

Designing Pavements to Support the Heavy Loads in the Energy Development Areas

Technical Report 0-6839-P1 & P2

Cooperative Research Program

TEXAS A&M TRANSPORTATION INSTITUTE COLLEGE STATION, TEXAS

in cooperation with the Federal Highway Administration and the Texas Department of Transportation http://tti.tamu.edu/documents/0-6839-P1-P2.pdf

TxDOT Project 0-6839 Workshop: Designing Pavements to Support the Heavy Loads in the Energy Development Areas

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Texas A&M Transportation Institute

Project 0-6839 Research Team

Austin, Tx; February 26, 2018

Outline



- Project overview and objectives
- Concrete pavement options
- □ Flexible pavement options
- Implementation plan



Project Overview



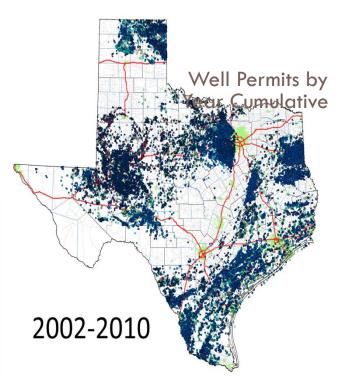
- Energy development areas
 - Heavy traffic and sever failure



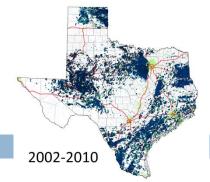








Project Overview





- "Features" and challenges of pavement design
 - Early opening requirements (no detours; end of day)
 - Weak/thin existing materials (most FM roads)
 - Excessive traffic loads (50-60% overload)
 - Available funds (limited fund vs. miles and miles)

Project Main Objectives

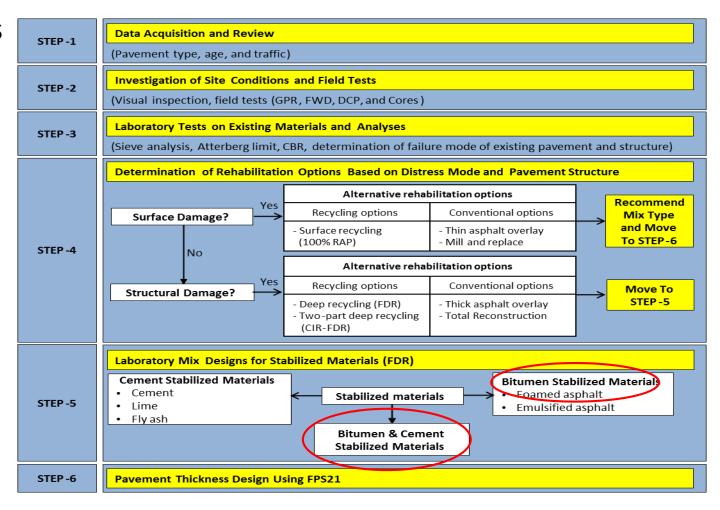


- Develop materials options suitable for early trafficking
- Recommend pavement designs that are structurally adequate for overloaded vehicles
- Work with Districts to design, construct, and monitor test sections with new materials and design approaches

Selecting rehabilitation options



□ 6 steps

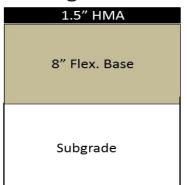




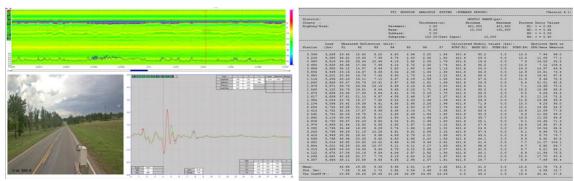
Case Study: FM906 in Paris District, Texas

Project Info. (Step-1)

- From FM 196 to US 271
- 4.5 miles long (net)
- AADT (2015): 904
- Future AADT (2035): 1,810
- Truck Percent: 4.3
- Speed Limit: 55 MPH
- Number of Lanes: 2
- Existing Structure



Field Survey and Test (Step-2)



GPR Survey

FWD Test

Test Pit



Material Collection and Laboratory Test (Step-3)





