 1932, there were various PCC resurfacing projects, as it was called at the time, over brick and PCC pavement. Most of these designs included welded wire fabric on top of the existing pavement. Unfortunately, there are not any records indicating the performance of any of the projects.

Several research projects were placed over the years. The first whitetopping project was placed on the Storm Lake airport in 1971. This overlay is still in service with several areas of patched panels. In 1973, a research project<sup>1</sup> in Greene County kicked off the modern era of bonded and unbonded overlays. In 1994, thin bonded overlay research project with 65 test sections was placed on IA 21 in Iowa county. In 2002, an overlay and widening research project with over 100 test sections was placed on IA 13 in Delaware county.

#### **Objectives**

A recent 2017 study<sup>2</sup> by the National Concrete Pavement Technology Center at Iowa State University mainly concentrated on PCC overlays on the secondary system. Using pavement management data, the research concluded that 89% of all overlays in Iowa are good to excellent. Another 2014 study<sup>3</sup> was conducted by the National Concrete Pavement Technology Center on performance of overlays in the United States. The objective of this study was to perform a review of concrete overlays performance on the interstate and primary system and determine any best practices or lessons learned. A map and project information can be found in Appendix A. Construction history and project reviews can be found in Appendix B.

#### Concrete Overlay History

lowa has a long history of PCC pavement overlays. As early as 1932, there were various PCC resurfacing projects, as it was called at the time, over brick and PCC pavement. Most of these designs included welded wire fabric on top of the existing pavement. Unfortunately, there are not any records indicating the performance of any of the projects. In 1949, a 6-inch PCC resurfacing project on US 30 in Benton County was one of the early projects to utilize widening with the overlay. Designs on these early PCC resurfacing projects may be found in Appendix C.

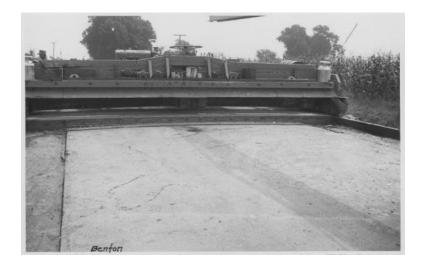


Figure 1 – Benton County US 30 Bonded Overlay 1949

#### Concrete Overlay Designs

The Iowa DOT classifies concrete overlays in the following categories:

- Whitetopping PCC over HMA Pavement
- Unbonded PCC over Composite Pavement
- Bonded PCC over PCC Pavement.

There have been multiple design features incorporated and changed over the years. A variety of joint spacing and thicknesses have also been utilized.

### Review of Iowa County IA 21 Research Project

In 1994, Dr. James Cable developed a research project<sup>4</sup> on IA 21 in Iowa County. This whitetopping research project included 65 different test sections with thicknesses of 2, 4, 6, and 8 inch and joint spacing of 2, 4, 6, 12, and 15 feet. Also, three different types of base preparations were used on the project, including patching and scarifying, patching only, and cold in place recycling. A few sections were placed with monofilament or fibrillated polypropylene microfibers. The test section layout may be found in Appendix D.

A two-year review in 1996 showed some distress in the 2-inch sections. Eventually, some of the 2-inch sections were patched and later overlaid with HMA in 2002, 2006, and 2009. However, a 2023 review showed that most of the 4-inch test sections and all of the 6-inch test sections were in good condition, regardless of joint spacing. Approximately, four miles of the 4-, 6-, and 8-inch PCC overlay sections are performing well, after 29 years of service.

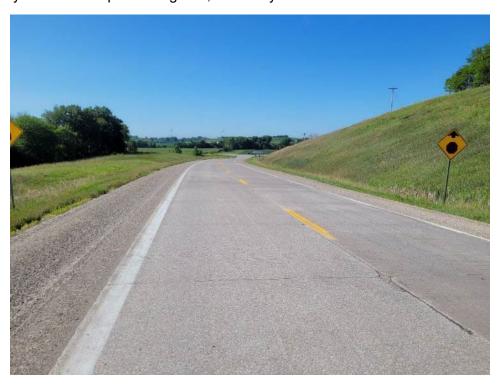


Figure 2 – 6-inch Overlay 12 x 12 ft. Joint Spacing

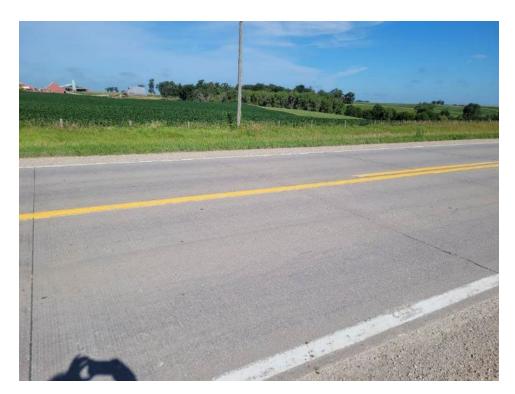


Figure 3 – 8-inch overlay 12 x 15 ft. Joint Spacing



Figure 4 – 6-inch overlay 6 x 6 ft. Joint Spacing



Figure 5 – 4-inch overlay 4 x 4 ft. Joint Spacing

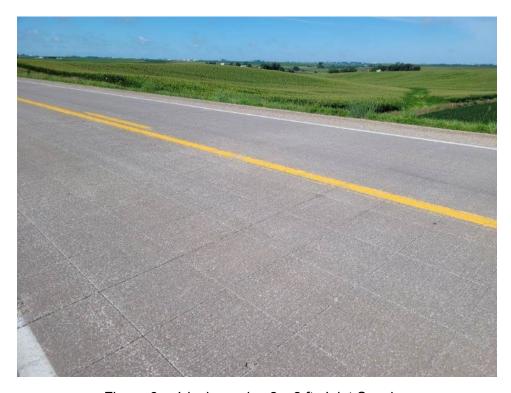


Figure 6 – 4-inch overlay 2 x 2 ft. Joint Spacing

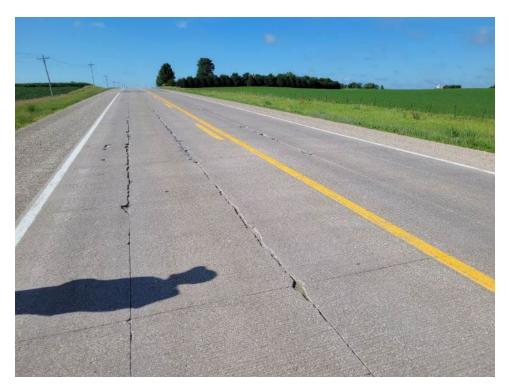


Figure 7 – 4-inch Overlay 4 x 4 ft. Joint Spacing exhibiting some joint deterioration.

A possible reason why the test sections still are performing very well may be because the overlay was placed the same width as the existing pavement, without widening or tied shoulders. Another factor may be for the test sections, 4 inches or thicker test, all joints were sealed.

#### Review of Delaware IA 13 Overlay Research Project

In 2002, Dr. Cable developed another research project<sup>5</sup> on IA 13 in Delaware County. This project was a widening and unbonded overlay of a composite pavement. The old widening units were removed and a 6 foot by 8-inch-thick widening was added to each side. There were 191 test sections of 3.5-inch and 4.5-inch overlay, varying panel size, macro fibers, microfibers, tied shoulders, and untied shoulders.

The longitudinal joint former was used to form the joint between the widening units and overlay. There were issues with cracking within a short time period, due to lack of a formed joint. This coupled with the widening unit heaving caused issues on this overlay. In areas, the widening units are inversely sloped toward the outside wheel path making it difficult to remove snow.



Figure 8 – Delaware IA 13 Unbonded Overlay Construction



Figure 9 – Delaware IA 13 Widening Unit Heaving

#### Review of Sac IA 175 Unbonded Overlay Experimental Project

Another experimental overlay was placed on IA 175 in Sac County in 2007. This project was a 4.5 inch unbonded overlay with 2 feet by 8-inch-thick widening on each side. The existing pavement had 2-foot HMA widening on each side in the 1980s. Engineering fabric was used over the HMA widening units and no visible cracking was observed in the pavement prior to the PCC overlay. No reinforcing steel was used to tie the widening units to the overlay.

Cracking occurred in the outside wheel paths in less than two years. It was assumed that the old HMA widening unit was heaving causing the cracking. However, it was later discovered that the outside widening unit was heaving that cause the cracking. This problem led to the use of the 60-inch reinforcing steel bar across the old widening and tied to the new widening.

The district sealed all the cracking and there has not been a lot of patching required. Even with all the cracking, the overlay still rides fairly well at the time of this report.



Figure 10 – Sac IA 175 UBOL Left Shoulder Crossslope 1.66%



Figure 11 – Sac IA 175 UBOL Left Shoulder Crossslope 1.13%



Figure 12 – Sac IA 175 UBOL Left Shoulder Crossslope 1.13%

#### Review of Whitetopping Overlays

Review of the Iowa County IA 21 whitetopping overlay showed how well some of the test sections are performing after 29 years.

Another whitetopping project that has been in service for more than 40 years is the Adair County I-80 westbound overlay, in 1979. The design was to mill 8 inches of HMA and replace with 10 inches of PCC. Overall, the overlay is in good condition with a few patched areas. The pavement was diamond ground in 2020.



Figure 13 – Adair I-80 WB Whitetopping Overlay Paved 1979 (Picture 2023)

Review of other whitetopping overlays include Montgomery US 71 and Cass US 71 projects. Built in 2006 and 2007, respectively, both 8-inch overlays are in very good condition. The Montgomery County whitetopping has HMA shoulders and the Cass County whitetopping has tied PCC shoulders.

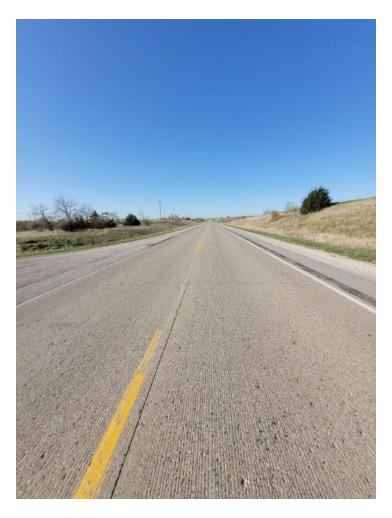


Figure 14 – Montgomery County US 71 Built 2006 (Picture 2023)

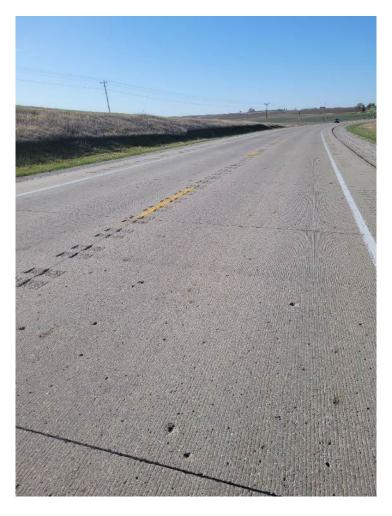


Figure 15 – Cass US 71 Built 2007 (Picture 2023)

One project to note that includes both whitetopping and unbonded overlay sections is on US 71 in the Clay/Dickinson counties. It is interesting to note that the whitetopping areas of the overlay are performing well, while areas of the unbonded overlay are experiencing longitudinal cracking and panel movement. The sections of whitetopping overlay used a 36-inch tie bar, while the sections of unbonded overlay have a 6-foot #5 reinforcing tie steel.



Figure 16 – Clay/Dickinson US 71 Whitetopping Overlay Built 2012 (Picture 2023)



Figure 17 – Clay/Dickinson US 71 Unbonded Overlay Built 2012 (Picture 2023)



Figure 18 – Clay US 71 Whitetopping Built 2015



Figure 19 – Clay US 71 Whitetopping Built 2016

#### Review of Unbonded Overlays

Many of the unbonded overlays have been placed over old 1920s and 1930s pavements that were built 18 to 20 foot wide and were widened and resurfaced in the 1950s. After the early cracking found on the Sac IA 175 overlay, it was decided to staple a 60-inch reinforcing bar across the old widening unit into the new widening/shoulder. With the exception of the US 71 Clay unbonded overlays placed in 2015 and 2016, nearly all of the unbonded overlays exhibit some type of longitudinal cracking and/or panel movement in the interior slabs.

Many of the unbonded overlays placed between 2002 and 2014 were thin at 5 inches or less. Also, between 2004 and 2014, all joints were left after sawing without any joint filler material. This likely caused issues with joint infilling, which led to problems with panels moving.



Figure 20 – Osceola IA 9 panel movement



Figure 21 – Grundy IA 14 Longitudinal Cracking and Shattered Panels

A thicker overlay did not seem to prevent the cracking with the longer reinforcing tie steel. On the Dallas US 169 unbonded overlay, this 7 inch unbonded overlay with 12 x 12-foot panels and tied shoulders with a 60-inch reinforcing bar exhibits quite a bit of longitudinal cracking. Most of the cracks have been sealed and there is very little patching. The cracking does not seem to affect the ride of the pavement, currently. The as constructed shoulder cross slope was 2 percent. Checking shoulder cross slope near the areas with longitudinal cracking indicated shoulder cross slope anywhere from 0.9% to 1.48%. Thus, the shoulders appear to heave, causing cracking off the end of the 60-inch reinforcing steel.



Figure 22 – Dallas US 169 UBOL Longitudinal Cracking



Figure 23 – Dallas US 169 UBOL Shoulder Cross Slope 1.40%

However, on the Clay US 71 overlays placed in 2015 and 2016, which include both whitetopping and unbonded overlays, both overlay types are in very good condition. As noted earlier, the unbonded overlay placed on Clay/Dickinson US 71 had a few areas of longitudinal cracking. The main difference between the 2012 overlay and the newer overlays was the joint spacing. The 2012 unbonded overlay has 6 x 6-foot panels, with a 6-foot #5 reinforcing steel over the old widening into the tied 4 x 6-foot shoulder. While the 2015 and 2016 overlays have 8 x 9-foot panels with a 6-foot #5 reinforcing steel across the old widening into the 8 x 7-foot shoulder. This design places the sawed joint directly over the old 18-foot original pavement edge.

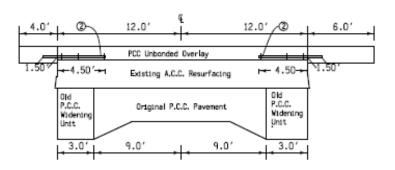


Figure 24 – Clay/Dickinson US 71 2012 UBOL typical

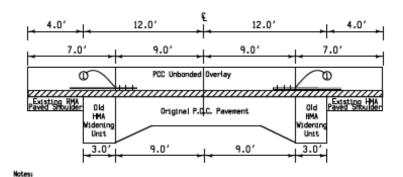


Figure 25 – Clay US 71 2015-16 UBOL typical



Figure 26 – Clay US 71 2015 UBOL



Figure 27 - Clay US 71 2016 UBOL

The 9 inch unbonded overlays in Fremont and Mills counties placed in 2009 are both performing very good. There are no patches or longitudinal cracking noted during a review in 2023. Interesting to note, the driving lane was widened to 14 foot, or 2 foot over the existing shoulder. No cracking was noted, however, HMA shoulders were used, so there was no tie steel used between the shoulder and the overlay.



Figure 28 – Fremont County I-29 NB Unbonded Overlay (2023)



Figure 29 – Mills County I-29 NB Unbonded Overlay (2023)

#### Review of Bonded Overlays

Many of the early bonded overlays were placed on existing pavements in poor condition. Thus, any issues in the underlying pavement reflected through the overlay in short time. Also, many of the early bonded overlay projects were fast track projects<sup>6</sup>, utilizing Type III cements and insulating blankets for rapid strength gain. However, the very high temperatures and rapid hydration of the Portland cement resulted in non-durable paste, leading to durability issues.

A bonding grout was required for bonded PCC overlays until the April 2003 specification revision. Issues with bonding grout drying out can cause debonding of the overlay. It was found that when the existing PCC surface is in surface saturated dry condition, bonding with the PCC overlay is more than adequate.

Another issue affecting performance of bonded PCC overlays is the alignment of sawed joints in overlay with those in the existing pavement. All joints must be aligned directly over the existing joints to prevent random cracking. All joints need to be sawn full depth of the overlay and transverse joints need to be sawed as wide as the existing joints.

Currently, there is only one bonded concrete overlay projects remaining on the interstate and primary system. Placed in 1994, the Franklin County IA 3 bonded overlay project is the last remaining bonded overlay on the primary system. This bonded overlay was also a fast-track project, however, it was placed in cooler conditions in the fall and likely the concrete temperatures were not as elevated. Thus, the pavement exhibited better performance than the fast-track overlays placed in the summer.



Figure 30 – Poweshiek I-80 Bonded Overlay 1984 with Grout on Existing Pavement



Figure 31- Franklin Co. IA 3 Bonded Overlay

The existing concrete pavement should be in fairly good condition prior to placement of a bonded PCC overlay. Otherwise, any defects in the existing pavement will be mirrored through the overlay, in a very short time period.

#### Existing Pavement Prior to Overlay Design

For whitetopping and unbonded overlays, the condition of the HMA creates a critical component impacting the performance of the PCC overlay. The existing HMA layer may be oxidized with large number of thermal cracks. If the thermal cracks are unable to be removed by milling or filling, these thermal cracks can lock in the overlay, causing cracking in the PCC overlay.

On several projects, after milling was completed and haul trucks drove on the surface, the existing HMA layer deteriorated in areas. On the Woodbury I-29 unbonded overlay project, the HMA layer de-bonded from the PCC surface below during milling operations. HMA resurfacing was added as extra work to alleviate some of these issues. A non-woven geotextile interlayer has also been used to fix short areas of deterioration.



Figure 32 – HMA deterioration on US 71 Cass Whitetopping Project

The condition of existing PCC pavement, prior to a bonded PCC overlay, was discussed in the Review of Bonded Overlays section.

#### Design Features of Overlays

During this review, several design features were noted that have impact on the performance of the overlay. One of the best features noted was using full depth pavement at the beginning (BOP) or end (EOP) of the project and transition between overlay types. Using full depth pavement at the BOP or EOP is especially important if the transition pavement it HMA. Using full depth pavement prevents the panels from migrating due to traffic.



Figure 33 – Clay/Dickinson US 71 NB panels migrating in driving lane at BOP against HMA pavement.

Using full depth pavement transition between overlay types also improves performance of the overlay. This is especially needed if there is a difference in joint spacing. For instance, an unbonded overlay with  $6 \times 6$ -foot panels butted to a whitetopping overlay with other joint spacing, such as  $12 \times 12$  foot or  $9 \times 8$  foot. The full depth panel prevents misaligned joints from extending into adjacent panels.

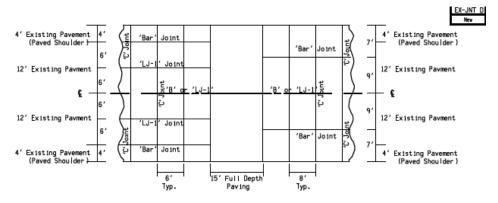


Figure 34 – Full depth transition between overlay types typical



Figure 35 - Full depth transition panel US 71 Clay County

A typical design feature of unbonded overlays where the existing pavement had been widened and widened again with the overlay that has caused performance issues is the use of 60-inch reinforcing steel stapled to the existing pavement over the widening unit, extending into the new widening. This seems to cause issues on nearly all of these unbonded overlays where it has been used.

On many of the projects that exhibit longitudinal cracking in the wheel path, it has been observed that the shoulder has heaved and is not at the cross slope as placed. Many times, the shoulder is approximately 1 percent lower cross slope that from that as placed. Apparently, this tends to raise the outside panel resulting in a crack developing off the end of the tie steel in the wheel path.

Prior to 2011, a #4 reinforcing steel bar 60 inches long was placed over the existing widening and into the new widening unit. From 2011 to 2014 a #5 bar was used and a #4 from 2015 and later. It was noted that the #5 bar may be too rigid and standards were changed back to the #4 bar. Although, it does not appear that changing to a smaller diameter bar has helped eliminate longitudinal cracking. Most all are stapled to the existing pavement and panels throughout the middle are moving with direction of traffic.



