DEPARTMENT OF TRANSPORTATION

Estimation of Metro Freeway System Reliability and Resilience

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This study has estimated and analyzed the travel-time reliability and traffic-flow performance trends of the freeway corridors in the Twin Cities metro area of Minnesota. First, TeTRES (Travel-Time Reliability Estimation System), developed in the previous study, was enhanced by adding the estimation module of the traffic-flow performance measures for selected routes. Next, the TeTRES database was populated with the external-operating condition data collected from 2010 to 2020. The enhanced TeTRES was then applied to a total of 48 directional corridors in the metro freeway network and the travel-time reliability for each corridor under different operating conditions was estimated and analyzed along with the traffic-flow performance measures for 2016 – 2020 period. In particular, a newly developed vulnerability index, which combines 95 th percentile buffer index and 95 th percentile travel rate of each route, was applied to determine yearly-reliability trends under different operating conditions for each corridor. The vulnerability index was also applied to identify the most vulnerable bottleneck section within each directional corridor using the 2019 data under all conditions. Finally, a preliminary study to assess the operational resilience of freeway corridors was conducted in this study by formulating the corridor-wide operational resilience with data from a total of six directional corridor routes in the metro freeway network.			
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ESTIMATION OF METRO FREEWAY SYSTEM RELIABILITY AND RESILIENCE

FINAL REPORT

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EXECUTIVE SUMMARY

A key element in developing and maintaining a reliable freeway system is the capability to monitor and assess corridor-wide travel-time reliability on an ongoing basis. The previous phase of this research developed a comprehensive computer system, TeTRES (Travel-time Reliability Estimation System), which can be used to efficiently collect and integrate a large volume of data from multiple sources, such as traffic, weather, and incident databases, and estimate a set of reliability measures for given corridors and time periods under specified operating conditions.

This report summarized the results from the current research effort to apply TeTRES for analyzing the travel-time reliability trends at the major corridors in the Twin Cities metro freeway network. First, a new module to calculate the traffic-flow measures of effectiveness (MOEs) has been added to TeTRES, so that an integrated analysis using both travel-time reliability and traffic-flow MOEs can be performed in an efficient way for selected corridors and periods. Next, various sets of historical data for the metro freeway network, including non-traffic external condition data, such as weather, incidents, special events, and work zones, were collected from 2012 until 2020. The collected data were processed and loaded into the TeTRES database. Furthermore, a total of 116 directional routes were identified in cooperation with the Regional Transportation Management Center, MnDOT, and for each route, the travel times were calculated for every five-minute interval from 2012 to 2020 and saved in the traveltime database in TeTRES. The calculated travel times were linked to external operating conditions, e.g., weather and incident conditions, and stored in the TeTRES database. The expanded database of TeTRES was then applied to estimate a set of monthly and yearly travel-time reliability measures for 48 directional routes in 23 corridors in the metro network under different operating conditions from 2016 until 2020. These estimation results were then analyzed and the effects of different operating conditions, sch as weather, incidents, and work zones, on travel-time reliability were identified for each route. In particularly, a newly developed vulnerability index, which combines 95th percentile buffer index and 95th percentile travel rate of each route, was applied to determine yearly reliability trends under different operating conditions for each route. The vulnerability index was also applied to identify the most vulnerable bottleneck section within each directional route using the 2019 data under all conditions.

Finally, a preliminary study to assess the operational resilience of freeway corridors was conducted in this study using the data from a total of six directional corridor routes in the metro freeway network. Using the collected traffic and incident data, the congestion start/recovery process of each route was analyzed and a model to quantify the corridor-wide operational resilience (CORI) of a given corridor was formulated and applied to the sample directional routes for the weekday-peak periods under dry-weather conditions. The resulting CORI estimates of the sample directional routes indicated that the southbound routes show consistently stronger resilience with more stable day-to-day variations than those of the northbound route in a same corridor. Furthermore, the average resilience values of all the sample routes were significantly different from each other at a 95% confidence level. Finally, the potential relationship between the operational resilience and the geometric structure of each sample route was analyzed by quantifying the level of potential geometric-friction in each directional route in

terms of handling corridor-wide, through-traffic flows. The quantification of the geometric friction was based on the geometry data easily measurable from the field. The resulting geometric-friction levels showed clear correlation with the operational resilience measures of each sample route. For example, the southbound routes in the sample corridors used in this study showed less geometric-friction levels with stronger operational resilience than the northbound routes in the same corridors. This indicates the promising possibilities of the proposed corridor-wide operational resilience measures for accurately understanding the main sources of reliability issues and improving operational effectiveness of given corridors.

CHAPTER 1: INTRODUCTION

1.1 BACKGROUND AND RESEARCH OBJECTIVES

A reliable and resilient freeway network, which can absorb, recover and adapt to various operating conditions, is of critical importance in sustaining the way of life and economic vitality of the Twin Cities metro area. A key element in developing and maintaining such a reliable freeway system is the capability to monitor and assess corridor-wide travel-time reliability, a major performance measure for quantifying the operational effectiveness of a freeway network.

The previous phase of this research developed a comprehensive computer system, Travel-time Reliability Estimation System (TeTRES), which can efficiently process a large amount of data from multiple sources, such as MnDOT traffic-data archives, Computer Aided Dispatch (CAD)-incident database, and the National Oceanic and Atmospheric Administration (NOAA) weather-data storage, and estimate a set of travel-time reliability measures for given corridors and time periods under userspecified operating conditions. This research expands TeTRES by adding the capability to calculate traffic-flow measures of effectiveness (MOEs) for given routes. Furthermore, the enhanced TeTRES is applied to analyze the reliability trends and/or issues of individual corridors in the metro freeway network. The specific objectives of this study include:

- Expansion of TeTRES with the addition of the traffic-flow module of the Traffic Information and Condition Analysis System (TICAS), for an integrated analysis of travel-time reliability and traffic-flow measures of effectiveness for given routes.
- Population of TeTRES database with various types of historical data for the 2012 2020 period from multiple sources relevant to the estimation of travel-time reliability measures for selected corridors under given operating conditions.
- Estimation of travel-time reliability measures for the major corridors in the metro freeway
 network for the selected periods and identification of bottleneck sections in each corridor using
 the travel-time reliability measures.
- Preliminary study for assessing operational resilience of sample freeway corridors.

1.2 REPORT ORGANIZATION

Chapter 2 describes the internal structure of the enhanced TeTRES with the newly added traffic-flow MOE module. The enhancements of administrative and user clients to facilitate the extraction of MOEs are also explained in this chapter. The collection and processing of the historical data, both traffic and non-traffic data, needed to estimate travel-time reliability measures under various operating conditions are explained in Chapter 3. The populated database of TeTRES is used in Chapter 4 to estimate the monthly and yearly values of the travel-time reliability and traffic-flow measures for the major corridors

in the metro freeway network. The identification and prioritization of the bottleneck sections in each corridor are also described in Chapter 4. The preliminary study to model and assess the operational resilience of sample freeway corridors is reported in Chapter 5. Finally, Chapter 6 summarizes the benefits of the current research, implementation steps. and conclusions.

CHAPTER 2: INTEGRATION OF TETRES AND TRAFFIC-FLOW MOE CALCULATION PROCESS

2.1 INTRODUCTION

In this chapter, TeTRES is enhanced with the addition of the traffic-flow Measures of Effectiveness (MOEs) calculation functions of TICAS, Traffic Information, and Condition Analysis System, developed at the University of Minnesota Duluth, so that an integrated analysis of travel-time reliability and trafficflow performance measures can be conducted for given corridors. Specifically, the existing travel-time estimation module of TeTRES is modified to incorporate the additional calculations of various trafficflow MOEs, which are then stored in the TeTRES database, which is also expanded in this study. In the enhanced TeTRES, the calculation of traffic-flow MOEs is performed on a need basis, i.e., users can choose the MOE calculation option for selected routes and time periods. To facilitate the MOE calculation and extraction process, the admin client is revised to allow the administrator to calculate the MOE values for predefined routes for a specific time range with the option of modifying parameter values needed for MOE calculation. The user client is also enhanced with the functions necessary for the traffic-flow MOE calculation/extraction processes. In particular, the MOE-output module, developed and inserted into the User client in this study, extracts the MOE values for selected corridors from the database for user-specified operating conditions and generates a set of the output files in a spreadsheet format. The rest of this chapter summarizes the architecture of the enhanced TeTRES, the processes to calculate, store and extract the traffic-flow MOE values for given routes.

2.2 INCORPORATION OF TRAFFIC-FLOW MOE CALCULATION PROCESS INTO TETRES

2.2.1 OVERVIEW OF THE ENHANCED TETRES ARCHITECTURE

Figure 2.1 shows the architecture of the enhanced TeTRES, whose highlighted modules incorporated the new processes and functions, developed in this chapter, to calculate, store, and extract the MOE values for pre-defined routes. Specifically, the following modules have been updated with the new MOE-related processes and functions:

- Travel-Time Database, which is expanded to store a set of MOEs calculated for each route.
- Travel-Time and Reliability Calculation Module, where the traffic-flow MOE calculation functions are called and MOEs for each route are calculated.
- Admin-Client, where the admin users calculate the MOE values for the predefined routes for a specific time range with the option of modifying different parameters of MOE calculation.
- User-Client, where the new MOE output module extracts the MOE values for user-specified operating conditions and generates a set of the output files in a spreadsheet format.

2.2.2 TYPES OF TRAFFIC-FLOW MEASURS OF EFFECTIVENESS

This section summarizes the types of the traffic-flow MOEs incorporated into TeTRES in this study. Figure 2.2 shows the space-discretization scheme adopted in TeTRES, where a freeway corridor is divided into 0.1-mile segments and a set of MOEs are estimated for each segment with the macroscopic flow parameters, i.e., flow rate (q), density (k) and speed (u), for every time interval. In particular, the values of q, k, and u for each 0.1-mile segment are determined by interpolating the measurements from the field detectors installed at fixed locations on freeway corridors. Currently, q, k, and u values for each 0.1-mile segment are estimated for every 5-minute interval for the entire freeway network in Twin Cities, Minnesota. Those q, k, and u values for each segment are used to determine the various types of traffic-flow MOEs for a given route and period. The specific types of traffic-flow MOEs, coded into the MOE module, and their definitions used in this study are as follows:



Figure 2.1: Architecture of Enhanced TeTRES with Traffic-Flow MOE Calculation

2.2.3 Vehicle-Miles Traveled (VMT)

VMT measures the amount of travel for all vehicles in a given roadway during a given time period. In TeTRES, the VMT for a given route is calculated every time interval, t, e.g., 5 minutes in the current version, by adding up all '0.1-mile segment VMTs' in a given route as follows:

VMT for segment i during t

= $[density (k)]_{i,t}$ * segment length (= 0.1 mile) * $[speed (u)]_{i,t}$ * time-interval (=5/60 hr)

VMT for a Route during t = Sum of VMTs for all the segments in a route during t

2.2.4 Vehicle-Hours Traveled (VHT)

VHT measures the total amount of time spent by all vehicles in a given roadway over a given period of time. It's a measure of the quality of traffic performance of a given route and estimated for each time interval as follows:

VHT for segment i during t = $[density (k)]_{i,t}$ * segment length (=0.1) * time-interval (=5/60 hr)

VHT for a route during t = Sum of VHTs for all segments in a route during t

2.2.5 Delayed-Vehicle Hours (DVH)

DVH measures the total delay experienced by all the vehicles in a given route over a given period of time and estimated as follows:

DVH for Segment i during t = [(Estimated Actual-Travel Time)_{i,t} – (Free-flow travel Time)_i] *

[flow rate (q)]_{l,t} * time interval (=5/60 hr)

DVH for a route per time interval = Sum of DVHs for All Segments in a Route per time interval

2.2.6 Lost VMT for congestion (LVMT)

LVMT is a measure of the lost capacity, because of congestion, of a given route over a given time period. It measures the inefficiencies of a given roadway traffic system and estimated by converting the lost capacity into VMT for each segment every time interval as follows:

If (density of segment i per time interval) > critical density,

Then

LVMT_i = (capacity * number of lanes – Total flow rate)I,t * time interval * segment length (=0.1 mile)

Else, LVMT_i= 0

LVMT for a route per time interval = Sum of LVMTs for all segments in a route during t

2.2.7 Unused VMT (UVMT)

UVMT measures the level of unused capacity of a given route for a given time period. UVMT can happen because of the lack of traffic demand or upstream bottlenecks and estimated for each segment by converting the unused capacity into VMT as follows:

If Density of segment i during t <= Critical Density,

Then UVMT for Segment i during t = [capacity * (number of lanes)i – (Total flow rate)i,t] * time interval

(=5/60hr) * segment length (=0.1 mile),

Else, UVMT i = 0

UVMT for a Route during t = Sum of UVMT for All Segments in a Route during t

2.2.8 Congested Miles (CM)

CM measures the amount of congested roadways of a given roadway for a given time period. A segment i is defined as congested when its speed during t is less than a pre-specified congestion-speed vale and the CM for a given route during t is calculated as the sum of all the congested segments during t as follows:

If speed of segment i during t < congestion _threshold speed,

then CM_i = Segment length (=0.1mile), Else CM_i = 0

CM for a given route during $t = \sum CM_i$ for all segments in a given route during t

2.2.9 Congested-Mile Hours (CMH)

CMH measures the extent of congestion for a given roadway during a given time period by adding congested-time duration to CM. The specific formula for CMH for a given route is as follows:

If speed of segment i during t < congestion_ threshold speed,

then CMH_i = Segment length (=0.1mile) * time interval (=5/60 hr), Else CMHi = 0

CMH for a given route during $t = \sum CM_i$ for all segments in a given route during t

2.2.10 Speed Variations (SV)

SV measures the variance of the speed of a given route for a given time period. It's estimated with the speed values of all segments during t for a given route during t, i.e.,

SV for a route during t = Var[Speed values of all segments for a given route during t]

In the enhanced TeTRES, average speed, maximum speed, minimum speed, speed difference (maximum speed - minimum speed) and acceleration are also calculated along with speed variance.

2.2.11 Number of Vehicles Entered and Exited

```
TVE (j): Total number of Vehicles Entered a given corridor during time (j)
```

```
= Sum [Upstream Boundary Flow Rate(j), q<sub>r</sub>(i,j)] for all ramp (i), where q<sub>r</sub>(i,j) = flow rate of ramp i during j
```

TVE: Total number of Vehicles Entered during a given period

```
= Sum [TVE(j)] for all (j)
```

TVX (j): Total # of Vehicles exited during (j)

= Sum $[q_x(i,j), Downstream Boundary Flow (j)]$ for all exit ramp *i*, where $q_r(i,j) = flow$ rate of ramp *I* during *j*

TVX: Total number of vehicles Exited from during a given period

= Sum [TVX(j)] for all (j)



Figure 2.2: Current space-discretization and flow-parameter estimation scheme in TeTRES

Field Detector

2.2.12 Parameters for Traffic-Flow MOE calculation

The following three parameters need to be predefined to calculate the MOEs defined above:

Congestion Threshold Speed

For calculating Congested Miles (CM) and Congested-Mile Hours (CMH) the value of this parameter needs to be defined. The default value used by TeTRES for this parameter is 45 mph.

Lane Capacity

For calculating Lost VMT for congestion (LVMT) and Unused VMT (UVMT) the value of this parameter needs to be defined. The default value used by TeTRES for this parameter is 2200 veh/hr/lane.

Critical Density

For calculating Lost VMT for congestion (LVMT) and Unused VMT (UVMT) the value of this parameter needs to be defined. The default value used by TeTRES for this parameter is 40 veh/mile/lane.

2.3 EXPANSION OF TETRES DATABASE TO STORE TRAFFIC-FLOW MOE VALUES

The first step to incorporate MOE values into TeTRES is to expand the existing database to store those values. The TeTRES database has a specific structure for storing travel-time data for each route for every five minutes. For each year, a separate travel-time table is generated automatically by the system with the following name format: "tt_<year>". For instance, for 2020, the name of the travel-time table is "tt_2020" in the TeTRES database. Figure 2.3 shows the structure of the expanded, yearly travel-time table to store the MOE values.

class TravelTime(object):

```
id = Column(Integer, primary_key=True, unique=True, autoincrement=True)
route_id = Column(Integer, ForeignKey('route.id'), nullable=False)
route = relationship("TTRoute")
time = Column(DateTime, nullable=False)
tt = Column(Float, nullable=False)
vmt = Column(Float, nullable=True)
vht = Column(Float, nullable=True)
dvh = Column(Float, nullable=True)
lvmt = Column(Float, nullable=True)
uvmt = Column(Float, nullable=True)
cm = Column(Float, nullable=True)
acceleration = Column(Float, nullable=True)
meta_data = Column(UnicodeText, nullable=True)
```

Figure 2.3: Expanded Yearly Travel-Time Table

The description of each column of the above table is provided below:

- *id:* This is the primary key of the table which is a unique auto-incrementing integer.
- *route_id:* This column stores the reference route id.
- *time:* This column keeps track of the exact date time.
- *tt:* The travel time in minutes is stored in this column.
- *vmt:* The MOE, Vehicle-Miles Traveled (VMT), is stored in this column.
- *vht:* The MOE, Vehicle-Hours Traveled (VHT), is stored in this column.
- *dvh:* The MOE, Delayed-Vehicle Hours (DVH), is stored in this column.
- *lvmt:* The MOE, Lost VMT due to congestion (LVMT), is stored in this column.
- *uvmt:* The MOE, Unused VMT (UVMT), is stored in this column.
- *cm:* The MOE, Congested-Miles (CM), is stored in this column.
- cmh: The MOE, Congested-Mile Hours (CMH), is stored in this column.
- *acceleration:* The MOE, acceleration, is stored in this column.
- meta_data: This column stores a JSON string having a key-value pair structure that stores the following meta-information for the specific route and time:

- o Flow
- o Speed
- o Density
- o Lanes
- o Speed Limit
- Speed Average
- o Speed Variance
- Speed Max
- o Speed Min
- Speed Difference
- Number of Vehicles Entered
- o Number of Vehicles Exited
- MOE Lane Capacity
- MOE Critical Density
- MOE Congestion Threshold Speed

To facilitate the need-based MOE calculation process by users, a table named

"RouteWiseMOEParameters" was created to track the status of MOE calculations for different routes and for different time ranges. This table allows users to choose the MOE calculation option for the selected routes and time ranges. Figure 2.4 shows the structure of this table.

class RouteWiseMOEParameters(Base):	
tablename = 'route wise moe parameters'	
id = Column(Integer, primary key=True, unique=True, autoincrement=True)	
reference tt route id = Column(Integer, ForeignKey('route.id', ondelete='CASCADE'), nullable=False, index=Tr	ue)
reference tt route = relationship(TTRoute)	
<pre>moe lane capacity = Column(Float, nullable=False)</pre>	
<pre>moe_critical_density = Column(Float, nullable=False)</pre>	
<pre>moe_congestion_threshold_speed = Column(Float, nullable=False)</pre>	
<pre>start_time = Column(DateTime, nullable=True)</pre>	
end_time = Column(DateTime, nullable=True)	
<pre>update_time = Column(DateTime, nullable=False)</pre>	
<pre>status = Column(VARCHAR(255), nullable=True)</pre>	
<pre>reason = Column(VARCHAR(255), nullable=True)</pre>	

Figure 2.4: Route-Wise MOE-Parameters Table

The definition of each column in the above table is described below:

- *id:* This is the primary key of the table which is a unique auto-incrementing integer.
- *reference_tt_route_id:* This column stores the reference route id.
- *moe_lane_capacity:* Lane capacity value used for the current MOE calculation
- *moe_critical_density:* Critical density value used for the current MOE calculation
- *moe_congestion_threshold_speed:* Congestion Threshold Speed value used for the current MOE calculation
- **start_time:** MOE calculation is performed for a specific route for a specific time range. This column keeps track of the exact starting date-time of the time range.
- **end_time:** MOE calculation is performed for a specific route for a specific time range. This column keeps track of the exact ending date-time of the time range.
- **update_time:** This column keeps track of the time when the MOE calculation is triggered for the specific route for the specific time range.
- *status:* This column keeps track of the current status of the calculation.
- **reason:** This column stores the reason for the failure in case the current MOE calculation is failed for some reason.

Further, the above table also keeps track of the values of the key parameters for the MOE calculation, e.g., lane capacity and critical density, so that it can be clear which parameter values were used for which route-MOE calculations, while the specific values of those parameters can be set in the admin client.

2.4 ENHANCEMENT OF ADMIN CLIENT TO INCORPORATE MOE CALCULATION PROCESS

In this study, the admin client of TeTRES is enhanced to handle the traffic-flow MOE calculation process, including the specification of the key parameter values, such as lane capacity and critical density, for computing the MOEs, for selected routes and time periods. Specifically, two sub-tabs were developed and inserted into the main window of the admin client. They include:

- 'Default Parameter' sub-tab, which enables user to enter/change the default values of the MOE parameters,
- 'Route Wise MOE Parameters', where users can enter specific values of the MOE parameter values for selected routes and time periods.

Figure 2.5 shows a new sub-tab, "Default Parameters", which can be accessed through System Configuration \rightarrow Categorization Parameter Settings \rightarrow MOE Parameters tab in the main window of the admin client. As shown in this figure, the default values of the key MOE parameters can be updated in this sub-tab, while changing the default values of these parameters do not have any impact on the existing MOE values calculated with previous default-parameter values.

Iravel Time Reliability Estimation System Client - 1.0.0					
File Tools Help					
Route Config Operating Condition Data Input Data Change Log System Configurations					
Periodic Job Setting Categorization Parameter Setting Current Database Population					
Incident Workzone Special Event MOE Parameters					
Default MOE Paramters Route Wise MOE Parameters					
Critical Density k0.0 vehs/mile/lane					
Lane Capacity 2200.0 vehs/hr/lane					
Congestion Threshold Speed 45.0 miles/hr					

Figure 2.5: Default Parameters Tab

Figure 2.6 shows the screenshot of the 'Route Wise MOE Parameters' sub-tab, where an admin-client user can start calculating the MOE values for a specific route for a specified time interval.

🖢 Travel Time Reliabi	ility Estimation	System Client - 1.0.0									-		>
e Tools Help													
Route Config Operating Cor	ndition Data Input D	ata Change Log System Configu	ations										
Periodic Job Setting Catego	rization Parameter Se	tting Current Database Populati	on										
Incident Workzone Spe	cial Event MOE Para	meters											
Default MOE Paramters	Route Wise MOE Para	meters											
Route	Select One	V	Route	Critical Density	Lane Capacity	Congestion Thresh	Start Date	End Date	Update Time	Status	Reas	on	
			02 I494 to StPaul D	40.0	2200.0	45.0	2019-08-01 00:00:00	2019-08-02 00:00:00	2020-09-13 12:08:45	Completed			
			02 I694 to StPaul D	40.0	2200.0	45.0	2019-08-01 00:00:00	2019-08-03 00:00:00	2020-08-19 19:48:55	Completed			
Critical Density	40.0	vehs/mile/lane	02 I694 to StPaul D	40.0	2200.0	45.0	2019-08-01 00:00:00	2019-08-07 00:00:00	2020-08-19 19:52:03	Completed			
			02 I694 to StPaul D	40.0	2200.0	45.0	2019-08-01 00:00:00	2019-08-07 00:00:00	2020-08-19 20:07:02	Completed			
Lane Capacity	2200.0	vehs/hr/lane	01 I35W to I94	40.0	2200.0	45.0	2019-08-01 00:00:00	2019-08-30 00:00:00	2020-08-19 19:11:26	Completed			
Congestion Threshold Spe	eed 45.0	miles/hr											
Start Date		•											
End Date		*											

Figure 2.6: Route Wise MOE Parameters Tab

As shown above, there are six fields that need to be filled in this tab, while the 3 MOE parameters, i.e., critical density, lane capacity and congestion threshold speed, automatically show the default values specified in the 'Default Parameters' tab:

- Route
- Critical Density
- Lane Capacity
- Congestion Threshold Speed
- Start Date
- End Date

First, the user needs to choose a route from a set of existing defined routes. The user also can change the values of those 3 MOE parameters in this tab if necessary. The newly updated parameter values will only be effective for those selected route and time periods. Then the user needs to select the start and end date. Based on these values, the server will start calculating MOE values for a selected route and the status of the MOE calculation process can also be shown in the 'status' column of the same tab. Figures 2.7 and 2.8 show some of the codes developed for enhancing the admin client and tracking route-wise MOE parameters. Figure 2.7 shows the first function that's invoked by the admin client to calculate the MOE values for a specific route and period, while the major functions in the server to process the 'Route-wise MOE calculation' are listed in Figure 2.8.