Sieve Analysis / Plastic Index / Proctor Test

Gradation % Passing				
Sieve	Existing Base	New Base		
1 ¾ "	100	100		
1 ¼ "	99.0	95.4		
3/4 "	90.5	78.5		
3/8 "	66.0	57.7		
# 4	55.3	44.1		
# 40	29.0	28.2		
Plasticity Index	7	4		
Combined Materials		Dry Density		
Combined Materials	OMC (%)	(pcf)		
75% Existing Base & 25% RAP	5.4	133.0		
42% Existing Base, 33% New Base, & 25% RAP	6.0	131.1		



Rehabilitation Method Selection (Step-4): We select FDR for this study

Laboratory Mix Designs on FDR Mixes (Step-5)

Design #	Material %	%RAP	Foamed % (PG64-22)	Emulsion % (CSS-1H)	Additive
1	75% EB	25%	2.4	-	0%
2	75% EB	25%	2.4	-	1% Cement
3	42% EB 33% NB	25%	2.4	-	0%
4	42% EB 33% NB	25%	2.4	ı	1% Cement
5	75% EB	25%	-	4	0%
6	75% EB	25%	-	4	1% Cement
7	42% EB 33% NB	25%	-	4	0%
8	42% EB 33% NB	25%	-	4	1% Cement

8 FDR Mixes

Design #	Material %	%RAP	Foamed % (PG64-22)	Emulsion % (CSS-1H)	Additive	Dry IDT (psi)	Wet IDT (psi)
1	75% EB	25%	2.4	-	0%	78.9	1.7
2	75% EB	25%	2.4	2 - 5	1% Cement	73.3	33.5
3	42% EB 33% NB	25%	2.4	-	0%	71.3	2.9
4	42% EB 33% NB	25%	2.4	-	1% Cement	49.3	37.9
5	75% EB	25%	-	4	0%	76.4	50.2
6	75% EB	25%	-	4	1% Cement	53.2	41.1
7	42% EB 33% NB	25%	-	4	0%	67.5	42.7
8	42% EB 33% NB	25%	-	4	1% Cement	56.0	49.5



Moisture Conditioning





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Moisture Conditioning





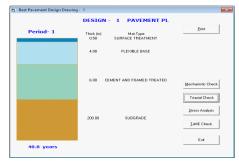
Pavement Thickness Design using FPS 21(Step-6)

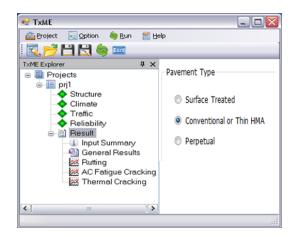
	FM 99	FM 906	FM 541
AADT (2015)	3,352	904	697
Future AADT (2035)	6,710	1,810	1,400
Truck Percent (%)	4.9	4.3	22.8
Speed Limit (MPH)	60	55	55
Number of Lanes	2	2	2
18 kip ESAL for 20-year (millions)	1.144	0.271	1.111
Variables used for ESAL calculation	Design years=20 Dir. Distribution=0.5 Lane Distribution=1.0 Growth Rate (%)=3.50 Truck Factor= 1.35	Design years=20 Dir. Distribution=0.5 Lane Distribution=1.0 Growth Rate (%)=3.53 Truck Factor= 1.35	Design years=20 Dir. Distribution=0.5 Lane Distribution=1.0 Growth Rate (%)=3.54 Truck Factor= 1.35
Subgrade Modulus (ksi)	19.5	10	14.3



- FPS21
 - Modulus
 - Traffic: ESALs
 - Design life
- Texas Triaxial check
 - One pass shear failure
- TxME check
 - Load spectra
 - Rutting
 - Cracking