Figure 36 – Typical tie steel on unbonded overlay with existing widening.



Figure 37 – Closeup of steel, stapled to existing pavement.



Figure 38 – Shoulder slope 3.3%, Paved 4%



Figure 39 – Typical cracking off end of steel in wheel path.

Observations indicate that the outside panels and shoulder appear to be locked in place with the steel stapled to the existing pavement. On several projects, the center panels are moving with traffic. On two lane pavements, there are areas where the panels are moving at each other and

becomes an area where the panels buckle. Note in Figure 38 that the panels have moved approximately 1 inch from the left and approximately 1 ½ inches from the right, resulting in the between joint to buckle.



Figure 40 – Interior panels moving towards each other and blowup.

#### Future Overlay Design Details

Several alternate designs have been placed in the last few years that likely will address the issues found with the 60-inch reinforcing steel. One method used on the Marshall County IA 14 project was a 36-inch reinforcing steel was stapled across the existing widening unit. The overlay was placed full width and the shoulders were not tied.

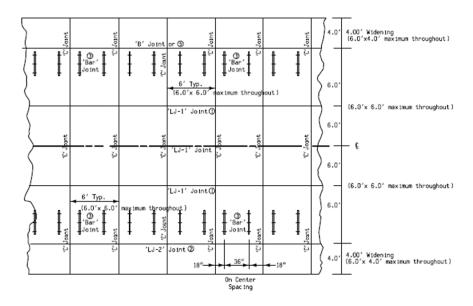


Figure 41 – Marshall County IA 14 Overlay Typical



Figure 42 – Marshall County IA 14 Overlay

The use of macro fibers has been tried successfully on a few county projects in Worth and Buchanan counties. Based on the success of those projects, test sections utilizing macro fibers and various joint spacing were placed on the Woodbury IA 31 whitetopping overlay in 2020.

Begin Sta.	End Sta.	Length feet	PCC SY	PCC CY	Fiber Ibs	Transverse Sawcut Spacing FT	Longitudinal Sawcut Spacing FT
43+76	50+00	624	2079.8	359.7	1438.7	6	6
142+00	152+00	1000	3333.0	576.4	2305.6	15	12
152+00	162+00	1000	3333.0	576.4	2305.6	12	12
162+00	172+00	1000	3333.0	576.4	2305.6	9	12
172+00	182+00	1000	3333.0	576.4	2305.6	9	6
182+00	192+00	1000	3333.0	576.4	2305.6	6	6
423+00	441+00	1800	5999.4	1037.5	4150.1	6	6

Figure 43 – Woodbury IA 31 Overlay 2020 Macro Fiber Test Sections

In 2022, the adjacent Cherokee IA 31 whitetopping project was placed with 4 lbs. macro fibers per cubic yard. The shoulders were tied with a  $\#4 \times 36$ " reinforcing steel bar at 30-inch center to center. The joint spacing utilized 12 x 12-foot panels.

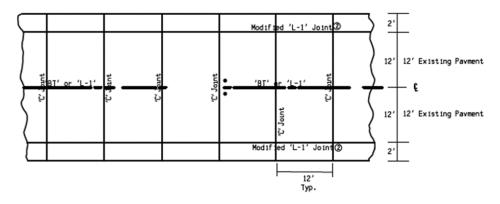


Figure 44 – Cherokee IA 31 Whitetopping with Macro Fibers

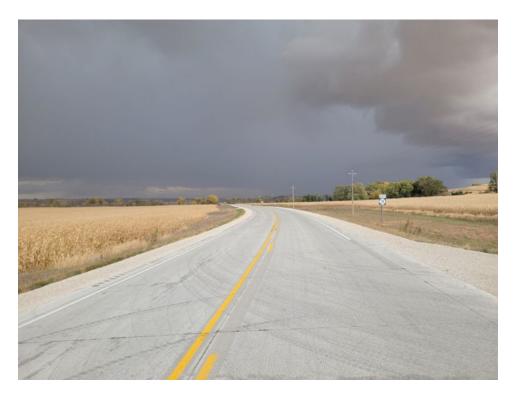


Figure 45 – Cherokee IA 31 Whitetopping with Macro Fibers

In 2022, an unbonded overlay on US 63 in Tama and Blackhawk counties was placed with 5 lbs. of macro fiber per cubic yard. No reinforcing steel was used in the project. The existing pavement was rubblized in 1998, prior to an HMA overlay. The joint spacing utilized 6 x 6-foot panels.

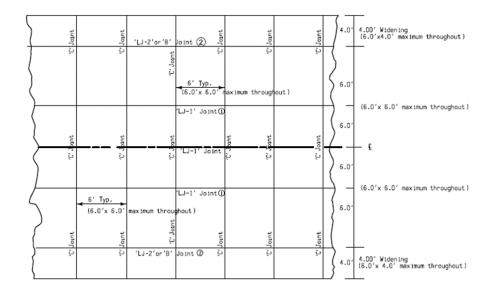


Figure 46 – Cherokee IA 31 Overlay Typical

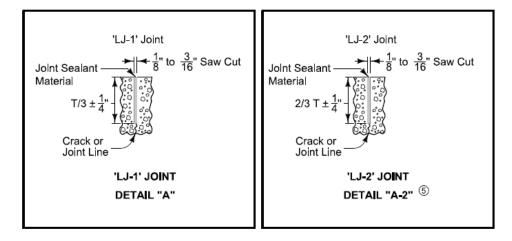


Figure 47 – LJ-1 and LJ-2 Joint Typical



Figure 48 – Tama/Blackhawk US 63 Overlay with 5 lbs. macro fiber per cubic yard.

In 2023, the Plymouth IA 3 whitetopping project utilized 4 lbs. of macro fiber per cubic yard. Similar to the Cherokee IA 31 whitetopping, the modified L-1 joint was used to tie the shoulders. Joint spacing was increased to 12 x 12-foot panels.

Utilizing fibers should be the future design for overlays. The fibers allow wider joint spacing, keep any cracking tight, reduce panel movement, and may eliminate the need for reinforcing to tie shoulders or bridge old widening units. These projects should be monitored regularly for performance and incorporated into current overlay designs.

#### **Overlay Construction**

During construction of overlays, the superelevated curves are typically corrected to the proper cross slope. When this is done with the overlay, the concrete can become much thicker than the design thickness on the outside of the curve. On the Woodbury I29 northbound overlay in 2008, the thickness on the outside of the curve reached up to 15 inches in some locations. The design was a 9 inch unbonded overlay, requiring the longitudinal joints to be sawed at T/3, or 3 inches. Sawing at 3 inches was not adequate on the thickened outside edge of the pavement, which should have been 5 inches based on the concrete placed.

This led to longitudinal cracking right away. Cracking also occurred in the inside lane from due to the widened slab over the shoulder and differential settlement. Approximately 3763 feet of cracking in the left lane and 9242 feet of cracking in the right lane was cross stitched.

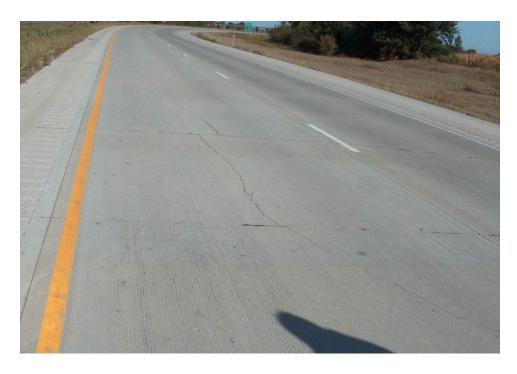


Figure 49 – Woodbury I-29 NB Longitudinal Cracking – passing lanes.

Two similar projects were let the next construction season on I-29 in Fremont and Mill counties. Fortunately, the same contractor placed those overlays and developed a spreadsheet with pavement depths in order to assure the longitudinal joints were sawed at T/3.

## **Patching Overlays**

Patching of PCC overlays falls into to two categories, patching the overlay and full depth patching. When there are just a shattered panel or two, the overlay itself may be patched. However, many times the reason for the shattered panel is because the overlay is typically less than design thickness at that location. If may be possible to remove some of the HMA, if thick enough, to achieve a thicker patch replacement. If the HMA is thin or has deteriorated, a full depth patch may be required.



Figure 50 – Shattered panel. Typically overlay is thin in these areas.

If panels are moving, a full depth patch to the bottom of the existing pavement is recommended to prevent further movement. Many times, the old existing pavement may have longitudinal steel making it difficult to saw through the old pavement at the depth below. On the Clay/Dickinson US 71 project, the patch was pinned to the existing pavement below. These overlays should continue to be monitored to see if the panel movement has reduced significantly.

Full depth patches were placed approximately every 500 to 1000 feet on the Osceola IA 9 project in 2023 to prevent movement. West of US 59 the pavement seemed to exhibit more movement that east of US 59. A saw and seal project were also let on the Osceola IA 9 project. This project should be monitored for performance after the patching and sealing project.



Figure 51 – Full depth PCC patch in overlay



Figure 52 – Patch, existing original pavement left in place and pins.

Regardless of the type of patching, it is imperative to not add room on the ends to the patch. This allows more movement of the panels, leading to further patching issues. Patches should be placed tight to the next panel.



Figure 53 – Panel placed with1 inch gap. Do NOT leave space.

### **Observations and Conclusions**

Based on review of overlays on the interstate and primary system, the following observations were noted:

- Nearly all whitetopping overlays are in good condition, with minimal cracking and very little panel movement.
- There is substantially less cracking when the overlay is placed the same width as the existing pavement.
- Longitudinal joints sawed less than T/3 based on concrete placed causes cracking.
- The 60-inch reinforcing steel over existing widening and tie shoulders has caused longitudinal cracking on nearly all overlays where it was used.
- There are less issues with cracking when the smaller 36-inch reinforcing steel is used.
- Condition of the HMA interlayer may impact project progress if issues are found during milling.
- Unfilled joints cause issues with incompressible material and may be the cause of buckling with panel movement.
- Leaving room for movement with patches promotes further panel movement.

### Recommendations

Based on observations during review of concrete overlays on the interstate and primary system, the following recommendations are discussed below:

- Place overlay same width as existing pavement.
- Use 6-inch minimum thickness.
- Use maximum size reinforcing steel of #4 x 36-inch length.
- Use macro fibers at 4 lbs./cy. Fibers help reduce cracking and panel movement.

- When fibers used, joint spacing may increase to 12 x 12 ft on 6-inch for whitetopping overlays and 9 x 8 ft with 7 x 8 ft. shoulders on 6-inch unbonded overlays with existing widening (18-foot original pavement).
- · Fill all joints to prevent infilling.
- Saw longitudinal joints T/3 based on thickness placed.
- Use full depth transition sections to existing pavement and overlay type change.
- Use full depth patches at locations with panel buckling to prevent further movement.

Since there have been some more recent changes with rehabilitation and designs with fibers, it is recommended to monitor the following projects to see how these changes impact long term performance.

- Osceola IA 9 overlay with full depth patches and joint filling.
- Tama/Blackhawk US 63 overlay with fibers and no reinforcing steel.
- Marshall IA 14 overlay, reinforcing stapled over old widening unit and untied shoulders.
- Plymouth IA 3 overlay with fibers and shoulders tied.

The National Concrete Pavement Technology Center has developed several guides for developing an overlay design<sup>7</sup> and specification requirements<sup>8</sup>. Use of these guides as well as recommendations found during this study should be implemented to improve overlay performance.

### References

<sup>1</sup>Betterton, R.H. and Knutson, M.J., 1973, *Five Year Performance Study of Fibrous Concrete Pavement Overlay Research in Greene County, HR-165*, Iowa Department of Transportation, Ames, Iowa

<sup>2</sup>Gross, J., King, D., et al, 2017, *Concrete Overlay Performance on Iowa's Roadways*, *IHRB TR-698*, National Concrete Pavement Technology Center, Iowa State University, Ames, IA.

<sup>3</sup>Fick, G. and D. Harrington. 2014. *Performance History of Concrete Overlays in the United States*. National Concrete Pavement Technology Center, Iowa State University, Ames, IA.

<sup>4</sup>Cable, J. K., J. M. Hart, and T. J. Ciha. 2001. *Thin Bonded Overlay Evaluation Construction Report*. Civil and Construction Engineering, Iowa State University, Ames, IA.

<sup>5</sup>Cable, J.K., et al, *Design and Construction Procedures for Concrete Overlay and Widening of Existing Pavements, TR-511*, 2005, Center for Portland Cement Concrete Pavement Technology, Iowa State University, Ames, Iowa

<sup>6</sup>Calvert, G., Lane, O.J., Andersen, C., 1990, *Thin Bonded Overlay with Fast Track Concrete, HR-531*, lowa Department of Transportation, Ames, Iowa

<sup>7</sup>Harrington, D. and G. Fick. 2014. *Guide to Concrete Overlays, Third Edition.* National Concrete Pavement Technology Center, Iowa State University, Ames, IA.

<sup>8</sup>Fick, G. and D. Harrington. 2015. *Guide Specifications for Concrete Overlays*. National Concrete Pavement Technology Center, Iowa State University, Ames, IA.

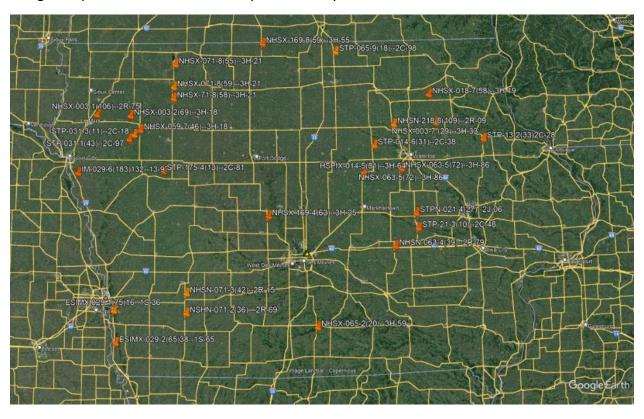
## Acknowledgements

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# Appendix A – Iowa Primary Overlays Project Information

## Google Map of Interstate and Primary PCC Overlays



# Interstate & Primary Overlay Project History Table

	CONTRACTOR	000	KOC	200	MAN	OPC	₽	cvc	) ) )	220	FOC	FJ	cvc	MAN	5 <u>5</u>	MAN	cvc	FLY	200	MAN	GOD	cvc	FLYNN
	PROJECT NOTES	10"x24"	3;replaced no later than 1990 (new PCC)	10"x12.5'; overlaid with 4" HMA in 2009	4; overlaid with 4" HMA in 2007	4" Bonded Fast Track RESEARCH Project	6; replaced no later than 2005 (new PCC)	5.0 Cl.C; replaced sometime after 2011 (new PCC)	5.0 cl.F; replaced 2011?	5.0 Cl.C; replaced 2011?	5.0 Cl.C; replaced 2003? (New PCC)	5.0 Cl.C; SB lane replaced sometime after 2011 (New PCC)		Var. x24'; DOT RESEARCH Project Thin sections resurfaced	3.5-4.5"x.28" RESEARCH Project	8" 24" 3" milled	4.5" 28" 1985 HMA widening w eng. Fabric over joint	8" x 32" (12" with 4" tied lanes) 3" Milled	2" Miled 9" 26', FD inside Shldr. paved integral. 7" Out Shldr. 1d' driving lane over 12' existing & Shlder.	8.5" x 32' (12' with 4' tied lanes)	1" HVA Interlayer 9" x 26" (6' tied FD PCC Shoulders 10 Pt HVA outside) CD loints	HMA Milled Off, 1" New HMA 9" x 26" (6' tied PD PCC Shoulders + 10ft HMA outside). CD Joints	5.5'%28' (4.5%4.5%5' long joints) Rt bars 6' @30" CTR 9.5' x 2' Widening
	JOINT SPACING (ft)	20	20	20	20	20	20	20	20	20	20	20	20	Various	4 to 6?	14	7	41	15	14	15	15	'n
	OVERLAY THICKNESS (in)	10	m	10	4	4	g	s.	50	N.	5	5	m	2,4,6	3.5 to 4.5	10	4.5	69	56	8.5	on .	on.	5.5
	ORIG. THICK/ WIDTH				10" PCC							8" PCC	9" PCC	3" HMA 1964 0.5" HMA 7" CTB 1961	3" HIVA 1982 2" HIVA 1964 10-7-10 - 18'	2" HMA 1981 9" HMA 24' 1972	4" H MA 24' 1986 10-7.5-10 -20' 1939	2" HMA 1981 9" HMA 24' 1972	3" HMA 1989 10" -24" PCC 1959	11.5" 24" HMA	8" -24" CRC/4"CTB	5" HMA 8" CRC-24" /4" ATB	1.5" HMA 1983 3" HMA 1961 3' PCC Widen 1956 10-7-10-18'
	OVERLAY CONST.	1979	1979	1981	1984	1986	1988	1989	1990	1990	1991	1992	1994	1994	2002	2006	2006	2007	2008	5005	2009	2009	5005
CC Overlays	ORIG CONST.		1966		1964	1949	1966	1972	1973	1973	1967	1967	1970	1961	1937	1972	1939	1972	1959	1998	1972	1971	1931
Primary P	S.Y.		63,246		127,900	95,773	53,600	137,390	19,212	142,863	65,259	111,169	40,260			177,035	122,246		236,138	279,940	66,170	102,473	136,497
terstate &	LENGTH (Miles)	5.000	4.492	8.100	9.084	6.800	4.134	7.250	1.000	7.230	4.022	6.052	2.900	7.000	9.545	12.574	8.577	10.560	14.144	7.750	4.240	5.700	8.700
lowa DOT Interstate & Primary PCC Overlays	DESCRIPTION	L80 WBL MP 93.86 to MP 97.25	H80 EBL Avoca to Shelby	H80 EBL (rt. Lane only) MP 85.75 to MP 98.86	H80 fr 1 mi. E of IA 145 E approx. 9 mi. to 1.25	U.S. 71 - JCt. IA 3, to Jct. IA 10	_	F280 SBL fr S if IA22, N to US 6	H280 [NBL] fr H80 SE to Mississippi river bridge	H280 [NBL] fr H80 SE to Mississippi river bridge	H35 N of intrchg, with Co. rd D41 N to intrchg, IA	H35 fr 2 mi. N of IA175 interchg., N approx. 1 mi. N of Co. rd D41 intrchg.	From .3 mi. W. of JCT S41, E. 1.8 mi. & 2 locs. At H35 intrchg.	IA 21 fr JCt. of US 6 N to JCt. of IA 212	IA 13 fr NCL of Manchester N to IA 3	U.S. 71 from US34 North to Cass Co. line	IA175 from Ida Co. line E to W. Jct US71	U.S. 71 from Montgomery Co. line N. to Co. Rd G- 43	F29 from Monona Co . Line N to Sergeant Bluff Interchange (NBL)	U.S. 218 Along Waverly By-Pass (MP 198.57 to 206.16)	1-29, from 1.5 mile N. of Co. Rd. 1-26 N. 1.5 mile N. of Co. Rd. 1-24 (PCC overlay NBL/HMA presurface SR)	P29 from 3 miles N. of N. Jct US34 N. to Pottawattamie Co. line (NBL)	IA 9 from E. of relocated IA 60 E. to W. of Jct Co. Rd L-38
	Route	1-80	85	F80	F80	U.S. 71	087 180	F280	H280	H280	H35	H35	IA 3	IA 21	IA 13	U.S. 71	IA 175	17.2.U	F29	U.S. 218	F29	F29	9 6
	PROJECT NO.	HR-80-2/8208514-01	HEACIR-80-1(126)	IM-80-2(89)8614-01	IR-80-5(106)	FR-71-7(32)		IR-280-9(97)	IR-280-8(98)	IR-280-8(98)	R-35-5(54)13312-40	IR-35-5(56)13312-40	NHS-3-5(50)-19-35	STP-21-3(10)-2C-48	STP-13-2[33]2C-28	NSHN-071-2(36)—2R-69	STP-175-4(13)20-81	NHSN-071-3(42)2R-15	IM-029-6(183)132-13-97	NHSN-218-8(109)2R-09	ESIMK-029-1(75)16-15-36	ESINK-029-2(65)3815-65	STP-009-2(21)-2C-72
	County	Adair	Pottawattamie	Adair	Poweshiek	Buena Vista	Pottawattamie	Scott	Scott	Scott	Hamilton	Hamilton	Franklin	ewoj	Delaware	Montgomery	38 28	Cass	Woodbury	Bremer	Fremont	Mils	Osceola
	District County	4	4	7	1	3	4	9	9	9	1		2	9	9	4	m	4	m	2	4	4	2
	Overlay Type	WT	BOL	WT	BOL	BOL	BOL	BOL	BOL	BOL	BOL	BOL	BOL	WT	NBOL	W	UBOL	WT	UBOL	WT	UBOL	NBOL	UBOL
	Existing Pavement	HMA	5	НМА	PCC	22	PCC	5	SC	SC C	202	PCC	PCC	НМА	Comp	НМА	Comp	НМА	Comp	HMA	Comp	Comp	Сотр