Figure 2.7: First Function Called in TeTRES Server for MOE Calculation



Figure 2.8: Major Server Functions for Processing Route-specific MOE Calculation

2.5 ENHANCEMENT OF THE TRAVEL-TIME AND RELIABILITY CALCULATION MODULE

To calculate and store the traffic-flow MOE values for selected routes, in this study, a set of new functions have been developed and inserted into the TeTRES server. After the admin client initiates the MOE calculation for specific routes and periods, the server starts calculating the MOE values and stores them in the expanded TeTRES database in the background. Figure 2.9 shows the functions initiating the MOE calculation module. The main function that handles all the MOE calculations is named "calculate_tt_moe_a_route", whose code snippet is listed in the Appendix. The code snippets of the individual functions used to calculate specific MOE values are also included in the Appendix B.

def	update moe values(rw moe param ison, db info =None , *args, **kwargs):	
	ry more param ison['ry more start date'] = datetime.datetime.strotime(ry more param ison['ry more start date'].	
	\$\V_\$m_\$d \$\theta\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	
	rw moe param ison['rw moe end date'] = datetime.datetime.strotime(rw moe param ison['rw moe end date'].	
	'%/-%m-%d'%H%%'_date()	
}	_update_moe_values(rw_moe_param_json, db_info=db_info, *args, **kwargs)	
def	_update_moe_values(rw_moe_param_json,	
3	db_info=None, *args, **kwargs):	
	logger = getLogger(name)	
-	logger.debug('>> Updating MOE Values')	
	<pre>worker_process_to_update_moe_values(rw_moe_param_json['rw_moe_start_date'], rw_moe_param_json['rw_moe_end_date'],</pre>	
	db_info, rw_moe_param_json=rw_moe_param_json)	
]	logger.debug('<< End of Updating MOE Values')	
def	_worker_process_to_update_moe_values(start_date, end_date, db_info, **kwargs):	
	from pyticas_tetres.db.tetres import conn	
	from pyticas.tool import tb	
	logger = getLogger(name)	
	stime = datetime.time($0, 0, 0, 0$)	
	etime = datetime.time(23, 55, θ , θ)	
	<pre>daily_periods = period.create_periods(start_date, end_date, stime, etime, cfg.TT_DATA_INTERVAL, target_days=[0, 1, 2</pre>	, 3, 4, 5, 6], remove_holiday =False)
	logger.debug('>>> Starting Multi Processing (duration= %s to %s)' % (start_date, end_date))	
	rw_moe_param_json = kwargs.get("rw_moe_param_json")	
	<pre>reference_tt_route_id = rw_moe_param_json.get('reference_tt_route_id')</pre>	
	if db_into:	
	conn.connect(db_into)	
	da_route = TTRouteDataAccess()	
	ttri = da_route.get_by_id(reference_tt_route_id)	
3	if not ttri:	
	logger.debug('route is not found (%s)' % (reference tt_route_id))	
	return	
1	for pinx, pro in enumerate(daily_periods):	
	it pro is wone:	
	ud_ioute.ctose_session()	
	dy route close session()	
	travaling cloudest travely route(and thri decession-de route est session()	
	craverchine.carculate_tr_inote_a_foureprint, tubesta_True	
	ac collect()	
	event Excention as ex:	
ľ	th traceback(ex)	
	continue	
L		

Figure 2.9: Functions for Facilitating MOE calculation

2.6 ENHANCEMENT OF USER CLIENT TO EXTRACT MOE VALUES

Figure 2.10 shows the data-flow process between the TeTRES server and the user-client, which has been modified in this task to allow the user to specify if the calculated and stored MOE values for selected routes would be extracted from the database and written to a separate spreadsheet file. This action is invoked in the main window of the user client by clicking the Estimate button with the '*Include MOE*' *Spreadsheet*' checkbox selected, as shown in Figure 2.11. By clicking the checkbox, the user client sends

a signal to the server, which creates and populates the spreadsheet file, "traveltime-moe-data.xlsx", where all the MOE values calculated for selected routes are contained.



Figure 2.10: Data-Flow Process Between User-Client and Server for MOE Extraction and Writing



Figure 2.11: Screenshot of User-Client with Added Checkbox for Extracting MOE values

Currently, the following traffic-flow MOE measures are stored every 5 min-interval in the Excel spreadsheet file for each selected route and period:

- Travel Time
- Speed
- VMT
- VHT
- DVH
- UVMT
- *CM*
- *CMH*
- Acceleration
- Number of Vehicles Entered
- Number of Vehicles Exited
- Speed Average
- Speed Variance
- Max Speed
- Min Speed
- Speed Difference

Figures 2.12 - 2.14 include some of the codes developed for extracting MOE values and writing them to a spreadsheet file.

Package / File: user.panels.estimation.PanelEstimation.java
 private JCheckBox chkWriteMoeSpreadsheet;
Class: PanelEstimation
Method: initComponents {building Checkbox}
 chkWriteMoeSpreadsheet = new JCheckBox();
 chkWriteMoeSpreadsheet.setSelected(false);
 chkWriteMoeSpreadsheet.setText("Include MOE Spreadsheet"); .addComponent(chkWriteMoeSpreadsheet)
Class: PanelEstimation
Method: getRequestOption
 eri.write_moe_spreadsheet = this.chkWriteMoeSpreadsheet.isSelected();
Class: PanelEstimation
Method: loadRequestedInfo
 this.chkWriteMoeSpreadsheet.setSelected(prevEri.write_moe_spreadsheet);

Figure 2.12: Code Snippets of PanelEstimation Class Managing MOE Extraction Request



Figure 2.13: Snippets of Codes for Requesting and Receiving MOE values from Server
```
def write_moe_data_sheet(eparam, ext_filter_groups, wb):
      Created function to
                                      write moe data to spi
     :type eparam: pyticas_tetres.ttypes.EstimationRequestInfo
:type ext_filter_groups: list[pyticas_tetres.rengine.filter.ftypes.ExtFilterGroup]
:type wb:xlsxwriter.Workbook
     def clean(v):
          return v if v != math.inf and v and v > 0 else ''
     def cleanMOE(v):
           return v if v != math.inf and v and v >= 0 else ''
     for idx, ef in enumerate(ext_filter_groups):
          ws = wb.add_worksheet('MOE Data (OC=%d)' % idx)
          ws.write_row(0, 0, ['Operating Condition:', eparam.operating_conditions[idx].name])
          ws.write_row(0, 0, ['Operating Conditio
ws.write_row(1, 0, [
    ''moe-values', '', '', '',
    'Speed Variation', '', '', '',
    'MOE Parameters', '',
    'weather', '', '', '',
    'incident', '', '', '',
    'special-event', '', '', '',
    'snow-management'
           i)
          ws.write_row(2, 0, [
    'time', 'tt', 'speed', 'vmt',
    'vht', 'dvh', 'lvmt', 'uvmt',
    'cm', 'cmh', 'acceleration', 'number_of_vehicles_entered', 'number_of_vehicles_exited',
                'speed_average', 'speed_variance', 'speed_max_u', 'speed_min_u', 'speed_difference',
'moe_lane_capacity', 'moe_critical_density', 'moe_congestion_threshold_speed',
                'usaf', 'wban', 'precip_type', 'precip', 'precip_intensity', # weather
'type', 'impact', 'cdts', 'udts', 'xdts', 'distance', 'off_distance', # incident
'name', 'lane_config', 'closed_length', 'location', 'off_distance', # workzone
'name', 'distance', 'attendance', 'type', # special event
'truck_route', 'road_status', 'location', 'off_distance'
          j)
          for idx, extdata in enumerate(ef.whole_data):
               x = extdata.tti
               dts = x.time.strftime('%Y-%m-%d %H:%M')
               try:
                   meta_data = json.loads(x.meta_data)
               except Exception as e:
                    print(e)
                    meta data = {}
               moe_lane_capacity = cleanMOE(meta_data.get("moe_lane_capacity", 0))
               moe_critical_density = cleanMOE(meta_data.get("moe_critical_density", 0))
               moe_congestion_threshold_speed = cleanMOE(meta_data.get("moe_congestion_threshold_speed", 0))
               speed_average = cleanMOE(meta_data.get("speed_average", 0))
               speed_variance = cleanMOE(meta_data.get("speed_variance", 0))
               speed_max_u = cleanMOE(meta_data.get("speed_max_u", 0))
               speed_min_u = cleanMOE(meta_data.get("speed_min_u", 0))
               speed difference = cleanMOE(meta_data.get("speed_difference", 0))
               number_of_vehicles_entered = cleanMOE(meta_data.get("number_of_vehicles_entered", 0))
               number_of_vehicles_exited = cleanMOE(meta_data.get("number_of_vehicles_exited", 0))
               ws.write_row(idx + 3, 0, [
                    dts, clean(x.tt), clean(x.speed), clean(x.vmt),
                    cleanMOE(x.vht), cleanMOE(x.dvh), cleanMOE(x.lvmt), cleanMOE(x.uvmt),
                    cleanMOE(x.cm), cleanMOE(x.cmh), cleanMOE(x.acceleration), number_of_vehicles_entered,
                    number of vehicles exited.
                    speed_average, speed_variance, speed_max_u, speed_min_u, speed_difference,
                    moe_lane_capacity, moe_critical_density, moe_congestion_threshold_speed,
               1
                                + get_weather_values(extdata)
                                + get_incident_values(extdata)
                                + get_workzone_values(extdata)
                                + get_specialevent_values(extdata)
                                + get_snowmangement_values(extdata)
                                )
```

Figure 2.14: Code Snippets for Writing MOE values in a Spreadsheet file

CHAPTER 3: POPULATION OF TETRES DATABASE FOR METRO FREEWAY NETWORK

3.1 INTRODUCTION

In this chapter, the database of TeTRES is populated with the historical data, also collected in this study for estimating the travel-time reliability measures for the metro freeway network. The specific types and sources of the data collected and processed in this chapter are as follows:

- 1) Freeway traffic-detector data from RTMC, MnDOT
- 2) Weather data from NOAA (National Oceanic and Atmospheric Administration)
- 3) Incident data from CAD (Computer-Aided-Dispatch system, Department of Public Safety) and IRIS (Intelligent Roadway Information System, MnDOT)
- 4) Special-event data from the venues located in the metro area
- 5) Metro freeway Work-Zone data from Metro District, MnDOT
- 6) Winter road-surface data from Metro District, MnDOT

In the above list, the traffic-detector data are stored in a local hard disk in a structured format, while all other types of data are processed and stored in the TeTRES database (T-database), whose schema and table formats were explained in the previous phase of this research [1]. Further, the weather data from NOAA has been automatically downloaded and stored in the database by the weather-data processing module in TeTRES. For the incident data, the CAD/IRIS incident databases of the past periods were provided off-line by RTMC, MnDOT, and the incident-data loading module of TeTRES was applied to load them into the T-database. It can be noted that the incident-data loading module in TeTRES is also designed to directly load the incident data from those databases if they can be directly accessed by TeTRES in an integrated operating environment. The other types of data, i.e., special event, work zone and winter-road surface data, have been collected manually and organized in a set of Excel-formatted files for each data type. Those Excel files are then batch-processed by a set of the scripts, developed in this task, for an efficient population of the T-database. The rest of this chapter summarizes the types of the data collected and the process to populate the T-database.