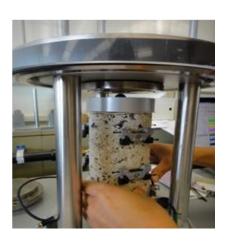


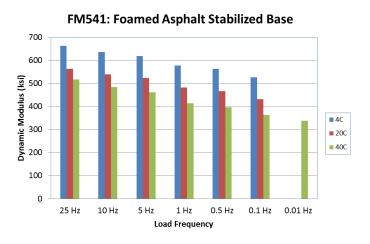


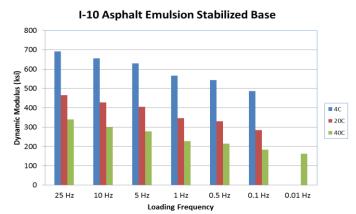


□ FDR materials moduli: laboratory measurement



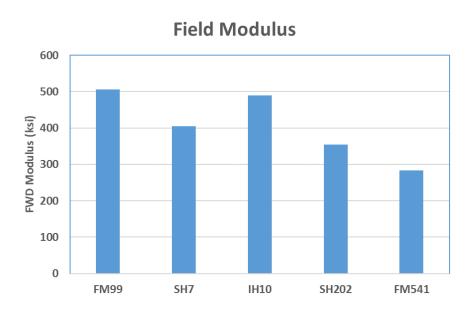


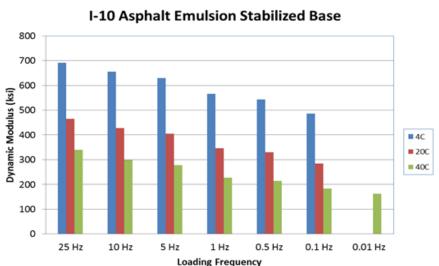






□ FDR materials moduli: lab vs. FWD (field)





Recommended modulus: 300 ksi



□ Traffic: load spectrum; Case study: FM468

Vehicle Class Distribution & Growth

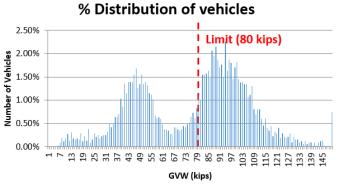
Vehicle Class	Pictorial View	Distribution (%)
Class04		0.94
Class05		3.24
Class06		2.87
Class07		0.40
Class08		2.51
Class09	000 00	86.88
Class10		2.97
Class11		0.00
Class12		0.08
Class13	0-00 00 0 0	0.11
	Sum of Distribution =	100.00%

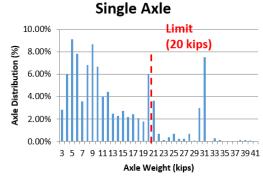


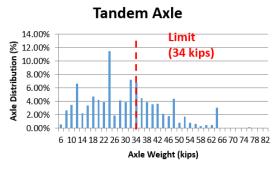
Traffic: load spectrum; Case study: FM468

GVW & Axle Load Distribution

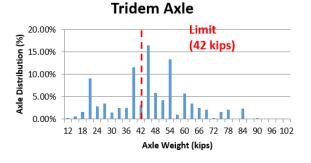
Portable WIM | 16-Days Traffic Data Collection | GVW = Gross Vehicle Weight

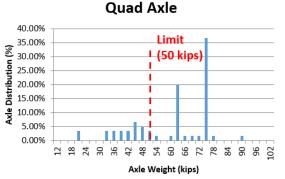






Axle Type	Daily Count
Steering Axles	326
Non-steering Single Axles	32
Tandem Axles	588
Tridem Axles	10
Quad Axles	2



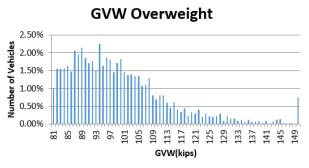


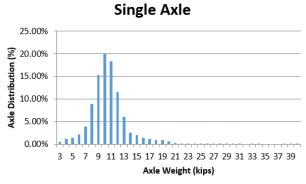


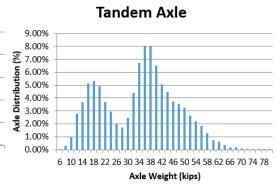
Traffic: load spectrum; Case study: FM468

Overloading & Overweight Data

Portable WIM | 16-Days Traffic Data Collection







55.75 % Overloaded Trucks Daily (GVW ≥ 80 kips)

1.39 % Overloaded Trucks Daily (Single Axle Weight ≥20 kips)

52.92 % Overloaded Trucks Daily (Tandem Axle Weight ≥34 kips)