# Interstate & Primary Overlay Project History Table (continued)

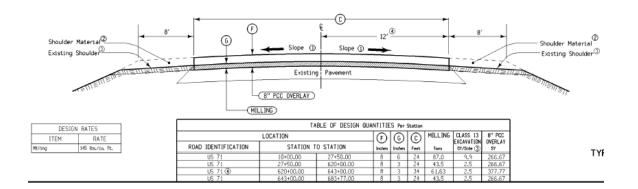
			iale &		i y Ove	eriay Fi	oject miste	ny rabie (t		,	
	cvc	cvc	cvc	MAN	MAN	OVC	O/C	0/C	Croell	Croell	Croell
	6" x 32' (9 x8' panels mainline & 7x8' panels shoulders) UBOL #41 bar 36" @ 60" UBOL 2" Miling	6" x 32" (6 x 6" ML and 4 x 6" Shider) WT #4 bars 36" @ 30" CTR WT 2" Milling	6" x 32' (9 xs" panels mainline & 7xs" panels shoulders; UBOL nt bars 36" @ 60" UBOL 2" Milling	6" x 22" (6 x 6" ML and 4 x 6" Shider) #4 bars 36" @ 30" CTR 2" HMA Bondbreaker	7" x 32' (12 x 12' ML and 4 x 12' Shider) #4 bars 36" @ 60" CTR Geotextile Interlayer	6' x 32' (6 x 6' ML and 4 x 6' Shider) 1" milling Shoulder/Widening not tied	© x 32' (6 x 6' ML and 4 x 6' Shider) # x 36' (6 x 6' ML and 4 x 6' Shider) # x 36' (6 x 6' ML and 4 x 6' Shider) File in the control of the	Fibers 4 lb/cy 3-Willing Euclid Tuf Strand SF Fibers #4 x 36° @ 30°c-c	6" x 32' (6 x 6' ML and 4 x 6' Shider) #4 x 36" @ 30"c-c 2" Milling 12 x 12 test section	5" x 32' (6 x 6' ML and 4 x 6' Shider) 3" Milling Fibers 5 lb/cy - Forta Ferro	6" x 36 (12 x12" ML& 6 x12" Shidr 2" Milling #4 x 36" @ 30"c-c Fibers 4 lb/cy Forta Ferro
	00	9	9	9	12	9	ω	12	9	9	12
	φ	9	9	5.5	7	ø	φ	ø	9	5	9
	1" ASC 1976 3" HMA 1962 10-7-10-18" 1931	3" HMA 9.5" TBB 1993	1" ASC 1976 3" HWA 1962 10-7-10 -18" 1931	2" HMA Interlayer 2016 8" PCC	3" BAC 1987 PC7 1940	3.5" HMA 1991 1" 1979 1.5" 1959 PC7 1932	4.5" HMA 1997 BSC 1986 2.5" BAC 1967 3" AAC, 6"BTB, 6"SAS 1955	4.5" HMA 1997 BSC 1986 2.5" BAC 1967 3" AAC, 6"BTB, 6"SAS 1955	2" HMA 2009 11" HMA 1995	4" HMA 1998 9" PCC 1973	2" HMA 2009 11" HMA 1995
	2015	2016	2016	2016	2018	2019	2020	2022	2022	2022	2023
CC Overlays	1931	1931	1931	1979	1940	1932	1955				
Primary P(	138,911	123895	123895	62080	239362	155401	137007	162607	181506	25286	75050
erstate &	7.740	6.95	6.95	3.46	13.02	11.28	82	10.17	8.72	13.14	5.36
lowa DOT Interstate & Primary PCC Overlays	US 71: From Co. Rd. B53 N to 15th	US 71: Buena Vista Co. Line N. to Co. Rd. B53	US 71: Buena Vista Co. Line N. to Co. Rd. B53	IA 21; From E66 N. to just S. US 30	US 169 From IA 141 N to US 30	IA 14 From Co. Rd. E18 N. to W. JCT IA 175	IA31 Fram E. JCT US 20 N. to Co. Rd. C66	Washta to US 59	Lemarsto Remsen	From Traer to 0.5 mi S of IA 58	Remsen to Cherokee Co Line
	U.S.71	US 7.1	US 7.1	IA 21	US 169	IA 14	IA 31	IA 31	IA 3	883	IA3
	NHSX-071-8[59]-3H-21	NHSX-71-8(58)3H-21	NHSX-71-8(58)3H-21	STPN-021-4(27)21-06	NHSX-169-4(63)-3H-25	HSPIX-014-5[81]3H-64	STP-031-1(43)2C-97	STP-031-3(11)2C-18	NHSX-003-1(106)2R-75	NHSX-063-5(72)-3H-86	NHSX-003-1(104)2R-75
	Clay	Clay	Clay	Benton	Dallas Boone	Marshall Grundy	Woodbury	Cherokee	Plymouth	Tama	Plymouth
	m	m	m	φ	4 11		m	m	m	1	m
		UBOL & WT		UBOL	UBOL	NBOL	IM.	TW.	TM	UBOL	WT
		Comp HMA		Comp	Comp	Comp	НМА	НМА	НМА	Comp	НМА

# Interstate & Primary Overlay Project History Table (continued)

Г													
	FLYNN	MAN	MAN	MAN	GPI	Croell	MAN	Croe	Croell	כאכ	KNIFE	FLN	cvc
	5' x 32' (5" x24' & 4' x 8" Widening) #4 bars 6' @ 30" CTR 4.5w4.5w7ft long JCT 0.5" Willied - Paved half width at time	4' x 32' (4" x 34' & 4' x 8" widening) #5 bars 6' @ 30' CTR 0.5" Miled 2%	6" x 24' (6' out side, 4' inside 8" PCC Shoulders) Shoulders tied with #5 x 6' @30" CTR 11/2 to 21/2" Milling	67.24 (6 outside, 4 inside 8" PCC Shoulders) Shoulders tied with #5 x 6' @30" CTR	4.5° x 30' with safety edge #5 bars 3' @ 30" CTR 5.5 x 5.5 x 4' long jts 1. HMA A'va 5" widenine	5" x 32" (5" x 24" & 4" x 9.5" widening) #5 bars 6" @ 30" CTR 4.5 x 5 @ 18" & 7" x 5" outside 1 1/2" Milling	5' x 32' (5' x 22' & 5' x 9" widening) #5 bars 36" @ 33" CTR 5.5 x 5.5 x 5 y long its 1' HMA	5' x 32' (5' x 24' & 4' x 8' widening) #5 bars 5' @ 30" CTR 5 x 5 x 6' long its Scarification to 2% cross slope	5' x32' (5" x20' & 6' x 9.5" widening) #5 bars 36" @ 30" CTR 5 x 5 x 6' long its Closed during paving	6" x 44" (6 x6" panels mainline & 5x5" panels shoulders) 6x6x5x5 long its 8x1 bars 6, 69 30" CTR	6" x32" (5 x6" ML and 6 x 6" Shider) 5 x 5 x 6" long its 44 bar 5" (9 20" CTR	5' x 36' (5" x 20' & 6' x 9.5" widening) #5 bars 36" @ 30" CTR 5 x 5 x 6' long jts Milled AC surface-Closed during paving	6"x32" (6x6" NL and 4x6' shider) WT #4 bars 36" @ 30" CTR WT 2" Milling
	s.	'n	<b>o</b>	ø	2.5	s.	5.5×5	ın	ın.	9	φ	IN.	v
	5	4	9	9	45	5	s.	S	s	9	9	s	Q
	2" HWA 1978 3" HWA 1961 1928-9 10-7-10 - 18'	3" HWA 1984 3" HMA widen 1978 3" HWA 1966 10-7-10 20' 1938	3" HWA 1992 3" HWA 1984 2" HWA 1969 6" PCC 1954	11" HMA 1992	2" HWA 1994 VL 8" PCC 1954	1.5" HMA 1984 3" HMA 1964 10-7-10 18' 1930	10-8.5-10 22' 1954	3" HMA 1994 & HMA Widen 3" HMA 1978 10-7.5-10 20'	3" HMA 1988 10-7-10 20' 1936	4.5" HMA 1991 3" HMA 1974 10-7.5-10 20' 1937	10-7-10 20' 1937	0.5" HMA 1962 1.5" HMA 1956 3" HMA 1949 8" PCC 1921 18"	3" HMA 9.5" TBB 1993
	2009	2011	2012	2012	2013	2013	2013	2014	2014	2014	2015	2015	2015
CC Overlays	1928-9	1938	1954	1992	1954	1930	1954	1940	1987	1991	1937	1921	1931
Primary F	192,078	350,267	362,471		119,469	154,787	154,994	144,302	132,579	309,384	145,951	124,630	138,911
erstate &	10.960	19.000	9.000		6.910	10.410	8.750	7.882	7.315	14.520	7.730	4.980	7.740
lowa DOT Interstate & Primary PCC Overlays	U.S. 65 from IA 9 in Manly N. to Co. Rd 105	U.S. 18 from ECL of Fred ericksburg E. to West Union	U.S. 71 from US 18 North to SCL of Milford -NBL	U.S. 71 from US 18 North to SCL of Miford -58L	IA 14 from .5 mile South of US 20 North to IA 57	IA 3 from West county Line to North City Limits of Oelwein	U.S. 65, From Wayne Co. Line N. to US 34	U.S. 169, E. Jct. IA9 N to Minnesota State Line	US 63 from NCL Monteauma North to Just south of I-80	US 59 from IDA Co. Line N. to IA 3	lowa 3: Plymouth co to ECL of Cleghorn	US 69: From Just N of 118th Ave N. to Just 5. of IA 210	US 71: From Co. Rd. B53 N to 15th St in Spencer
	U.S. 65	0.5.18	U.S.71	U.S.71	IA 14	হ ধা	U.S. 65	U.S. 169	U.S. 63	U.S. 59	E A1	U.S. 69	U.S. 71
	STP-065-9[18]2C-98	NHSX-018-7[58]-3H-19	NHSX-071-8[55]3H-21	NHSX-071-8(55)3H-21	STP-014-6(31)-2C-38	NHSX-003-7 (29)—3H-33	NHSX-065-2[20]-3H-59	NHSX-169-8[59]3H-55	NHSN-083-4(39)-2R-79	NHSX-059-7[46]3H-18	NHSX-003-2[69]3H-18	STPN-069-4[100]21-77	NHSX-071-8(59)-3H-21
	Worth	Chickasaw	clay	Clay	Grundy	Fayette	Lucas	Kossuth	Poweshiek	Cherokee	Cherokee	Polk	clay
	2	7	m	m	н	2	'n	7		m	m		m
	NBOL	UB OL	WTNB	UBOL SB	UBOL	NBOL	UBO.	UBO.	ngor	UBOL	UBOL & WT	ngor	UBOL & WT
	Comp	Comp	Comp	нма	dwo	camp	ompo	dwoo	dwo	Comp	Сотр НМА	Comp	Comp

Appendix B - Primary Overlays Project Construction History and Review

Year	2006	Overlay Type	Whitetopping
County	Montgomery	Design	8" – 24 ft. CD 14 ft. HMA
			Shoulders
Route	US 71	Milling	3" milling
Project	NSHN-071-2(36)—2R-69	Interlayer	Existing HMA
Location	U.S. 71 from US34 North	Tie Bars	n/a
	to Cass Co. line		



#### Notesc

- $\bigcirc$  Forished slope shall match existing pavement except that the maximum allowable slope is 3.0  $\times$ , minimum allowable slope is 2.0  $\times$ . Section may be modified as directed by the Engineer through areas of special shaping.
- Shoulder material as specified elsewhere in these plans. See Typical 7153M for more details.
- ① Top 4" of existing shoulder material will be removed from 80P to Sta 27+50. Top 1" of existing shoulder material will be removed from Sta. 27+50 to 60P. Shoulder will be reconstructed as per Typical 7153M. Per side per station.
- $\textcircled{\scriptsize 40}$  10' clumbing lane on right. Includes 400' tapers at each end.

Construction

Some issues with HMA failing during construction. Several areas patched with new HMA.







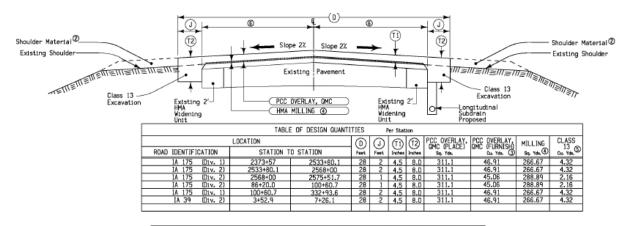
## Review 2023

Overall, in good condition. A few areas with cracking just off centerline (~4 to 5 locations). Some issues with centerline rumble strips went through transverse joints and were patched.





Year	2007	Overlay Type	UBOL
County	Sac	Design	4.5" x 28 ft. (7 x 7 ft. panels)
Route	IA 175	Milling	½" at CL – 2% cross slope
Project	STP-175-4(13)2C-81	Interlayer	Existing HMA
Location	Ida Co. line E to W. Jct	Tie Bars	None
	US 71		

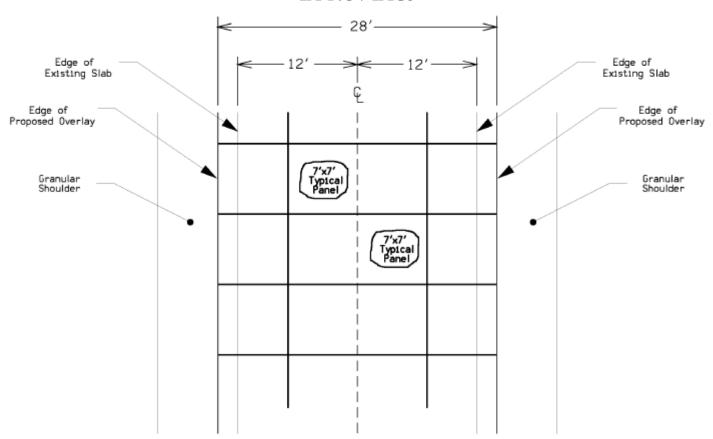


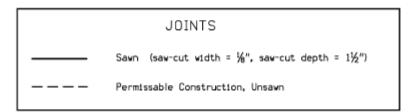
Notes

Refer to tabulation listing of superelevated curves and Standard Road Plans for additional requirements through superelevated curves. 2610 MODIFIED

- Shoulder naterial as specified elsewhere in these plans; refer to typical 7:35 on 8.01 for "Type '8' Granular Surfaced Shoulders."
- ③ Quantity includes PCC widening units to be placed with PCC overlay and based on 5 inch thickness for irregularities.
- (4) Mill the existing HMA pavement ½" deep at centerline, and to a ZX cross slope except in areas of superelevation and as shown onlyptcal MN-1.
- ⑤ Includes both sides.
- (6) Existing pavement width is 24' except in Div. 2 (corporate limits of Odebolt) where it is 26'.

# IA 175 / IA 39









## 2008 Cracking

Quite a bit of longitudinal cracking showed up the next year after construction. It was noted that the outside shoulders appeared to be heaving based on the cross slope.







Driving lane panel cross slope 2.09%



Shoulder panel cross slope 1.31%

## 2017 Patching Project

Many of the areas needing patching were not at design thickness of 4.5 inches. Note in the picture that the overlay is 3 inches where patching.



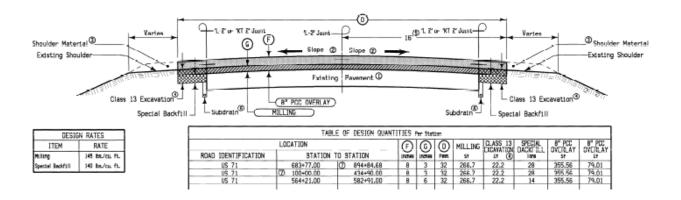
## 2021 Review

After most of the cracking occurred, the district filled the longitudinal cracks with hot pour sealant. Even with all the cracking that occurred early, the pavement continues to ride fairly will. There are likely areas that need to be patched.





Year	2007	Overlay Type	Whitetopping
County	Cass	Design	8" x 32 ft. (12 x 14 ft. CD)
Route	US 71	Milling	3" Milled
Project	NHSN-071-3(42)2R-15	Interlayer	Existing HMA
Location	Montgomery Co. line N. to Co. Rd G-43	Tie Bars	L-2 #5



Modtfled

#### Notes:

- (j) Existing povenent width is 24 ft.
- ② Finished slope shall netch existing powerent except that the next and allowable slope is 3.00 nizimum allowable slope is 2.00. Section may be notified as directed by the Figureer through arrow of special shaping.
- 3 Shoulder material as specified elsewhere in these plans; refer to Typical 7110M.
- 9" depth 4" vide, 6" to be replaced with Special Backfill for subdrain work
  prior to PCC ownlay, (Both sides)
- (5) Includes 4' PCC paved shoulder. Refer to Tysical 22094 for climbing lane details.
- Refer to Typical PCC-4 for details. See Tab 104-9 for locations.
- (T) Equation Sta. 694+894.66 Best Sta. 100+00.06 Alecod
- (ii) Refer to Detail Sheet U.C.3 for details.

### Construction

There were areas where the HMA was in poor condition and new HMA was placed. A shortage of haul trucks caused the paving machine to move slowly waiting for concrete. There are several areas where the grout box material was dropped into the pavement. These areas have major popouts from the light particles in the gravel source.





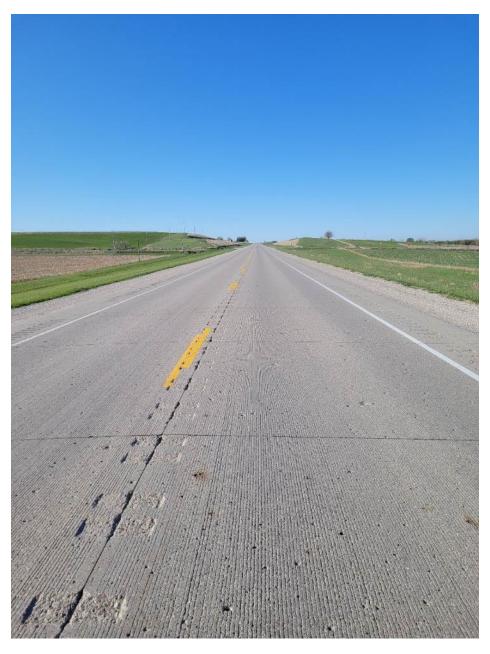


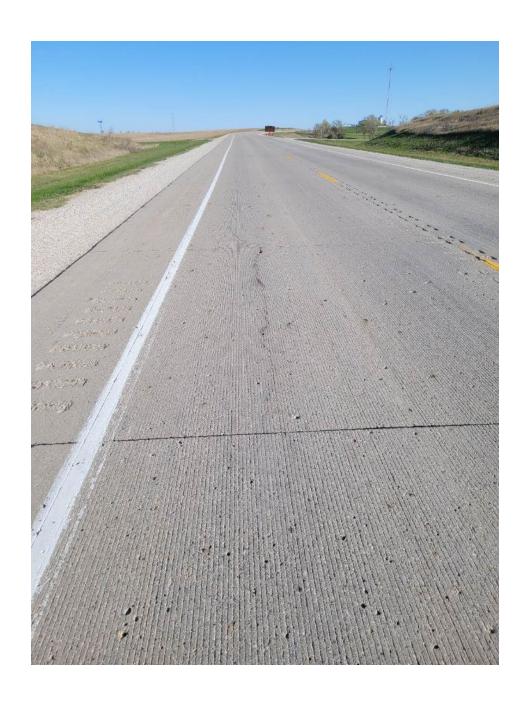




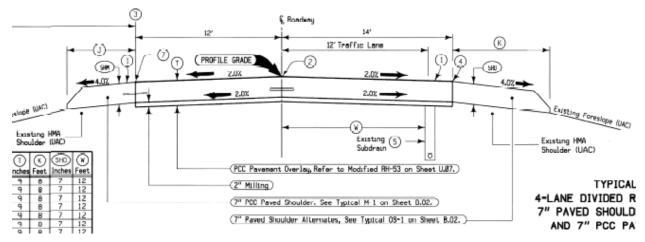
## 2023 Review

Overall, in good condition. There are approximately 3 to 4 areas with cracking at quarter point. There are also areas where milled rumble strips went through transverse joints and blew out edge.