3.2 COLLECTION AND PROCESSING OF SPECIAL EVENT DATA

First, the data for the metro-area special events, defined as those with large attendance big enough to affect the traffic conditions on the freeways linked to event locations, are collected manually using the publicly available data sources, such as fan-based websites, for the period of 1/2012- 3/2020. The collected data for each event includes event name, date/start/end times, attendance, and coordinates of event location. The following list shows the names of the special event locations and the sources of the event data collected for each location:

3.2.1 Mall of America Stadium

- Main events: National Football League games
- Data Sources:
 - o <u>https://www.pro-football-reference.com/boxscores/</u>
 - o <u>https://www.espn.com/nfl/team/schedule/_/name/min/</u>

3.2.2 Target Center

- Main Events: National Basketball Association Games
- Data Sources:
 - o <u>https://www.basketball-reference.com/teams/MIN/</u>

3.2.3 Target Field

- Main events: Major League Baseball games
- Data Sources:
 - o <u>https://www.baseball-reference.com/teams/MIN/</u>
 - o <u>https://www.espn.com/mlb/team/schedule/_/name/min/</u>

3.2.4 US Bank Stadium

- Main Events: National Football League Games
- Data Sources:
 - o https://www.pro-football-reference.com/boxscores/
 - o https://www.espn.com/nfl/team/schedule/ /name/min/

3.2.5 TCF Bank Stadium

- Main Events: College Football Games
- Data Sources:
 - o https://www.espn.com/nfl/team/schedule/_/name/min/

3.2.6 Xcel Energy Center

- Main Events: National Hockey League Games
- Data Sources:
 - o <u>https://www.hockey-reference.com/leagues/</u>
 - o <u>https://www.nhl.com/gamecenter/</u>

<u>https://www.arcticicehockey.com/2012/5/15/3001213/nhl-average-length-of-regular-season-ot-by-year</u>

All the collected data from the above sources have been organized in an Excel spreadsheet format for each year. Table 3.1 shows a portion of the sample spreadsheet-formatted, special-event data file, collected for the 2012-2013 season, at the Mall of America Stadium in Minneapolis, Minnesota, before its name was changed to US Bank Stadium.

DATE	START	END	TITLE	TYPE	ATTEND	LAT	LON
2012-09-09	12:00:00	15:15:00	MN Vikings Vs Jacksonville Jaguars	Football	56607	44.9738927	-93.2602443
2012-09-23	12:00:00	15:15:00	MN Vikings Vs San Francisco 49ers	Football	57288	44.9738927	-93.2602443
2012-10-07	15:15:00	18:35:00	MN Vikings Vs Tennessee Titans	Football	57652	44.9738927	-93.2602443
2012-10-21	12:00:00	15:00:00	MN Vikings Vs Arizona Cardinals	Football	61068	44.9738927	-93.2602443
2012-10-25	19:30:00	22:50:00	MN Vikings Vs Tampa Bay Buccaneers	Football	60860	44.9738927	-93.2602443
2012-11-11	12:00:00	15:00:00	MN Vikings Vs Detroit Lions	Football	64059	44.9738927	-93.2602443
2012-12-09	12:00:00	15:15:00	MN Vikings Vs Chicago Bears	Football	64134	44.9738927	-93.2602443
2012-12-30	15:20:00	18:40:00	MN Vikings Vs Green Bay Packers	Football	64134	44.9738927	-93.2602443
2012-08-17	19:00:00	22:15:00	MN Vikings Vs Buffalo Bills	Football	64121	44.9738927	-93.2602443
2012-08-24	19:00:00	22:15:00	MN Vikings Vs San Diego Chargers	Football	64121	44.9738927	-93.2602443
2013-09-22	12:00:00	15:30:00	MN Vikings Vs Cleveland Browns	Football	63672	44.9738927	-93.2602443
2013-10-13	12:00:00	15:00:00	MN Vikings Vs Carolina Panthers	Football	63963	44.9738927	-93.2602443
2013-10-27	19:30:00	22:30:00	MN Vikings Vs Green Bay Packers	Football	64134	44.9738927	-93.2602443
2013-11-07	19:35:00	22:35:00	MN Vikings Vs Washington Redskins	Football	64011	44.9738927	-93.2602443
2013-12-01	12:00:00	15:50:00	MN Vikings Vs Chicago Bears	Football	64134	44.9738927	-93.2602443
2013-12-15	12:00:00	15:30:00	MN Vikings Vs Philadelphia Eagles	Football	64087	44.9738927	-93.2602443
2013-12-29	12:00:00	14:55:00	MN Vikings Vs Detroit Lions	Football	64134	44.9738927	-93.2602443
2013-08-09	19:00:00	15:15:00	MN Vikings Vs Houston Texas	Football	64121	44.9738927	-93.2602443
2013-08-29	19:00:00	15:15:00	MN Vikings Vs Tennessee Titans	Football	64121	44.9738927	-93.2602443

Table 3.1: Sample Organized Data for Special Events at Mall of America Stadium in 2012-13

3.3 DATABASE POPULATION FOR SPECIAL EVENT DATA WITH BATCH-LOADING PROCESS

Figure 3.1 shows the special-event data input module of the TeTRES Admin Client, where an administrator can enter manually each event data while the location of an event place can be directly located on the map. In this study, a batch-loading process was developed for an efficient population of the T-database with all the special-event data collected and organized in a set of Excel-formatted files as shown in the previous section. Figure 3.2 shows a screenshot of the main module of the Python script, which was designed to read the Excel-formatted special-event data files and load them to the T-database. The resulting T-database contains the special-event data at the metro area from 1/2012 until 3/2020.

ork zone ope	cial Events Road Con	dition during Snow Event		
	Add Special E	Event	💰 Edit	
lter			Name	
All years		~	•	orth
pecial Event Lis	ŧ		Description	
Duration	Name	Attendance		
			· · · · · · · · · · · · · · · · · · ·	231488
			Start Date Time	
				230 9B
			End Date Time	88 231A
			Attendance	2318
			Coordinates (right click on the map)	CR 22
			Canad	
			Caricer Save	- + R 22
			MN 100	
				CP 152

Figure 3.1: Special-Event Data Input Screen of TeTRES Admin Client



Figure 3.2: Snippets of the Python Script for Uploading Special-Event Data to TeTRES Database

3.4 COLLECTION AND PROCESSING OF WORK-ZONE DATA

The work-zone data for TeTRES were collected from the construction-project maps provided by the Metro District, MnDOT, for the 2012-2020 period. Figure 3.3 shows the 2017 construction-project map, showing the information of each work zone, i.e., start/end locations and period, of all the roadway-construction projects implemented in the metro district in 2017. In this task, the information for each construction project on the freeways of the metro area were manually converted from the construction-project maps to a set of the Excel-formatted files, which were then used to populate the TeTRES database with the newly developed Python script. In particular, the detector station IDs corresponding

to the start/end locations of each project were identified for each direction by comparing each year's project map with the MnDOT detector-station map. Table 3.2 shows a sample Excel-formatted workzone data file containing the information for all the freeway work zones in 2017. Figure 3.4 also shows the Python script developed in this task to populate the T-database with those Excel-formatted workzone data files organized for each year. The resulting T-database contains the data for all the freewaywork zones in the metro network from 4/2012 to 9/2020.



Figure 3.3: Construction Project Map for 2017

Table 3.2: Sample Excel-formatted File for 2017 Work-Zone Data

Rote ID #	Memo	Start	End	Begin Station	End Station	IMPACT
SP# 1981-129	No Lane Config Available	07/27/2016	1/27/2017	S71	S71	MED
SP# 2771-37	No Lane Config Available	09/22/2014	06/30/2017	S1958	S1961	MED
SP# 2772-105	No Lane Config Available	01/23/2017	10/31/2017	S430	S442	н
SP# 2772-110	No Lane Config Available	01/23/2017	10/31/2017	S438	S438	ні
SP# 2772-113	No Lane Config Available	01/23/2017	10/31/2017	S430	S431	ні
SP# 2781-432	No Lane Config Available	03/01/2017	07/01/2018	S86	S128	н
SP# 2781-467	No Lane Config Available	09/11/2017	08/17/2018	S1813	S466	MED
SP# 2782-327	No Lane Config Available	08/07/2017	10/22/2021	S57	S565	MED
				S76	S76	
				S64	S64	
SP# 6280-390	No Lane Config Available	05/08/2017	09/22/2017	S832	S832	MED
SP# 6283-175	No Lane Config Available	04/18/2016	11/01/2017	S779	S780	н
SP# 6283-233	No Lane Config Available	04/18/2016	11/01/2017	S1045	S1045	н
SP# 6283-234	No Lane Config Available	04/18/2016	11/01/2017	S780	S1047	ні
SP# 6285-143	No Lane Config Available	04/11/2016	10/27/2017	S1461	S1086	ні
SP# 8205-141	No Lane Config Available	07/10/2017	10/13/2017	S1923	S1923	MED

```
import sys
 3 sys.path.append("server")
    import common
    from pyticas import ticas
6 from pyticas.infra import Infra
7 from pyticas_tetres import api_urls_admin
9 from tetres_data_populator.work_zone_data.work_zone_api_reader import WorkZoneAPIReader
10 from tetres_data_populator.work_zone_data.work_zone_api_writer import WorkZoneAPIWriter
11 from tetres_data_populator.work_zone_data.work_zone_excel_reader import WorkZoneExcelReader
13 if __name__ == "__main__":
14
        ticas.initialize(common.DATA_PATH)
        infra = Infra.get_infra()
        base_url = "http://localhost:5000"
        only_file_names = ["work_zone_2010.xlsx", "work_zone_2011.xlsx", "work_zone_2012.xlsx", "work_zone_2013.xlsx", "work_zone_2014.xlsx"]
        for only_file_name in only_file_names:
20
            work_zone_filename = "server/data_populator_libs/tetres_data_populator/excel_data/work_zone_data/{}".format(only_file_name)
            work_zone_excel_reader = WorkZoneExcelReader(filename=work_zone_filename)
            # adding work zone group data
            work_zone_api_writer = WorkZoneAPIWriter(infra=infra)
            work_zone_api_writer.post_data(excel_reader=work_zone_excel_reader,
                                           post_url=base_url + api_urls_admin.WZ_GROUP_INSERT,
                                           list_url=base_url + api_urls_admin.WZ_GROUP_LIST,
                                           data_type="work_zone_group",
                                           api_reader_class=WorkZoneAPIReader)
            work_zone_api_writer.post_data(excel_reader=work_zone_excel_reader,
                                           post_url=base_url + api_urls_admin.WZ_INSERT,
                                           list_url=base_url + api_urls_admin.WZ_LIST_ALL,
                                           data_type="work_zone",
                                           api_reader_class=WorkZoneAPIReader)
            work_zone_api_writer.populate_work_zone_years(api_reader=WorkZoneAPIReader())
```

Figure 3.4: Snippets of the Python script for Processing Work-Zone Data Files

3.5 COLLECTION AND PROCESSING OF WINTER ROAD-CONDITION DATA

Tables 3.3 and 3.4 show a portion of the plow-route and snow-event data provided by the Metro District, MnDOT for the 2016-19 period. First, the detector station IDs for the start and end location of each plow route in the metro-freeway network are identified by visually matching the MnDOT detectorstation map with the plow-route locations from Table 3.3. The station IDs for each plow route were then organized in an Excel-format. Table 3.5 shows a portion of the Excel-formatted file with the station data for each plow route. Next, the detailed information for each snow event on the metro freeways, i.e., event date/time, route ID and bare-lane lost/regain times, were extracted from the MnDOT snow-event data shown in Table 3.6 and stored in a set of the Excel-formatted files. Table 3.6 includes a converted snow-event data file. Those Excel-formatted files with the converted snow-event data were then used for populating the TeTRES database. Figure 3.5 shows the snippets of the Python script developed in this study to populate the TeTRES database with those Excel-formatted snow-event data.