Over-Weight Summary	Daily Overweight Count (% of Total)	Maximum Overweight Recorded	Legal Limit	%age Overweight
GVW Overweight (≥ 80 kips)	182(55.75%)	411 kips	80 kips	414%
Single Axles (≥ 20 kips)	5(1.39%)	41kips	20 kips	105%
Tandem Axles (≥ 34 kips)	311(52.92%)	80 kips	34 kips	135.3%
Tridem Axles (≥ 42 kips)	4(41.03%)	90 kips	42 kips	114.3%
Quad Axles (≥ 50 kips)	1(48.48%)	50 Kips	50 kips	80%

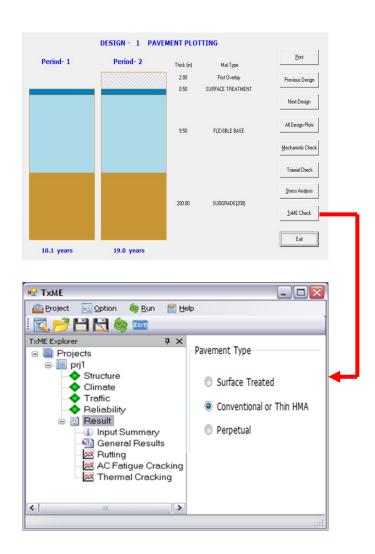


□ Traffic: load spectrum vs. ESAL

Highway ID	Station ID	AADTT	ESAL from TxME Load Spectra (20 years)
I35	513	10,867	49,650,718
I10	502	8,005	32,748,557
I20	526	7,704	50,529,653
I45	539	6,834	37,354,536
I35	531	6,299	26,717,107
I20	544	5,767	28,243,048
US287	506	4,182	36,010,559
US287	528	3,247	17,228,683
SH114	527	2,656	13,479,223
SH130	532	2,269	7,682,393
US59	535	2,000	5,656,394
US82	530	919	3,120,864
US96	142	846	4,337,616
SH121	546	550	1,976,022
SH6	Portable WIM	474	1,830,420
US82	543	372	1,310,763
FM468	Portable WIM	1,062	11,437,641
FM3129	541	251	1,652,034
FM2223	800	142	516,928