Year	2008	Overlay Type	UBOL
County	Woodbury	Design	9" x 26' FD Inside Shoulder
			integral. 7" Outside Shoulder.
Route	I-29 NB	Milling	2" Milling
Project	IM-029-6(183)13213-97	Interlayer	Existing HMA
Location	Monona Co. Line N to	Tie Bars	#5
	Sgt. Bluff		



- NBML.
- (1) Shoulder rumble strip required. Refer to Sheets U.08 and U.10 for details.
- (2) "L-2" joint required. See Standard Road Plan RH-51.
- 3 The median shoulder may be placed with the 26-ft. wide 1-29 NB mainline payerent.
- (4) For the PCC alternate, use "L-1", "KT-1", or "8T-1"joint. See Standard Road Plan RH-51.
- (5) UAC and protect existing subdrains.
- B) Profile grade for this project is located on the ceterline of the NB lanes.
- Use "L-2", "K-2", or "BT-2" juints. See Standard Road Plan RH-51.

\_\_\_\_

During milling of the HMA, there were a few areas where the HMA peeled off the existing concrete.







Section 2 Cross stitching 9242 feet right, 3763 feet left.



Year	2009	Overlay Type	UBOL
County	Osceola	Design	5.5"x28' (4.5x4.5x5' long joints)
Route	IA 9	Milling	None
Project	STP-009-2(21)—2C-72	Interlayer	Existing HMA Surface
Location	IA 60 E. to L-58	Tie Bars	#4 bars 6' @30" CTR

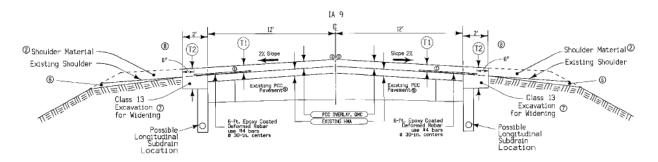
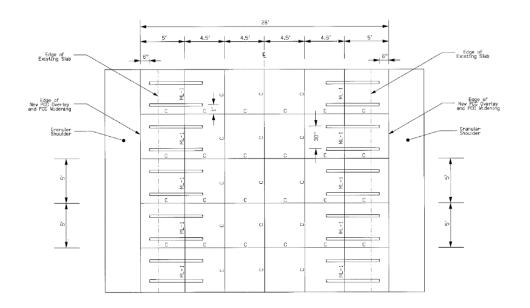


TABLE	OF DESIGN QUANT	ITIES		er Stat	ton			
LOCATION		(0)	(1)	(11)	(12)	PCC OVERLAY,	PCC OVERLAY,	CLASS 13
STATION T	O STATION	Feet	Feat	Inthes	Inches	Sq. Yds.	PCC OVERLAY, QMC (FURNISH) On You (2) 56.79 56.79 56.79	Car Yes.
188+95	360+31.4	28	2	5.5	9.5	311.11		4,94
760+99,47	839+98.21	28	2	5.5	9.5	311.11	56.79	4.94
848+89,11	935+58.5	28	2	5.5	9.5	311.11	56.79	4.94
943+71.5	1043+25	28	2	5,5	9,5	311.11	56.79	4.94
	STATION T 188+95 760+99.47 848+89.11	STATION TO STATION   188+95   360+31.4   760+93.47   839+99.21   848+89.11   935+89.5	STATION TO STATION Feet 188+95 360+31.4 28 760+99.47 839+99.21 28 848+89.11 935+58.5 28	DOCATION   DOCATION   STATION   Text   Feet   Fee	DOCATION   DOCATION   DOCATION   Tent   Docation   Do	DOCATION	DOCATION	DOCUMENT   CONTINUE   CONTINUE

TYPICA UNBONI

### Notes:

- ① Longitudinel joints shall be located at centerline, 4.5-ft Lt. and Rt. of centerline and 9.0-ft. Lt. and Rt. of centerline. A nodified 1-ft Nt.-D joint shall be located at 9.0-ft. Lt. and Rt. of centerline with a 5-ft. long retinforcing bar. Increases joints shall be located at 5.0-ft. spectrgs.
  - Each G-Ft, rebar shall be attached to the existing HMA surface in at least 3 locations. Connecters shall be ne less than 12-bn, apart and ne more than 15-bn, apart. See Sheet UDI For a plan value of the required joint layout.
- Shoulder naterial as specified elsewhere in these plans; refer to typical [7135] on Sheet B.1 For "Type 16" Granular Surfaced Shoulders."
- 3 Quantity is estimated using a 0-in, thickness over the existing 24-ft, wide parement and a 10-in, thickness for the integral 2-ft, vibrating units.
- ① Includes both sides.
- (5) Existing total pavement width is 24'.
- (a) Earth Shoulder Construction: Place and compact Class 13 Exception in this area to build up the shoulder and eliminate the accordary ditch along the edge of the editing shoulder. Slope shall be adjusted as directed by the Engineer.
- If My existing HMA eigerut or fillet naterial within 2-ft, of the edge of the existing personnel shall be considered Class 13 Exception except at paved county read intersections.
- (S) Existing HMA fillet material that is located 2-ft. or more from the edge of guisting pavesers will be tabulated as Additional Diese L3 Desivation at Entrances and Gravel Road Intersections. See Tab AUGO, L3 on Sheet C.8.



Construction

Some issues with late sawing causing cracking off end of saw joints.





Panels moving, especially west of US 59. Maintenance placed patches with foam on either said to accommodate slab movement. We noted that now the slabs will move even more. District will let a patching project to ad full depth patches in areas to prevent slab migration.







Longitudinal cracking

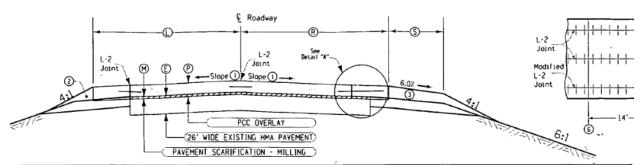


PCC overlay in better condition east of US 59.

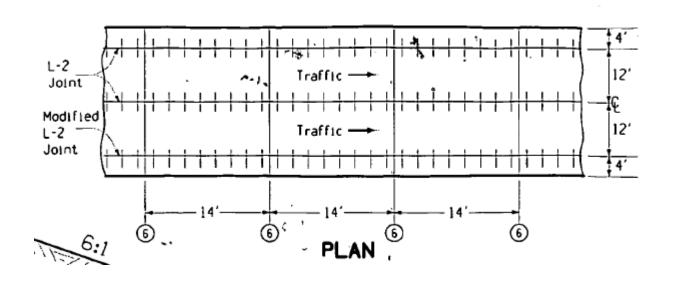


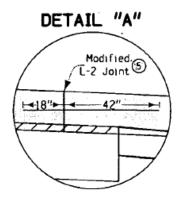
Project let in 2023 included 4293.3 SY of 10-foot x 28-foot x 17-inch full depth patching and 8.9 miles of joint sealing for the entire project. Full depth patches were selected try to stop panel movement.

Year	2009	Overlay Type	WT
County	Bremer	Design	8.5" x 32' (12' with 4' Shoulder.
Route	US 218	Milling	Scarify Existing
Project	NHSN-218-8(109)2R-09	Interlayer	Existing HMA
Location	Waverly Bypass	Tie Bars	Modified L-2 5' bar



		Design	Quant	ities	<b>①</b>						• Not a Bid Item
Location					(H)	(L)	®	(\$)	PCC Pavement	*Pavement Scarification	Pavement Scarification
Road [dentification	Station T	o Station	Inches	Inches	Inches	Feet	Feet	Feet	Sq. Yds.	Sq Yds.	Tons
Northbound Lanes											
U.S. 218	319+44.00	328+25.00	8.5	11.5	1.5	16	16	6	3132	2838	223
U.S. 218	336+75.00	496+66.38	8.5	11.5	1.5	16	16	6	56,858	46,197	3538
U.S. 218	506+25.00	514+52.97	8.5	11.5	1.5	16	16	6	2944	2392	188
U.S. 218	523+36-66	550+63.78	8.5	11.5	1.5	16	16	6	9696	7878	227
U.S. 218	559+64.78	658+85.00	8.5	11.5	1.5	16	16	6	35,236	28,629	2255
U.S. 218	666+85.00	713+85.00	8.5	11.5	1.5	16	16	6	16,711	13,578	1069
Southbound Lanes											
U.S. 218	311+26.00	496+66.38	8.5	11.5	1.5	16	16	6	65,921	55,854	4399
U.S. 218	506+25.00	514+83.43	8.5	11.5	1.5	16	16	6	3052	2480	195
U.S. 218	523+67.12	550+63.78	8.5	11.5	1.5	16	16	6	9588	7790	613
U.S. 218	559+64.78	658+85.00	8.5	11.5	1.5	16	16	6	35,236	28,629	2255
U.S. 218	666+85.00	713+85.00	8.5	11.5	1.5	16	16	- 6	16,711	13,578	1069
		TOTALS							255,085	209,843	16,131

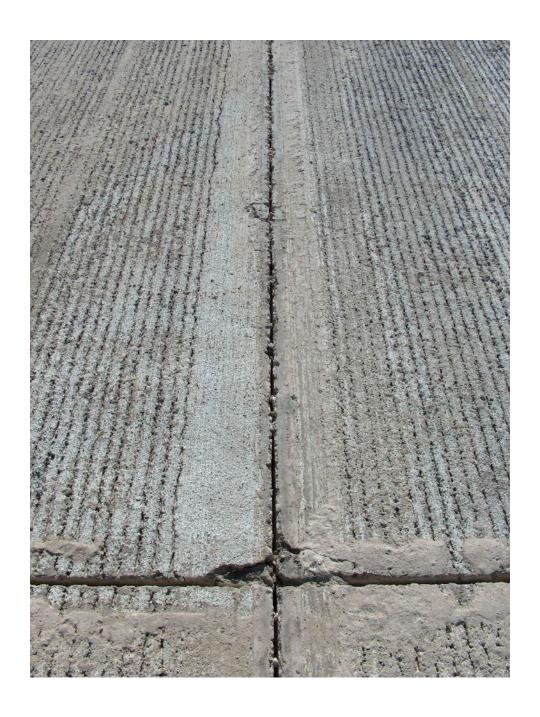




# Construction







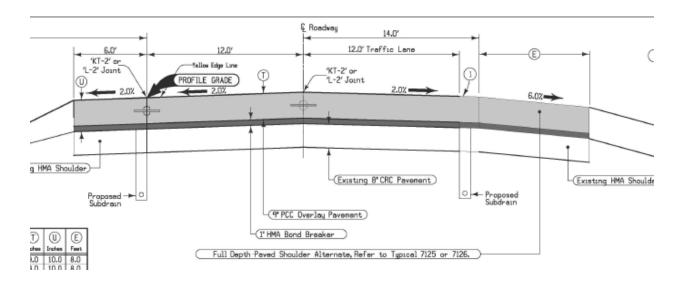
Overall, in good condition. An area on the north end of the southbound lanes exhibits some issues at the joints. There are a few random areas of longitudinal cracking in the driving lane.





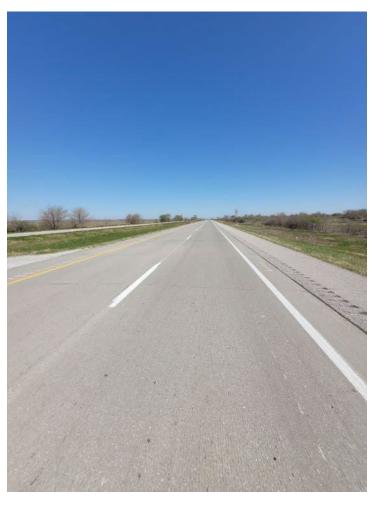


Year	2009	Overlay Type	UBOL
County	Fremont	Design	9" x 26' (8' & 6' HMA
			Shoulders) CD
Route	I-29 NB	Milling	n/a
Project	ESIMX-029-1(75)161S-36	Interlayer	1" HMA New
Location	1.5 mile N. of Co. Rd. J-26 N. 1.5 mile N. of Co. Rd. J- 24	Tie Bars	L-2



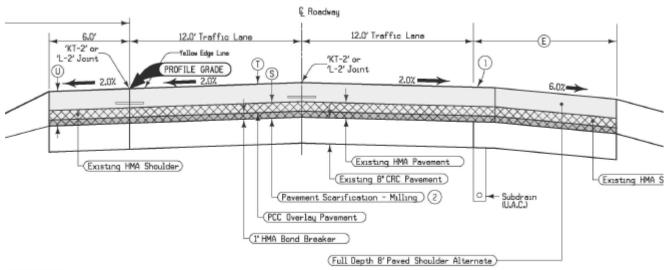
lt	LOCATION	LOCATION NORTHBOUND LANES							(E)
IL	ROAD IDENTIFICATION	STATION T	D STATION	Feet	Feet		Inches	Inches	Foot
	I-29 NBL	886+00	1016+00	64	UAC		9.0	10.0	8.0
Ш	I-29 NBL	1020+00	1043+50	64	UAC		9.0	10.0	8.0
Ш	I-29 NBL	1047+30	1120+00	64	UAC		9.0	10.0	8.0
	NOTE: Station equat	ion: Sta. 975+23	.92 BacK = Sta.	9851	48.4	0 Ahe	ad.		

2023 Review
Pavement in very good condition. No issues with cracking.





Year	2009	Overlay Type	UBOL
County	Mills	Design	9" x 26' (8' & 6' HMA
			Shoulders) CD
Route	I-29	Milling	4-5" Existing HMA removed
Project	ESIMX-029-2(65)381S-65	Interlayer	1" New HMA
Location	3 miles N. of N. Jct US34 N. to Pottawattamie Co.	Tie Bars	L-2
	line		

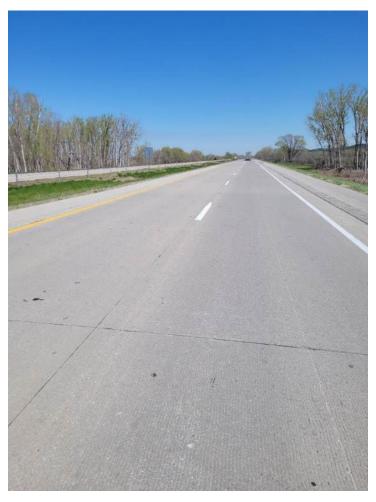


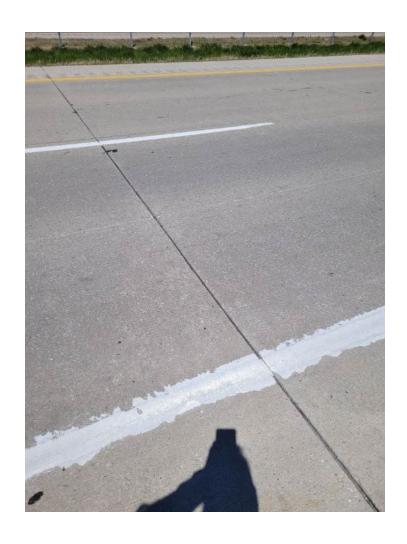
	_	
25.	00	(3)
	I (III) I	(E)
1		(
		Parent.

LOCATION	LOCATION NORTHBOUND LANES							E
ROAD IDENTIFICATION	STATION T	D STATION	Feet	Feet	Inches	Inches	Inches	Feet
Mills Co.	EQN: STA. 865+69.2	0 = STA. 860+71.80						
1-29 NBL	701+31.00	862+65.16	50	UAC	5.0	9.0	10.0	10.0
	863+58.63	944+44.40	50	UAC	5.0	9.0	10.0	10.0
Pott. Co.		O = STA. DO+DO.OO						
1-29 NBL	00.00	60+24.60	50	UAC	4.0	9.0	10.0	10.0

2023 Review

Pavement in very good condition. No issues with cracking.





Year	2009	Overlay Type	UBOL
County	Worth	Design	5.5"x28' (4.5x4.5x5' long joints)
Route	US 65	Milling	0.5" Milling – Paved Half Width
Project	STP-065-9(18)2C-98	Interlayer	Existing HMA
Location	IA 9 N. to Co. Rd 105	Tie Bars	#4 bars 6' @ 30" CTR

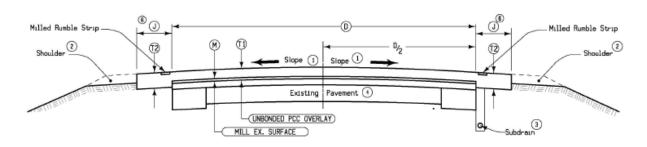


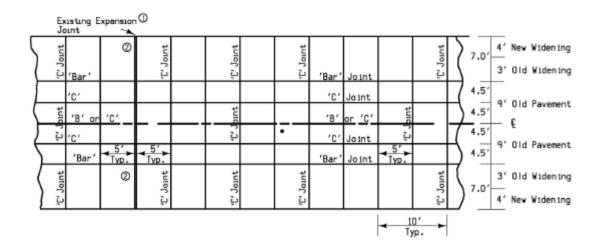
TABLE OF DESIGN QUANTITIES Per Location (5)											
	LOCATION		M	1	0	12	(1)	PCC OVERLAY QMC (Furnish)	PCC OVERLAY QMC (Place)	REMARKS	
ROAD IDENTIFICATION	STATION T	O STATION	Inches	Inches	Feet	Inches	Feet	Du. Yde.	Sq. Yds.	TO THE	
US 65	2329+00.05	2331+25.05	0.5	5.0	24	B.D	4	129.2	800.0	225' to BOP	
US 65	2331+25.05	2434+22,00	0.5	5.0	24	8.0	4	4405.1	28137.9	BOP to 1st Bridge Approach	
US 65	2436+14.00	2447+03.05	0.5	5.0	24	B.D	4	626.0	3872.2	End 2nd Bridge Approach to 1st Reconstruction	
US 65	2462+79.00	2575+61.00	0.5	5.0	24	8.D	4	6460.7	40113.8	End 1st Reconstruction to Town Section	
US 65	2575+61.00	2629+28.00	2.0	5.0	24	B.D	4	2482.0	19,082.7	Town Section	
US 65	2629+28.00	2667+38.00	0.5	5.0	24	B.D	4	2120.5	13,546.7	Town Section to 3rd Bridge Approach	
US 65	2669+28.00	2767+06.00	0.5	5.0	24	8.0	4	5427.7	34,766.2	4th Bridge Approach to 5th Bridge Approach	
US 65	2786+82.99	2908+06.00	0.5	5.0	24	B.D	4	6625.8	43,104.0	End 2nd Reconstruction to 225' from EOP	
US 65	2908+06.00	2910+31.00	0.5	5.0	24	8.0	4	129.2	0.008	225' to EOP	
370th St.	Total Area =	2054.5	-					50.7	228.3	Quantity subtracted to get TOTAL	
390th St.	Total Area =		-	_				50.7	228.0	Quantity subtracted to get TOTAL	
430th St.	Total Area =							50.7	228.3	Quantity subtracted to get TOTAL	
100411 001	TOTAL NICE	200710						5017	22010	additively subtracted to get ToTAL	
TOTAL:								29,569.0	183,448.9		
			_	_							

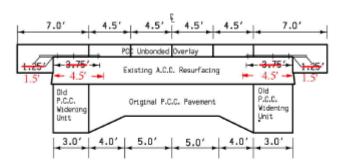
otes: Modified

Finished slope shall match existing pavement except that the maximum allowable slope is 3.0% minimum allowable slope is 2.0%. Section may be modified as directed by the Engineer through areas of special shaping.