It can be noted that, according to the MnDOT 'Snow and Ice Event/Bare Lane Training, 2016-2017' [2], the 'bare lanes' are defined as those with 'all driving lanes are 95% free of snow and ice between the outer edges of the wheel paths and have less than 1 inch of accumulation on the center of the roadway'. Using the bare-lane lost/regain time data for each snow-event, TeTRES identifies 'Lost' or 'Normal' freeway segments, which are linked to each travel-time route defined by the TeTRES Admin Client, so that the travel-time reliability for given routes under the 'lane-lost' or 'normal' conditions can be estimated. In this study, the snow-event data from 2012 until 2019 have been collected, processed, and uploaded to the T-Database.

Route_NM	TRoad_Cla	Route_ 💌	Hwy_NM	From_Loc	To_Loc 💌	MP_Begin	True_Mi_ 🔻	MP_Enc 🔻	True_Mi_ 🔻
TP5A0101	SC	2016	US10	3 exit ramp to US 169 NB/MN 101	S end of BR#2003 BNSF RR	214.553	216	230.402	232
TP5A0471	UC	2016	MN47	st Junction US10/East Junction US	Pederson Dr./Cty 81	20.662	21	34.802	35
TP5A0651	SC	2016	MN65	Aain St CSAH-14 X-ING/BR# 0205:	Anoka Co. Rd. 24/237th Ave NE in East Bethel	15.42	17	29.389	31
TP5A1691	SC	2016	US169	yton Rd, N Jct CSAH-12, LT Miller u	nction T.H. 10, Junction MN 47/BR #9713, Ferry St in A	145.715	143	146.874	144
TP5B0941	SC	2016	194	1 County line, NW end of Bridge 27	EB exit loop to CSAH-81	205.367	206	220.541	221
TP5B1011	SC	2016	MN101	on Hennepin Co. Rd. 81 in Rogers Jo	urne County Line (North end of Bridge 86005 over Mis	39.64	40	46.35	46
TP5B1691	SC	2016	US169	Junction T.H. 55 in Plymouth	Dayton Rd N Jct CSAH-12, LT Miller RD	130.94	129	145.715	143
TP5B4941	SC	2016	1494	BR #27905 Under Fish Lake Road	Junction I-94 (Fish Lake Interchange)	27.15	27	27.973	28
TP5B6101	UC	2016	MN610	WB EXIT RAMP TO WB CSAH 81	WB exit ramp to West Broadway CSAH 103	2.486	2	6.122	6
TP5C0121	SC	2016	US12	lependence, W Urban boundary N	Junction I-394/I-494 in Minnetonka	145.522	145	156+1.014	157
TP5C0551	UC	2016	MN55	27044; Wright-Hennepin Co/N enc	East end BR# 27785 over I94	165.336	165	189.677	190
TP5C3941	SC	2016	1394	Junction I-494 in Minnetonka	Junction T.H. 100 in Golden Valley	0	0	5.925	6
TP5C4941	SC	2016	1494	Junction MN 7 - Bridge 27V6O	BR# 27905 Under Fish Lake Road	16.259	16	27.15	27
TP5E0010	SC	2016	US10	tween I694EB ent ramp and MN5	East junction of US10/I694 (EB)	240.592	243	240+01.183	243
TP5E0941	SC	2016	194	EB exit loop to CSAH-81	Junction MNTH - 252	220.541	221	224.977	226
TP5E1001	SC	2016	MN100	Junction T.H. 62 in Edina	Junction I-694 in Brooklyn Center	2.139	2	16.158	16
TP5E6941	SC	2016	1694	Junction I-94 in Brooklyn Center	W end BR# 9209/9210 over Island Lake Channel	34.197	34	43.45	43
TP5F0101	SC	2016	US10	E end of BR# 9722 EB over BNSF R	I-35W North Jct.	230.402	232	237.035	239
TP5F0101	SC	2016	US10	Junction I35W NB/US910A	Between MN51 and junction with I694	238.393	240	240.592	243
TP5F0101	UC	2016	US910A	Cty Rd H Crossing, CSAH 9&10	Jct I-35W NB, US 10	237.906	240	238.393	240
TP5F0471	UC	2016	MN47	Anoka County Line at Minneapolis	E Junction USTH-10	5.37	5	13.076	13

Table 3.3: Sample Data for Plow-Route Locations from MnDOT

Table 3.4: Sample Snow-Event Data from MnDOT

Project Id 🚽	Project Maintenance Route	Service Level Name	Event Begin Date Time	Event End Date Time	Event Duration Hours	Lane Lost Date Time 🚽	Lane Regain Date Time 🔻
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	12/4/2016 12:00:00 AM	12/4/2016 5:30:00 AM	5.5	12/4/2016 1:00:00 AM	12/4/2016 6:00:00 AM
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	12/10/2016 12:00:00 PM	12/11/2016 11:00:00 AM	23.0	12/10/2016 5:00:00 PM	12/11/2016 11:00:00 AM
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	12/15/2016 9:45:00 PM	12/17/2016 6:30:00 AM	32.8	12/15/2016 10:15:00 PM	12/19/2016 10:00:00 AM
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	12/23/2016 10:30:00 AM	12/23/2016 3:45:00 PM	5.3		
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	1/2/2017 2:30:00 PM	1/3/2017 3:30:00 AM	13.0		
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	1/9/2017 11:30:00 AM	1/9/2017 4:30:00 PM	5.0	1/9/2017 12:00:00 PM	1/9/2017 9:30:00 PM
TP5P1691	MN282/2ND ST-JCT MN 41/CSAH78	SUPER COMMUTER	1/10/2017 4:00:00 AM	1/10/2017 2:30:00 PM	10.5	1/10/2017 5:30:00 AM	1/10/2017 5:15:00 PM

Table 3.5: Sample Plow-Route Location Data with Detector Station IDs

Station name	Plow ID	Starting Station	Ending Station
	TP5H0521	S1943	S122
	TP5H0941	S170	S2203
	TP5H35W1	S565	S573
Camden	TP5H3941	S281	S291
	TP5J0051	S495	S497
	TP5J0551	S533	S818
	TP5J0621	S301	S1135
	TP5J0651	S560	S1810
	TP5J0771	S920	S531
	TP5J1001	S375	S378
	TP5J35W1	ST3501	S23
Cedar	TP5J4941	S474	S494
	TP9F0361	S590	S597
	TP9F0521	S1176	S1178
	TP9F0611	S1039	S1944
	TP9F0941	S2203	S1044
	TP9F2801	S1466	S1471
	TP9F35E1	S838	S1489
Maryland	TP9F6941	S1078	S1454
	TP9B0351	S1591	S1511
Forest Lake	TP9B35E1	S1485	S1504
North Branch	TP9A0351	S1512	ST3583
	TP9K0361	S599	S1852
	TP9K0941	S1045	S1358
	TP9K4941	S1029	S1034
Oakdale	TP9K6941	S1398	S1028

Table 3.6: Sample Snow-Event Data for Freeway-based Routes

Event Start	Event End	Affected Routes	Lane Lost Time	Lane Regain Time
11/18/2016 2:30:00 PM	11/18/2016 10:00:00 PM	TP9K0941		
		TP5E1001	11/22/2016 10:00:00 PM	11/23/2016 4:30:00 AM
11/22/2016 7:00:00 AM	11/23/2016 11:00:00 AM	TP5E6941	11/22/2016 10:00:00 PM	11/23/2016 3:30:00 AM
		TP5B0941	11/22/2016 11:00:00 PM	11/23/2016 5:00:00 AM
11/22/2016 7:30:00 AM	11/23/2016 5:00:00 AM	TP5B1691	11/22/2016 11:00:00 PM	11/23/2016 5:00:00 AM
		TP5J0551	11/22/2016 9:00:00 PM	11/23/2016 2:30:00 AM
11/22/2016 8:00:00 AM	11/23/2016 1:00:00 AM	TP5J0771	11/22/2016 9:00:00 PM	11/23/2016 2:30:00 AM
11/22/2016 9:00:00 AM	11/23/2016 1:00:00 AM	TP9K0941	11/22/2016 7:00:00 PM	11/23/2016 2:30:00 AM
11/22/2016 2:00:00 PM	11/23/2016 2:00:00 AM	TP9P35E1	11/22/2016 9:00:00 PM	11/23/2016 2:30:00 AM
11/22/2016 2:00:00 PM	11/23/2016 1:00:00 AM	TP9P35W1	11/22/2016 8:00:00 PM	11/23/2016 1:30:00 AM
11/22/2016 3:00:00 PM	11/23/2016 1:00:00 AM	TP9M4941	11/23/2016 4:00:00 AM	11/23/2016 9:30:00 AM
11/22/2016 3:00:00 PM	11/23/2016 2:00:00 AM	TP9F0941	11/22/2016 10:00:00 PM	11/23/2016 1:30:00 AM
11/22/2016 3:00:00 PM	11/23/2016 3:30:00 AM	TP9F35E1	11/22/2016 11:00:00 PM	11/23/2016 2:00:00 AM
11/22/2016 3:15:00 PM	11/23/2016 3:30:00 AM	TP9F6941	11/22/2016 11:00:00 PM	11/23/2016 2:00:00 AM
11/22/2016 5:00:00 PM	11/23/2016 12:00:00 PM	TP9B35E1	11/22/2016 6:00:00 PM	11/23/2016 4:00:00 AM
		TP5M1691	11/23/2016 1:00:00 AM	11/23/2016 4:00:00 AM
11/22/2016 6:00:00 PM	11/23/2016 2:15:00 AM	TP5M2121	11/22/2016 7:00:00 PM	11/23/2016 2:45:00 AM
11/22/2016 6:30:00 PM	11/23/2016 10:00:00 AM	TP5P1691	11/22/2016 6:00:00 PM	11/23/2016 5:30:00 AM
11/22/2016 7:30:00 PM	11/23/2016 11:30:00 AM	TP9N0611	11/22/2016 7:30:00 PM	11/23/2016 2:00:00 AM

1	import sys
2	
з	sys.path.append("server")
4	import common
5	from pyticas import ticas
6	from pyticas.infra import Infra
7	<pre>from pyticas_tetres import api_urls_admin</pre>
8	
9	from tetres_data_populator.snow_event_data.snow_event_api_reader import SnowEventAPIReader
10	from tetres_data_populator.snow_event_data.snow_event_api_writer import SnowEventAPIWriter
11	from tetres_data_populator.snow_event_data.snow_event_excel_reader import SnowEventExcelReader
12	
13	ifname == "main":
14	ticas.initialize(common.DATA_PATH)
15	<pre>infra = Infra.get_infra()</pre>
16	
17	base_url = "http://localhost:5000"
18	
19	only_file_name = input("Enter the name of the snow event excel file: ")
20	<pre>snow_event_filename = "server/data_populator_libs/tetres_data_populator/excel_data/snow_event_data/{}".format(only_file_name)</pre>
21	<pre>snow_event_excel_reader = SnowEventExcelReader(filename=snow_event_filename)</pre>
22	
23	# adding snow route data
24	snow_event_api_writer = SnowEventAPIWriter(infra)
25	<pre>snow_event_api_writer.post_data(excel_reader=snow_event_excel_reader,</pre>
26	<pre>post_url=base_url + api_urls_admin.SNR_INSERT,</pre>
27	list_url=base_url + api_urls_admin.SNR_LIST,
28	data_type="snow_route",
29	api_reader_class=SnowEventAPIReader)
30	
31	# adding snow event data
32	<pre>snow_event_api_writer.post_data(excel_reader=snow_event_excel_reader,</pre>
33	<pre>post_url=base_url + api_urls_admin.SNE_INSERT,</pre>
34	list_url=base_url + api_urls_admin.SNE_LIST,
	data_type="snow_event",
	api_reader_class=SnowEventAPIReader)
38	# adding snow management data
	<pre>snow_event_api_writer.post_data(excel_reader=snow_event_excel_reader,</pre>
40	<pre>post_url=base_url + api_urls_admin.SNM_INSERT,</pre>
41	list_url=base_url + api_urls_admin.SNM_LIST_ALL,
42	data_type="snow_management",
43	api_reader_class=SnowEventAPIReader)

Figure 3.5: Snippets of the Python Script for Populating Snow-Event Data

3.6 DATA CATEGORIZATION FOR DEFINED ROUTES

As described in the previous sections, in this study, various types of historical data needed for TeTRES were collected, processed, and loaded into the TeTRES database, which contains the following data sets:

- Special event data for 1/2012-3/2020
- Work zone data from 4/2012 9/2020
- Winter road condition data from 10/2012 4/2019
- Weather data from NOAA for 2010 9/30/2020
- Incident data from CAD/IRIS database for 2010 9/30/2020
- Traffic-detector data (stored separately in a local hard disk): 2010 9/2020.