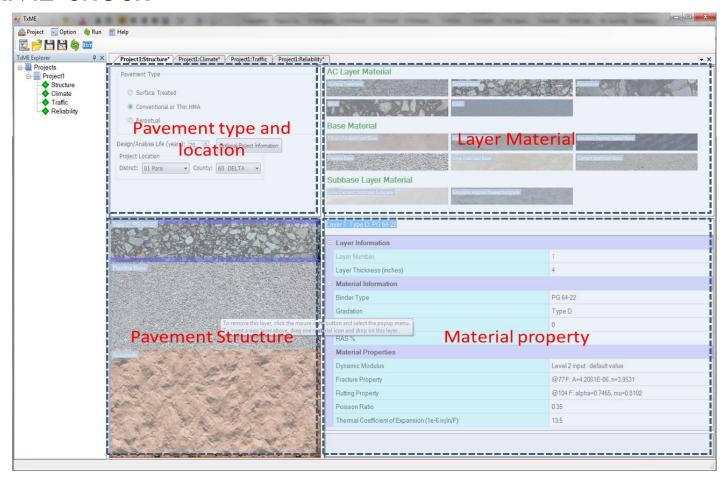


- □ TxME check
 - load spectrum



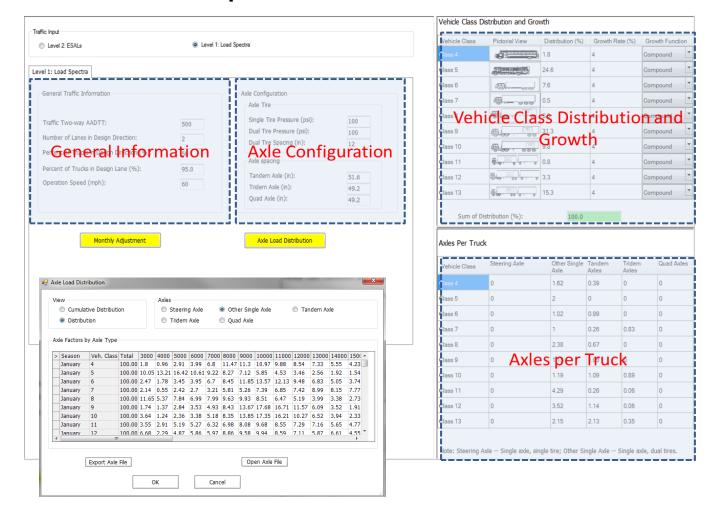


□ TxME check



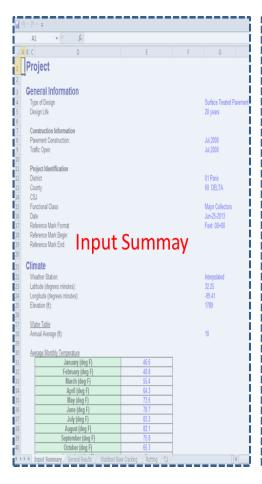


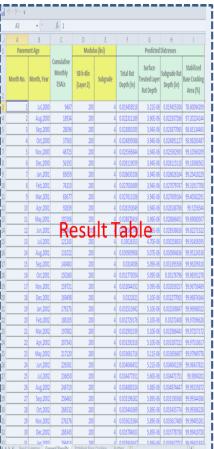
TxME check: load spectrum

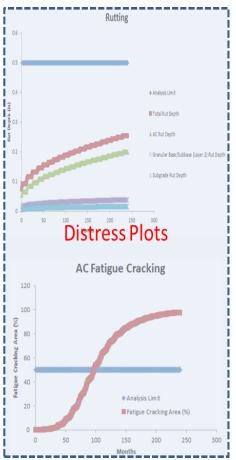




□ TxME check: performance prediction









□ TxME check

Influence of Material Properties

Variable inputs

- Fracture properties of dense grade type-D:
 20, 40, and 100 overlay cycles
- FDR Modulus (ksi): 300, **150**, 75

Fixed inputs

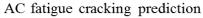
- Traffic: Traffic spectra from Station 535
- Subgrade Modulus (ksi): 14.3.
- Climate: San Antonio, TX

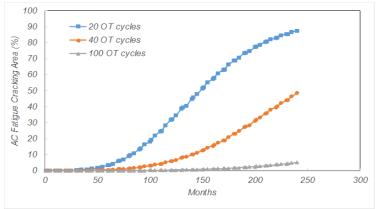
FM541 Design #3



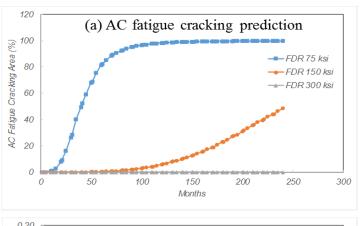


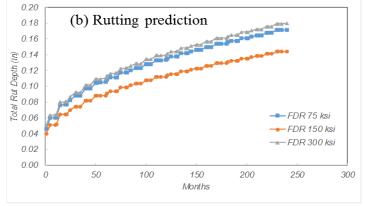
□ TxME check





Influence of Fracture Property of Mix





Influence of Modulus of FDR Mix



- □ FM541: foamed asphalt stabilization
- SH202: foamed asphalt stabilization
- □ 110: asphalt emulsion stabilization
- □ SH7: foamed asphalt stabilization
- □ FM99: foamed asphalt stabilization
- □ US281/SH123: concrete pavement



□ FM541: foamed asphalt stabilization















After 2 years: no cracking; average rut depth: 2.9 mm







□ SH202: foamed asphalt stabilization



After 1.5 years: no cracking; average rut depth: 5.4 mm







□ 110: asphalt emulsion stabilization







After 1.5 years: no cracking; average rut depth: 6.4 mm







SH7: foamed asphalt stabilization



After 1.5 years: no cracking AND no rutting



□ FM99: foamed asphalt stabilization



After 3.5 years: limited longitudinal cracking; rut depth: 4 mm





Implementation plan



- Develop and teach workshops
 - Rehabilitation options
 - Mix design
 - Structural design
- Construct sections: foamed vs. emulsion
- Continue to monitor existing field test sections
- □ Document US281/SH123 construction



Thank You All!

Questions???