Refer to tabulation listing of superelevated curves and Standard Road Plans for additional requirements through superelevated curves.

- Refer to other drawings for details of shoulder design and construction.
- Refer to Standard Road Plan RF-19C. Subdratn on one side only.
- 4 PCC Pavement with PCC Widening Units and HMA Resurfacing
- (5) Quantities include partially paved shoulders based on T2 of 8.5 inches for irregularities.
- (6) See Typicals TL-1, TL-2 and 'J' Sheets for locations where (2) is omnitted for turn lanes.





Notes

- $\bigcirc$  Extend existing expansion joints in kind in new pawement.
- Place OF joint in overlay.

Construction

Overlay was placed half width at a time.





2013 Review

Areas of cracking and broken panels.





2021 Review

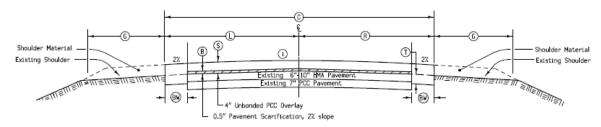
Areas of blowups due to panel movement.





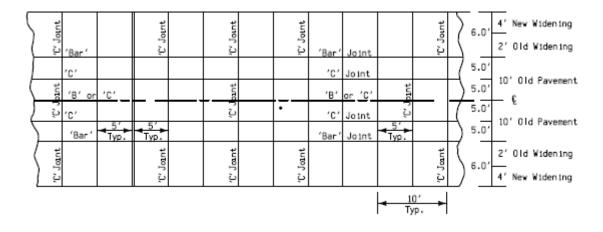
- Patching project let in 2023 with 2536.4 SY of 10-inch full depth patching and 293 patches by count.

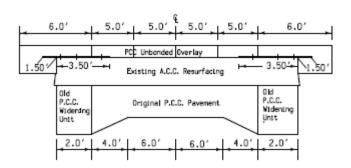
Year	2011	Overlay Type	UBOL
County	Chickasaw-Fayette	Design	4" x 32' (4" x24' & 4' x 8"
			widening) - Paved half width
Route	US 18	Milling	0.5" Milled 2%
Project	NHSX-018-7(58)3H-19	Interlayer	Existing HMA
Location	ECL of Fredericksburg E. to West Union	Tie Bars	#5 bars 6' @ 30" CTR



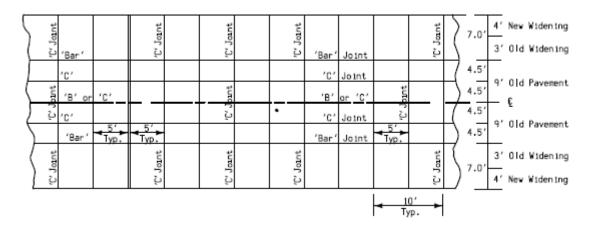
	Location		(§)	B	(a)	(L)	R	Overlay Quant	PCC	(T)	€	6	(5) Granular	
Road Identification	Station To	Station	Inches	Inches	Feet	Feet	Feet	Class 13 Cu. Yds.	Overlay Cu. Yds.	Inches	Feet	Feet	Shoulder	Remarks
US 18	346+55.4	416+37	4	0.5	32	16	16	12.35	49.38	4	4	6	33.97	
US 18	420+08	78+66.0*	4	0.5	32	16	16	12.35	49.38	4	4	6	33.97	
US 18	83+74	99+92.9	4	0.5	32	16	16	12.35	49.38	4	4	6	33.97	
US 18	99+92.9	100+42.9	Vari	0.5	32	16	16	12.35	54.32	4	4	6	35.87	
US 18	100+42.9	108+68.8	5	0.5	32	16	16	12,35	59.26	4	4	6	37.77	***
US 18	108+68.8	109+18.8	Vari	0.5	32	16	16	12.35	54.32	4	4	6	35.87	
US 18	109+18.8	340+33	4	0.5	32	16	16	12.35	49.38	4	4	- 6	33.97	
US 18	346+82	701+87.27	4	0.0	32	16	16	12.35	49,38	4	4	- 6	33,97	

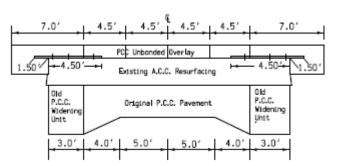
Sta. 346 to W-14





#### W-14 to EOP





Construction

Placed half width other lane open to traffic. Used a modified drop off.







# 2017 Cracking





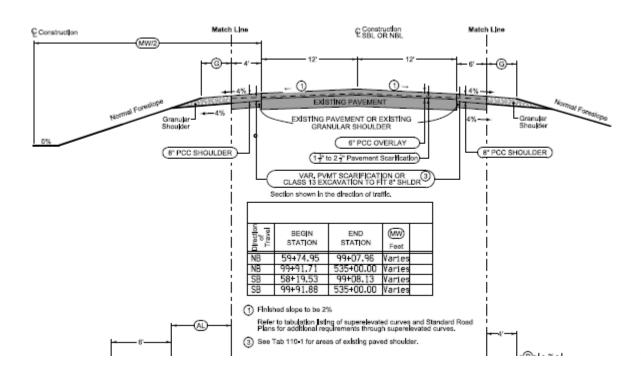




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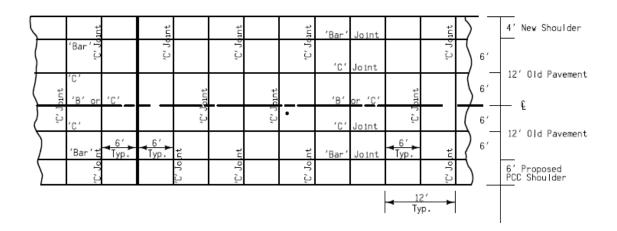
A patching project was let in 2023 with 10,221.09 SY of full depth patching by area and 1680 SY of full depth patches by count. The vast majority of patches were only the 4.5-inch overlay, with a few areas of 10 inch and 17-inch full depth.

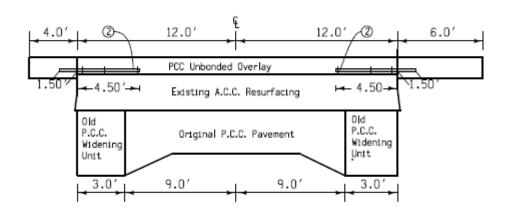
Year	2012	Overlay Type	WT & UBOL
County	Clay	Design	6" x 24' (6' outside, 4' inside 8"
			PCC Shoulders)
Route	US 71 SB	Milling	1 1/2 to 2 1/2" Milling
Project	NHSX-071-8(55)3H-21	Interlayer	Existing HMA
Location	US 18 N to SCL of	Tie Bars	#5 x 6' @30" CTR - UBOL
	Milford		#5 x 3' @30" CTR - WT



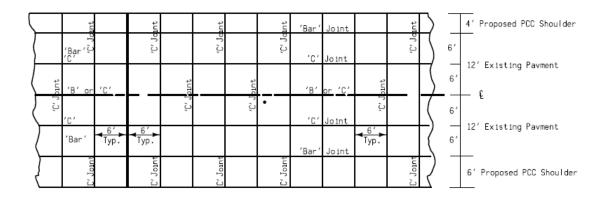
Milepost	Direction	Existing Pavement
		Туре
207.92-212.88	NB	Composite
212.88-217.37	NB	HMA
207.92-212.88	SB	HMA
212.88-214.55	SB	Composite
214.55-215.33	SB	HMA
215.33-216.08	SB	Composite
216.08-217.37	SB	HMA

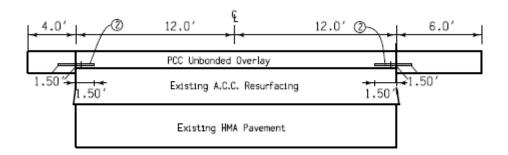
Over existing composite pavement





### Over existing HMA pavement





# Construction





UBOL sections - Longitudinal cracking off end of reinforcing in the outside wheel path. Shoulder heaving. Whitetopping overlay sections in very good condition.







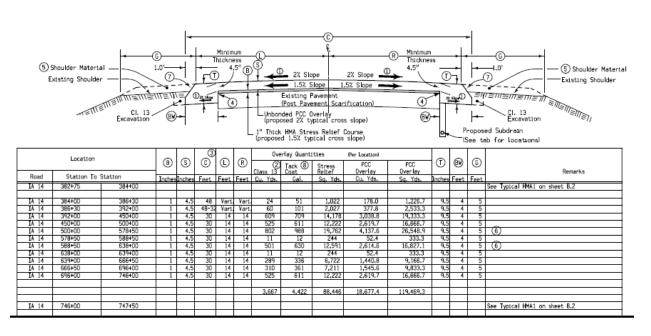
Shoulder cross slope 3.3%, Design 4%.

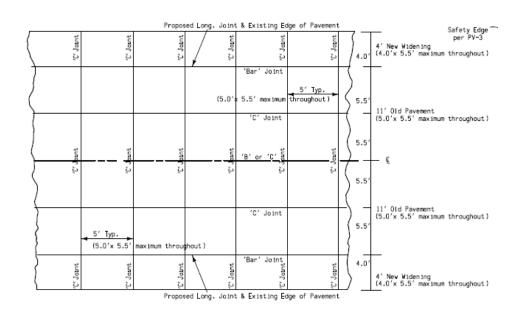


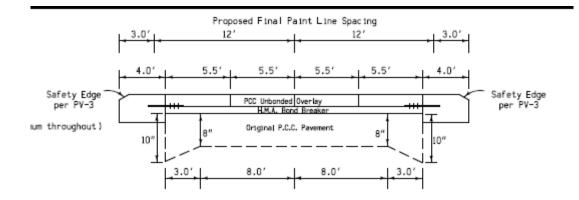


Whitetopping section

Year	2013	Overlay Type	UBOL
County	Grundy	Design	4.5" x 30' with safety edge; 5.5'
	_		x 5.5- x 4' long joints
Route	IA 14	Milling	1" HMA
Project	STP-014-6(31)2C-38	Interlayer	Existing & New HMA
Location	From 0.5 miles S of US	Tie Bars	#5 bars 3' @ 30" CTR
	20 N to IA 57		_







Tie steel was glued down with epoxy.



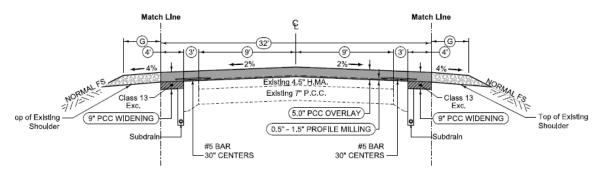


2021 Review
A few broken panels in wheel paths. Some of these areas have been patched.





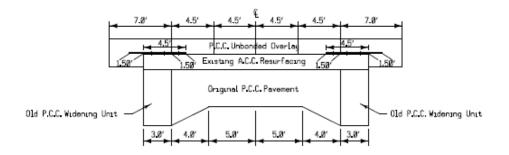
Year	2013	Overlay Type	UBOL
County	Fayette	Design	5" x 32' (4.5 x 5' panels 24'
			wide & 4' x 9.5" widening)
Route	IA 3	Milling	1 1/2" Milling
Project	NHSX-003-7 (29)3H-33	Interlayer	Existing HMA
Location	County Line to NCL of Oelwein	Tie Bars	#5 bars 6' @ 30" CTR



	3R_Overlay_ MODIFIED
STATION T	O STATION
677+22.20	785+50.50
791+45.50	1002+44.54
1007+77.46	1124+00.00

Longitudinal joint; See B.6 for Jointing Details Transverse joint: See B.6 for Jointing Details

tuon 'Bar'	Joint	Joint	Joint	'Bar'	Joint	7' 4' New Widening 3' 01d Widening
رن (	ڕؘ	ڼ	پَ	'C'	پَ	4.5' 9' Old Pavement
tuop 'B' or 'C'		Joint		'B' or 'C'	Joint	4.5'
ن ر <sub>ز</sub> <del>5′ ر</del>	5' . Typ.	رر,		′C′ Typ		4.5' 9' Old Pavement
Yoru 'Bar'	Joint	Joint	Joint	'Bar'	Joint	4.5'
,'C,	,C,	,C,	,۵,		رد,	3' Old Widening 4' New Widening
(		,	'	-	18' Typ. ►	



During milling operations, the mill removed HMA on the outside edge of the pavement in areas. Fabric interlayer was used to cover the exposed PCC pavement.





2017 – Cracking at 4 ft from edge. Location of old edge of widened pavement.

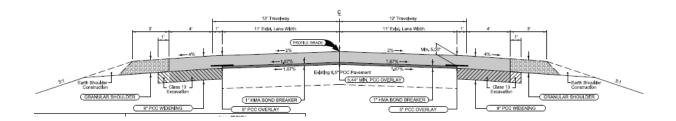


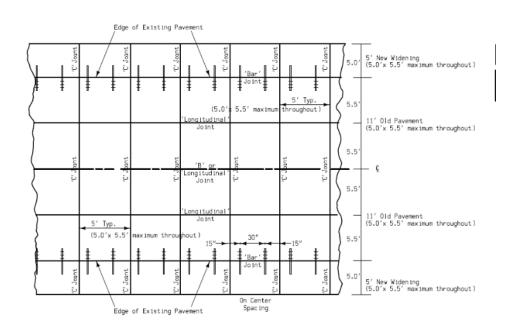
2021 Review
Cracking along paint line. Some patching.

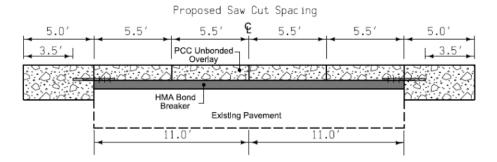




Year	2013	Overlay Type	UBOL
County	Lucas	Design	5" x 32' (5" x 22' & 5' x 9"
			widening)
Route	US 65	Milling	1954 8.5" PC8
Project	NHSX-065-2(20)3H-59	Interlayer	1" HMA Bond Breaker
Location	Wayne Co. Line N. to	Tie Bars	#5 bars 36" @ 30" CTR
	US 34		













### 2023 Review

Several areas with patches. Patches were full lane width, with no midpanel longitudinal joint. Several areas with outside wheel path cracking. Shoulder cross slope was 1.0 to 1.5%. Fairly long areas in good condition as well.



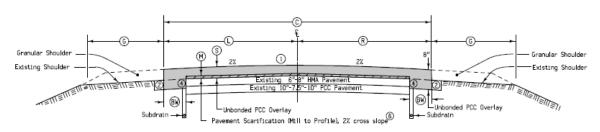




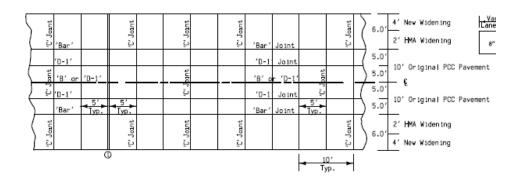


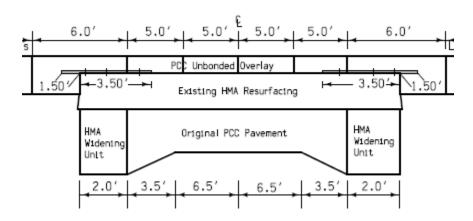


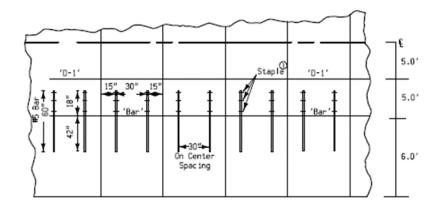
Year	2014	Overlay Type	UBOL
County	Kossuth	Design	5" x 32' (5" x 24' & 4' x 8"
		_	widening)
Route	US 169	Milling	Profile to 2% cross slope, 1"
Project	NHSX-169-8(59)3H-55	Interlayer	Existing HMA
Location	IA 9 N to Minnesota	Tie Bars	#5 bars 5' @ 30" CTR
	State Line		_



Γ		Location				0		_	Overlay	Quantities	Per Station			0	
L		Cocation		(S)	(4)	(C)	(0)	®				Scarification	⊛	( <u>G</u>	Dt
Γ	Road Identification	Station To	Station	laska.	Inches	Feet	Feet	Feet	Excavation Cu. Yds.	Overlay Sq. Yds.	Furntished Cu. Yds.	Overlay Sq. Yds.	Feet	9	nellants
F						reet	reet	rees					Leer.	reet	
L	US 169	4+00.0	344+65.0	5	VAR.	32	16	16	9.3	355.6	56.8	266.7	4	2	
	US 169	349+90.0	410+94.6	5	VAR.	32	16	16	9.3	355.6	56.8	266.7	4	2	
Γ	US 169	410+94.6	411+74.6	5	VAR.	32	16	16	9.3	355.6	56.8	266.7	4	2	SUPERELEVATION TRANSITION
[	US 169	411+74,6	415+05.0	5	VAR.	32	16	16	9.3	355.6	56.8	266.7	4	2	SUPERELEVATED















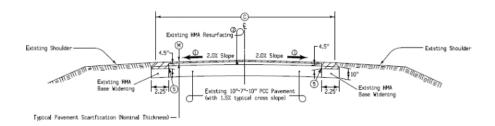
2023 Review

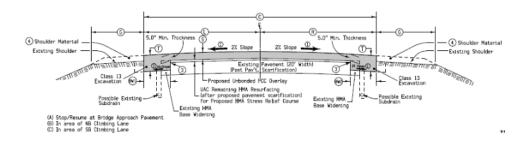
Areas of broken panels with Durapatch.

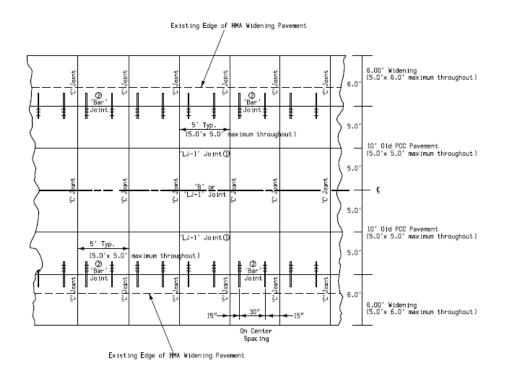


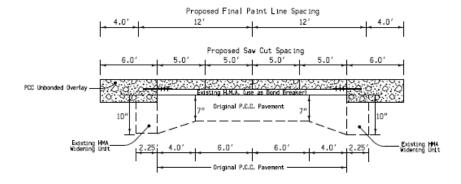


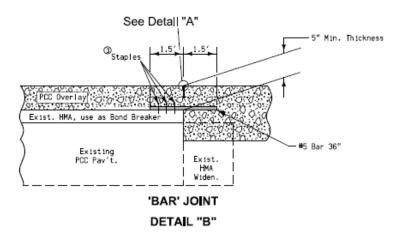
Year	2014	Overlay Type	UBOL
County	Poweshiek	Design	5" x 32' (5" x 20' & 6' x 9.5"
			widening) 5 x 5 x 6' long joints
Route	US 63	Milling	0.5 to 1.5"
Project	NHSN-063-4(39)2R-79	Interlayer	Existing HMA
Location	Montezuma N to Just	Tie Bars	#5 bars 36" @ 30" CTR
	south of I-80		







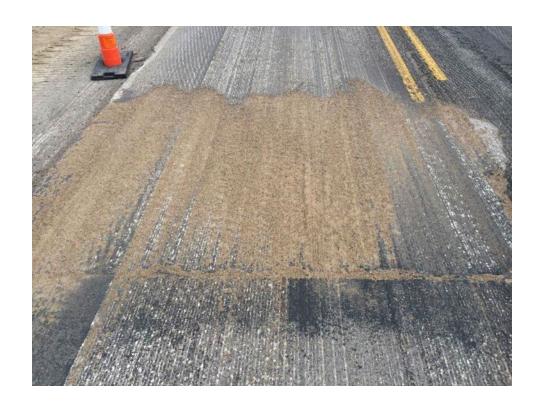




HMA was thin in some areas. 1482 tons of HMA added for stress relief layer. Issues with center line rumble strip blowing out joints.