It needs to be noted that the traffic-flow data from the field detector stations have been collected separately and stored in a local hard disk in a structured directory for the period of 2010 – 2020. In this study, the travel times of 116 routes, defined in the previous phase for the travel-time reliability analysis, have also been calculated for every 5-minute interval from 1/1/2010 until 9/30/2020 and stored in the travel-time database, a subset of the TeTRES database. Figure 3.6 shows the screenshots of the TeTRES admin client showing the list of currently defined routes. Finally, the categorization of the non-traffic data, e.g., weather and incident, collected/processed in this chapter, was performed and the categorized external data were linked to each route's travel-time data. Figures 3.7 – 3.9 include the sample database tables showing the links between the travel-time and non-traffic data for the selected routes. The resulting TeRES database, which contains each route's 5-minute travel-time data linked to

the non-traffic external condition data during the same 5-minute interval, can be applicable for estimating the reliability measures under different operating conditions for selected routes and periods. The populated TeTRES database can be accessed at the following link: https://drive.google.com/open?id =1iA8_27j_4Htw9oAtrWSnNbjgo_X23W5m

The state of the second s	- 5	1.05 (ND)	01 CRZ0 to Court Collit			1	
tich		I-35 (NB)	02 North Split to Wyoming	1-694 (EB)	04 1H36 to 194	T.H.36 (WB)	03 I694 to I35E
On an other to be to the		I-35 (SB)	01 Wyoming to North Split	1-694 (EB)	01 194 to TH36	T.H.5 (EB)	01 I494 to TH55
- Montaining press		I-35 (SB)	02 South Split to CR70	1-694 (WB)	02 TH36 to 135E	T U E (MD)	
tilde Think the	le (Tydy	I-35E (NB)	01 South Split to I494	1-694 (WB)	03 135E to 135W	1.H.5 (WB)	U1 TH55 to 1494
	300	I-35E (NB)	02 I494 to StPaul Downtown	1-694 (WB)	04 135W to TH100	T.H.52 (NB)	01 TH55 to I94
the state	3015	I-35E (NB)	03 StPaul Downtown to I694	I-694 (WB)	05 I94 to I35E	T.H.52 (SB)	01 194 to TH55
M 前面 网络 10	0 30*2	I-35E (NB)	04 I694 to North Split	I-694 (WB)	06 I35E to I494		
WINNI TO NO. 1920	hat	I-35E (SB)	01 North Split to I694	I-94 (EB)	01 TH101 to I494	T.H.61 (NB)	01 Hastings to I494
一 一 二 加速	301 2	I-35E (SB)	02 I694 to StPaul Downtown	I-94 (EB)	02 I494 to TH252	T.H.61 (NB)	02 I494 to I94
Re man		I-35E (SB)	03 StPaul Downtown to 1494	I-94 (EB)	03 TH252 to I35W	T U Ct (00)	04 104 - 1404
R8 Crowe	500 (A)	I-35E (SB)	04 I494 to South Split	I-94 (EB)	04 I35W to I35E	T.H.61 (SB)	01 194 to 1494
ita prezile	300 100	I-35W (NB)	01 Sourth Split to I494	I-94 (EB)	05 I35E to I694	T.H.61 (SB)	02 I494 to Hastings
169 Birtitis	1 Sol-34	I-35W (NB)	02 I494 to Minneapolis Downtown	I-94 (EB)	06 I694 to TH95	T U 610 (EP)	01 194 to US169
108 54 0 mm	Solar Solar	I-35W (NB)	03 Minneapolis Downtown to 1694	I-94 (EB)	07 I94 to I35E	1111010 (LD)	01134 (0 03105
IEB 02483	hat sake	I-35W (NB)	04 I694 to North Split	I-94 (EB)	Opposite Direction of samplei-94v	T.H.610 (EB)	02 US169 to TH47
103 Brotoni 103 Gadelan	A CONTRACTOR OF	I-35W (SB)	01 North Split to I694	I-94 (EB)	SAMPLE I-94(EB)	T.H.610 (WB)	01 TH47 to US169
168 Chirord	A BARRIER	I-35W (SB)	02 I694 to Minneapolis Downtown	I-94 (EB)	SAMPLE I-94(EB) 2.0	T 11 C40 (1910)	00.00460.00.004
168 Differit 4		I-35W (SB)	03 Minneapolis Downtown to I494	I-94 (EB)	SAMPLE I-94(EB) 3.0	1.H.610 (WB)	02 US169 to 194
1511 58-8 shier		I-35W (SB)	04 I494 to Sourth Split	I-94 (WB)	01 TH95 to I694	T.H.62 (EB)	01 I494 to I35W
Ibil Sesector a	era	I-394 (EB)	01 CR101 to US169	I-94 (WB)	02 I694 to I35E	T U 62 (ED)	02 125W to THES
10/1 0200H	2023/18 ²⁰ - 1	I-394 (EB)	02 US169 to Minneapolis Downtown	I-94 (WB)	03 I35E to I35W		02 135W to 11155
ISI 224 dier		I-394 (EB)	03 CR101 to Minneapolis Downtown	I-94 (WB)	04 I35W to TH252	T.H.62 (EB)	03 I494 to TH55
10/1 08 ord	200 C	I-394 (WB)	01 Minneapolis Downtown to US169	I-94 (WB)	05 TH252 to I494	T.H.62 (WB)	01 TH55 to 135W
ali Constantia (ci		I-394 (WB)	02 US169 to CR101	I-94 (WB)	06 I494 to TH101		
193 Division	(1) 1 2 2 2 1	I-394 (WB)	03 Minneapolis Downtown to CR101	I-94 (WB)	Opposite Direction of SAMPLE I-94(EB)	T.H.62 (WB)	02 I35W to I494
1811 Désététete		I-494 (EB)	01 I94 to I394	I-94 (WB)	SAMPLE I-94(EB) *OD*	T.H.62 (WB)	03 TH55 to 1494
IP1 Sesebet21		I-494 (EB)	02 I394 to US169	I-94 (WB)	samplei-94v	T 11 77 (ND)	O1 CR38 to TUC2
183 046	Size 115	I-494 (EB)	03 US169 to I35W	T.H.10 (EB)	01 US169 to I35W	1.H.// (ND)	01 CK38 t0 TH62
10 040 0 0		I-494 (EB)	04 I35W to I35E	T.H.10 (WB)	02 I35W to US169	T.H.77 (SB)	01 TH62 to CR38
18) 00:0 19) 050	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	I-494 (EB)	05 I35E to I94	T.H.100 (NB)	01 I494 to I394	U.S.169 (NB)	01 Canterbury Rd to 1494
14 280		I-494 (EB)	06 I394 to I35W	T.H.100 (NB)	02 I394 to I694		
183 023 191 047		I-494 (EB)	07 I35W to I94	T.H.100 (SB)	01 I694 to I394	U.S.169 (NB)	02 1494 to 1394
(e) 5545	Sec.	I-494 (WB)	01 I94 to I35E	T.H.100 (SB)	02 I394 to I494	U.S.169 (NB)	03 I394 to I694
1011 2012 L		I-494 (WB)	02 I35E to I35W	T.H.212 (EB)	01 TH41 to US169		
181 285 IL 1		I-494 (WB)	03 I35W to US169	T.H.212 (WB)	01 US169 to TH41	0'2'193 (MB)	04 1694 to 1H610
191 2442	in in	I-494 (WB)	04 US169 to I394	T.H.280 (NB)	01 I94 to I35W	U.S.169 (SB)	01 TH610 to I694
193 D-65	1 S	I-494 (WB)	05 I394 to I94	T.H.280 (SB)	01 I35W to I94	U.S. 169 (SB)	02 1694 to 1394
193 242	770 677 - 🎬	I-494 (WB)	06 I94 to I35W	T.H.36 (EB)	01 I35W to I35E	0.0.109 (00)	02 1004 00 1004
103 D164		I-494 (WB)	07 I35W to I394	T.H.36 (EB)	02 I35E to TH5	U.S.169 (SB)	03 I394 to I494
141		I-694 (EB)	01 TH100 to I35W	T.H.36 (EB)	03 I35E to I694	U.S.169 (SB)	04 I494 to Canterbury Rd
B Bet H	DALED D	I-694 (EB)	02 I35W to I35E	T.H.36 (WB)	01 TH5 to I35E		
		I-694 (EB)	03 I35E to TH36	T.H.36 (WB)	02 I35E to I35W	0.2.169 (SB)	UD 1694 to MN62

Figure 3.6: Screenshot of TeTRES Admin Client showing the List of Current Travel-Time Routes

Screenshot of Sample Route Database and Test Route

id [PK] integer	name character varying (255)	description /	corridor character varying (20)	route text /	reg_date timestamp without time zone
49	03 TH252 to 135W		1-94 (EB)	{'_class	2018-01-31 15:52:58:713387
48	05 TH252 to 1494		1-94 (WB)	{`_class_	2018-01-31 15:52:04.864296
47	02 1494 to TH252		1-94 (EB)	{'_class_	2018-01-31 15:52:03.665315
46	06 1494 to TH101		1-94 (WB)	("_class_	2018-01-31 15:51:09.804244
45	01 TH101 to 1494		1-94 (EB)	{'_class_	2018-01-31 15:51:07.817057
4	01 Minneapolis Downtown to		1-394 (WB)	{'_class_	2018-01-31 15:49:41.85985
43	02 US169 to Minneapolis Dow		1-394 (EB)	{'_class	2018-01-31 15:49:40.641613
42	02 US169 to CR101		1-394 (IVB)	("_class_	2018-01-31 15:49:03.010692
41	01 CR101 to US169		1-394 (EB)	{'_class_	2018-01-31 15:49:01.841141
40	01 194 to TH36		1-694 (WB)	("_class_	2018-01-31 15:47:11.751215