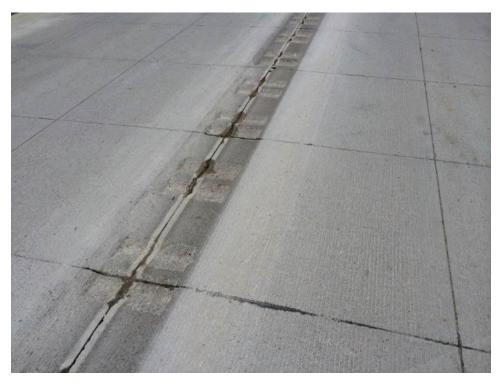




















2023 Review

Lots of areas with longitudinal cracking. Some areas with broken panels.









Year	2014	Overlay Type	UBOL
County	Cherokee	Design	6" 12x12 & 6x6 Panels – 10 ft.
_			shoulders. 44 ft. Total
Route	IA 3	Milling	2" Milling
Project	NHSX-059-7(46)3H-18	Interlayer	Existing HMA
Location	IDA Co. Line N. to IA 3	Tie Bars	#4 bars 6' @ 30" CTR

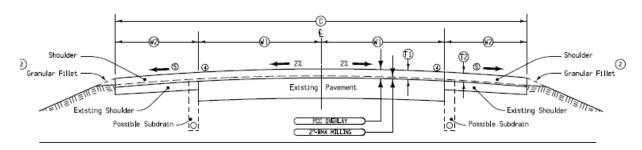
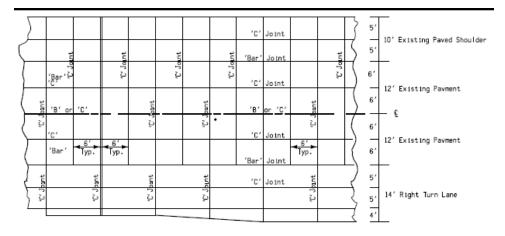


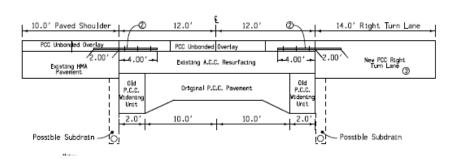
	TABLE OF DESIGN QUANTITIES Per Station										
G REMARKS	MILLING	PCC OVERLAY,	PCC OVERLAY, QMC (PLACE)	@ @ m @			(V1)			LOCATION	
nermons.	Tors	OMC (FURNISH)	GMC (PLACE) Sq. Yes.	Inches	Inches	Feet	Feet	SIDE	O STATION	STATION T	ROAD IDENTIFICATION
Ida County Line	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	810+63.40	810+00	US 59
	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	266+00.63	0+00	US 59
C63	26.58	22.22	133.33		6.0	_	12.0	Rt	269+72.45	266+00.63	US 59
C63	26.58	22.22	133.33		6.0	_	12.0	Rt	270+88.24	270+00.60	US 59
RR Guardrat	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	566+37.80	270+88.24	US 59
RR Guardrati	26,58	22.22	133.33		6.0	_	12.0	Rt	566+98.83	566+37,80	US 59
RR Guardrati	26.58	22.22	133.33		6.0	_	12.0	Rt	571+27.34	571+15.17	US 59
RR Guardrati	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	581+89.61	571+27.34	US 59
Washborn St. (S)	26.58	22.22	133.33		6.0		12.0	Rt	586+51.50	581+89.61	US 59
	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	595+35.61	586+51.50	US 59
Washborn St. (N)	26.58	22.22	133.33		6.0	_	12.0	Rt	597+11.61	595+35,61	US 59
Spring Rd HMA Ent.	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Rt	635+82,20	597+11.61	US 59
Linden St.	26.58	22.22	133.33		6.0		12.0	Rt	639+47.20	635+82,20	US 59
US 59 & [A 31 [nter	34.95	17.85	107.12		6.0	-	0-24.0	Rt-C	267+53	259+24	US 59
US 59 & [A 31 Inter	86.97	44.43	266.58		6.0	-	24.0	Rt-C	269+72.45	267+53	US 59
US 59 & [A 31 [nter	87.35	44.62	267.73		6.0	-	24.0	Lt-C	270+88	270+00.60	US 59
US 59 & [A 31 [nter	43.48	22.21	133.27		6.0	_	24.0-0	Lt-C	281+37	270+88	US 59
[da County Line	26.58	40.74	244,44	6.0	6.0	10.0	12.0	Lt	810+63.40	810+00	US 59
	26.58	40.74	244.44	6.0	6.0	10,0	12.0	Lt	52+00	0+00	US 59
C66	26.58	22.22	133.33		6.0	-	12.0	Lt	56+47	52+00	US 59
	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Lt	268+65.88	56+47	US 59
[A 31	26.58	22.22	133.33		6.0	_	12.0	Lt	269+72,45	268+65.88	US 59
EA 31	26.58	22.22	133.33		6.0	-	12.0	Lt	277+61.73	270+00.60	US 59
	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Lt	426+36,49	277461.73	US 59
C44	26.58	22.22	133.33		6.0	_	12.0	Lt	430+92,66	426+36,49	US 59
RR Guardrati	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Lt	566+86,66	430+92.66	US 59
RR GuardratI	26.58	22.22	133.33		6.0	_	12.0	Lt	566+98.83	566+86.66	US 59
RR GuardratI	26.58	22.22	133.33		6.0	_	12.0	Lt	571+75.57	571+15.17	US 59
RR Guardrati	26.58	40.74	244.44	6.0	6.0	10.0	12.0	Lt	632+87.20	571+75.57	US 59
Linden Street	26.58	22.22	133.33		6.0	_	12.0	Lt	639+47,20	632+87.20	US 59

#### Notes:

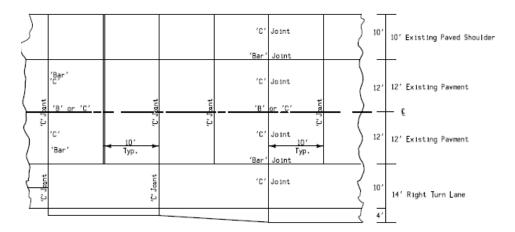
- (B) Sta. 810+00 to Sta. 426+36.49 Longitudinal joints shall be located at centerline, 6.0-ft Lt. and Rt. of centerline, 12.0-ft. Lt. and Rt. of centerline, and 17.0-ft Lt. and Rt. of centerline.A modified "L-1" (ML-1) joint shall be located at 12.0-ft. Lt. and Rt. of centerline with a 6-ft. long reinforcing bar. Transverse joints shall be located at 6.0-ft. spacings. See "EX-JNT A" on Sheet B.4.
- Sta. 426+36,49 to Sta. 639+47.20 Longitudinal joints shall be located at centerline, 12-ft Lt. and Rt. of centerline, A modified "L-1" (M-1) joint shall be located at 12.0-ft. Lt. and Rt. of centerline with a 6-ft. long reinforcing bar. Transverse joints shall be located at 10.0-ft. spacings. See "EX-INT B" on Sheet 8.4.
- 2 Shoulder material as specified elsewhere in these plans; refer to typical  $\fbox{7145}$  on Sheet B.3.
- 3 Quantity is estimated using a 6-in, thickness over the existing 24-ft, wide pavement and a 10-in, thickness for the integral 2-ft, widening units.
- (4) Refer to "EX-JNT" on B.4 for rebar placement.
- (5) Slope shall be 3% on "Turn Lanes" and 4% on shoulders.

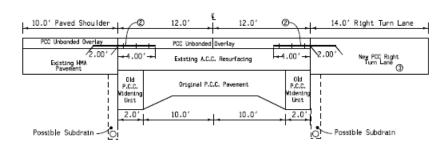
#### Sta. 810+00 to 426+36





Sta. 426+36 to 639+47



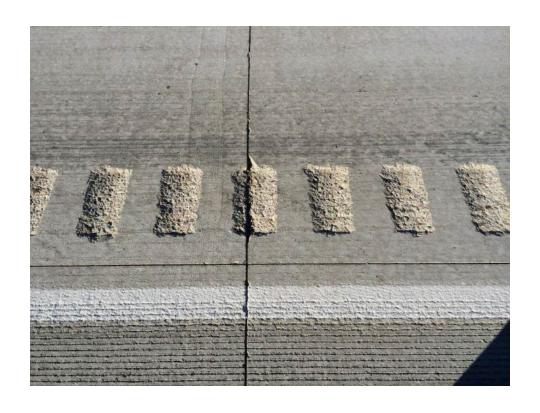


Construction

Placed half width.







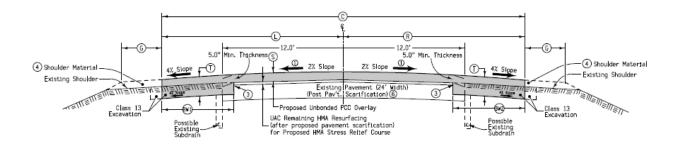
Review 2021

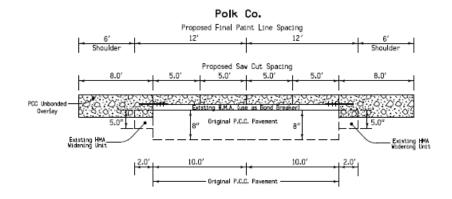
Areas of cracking at quarter point. Some broken panels.

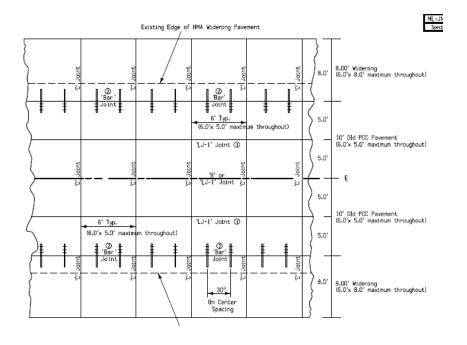




Year	2015	Overlay Type	UBOL
County	Polk	Design	5" x 36' (5" x 20' & 8' x 9.5"
			widening)
			5 x 5 x 6' long joints
Route	US 69	Milling	0.5" to 2" milling
Project	STPN-069-4(100)2J-77	Interlayer	Existing HMA
Location	118th Ave N. to Just S.	Tie Bars	#5 bars 36" @ 30" CTR
	of IA 210		







Placed half width. Some random cracking off sawed joints.



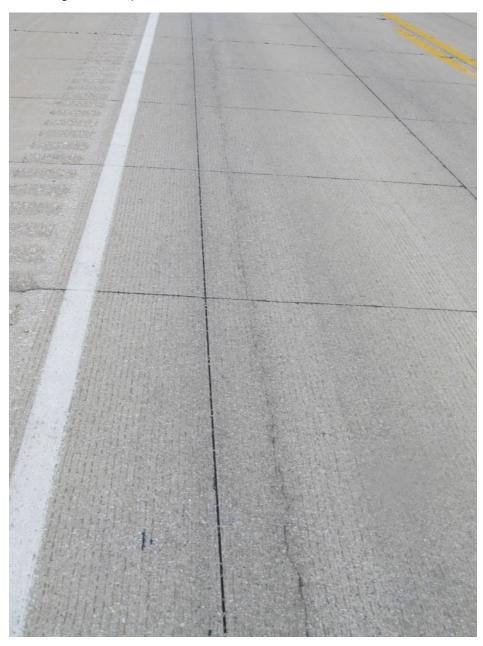






Review 2021

Longitudinal cracking in wheel path.



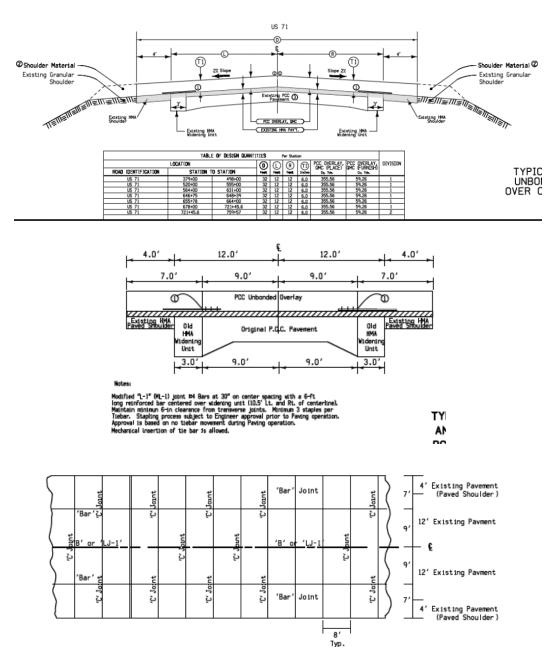




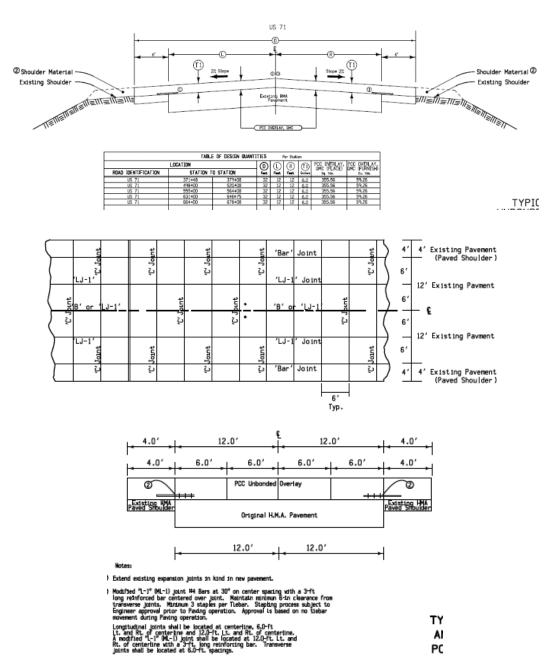
Shoulder cross slope 3.5%, Design 4%

Year	2015	Overlay Type	UBOL & WT
County	Clay	Design	6" x 32' (9 x8' panels ML & 7x8'
			panels shoulders) UBOL
			6" x 32' (6 x 6' ML and 4 x 6'
			Shoulder) WT
Route	US 71	Milling	2" Milling
Project	NHSX-071-8(59)3H-21	Interlayer	Existing HMA
Location	B53 N to 15th St in	Tie Bars	L-1 #4 @30" 6' – UBOL
	Spencer		L-1 #4 @30" 3' WT

#### Typical Cross Section - UBOL



#### Typical Cross Section WT



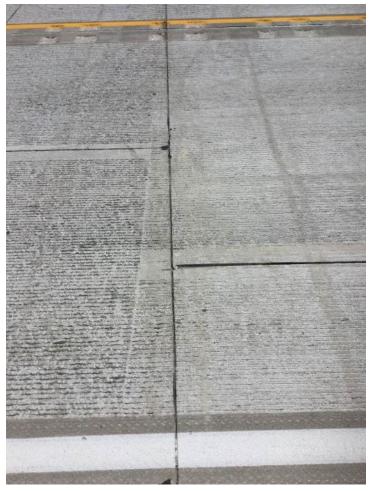
Milepost	Existing Pavement
	Type
188.13-190.29	Composite
190.29-192.21	HMA
192.21-193.80	Composite
193.80-195.06	HMA

Mechanical insertion of the bar is allowed.









2023 Review

Overall, both the UBOL and WT are in very good condition.



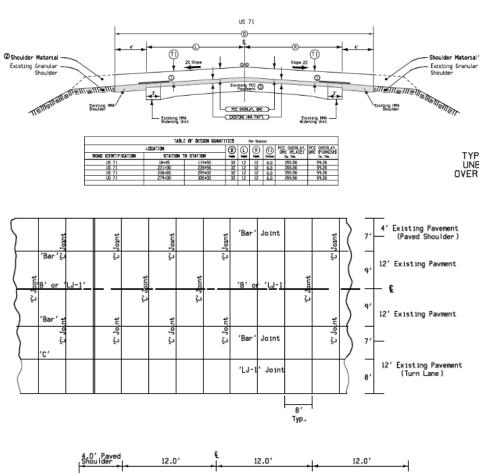
**UBOL** Overlay section

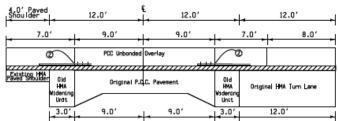


Whitetopping Overlay section

2016	Overlay Type	UBOL & WT
Clay	Design	6" x 32' (9 x8' panels ML & 7x8' panels shoulders) UBOL 6" x 32' (6 x 6' ML and 4 x 6' Shoulder) WT
US 71	Milling	2" Milling
NHSX-071-8(58)3H-21	Interlayer	Existing HMA
Buena Vista Co. Line N.	Tie Bars	L-1 #4 @30" 6' – UBOL L-1 #4 @30" 3' WT
	US 71 NHSX-071-8(58)3H-21	Clay  Design  US 71  NHSX-071-8(58)3H-21  Buena Vista Co. Line N.  Tie Bars

#### Typical Cross Section - UBOL





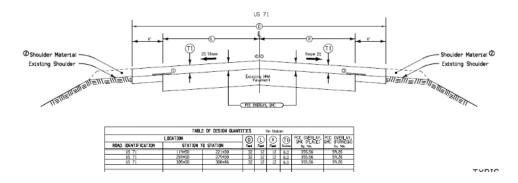
Notes

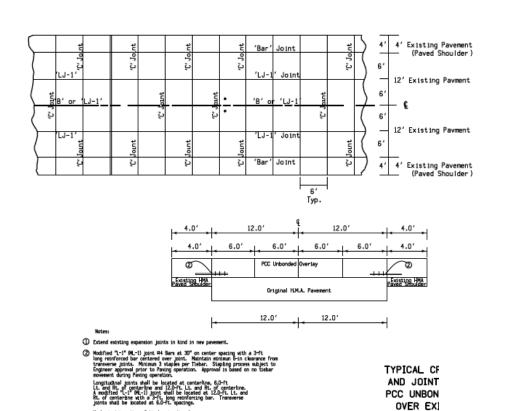
- ) Extend existing expansion joints in kind in new pavement.
- Modified 1-1" Mi-1 joint R4 Bars at 30" on center specing with a 6-ft joint period on the control over sidering self-one control on the control of the co

) See MOL-2 on Sheet B.2.

TYPICAL CRO AND JOINTI PCC UNBOND OVER EXISTIN

#### Typical Cross Section - WT









Review 2023

Overall, both the UBOL and WT are in very good condition.

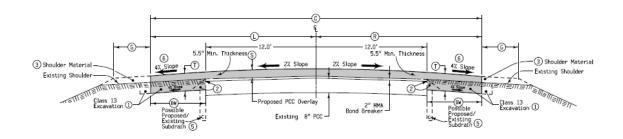


Whitetopping overlay section

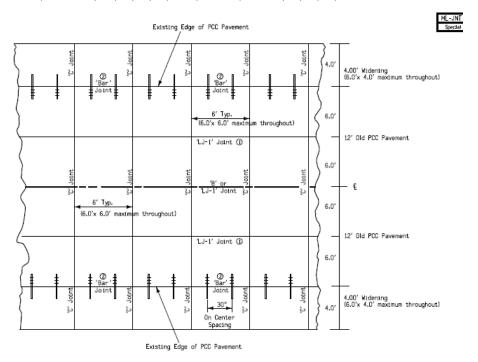


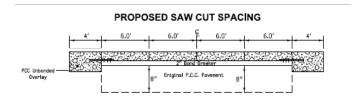
Unbonded overlay section

Year	2016	Overlay Type	UBOL
County	Benton	Design	5.5" x 32' (6 x 6' ML and 4 x 6'
_			Shoulder)
Route	IA 21	Milling	n/a
Project	STPN-021-4(27)2J-06	Interlayer	New HMA
Location	From E66 N. to just S.	Tie Bars	#4x36" @30" C-C
	US 30		



Location S C U R Overlay Quantities (Per Location) T S G Remarks														
[ (5) [ (6) ] (1) [ (6) ]	i	Location		)	(O	)	(	Ove	rlay Quantities	(Per Location)		_		
Class 13 Overlay Overlay	<u> </u>			(S)	(0)	Œ	(R)	Class 13	PCC Overlay	PCC Overlay	Θ	⊗	(6)	Remarks
Road Station To Station Inches Feet Feet Feet Cu. Yds. Cu. Yds. Sq. Yds. Inches Feet Feet	Road	Station T	Station To Station	Inches	Feet	Feet					Inches	Feet	Feet	
1A 21 72+65 247+25 5.5 32.0 16.0 16.0 1347 10,563 62,080 8 4.0 6.0 approx. MP 57.82 to MP 61.18	JA 21	72+65	2+65 247+25	5.5	32.0	16.0	16.0	1347	10,563	62,080	8	4.0	6.0	
17,460' @ 2x8.167sf/27= 10,563 CY PCC														17,460' @ 2x8.167sf/27= 10,563 CY PCC
17,460' e 2x1.042sf /27= 1348 CY Cl 13														17,460' @ 2x1.042sf /27= 1348 CY CI 13











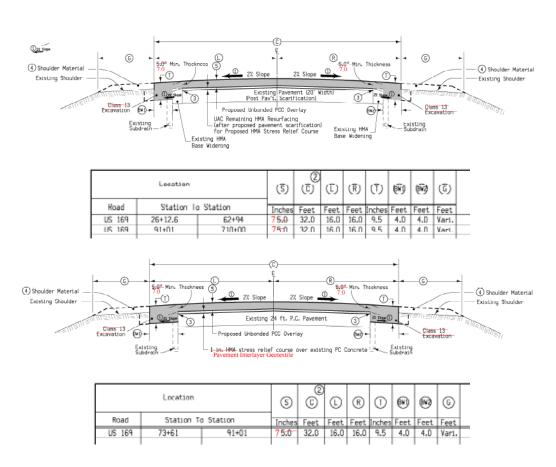
Review 2023

Overall, the overlay is in very good condition.

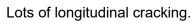




Year	2018	Overlay Type	UBOL
County	Dallas/Boone	Design	7" x 32' (12 x 12' ML and 4 x
			12' Shoulder)
Route	US 169	Milling	n/a
Project	NHSX-169-4(63)3H-25	Interlayer	Fabric Interlayer
Location	IA 141 N to US 30	Tie Bars	



## Review 2021









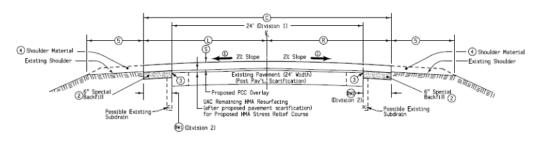


Shoulder cross slope 1.2%, Design 2%.

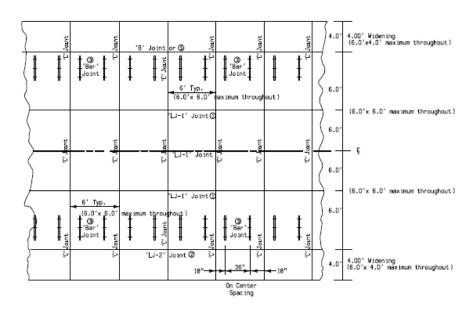
2023
Lots of longitudinal cracking. Most have been sealed.

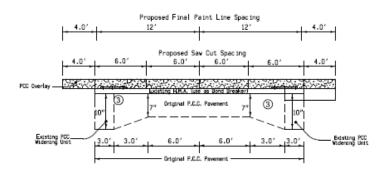


Year	2019	Overlay Type	UBOL
County	Marshall/Tama	Design	6" x 32' (6 x 6' ML and 4 x 6'
			Shoulder)
Route	IA 14	Milling	1" Milling
Project	HSPIX-014-5(81)3H-64	Interlayer	Existing HMA
Location	Co. Rd. E18 N. to W.	Tie Bars	Shoulder not tied
	JCT IA 175		



	Location								Division 1 Overlay Quantities Per Location		Division 2 Overlay Quantities For Location				
			(3)	(0)	0	®		89	PCC Overlay	PCC Overlay	Class 13	PCC Overlay	PCC Overlay	Special Backfill	Remarks
Road	Station To	Station	Inches	Feet	Feet	Foot	Feet	Feet	Cu. Yds.	Sq. Yds.	Cu. Yds.	Cu. Yds.	Sq. Yds.	Du. Yds.	1
EA 14	371+50,00	442+79.63	- 6	32	16	16	4	4	3169	19012	1745	1056	6337	1745	
	3+33,40	46471.50	- 6	32	16	16	4	4	1955	11728	1077	652	4009	1077	
	46+71.50	48451.50	- 6	32	16	18	4	4	1458	802	29	18	157	29	Rt. Turn Lane Taper
	48+51.50	52400.00	- 6	32	16	24			215	1292	119	52	215	119	Includes Rt. Turn Lane
	52400.00	95455.00	- 6	32	16	16	4	4	1875	11252	1033	645	3851	1033	
	102+57.40	520+00.00	6	32	16	16	4	4	18552	111315	10219	6184	37105	10219	











Review 2021

In very good condition. Shoulders do appear to be heaving, but no cracking due to no tie steel.





Sta 33+00 Mainline 1.9%



Sta 33+00

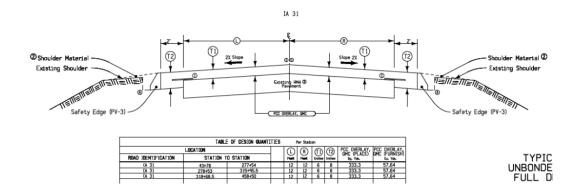


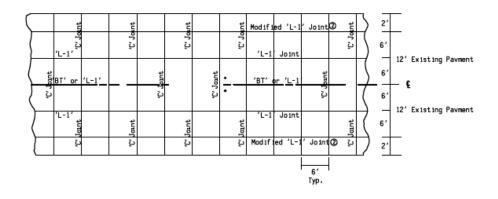
Station 412+00

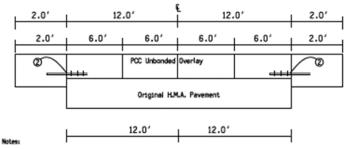
Still performing very well.



Year	2020	Overlay Type	WT
County	Woodbury	Design	6" x 32' (6 x 6' ML and 2 x 6'
			Shoulder)
Route	IA 31	Milling	3" Milling
Project	STP-031-1(43)2C-97	Interlayer	Existing HMA
Location	E. JCT US 20 N. to Co.	Tie Bars	Fiber Test Sections
	Rd. C66		







long reinforcing bar contered over joint. Mo long reinforcing bar contered over joint. Mo trainsverse joints. Minimum 3 staples per Tie Engineer approval prior to Paving operation. Movement during Paving operation. Mechanical insertion of tie bar is allowed.

TYI ٨N PC

#### Macro Fiber Test Sections

Begin Sta.	End Sta.	Length feet	PCC SY	PCC CY	Fiber Ibs	Transverse Sawcut Spacing FT	Longitudinal Sawcut Spacing FT	Remarks
43+76	50+00	624	2079.8	359.7	1438.7	6	6	Use Detail EX-JNT A on sheet B.2
142+00	152+00	1000	3333.0	576.4	2305.6	15	12	Use Detail EX-JNT B on sheet U.1
152+00	162+00	1000	3333.0	576.4	2305.6	12	12	Use Detail EX-JNT B on sheet U.1
162+00	172+00	1000	3333.0	576.4	2305.6	9	12	Use Detail EX-JNT B on sheet U.1
172+00	182+00	1000	3333.0	576.4	2305.6	9	6	Use Detail EX-JNT B on sheet U.1
182+00	192+00	1000	3333.0	576.4	2305.6	6	6	Use Detail EX-JNT B on sheet U.1
423+00	441+00	1800	5999.4	1037.5	4150.1	6	6	Use Detail EX-JNT A on sheet B.2

### Construction







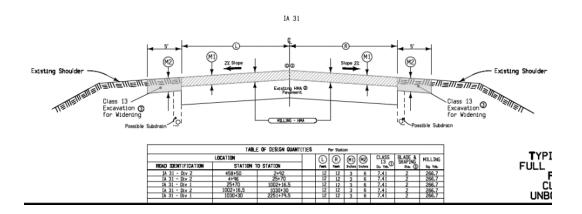
Review 2021

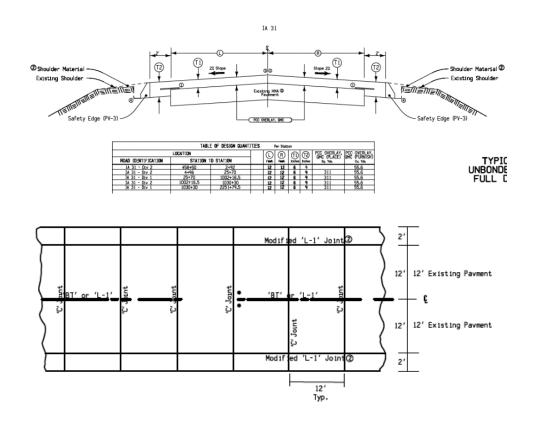
Overall, the overlay is in very good condition.

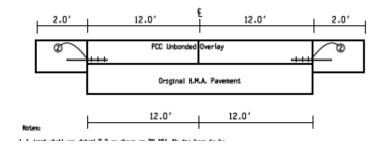




Year	2022	Overlay Type	UBOL
County	Cherokee	Design	6" x 28' (12' x12' ML and 2 x 12'
			Shoulder), Fibers – 4 lb/cy
Route	IA 31	Milling	3" Milling
Project	STP-031-3(11)2C-18	Interlayer	Existing HMA
Location	Washta to US 59	Tie Bars	L-1, 3 ft @ 30" centers







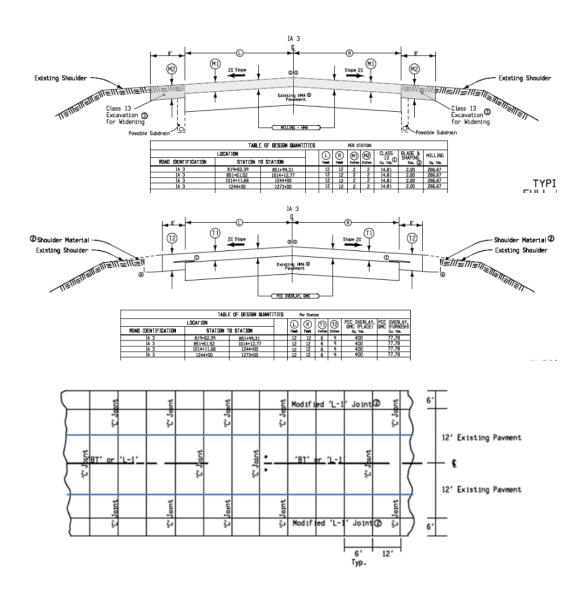
2022 Review

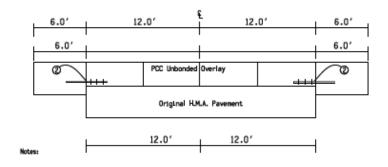
Overall, the overlay is in very good condition. A few areas with random cracking off sawed joints. Issues with center line rumble strip depth and crossing joint.





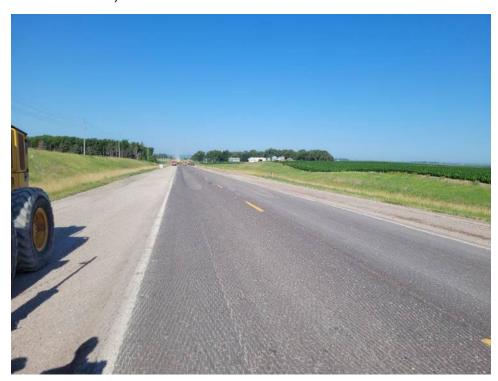
Year	2022	Overlay Type	WT
County	Plymouth	Design	6" x 36' (6 x 6' ML and 6 x 6'
_	_		Shoulder) 12x12 Test Section
Route	IA 3	Milling	2" Milling
Project	NHSX-003-1(106)2R- 75	Interlayer	Existing HMA
Location	Lemars to Remsen	Tie Bars	L-1, 3 ft @ 30" centers

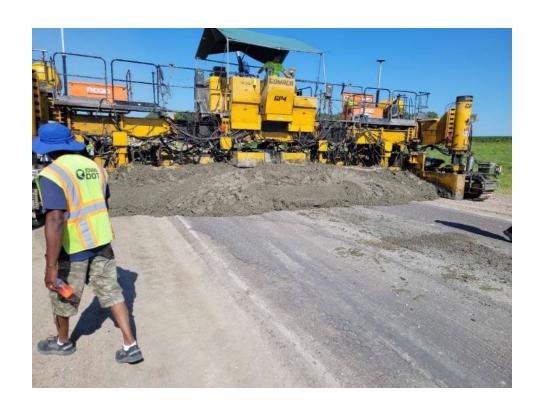




#### Construction

Overlay was placed full width to be able to place shoulders as soon as 325 psi maturity was reached. Goal was to pave  $\sim$ 1 mile per day and fully open each section within 2 days after paving. The test section of 12 x 12 foot panels is located between Otter Ave. and Oyens Ave. ( $\sim$ Sta 1028+00 to 1038+00).

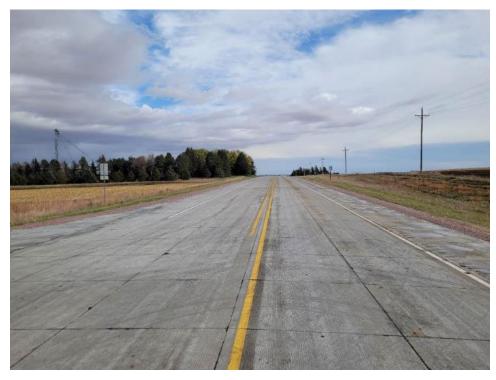






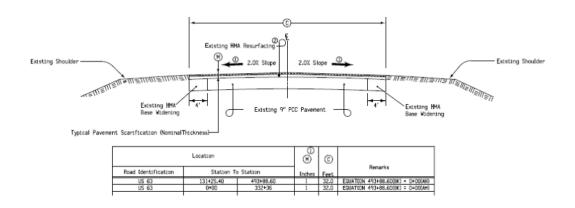
2022 Review

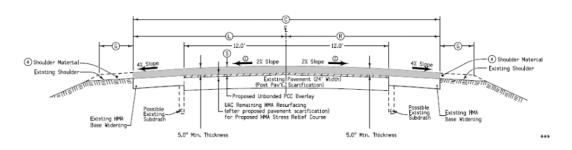
Overall, the overlay is in very good condition.

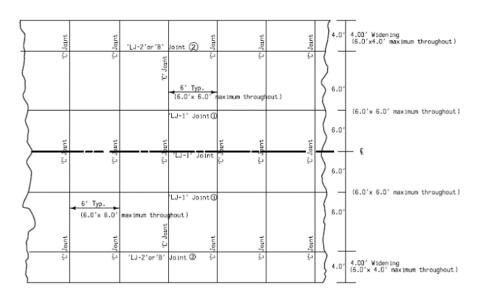


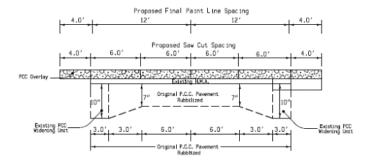


Year	2022	Overlay Type	UBOL
County	Tama/Blackhawk	Design	5" x 32' (6 x 6' ML and 4 x 6'
			Shoulder)
Route	US 63	Milling	1" Milling
Project	NHSX-063-5(72)3H-86	Interlayer	Existing HMA
Location	Traer to 0.5 mi S of IA	Tie Bars	Fibers 5 lb/cy – No Tie Steel
	58		-









### Construction







2023 Review

Overall, the overlay is in very good condition.





Year	2023	Overlay Type	WT
County	Plymouth	Design	6" x 36' (12 x 12' ML and 6' x 12'
			Shoulder)
Route	IA 3	Milling	3" Milling
Project	NHSN-003-1(104)3H-75	Interlayer	Existing HMA
Location	Remsen to Co. Line	Tie Bars	Fibers 4 lb/cy #4 x 36" at 30" C-C

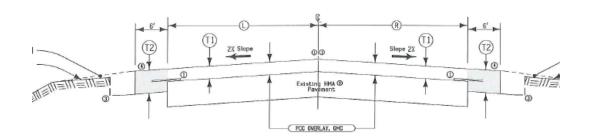
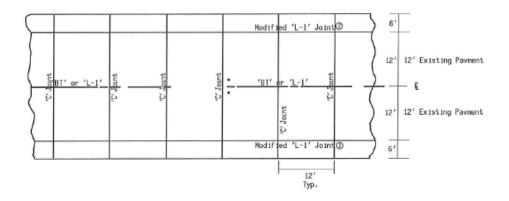
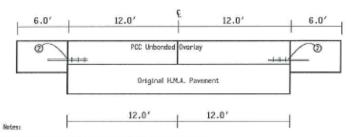


	TABLE	OF DESIGN QUANTITIES	1	Per Stati	on				
	LOCATION		(1)	(R)	(11)	(12)	PCC OVERLAY, QMC (PLACE)	PCC OVERLAY	
ROAD IDENTIFICATION	STATION T	O STATION	Feet	Feet Feet Inch		Inches	Eq. Yds.	Du. Yds.	
IA 3 DIV. 1	1315+30	1332+78	12	12	6	9	266.67	44.44	
IA 3 DIV. 2	1332+78	1596+74	12	12	6	9	266.67	44.44	





- $\mathbb D$  .l-1 joint shall use detail 0-3 as shown on PY-101. No tie bars to be used except as stated in note (2) .
- 2) An "L-I" joint shall be located at 12.0-ft. Lt. and Rt. of centerline with a 3-ft. long reinforcing bar.

Reinforced bars shall be #4 Bars at 30° on center spacing with a 3-ft long reinforcing bar centered over joint. Maintain minimum 6-in clearance from transverse joints. Minimum 3 staples per Tiebar. Stapling process subject to Engineer approval prior to Paving operation. Approval is based on no tiebar maximum during Paving operation. Mechanical insertion of the bar is allowed.