Screenshot of Sample Special Event Database

ið PK]integer /	name character varying (100)	description /	years character varying (255)	start, time timestamp without time zone
4283	Timberwolves Vis Orlando Magic	Basketball	2019	2019-01-04 19:00:00
4284	Timbervolves Vis Los Angeles Lakers	Basketbal	2019	2019-01-06 14:30:00
4285	Timberwolves Vis Dallas Mavericks	Basketbal	2019	2019-01-11 19:00:00
4286	Timberwolves Vis New Orleans Pelicans	Basketball	2019	2019-01-12 19:00:00
4287	Timberwolves Vis San Antonio Spurs	Basketball	2019	2019-01-18 19:00:00
4288	Timberwolves Vs Phoenix Suns	Basketbal	2019	2019-01-20 18:00:00
4289	Timberwolves Vs Utah Jazz	Basketball	2019	2019-01-27 18:00:00
4290	Timberwolves V's Memphis Grizzlies	Basketbal	2019	2019-01-30 19:00:00
4291	Timberwolves Vs Denver Nuggets	Basketbal	2019	2019-02-02 20:00:00

id [PK] integer	routejid integer /	time timestamp without time zone	tt double precision /	vmt double precision	speed /
97778	45	2019-01-04 16:30:00	5.85545206382096	2086.2	82.3760752270668
97779	45	2019-01-04 16:35:00	5.87992732741426	2116.45	82.2989290158374
97780	45	2019-01-04 16:40:00	5.71507418839543	2049.6	83.2125902026128
97781	45	2019-01-04 16:45:00	5.69638780490946	2211.8	83.3745615112639
97782	45	2019-01-04 16:50:00	5.7172046152492	2327.85	84.0223667845933
97783	45	2019-01-04 16:55:00	5.82023786197172	2102.35	82.5601028733427
97784	45	2019-01-04 17:00:00	5.83453121225218	2125.25	81.5638086084899
97785	45	2019-01-04 17:05:00	5.58564609957211	2096.5	85.1142388610745
97786	45	2019-01-04 17:10:00	5.77730176692019	2061.3	84.1184368748502
97787	45	2019-01-04 17:15:00	5.8904405575816	2119.85	82.0233266116708
97788	45	2019-01-04 17:20:00	5.6469704336962	2355.8	83.6540821243947
97789	45	2019-01-04 17:25:00	5.60816343208336	2056.05	84.8918851056209
97790	45	2019-01-04 17:30:00	5.81487751452415	2206.1	83.2995882080439
97791	45	2019-01-04 17:35:00	6.18979333581155	2179.65	79.0735918300638

id [PK] integer	tt_id integer	specialevent_id 🖋	distance double precision	event_type character (1)
1	97778	4283	11.2218440890994	A
2	97779	4283	11.2218440890994	A
3	97780	4283	11.2218440890994	A
4	97781	4283	11.2218440890994	A
5	97782	4283	11.2218440890994	A
6	97783	4283	11.2218440890994	A
7	97784	4283	11.2218440890994	A
8	97785	4283	11.2218440890994	A
9	97786	4283	11.2218440890994	A
10	97787	4283	11.2218440890994	A
11	97788	4283	11.2218440890994	A
12	97789	4283	11.2218440890994	A
13	97790	4283	11.2218440890994	A
14	97791	4283	11.2218440890994	A
15	97792	4283	11.2218440890994	A

Calculated Travel-Time database for Selected Route

Categorized Special Event Data linked to Travel-Time Data

Figure 3.7: Example Categorized Special Event Data Linked to Travel-Time of Sample Route

Sample Route Database and Test Route Database for WZ Groups

Sample WZ Group Database

Sample WZ

d Pr(integer	name character ranjin	g (255)	description text	corido: chaacterverying (21)	r <mark>tate</mark> 1 tet	rejidate tinestampi	itotinezee /	ii P(ites	iane / cheadernanjing (25)	/ <mark>tecipton</mark> Iet	I	anidas deside naying (25)	jes deadena	ning (23)		il P(itejs	/ <mark>icyaqid</mark> iteje	neto Ist	sitin freim vitotfreze	eljine I finelarp vitoz finezore	/ ^{tati}	ndê) Ist
1	a (4 (5)1 to (35)			148	Ldess.	. 279797	15333614726		9 940020	WPASS are construction		hl	117.NJ	19,222,221		1	q	13 Iola	. 779671000	22402350	_055	. (_dz.,
5	0.04831151763	Q		14/18	l' dess	216031	5510008		1, 946930	Bridge 1554 Rahab and 404 Im	poenets	h]	<u>3</u> 11			1	Ņ	10 Iola	77891M	0742350	(23	. (
	0 107/234-02	N		10/00	l dus	4110 11 11	10.01.00 719907		4 940000	Relay Smith		hl	216207			1	4	17 Ma	7590111	777913591	_025	. (. da
	9 VƏ INCƏLIDƏ	11		179 [0]	_000.	. MININ	12.32,367,13381			Ari Mani i			44.5		≯	. ,	0	18 Kilos	ANC N 10 MARA	1111 H 112 H) day	l da
	8 (57825)/9	4		194 (118)	_das.	2.8.8	55214,84296		Q 990054	Bróge 907 Rehab		0	201				a	H 10(3)	. UNHAIDUU	ALERI (221)	(_085	, (_035,
	7 (2)4915783	1		H4(B)	(<u>_</u> dass.	293	50124651		0 990040	Continuous Lighting		hl	218			1	8	18 Iola	780111	1111 36	_055	. (<i>b</i> a
	8 (614941b17H10	1		14/18	l' dess.	219131	1551088044		4 940933	Replace Fercing		hl	218			1	2	18 Iolai	27470100	21741335551	(_das	. (Ja
id (PK)	integer /	route_id integer	time timestamp wi	hout time zone	t double precis	ision /	vint / double precision /	speed double precisi	n /		id (PK) integ	er / tt_id integ	er /	workzone_ic integer	1	loc_typ integer	e /	distance double p	precision	off_distance double precision	,	
(r)	100 100	nteger /	omestamp wi	nout time zone	0000 e preci	S01	oouole precision	000010 precis	01		(PK) integ	er integ	200	integer	247	integer	1	double p	76015576551	double precision	1	
	289	4	8 2019-01-01 00	10:00	5.62915	263471121	228	74.05807	4657744			575	289		247		1	8.14	762915576551	-8.1476291557655	1	
	290	4	8 2019-01-01 00	15:00	5,25302	111221415	325.35	79.816	18977861			577	290		247		1	8.14	762915576551	-8.1476291557655	1	
	291	4	8 2019-01-01 00	20:00	5,25177	284249535	388.35	79.85467	13842797			579	291		247		1	8.14	762915576551	-8.1476291557655	1	
	292	4	8 2019-01-01 00	25:00	5,47918	145841439	501.35	76,85610	1708605			581	292		247		1	8.14	762915576551	-8.1476291557655	1	
	293	4	8 2019-01-01 00	30:00	5.33559	958971066	567.65	77,25990	3481804			583	293		247		1	8.14	762915576551	-8.1476291557655	1	
	294	4	8 2019-01-01 00	35:00	5,31874	390315846	592.95	79.32314	57180762			585	294		247		1	8.14	762915576551	-8.1476291557655	1	
	295	4	8 2019-01-01 00	40:00	5,356012	292957791	684.35000000001	78,23328	18359746	•		587	295		247		1	8.14	762915576551	-8.1476291557655	1	
	296	4	8 2019-01-01 00	45:00	5,35030	148444378	700.25	77.71534	4591252			589	296		247		1	8.14	762915576551	-8.1476291557655	1	
	297	4	8 2019-01-01 00	50:00	5.36938	945992467	578.85	78.14545	17351426			591	297		247		1	8.14	762915576551	-8.1476291557655	1	
	298	4	8 2019-01-01 00	55:00	5.38139	244263288	606.8	77.95996	13422867			593	298		247		1	8.14	762915576551	-8.1476291557655	1	
	299	4	8 2019-01-01 01	00:00	5.33992	885761705	643.1	78,74412	3065785			595	299		247		1	8.14	762915576551	-8.1476291557655	1	
	300	4	8 2019-01-01 01	05:00	5.31962	733117882	5753	77.91968	5148152			597	300		247		1	8.14	762915576551	-8.1476291557655	1	
	301	4	8 2019-01-01 01	10:00	5.38068	193419718	596.15	78.10980	9325396			599	301		247		1	8.14	762915576551	-8.1476291557655	1	

Calculated Travel-Time Database for Sample Route

Categorized WZ Data linked to Travel-Time Data of Sample Route

Figure 3.8: Example Categorized Work-Zone Data Linked to Travel-Time Data of Sample Route

Sample Route Database and Test Route

ið Piljinteger /	tane dasaterianjig(10) /	description / tet	pijid claasterverying(2)	ndel / tet	ndel / Et	regulate timestamp without time zure
31	Cedar-TPS,4941		19. K	(_das_	(_das,.	20246527141953210757
31	Maylard-TP9F0361		THFUST	(_des.,	(_des,_	ZOLIST WHEN HER
312	Maylard-TP9R0521		TRANSI	(_des	(_dess,_	XXHST WHEN BEEN
35	Maylard-TP9F0611		TP9F0611	(_das_	(_dess	200453714193925739
31	Maylard-TP9R1941		TRFN41	(_das_	(_des.,	2024537141941,4178
35	Maylard-TP952801		795231	(_d85_	(_des,_	2245274945279
38	Maylard-TP9535E1		1945321	(_des	(_des,_	20457141958299
31	Maylard-TP956941		79981	(_des	(_des,_	20045271439403469
33	Foetlale-TP9031		7803	(_das_	(_das,-	XARE VIEWOR

Sample Snow Event Database

il Minteper /	satjine inesanp vitozlinezne /	edine inetarp vitozinezne /	ngide instarp vitozi inszre
193	2030450000	20906507060	2024650 15121735607
194	2090478320	2019-01-1706-02:0	2024650 1512 (RBT62
195	20904804020	2019011901000	2020-05-07 15/12/21 39/644
195	2090480000	20901901000	2024650 15122846481
1937	201910-08122290	2019/01/1918/0200	2024650 151225451157
198	201901481730200	2019-01-18 19:00:00	2024650 151220,4649
199	21919211021	2019-01-02 14:00:00	202465015122946012

Sample Plow-Route Database

il Pi(inteps /	tane / deciption citaacter verying (101) / tect	∕ <mark>¢jä</mark> destererjoj∑j	tatél /	ndel 1et /	nçizle Tinestançı vilkost tine zone
30	Cedar-TP5,4941	16164	(_das_	(_dat, .	ZOHST MIKR 10757
31	Mayland-TP9F0361	THERE	(jæ,	(jas,	ZUANT NAKA KAO
30	Mayland-17997621	TH:FIS21	(_dag_	(_dat_	ZZAKU KINI MIKI
33	Maylard-17961611	TRANI	(_des	(_des,.	TENED WHEN DECK
34	Mayland - TP9F1341	THEM	(_das	(_dat,	ZORST NIMA OTO
35	Vaylard+TP95301	19531	(_das	(_dat,	TORIST MINASOTO
35	Mayland-TP9ESEE	1995/821	(_dag_	(_dag_	ZUNET NIME AND M
31	Mayland+TP9F6841	T999341	(_dag_	(Jøg.	ZORST HIBATAKIO
38	FoestLale-17980351	748351	(_das	(_dat_	ZZAKU KIMABOLO

7

Calculated TT-database for Sample Route

id PK[integer /	notejil integer /	time timestamp without time zone	tt double precision 1	vrat double precision 🖌	speed /
466124	27	2019-01-15 03:00:00	3.70314405666386	134.25	72.4148085040161
466125	IJ	2019-01-15 03:05:00	3.71704291589159	138.1	72,1702291155945
466126	IJ	2019-01-15 03:10:00	3.99017209760159	140.65	67.5782531730361
466127	27	2019-01-15 03:15:00	3.84012552184442	174.35	69.8455393609631
466128	27	2019-01-15 03:20:00	3.82254787114055	164.35	70.4599832311372
466129	27	1019-01-15 03:25:00	4.02315688994213	171.55	67.0101147039165
466130	27	2019-01-15 03:30:00	3.92415957010558	174.35	68.1641238554235
466131	27	2019-01-15 03:35:00	3.84003507076638	165.9	70.6809130656197
466132	27	2019-01-15 03:40:00	3.52543894824023	278.4	75.8519734757373
466133	IJ	2019-01-15 03:45:00	3.80921435233325	244.5	70.5465014109216
466134	IJ	2019-01-15 03:50:00	3.55646298517288	168.4	74,6100194020431

id [PK] integer	sroute_id integer	sevent_id /	lane_Jost_time timestamp without time zone	lane_regain_time timestamp without time zone	duration /
203	3 310	1933	2019-01-15 03:00:00	2019-01-15 08:00:00	5
203	4 354	1937	2019-01-18 12:30:00	2019-01-19 05:00:00	16.5
203	5 354	1939	2019-01-22 01:30:00	2019-01-22 15:30:00	14
203	3 352	1942	2019-01-22 02:30:00	2019-01-22 06:30:00	4
204	5 321	1947	2019-01-22 02:30:00	2019-01-22 09:00:00	6.5
204	2 317	1944	2019-01-22 02:30:00	2019-01-22 04:00:00	1.5
204	316	1944	2019-01-22 02:30:00	2019-01-22 04:00:00	1.5
203	312	1944	2019-01-22 02:30:00	2019-01-22 04:00:00	1.5
204	314	1944	2019-01-22 02:30:00	2019-01-22 04:00:00	1.5
203	7 345	1940	2019-01-22 03:00:00	2019-01-22 08:00:00	5
204	3 322	1945	2019-01-22 03:00:00	2019-01-22 09:30:00	6.5