T A PI

### Construction





### 2023 Review





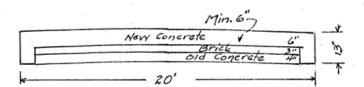
## Appendix C – Old Pavement Resurfacing Designs

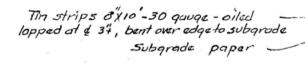
First used in letting of 5-10-32

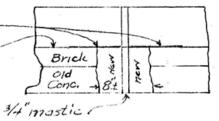
R-1

Dubuque FA 1480 8.365

Similiar to Design No RZ except for the details below



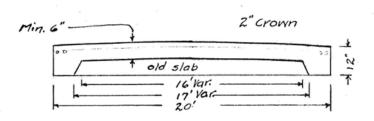


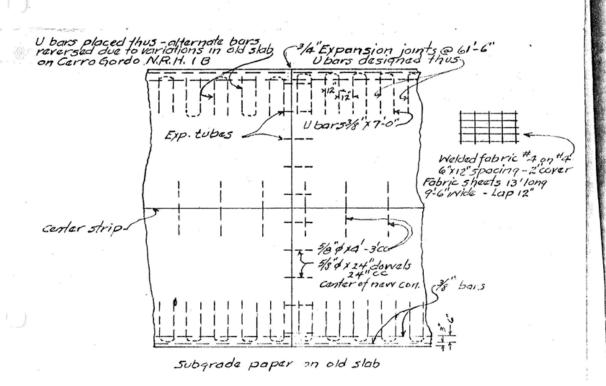


RZ

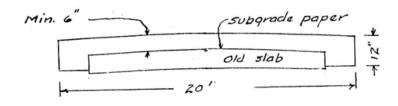
#### Design No. R-2 First used in letting of 9-12-33

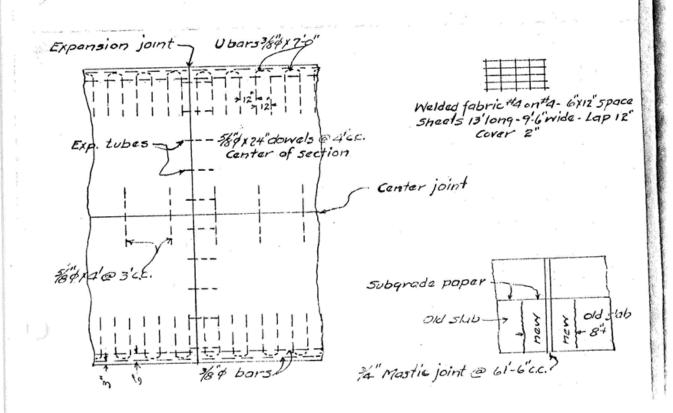
Carro Gordo NRH 18 1.059 Winnebago NRM 139 0.294 Woodbury P 45 1.8015





R-3

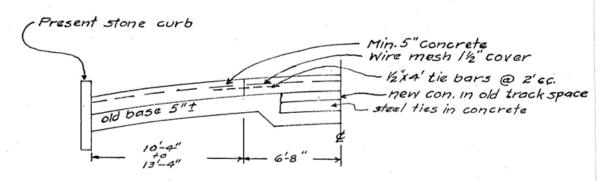


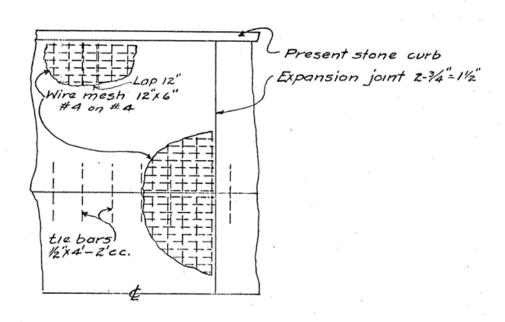


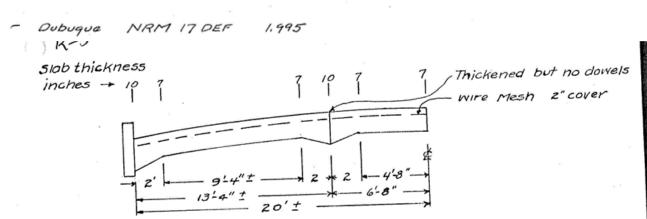


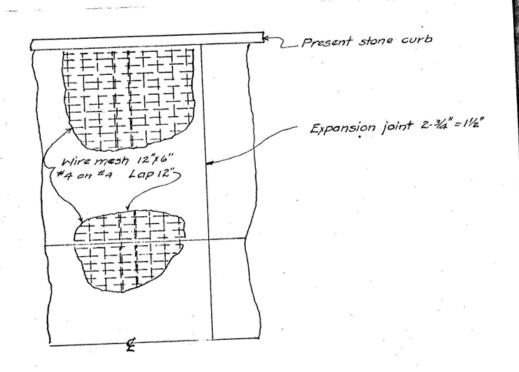
R-4

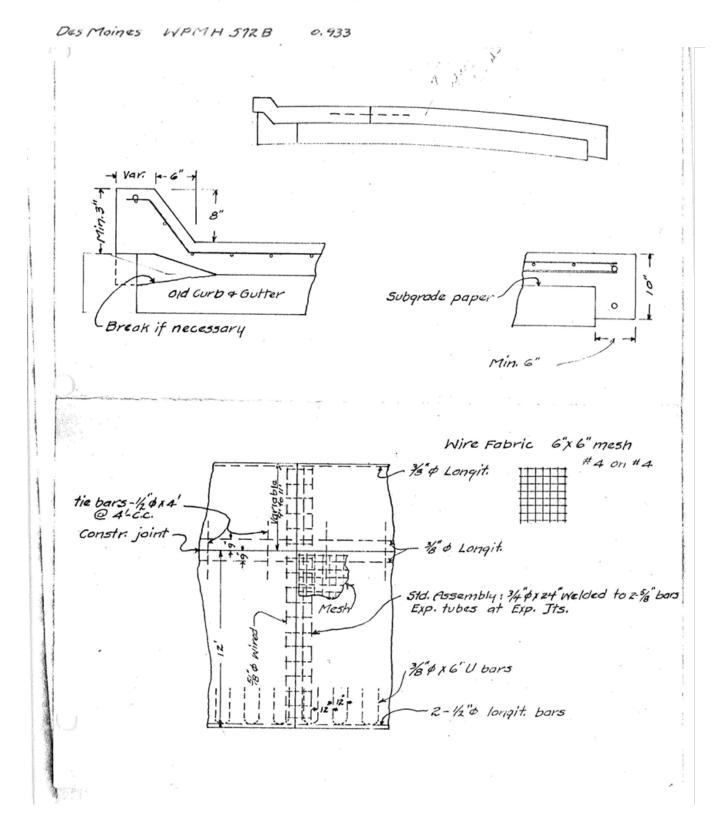
Dubuque NRM 17 DEF 1.995



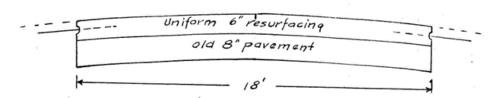








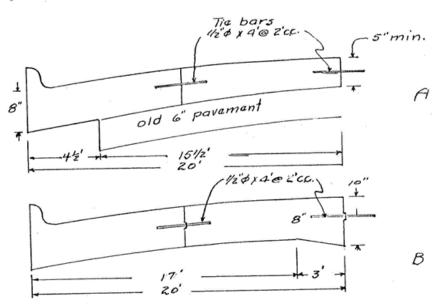
Das Moines DAWI. 2 2.045

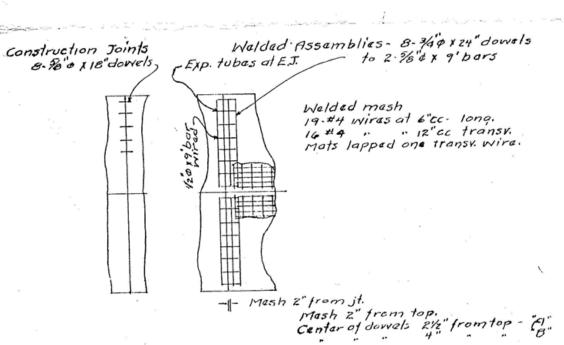


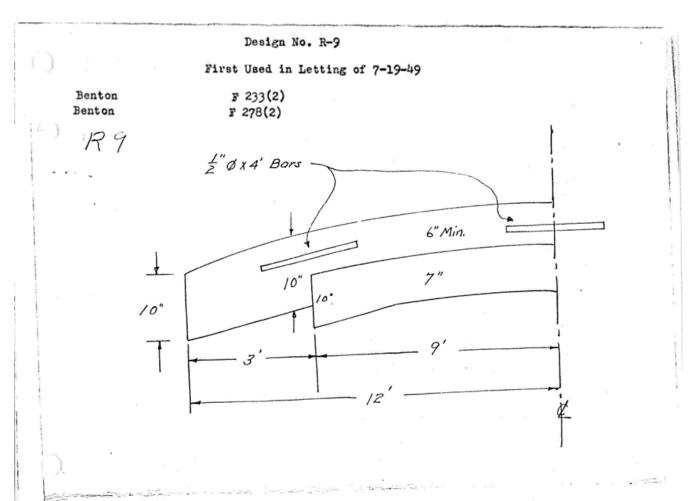
1/2 px4' & a'cc.

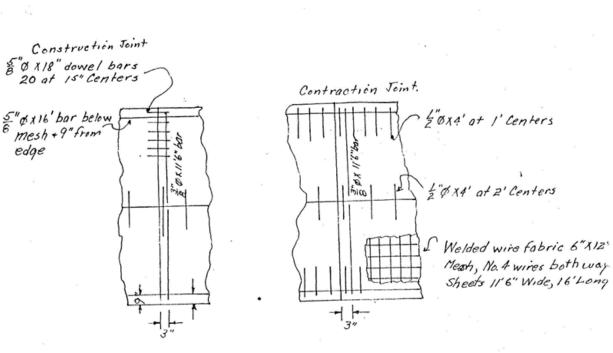


2-8









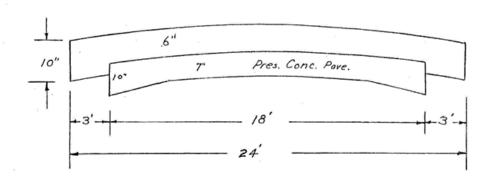
#### Design No. R-10

### First Used in Letting of8-1-50

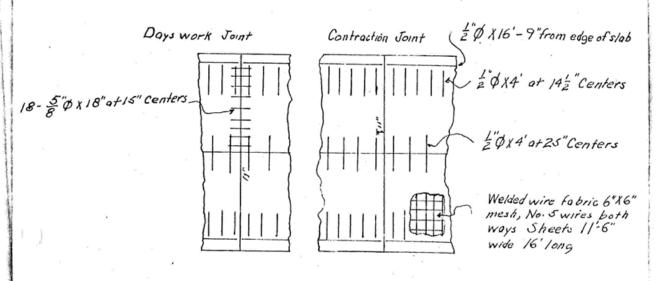
Linn

F-96(11) 4.312 Miles 1950

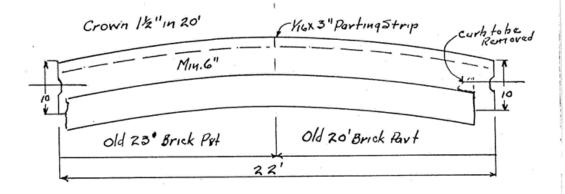
R10

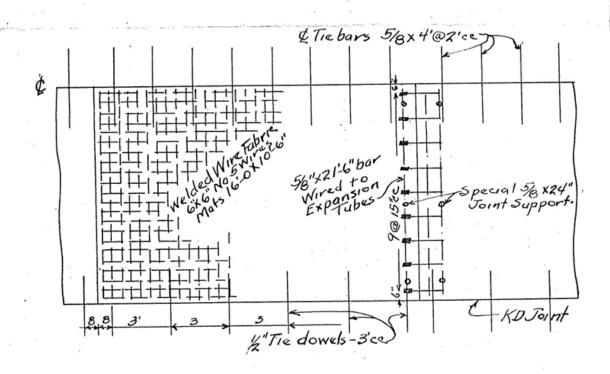


#### Contraction Joints at 16- 4" interv



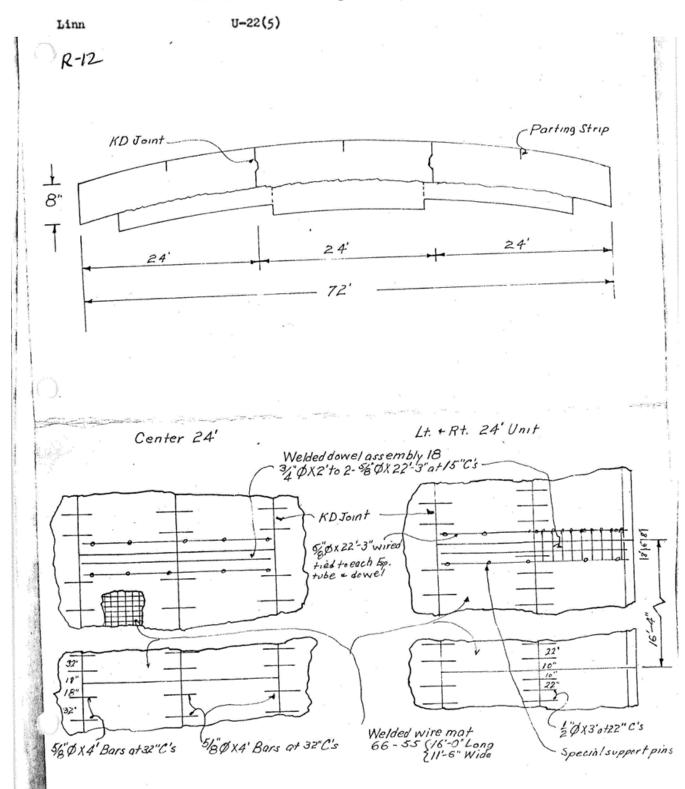


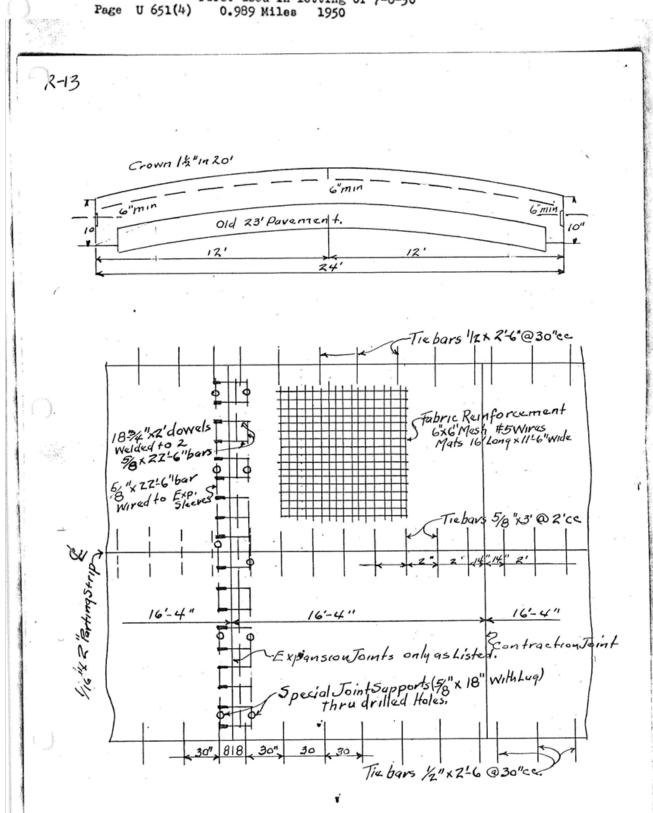




Design No. R-12

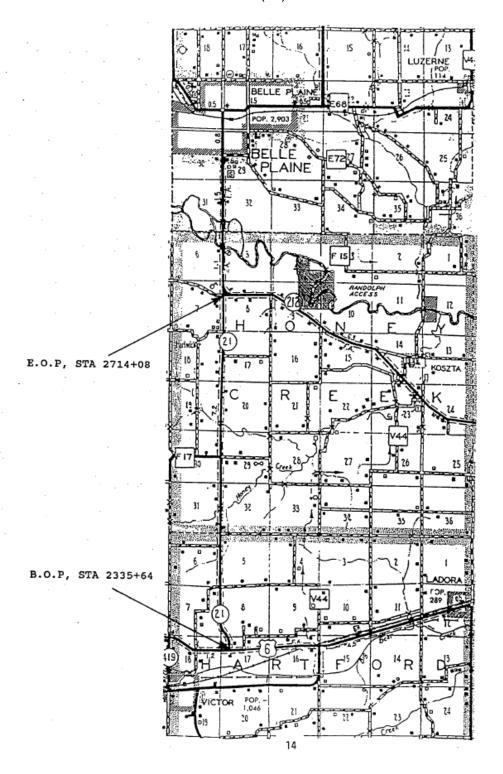
#### First Used in Letting of 6-6-50





Appendix D – Iowa County IA 21 Whitetopping Overlay Test Sections

### Whitetopping Research Location Iowa County IA 21 STP-21-3(10)—2C-48



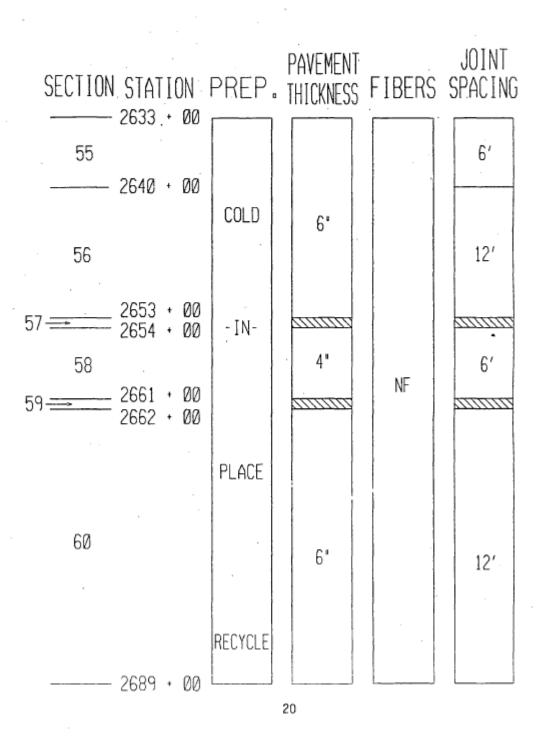
SECTION STATION		PAVEMENT THICKNESS 8"	FIBERS	JOINI SPACING
3 2349 + ØØ		6"		12'
4 5————————————————————————————————————	PATCH			6'
6 	AND	4*		2'
2371 · 00			F	4:
9 2378 + 00 2380 + 00	SCARIFY			2'
2387 · 00		2'		4'
2394 + 00 2396 + 00		15		

SECTION STATION I	PREP.	PAVEMENT THICKNESS F	FIBERS	JOINT SPACING
13 2403 + 00				6'
14		6"	F	12'
15===== 2414 · 00 2415 · 00	PATCH			200000
16		4.5"	ACC	N/A-
17===== 2425 + 00 2426 + 00	AND	7111111		200000
18 2433 + 00	-	6"		12'
19 20 ==== 2440 · 00 2441 · 00	SCARIFY		NF	6'
21 22===== 2448 + 00 2449 + 00	i	4"		2'
2456 + ØØ		2'		

SECTION STATION	PREP.	PAVEMENT THICKNESS	FIBERS	JOINT SPACING
2456 · 00 24 2458 · 00 25 2460 · 00	PATCH AND SCARIFY			
26	-			6'
2468 + 00		6"		
27				12'
2479 + 00		21111112		,,,,,,,,,,
29	PATCH	4 "	NF	4'
2487 + 00 2489 + 00				
31	-			15' ND
2496 + 00	ONLY	8"		
32				15′ D
33 2503 · 00 2505 · 00				
34		4.5"	ACC	N/A
2515 • 00				

	SECTION	STAT 2515 2516	] (	00	PREP.	PAVEMEN [HICKNESS	T 6 F	IBERS	(	JOINT SPACING
	36					6*		NF		6'
,	37	2538 2540		00 00	PATCH					
,	38	2547	+	00		2"				2'
40-		2554 2555	+	00 00	ONLY			F		4'
	41	2562	•	00						
,	42	2569		00		4"		,		2'
	43	2576	÷	00		18				6′

		STAT	I	ON ØØ	PREP.	PAVEMENT THICKNESS	FIBERS	
44		2576 2577		00	PATCH			
-	45 .	2585		00	ONLY	6'		12'
	46	2000		20	COLD			
47-	<b>→</b>	2593 2594	+	00 00	COLD	7777777		6'
	48				-IN-			
-	49	2601	•	00		4"	F	2'
-		2608	+	00	PLACE			
	50	0045		00				4'
-	51	2615 2617		00 00				
	: 52				RECYCLE		-	2'
-		2624	+	00		2"		
	53	2004		00			-	4'
-	54	- 2631 - 2633	•	00 00				



SECTION STATION	PREP.	PAVEMENT THICKNESS	FIBERS	JOINT SPACING
61 2689 + 00 2691 + 00	COLD			
62 2698 + 00		2"	NF	4'
<del>- 63</del> 2700 + 00	- IN-			
2704 + 00	PLACE			
65	RECYCLE	4.5"	ACC	N/A
2714 · 00				