Z

Snow-Event Database for Plow-Routes

id [PK] integer	tt_id integer ♂	snowmgmt, integer	_id 🖋	loc_type integer	distance double precision
181	466124		2033	6	1.72012236459678
182	466125		2033	6	1.72012236459678
183	466126		2033	6	1.72012236459678
184	466127		2033	6	1.72012236459678
185	466128		2033	6	1.72012236459678
186	466129		2033	6	1.72012236459678
187	466130		2033	6	1.72012236459678
188	466131		2033	6	1.72012236459678
189	466132		2033	6	1.72012236459678
190	466133		2033	6	1.72012236459678
191	466134		2033	6	1.72012236459678
192	466135		2033	6	1.72012236459678
193	466136		2033	6	1.72012236459678

Categorized Snow-Event Data Linked to Travel-Time for Sample Route

Figure 3.9: Example Categorized Snow-Event Data Linked to Travel-Time Data of Sample Route

CHAPTER 4: TRAVEL-TIME RELIABILITY TRENDS AND BOTTLENECK PRIORITIZATION FOR FREEWAY CORRIDORS

4.1 INTRODUCTION

In this chapter, the travel-time reliability trends of a total of 23 corridors were analyzed and the bottleneck sections within each directional route were identified. First, using the database populated in the previous chapter, a set of the reliability measures of the individual corridors in the metro freeway network during morning or afternoon peak periods were estimated under different operating conditions from 1/1/2016 until 9/30/2020. Figure 4.1 shows a total of 23 freeway corridors, whose boundaries have been determined in cooperation with the Regional Transportation Management Center, MnDOT. For each corridor, two directional routes, one for morning and the other for afternoon peak-period, were identified depending on the peak-period traffic patterns of each corridor, while the I-94 corridor between St. Paul and Minneapolis have four routes, i.e., two directional routes for both morning and afternoon peak periods. Therefore, there are a total of 48 directional routes, 24 routes for morning and 24 for afternoon peak periods. Table 4.1 includes the IDs of start and end stations of each directional route of each corridor.



Figure 4.1: Individual Corridors for Reliability Estimation

Table 4.1: Start/End Stations of Each Corridor Directional Route

Corridor	Description	Start	End
135VV (NB)	South Split to Minneapolis Downtown	S911	S566
135W (NB)	Minneapolis Downtown to North Split	S566	S1557
135VV (SB)	North Split to Minneapolis Downtown	S1567	S585
135W (SB)	Minneapolis Downtown to South Split	S585	S915
135E (NB)	South Split to Saint Paul Downtown	S870	S619
135E (NB)	Saint Paul Downtown to North Split	S619	S1504
135E (SB)	North Split to Saint Paul Downtown	S1531	S644
135E (SB)	Saint Paul Downtown to South Split	S644	S905
1394 (EB)	I494 to Minneapolis Downtown	S269	S291
1394 (VVB)	Minneapolis Downtown to I494	S262	S345
194 (EB)	TH101 to I494	S1115	S211
194 (EB)	I494 to Minneapolis Downtown	S211	S110
194 (EB)	Minneapolis Downtown to Saint Paul Downtown	S110	S499
194 (EB)	Saint Paul Downtown to Wisconsin Border	S499	S1358
194 (WB)	Wisconsin Border to Saint Paul Downtown	S1359	S500
194 (VVB)	Saint Paul Downtown to Minneapolis Downtown	S500	S1943
194 (VVB)	Minneapolis Downtown to I494	S1943	S213
194 (VVB)	1494 to TH101	S213	S1112
TH52 (NB)	TH55 to Saint Paul Downtown	S1166	S1178
TH52 (SB)	Saint Paul Downtown to TH55	S1151	S1163
1494 (EB)	1694 to TH212	S209	S473
1494 (EB)	TH212 to I35E	S474	S863
1494 (EB)	135E to 194	S863	S1026
1494 (VVB)	194 to 135E	S1029	S864
1494 (WB)	I35E to TH212	S864	S484
1494 (VVB)	TH212 to 1694	S484	S210
1694 (EB)	TH252 to 135E	S145	S1452
1694 (EB)	135E to 194	S1452	S1028
1694 (WB)	194 to 135E	S1027	S1459
1694 (WB)	1494 to TH100	S1459	S144
US169 (NB)	Canterbury Rd to TH610	S1617	S1966
US169 (SB)	TH610 to Canterbury Rd	S1795	S1626
TH100 (NB)	1494 to 194	S375	S1614
TH100 (SB)	194 to 1494	S1100	S421
TH10 (EB)	US169 to 1694	S940	S1825
TH10 (WB)	1694 to US169	S1949	S992
TH610 (EB)	194 to TH10	S1954	S966
TH610 (WB)	TH10 to 194	S995	S1961
TH36 (ÈB)	TH280 to 135E	S587	S600
TH36 (EB)	135E to 1694	S600	S1426
TH36 (VVB)	1694 to 135E	S1425	S608
TH36 (VVB)	135E to 135W	S608	S618
TH62 (EB)	1494 to 135VV	S301	S329
TH62 (EB)	I35W to Hiawatha	S329	S1135
TH62 (WB)	Hiawatha to I35W	S1136	S133
TH62 (WB)	135VV to 1494	S133	S369

Next, for each directional route of the individual corridors shown in Figure 4.1, the monthly and yearly travel-time reliability measures were estimated under different operating conditions using TeTRES and their trends were analyzed. It needs to be noted that, in this study, only regular non-holiday weekdays, i.e., Tuesdays, Wednesdays, Thursdays, were included for estimating travel-time reliability measures for each route. In addition, a set of the traffic-flow performance measures were also estimated and presented for each route. The specific measures and operating conditions used in this analysis are as follows:

- Travel-time Reliability Measures:
 - Buffer Index (95th%ile) = (95th%ile Travel Time Average Travel Time)/(Average Travel Time)
 - Planning Index (95th %ile) = 95th %ile Travel Time / Free-Flow Travel Time
 - Travel Rate (95th %ile), minutes/mile = 95th %ile Travel Time / Route Length
- Operating Conditions
 - Weather: All, Dry, Rain, Snow
 - Incident: All, No-Incident (N), Property Damage Only (PD), Severe/Fatal (INJ,FA)
 - Work Zone: All, No-WZ (N), Light Impact-WZ (L), Medium-Heavy Effect WZ (M,H)
- Peak Periods: Morning: 6:00 9:00 a.m. Afternoon: 3:30 6:30 p.m.
- Traffic-Flow Measures: VMT (Vehicle-Miles Traveled), VHT (Vehicle-Hours Traveled), DVH (Delayed Vehicle-Hours)

Further, to assess the overall reliability condition of a route, in this study, two travel-time reliability measures, i.e., buffer index (BI) and travel rate (TR), were combined in a BI-TR space as shown in Figure 4.2 and a vulnerability of a given route is quantified as the Euclidian distance between the origin and the data point of each route, i.e.,

Vulnerability Index (VI) of Route i = sqrt [(95^{th} percentile BI)_i² + (95^{th} percentile TR)_i²]

In this study, the above VI is estimated with the yearly measures of both BI and TR of each directional route under all conditions and used for the comparative analysis with other routes. Figure 4.2 also shows 5 levels of vulnerability in terms of BI and TR values of each route. Those vulnerability levels are used to capture the network-wide variation trends of the route vulnerability from 2016 to 2020. Finally, the potential bottleneck sections within each directional route were identified and the VI of each section was estimated with the 2019 data. The VIs of those potential bottleneck sections were then compared and the most vulnerable bottleneck section was determined for each route. The rest of this report summarizes the yearly variation trends of the travel-time reliability at each directional route and the identification of the most vulnerable bottleneck section in each directional route. The detailed estimation and analysis results of individual routes' travel-time reliability trends under different operating conditions are included in the Appendix.



Level	BI (95 th %-ile)	TR (95 th %-ile)
1	BI < 0.25	< 1.0
2	0.25 ≤ BI < 0.5	$1.0 \le TR < 2.0$
3	0.5 ≤ BI < 0.75	$2.0 \le TR < 3.0$
4	0.75 ≤ BI < 1.0	$3.0 \le TR < 4.0$
5	1.0 ≤ BI	4.0 ≤ TR

Figure 4.2: Vulnerability Index and 5 Levels

4.2 TRAVEL-TIME RELIABILITY TRENDS OF INDIVIDUAL DIRECTIONAL ROUTES IN METRO FREEWAY NETWORK

4.2.1 COMPARISON OF YEARLY RELIABILITY MEASURES OF INDIVDIDUAL DIRECTIONAL ROUTES

First, the travel-time reliability measures of each directional route were estimated with yearly data under all conditions using TeTRES from 1/1/2016 until 9/30/2020. Further, all the 48 routes were categorized into either morning or afternoon-route groups depending on the peak period of each route.

Figures 4.3 – 4.7 show the 95th percentile buffer index (BI) and the 95th percentile travel rate (TR) of each route from 2016 to 2020 in the BI-TR space for each route group. The vulnerability index (VI) of each route is also calculated and shown in the bar graphs as well.



Figure 4.3: Travel-Time Reliability and Vulnerability of Morning and Afternoon Routes (2016)



Figure 4.4: Travel-Time Reliability and Vulnerability of Morning and Afternoon Routes (2017)



Figure 4.5: Travel-Time Reliability and Vulnerability of Morning and Afternoon Routes (2018)





2019 - Route Vulnerability (Morning)





Figure 4.6: Travel-Time Reliability and Vulnerability of Morning and Afternoon Routes (2019)



Figure 4.7: Travel-Time Reliability and Vulnerability of Morning and Afternoon Routes (2020)

Tables 4.2 and 4.3 include the vulnerability indices for each of the morning and afternoon routes from 2016 to 2020. As indicated in Figures 4.3-4.7 and also in Tables 4.2 and 4.3, the afternoon routes show consistently more vulnerabilities than the morning routes. Further, the afternoon routes generally exhibit higher 95th percentile travel times per mile than the morning routes, while the buffer index, i.e., the travel-time variability, is mostly higher with the morning routes. The yearly variations of the route vulnerability levels for all the routes including both morning and afternoon groups are shown in Figure 4.8, which indicates the overall improving trends of the route vulnerability levels from 2016 to 2018, while in 2019 the number of the routes with higher vulnerability, i.e., above level 3, significantly increased. The same trends were also noticed from the traffic-flow measures, as shown in Figure 4.9, which shows that the total Vehicle-Miles Traveled were substantially decreased in 2019, while the total Delayed-Vehicle Hours increased for both morning and afternoon routes.

2016	i	2017		2018	В		2019		2020	
Route	VI	Route	VI	Route	VI		Route	VI	Route	VI
TH62 (TH55) WB M	2.6	TH62 (TH55) WB M	2.8	TH62 (TH55) WB M	2.9		TH36 (I35E) WB M	2.9	I94 (TH101) EB M	2.2
TH36 (I35E) WB M	2.5	TH36 (135E) WB M	2.5	TH36 (I35E) WB M	2.5		TH62 (TH55) WB M	2.7	TH62 (I35W) WB M	1.7
194 (1494) EB M	2.5	TH100 SB M	2.4	I94 (STPL) WB M	2.4		I94 (STPL) WB M	2.5	194 (STPL) WB M	1.6
TH100 SB M	2.3	194 (STPL) WB M	2.2	1694 (135E) WB M	2.3		135W (NS) SB M	2.3	1394 EB M	1.6
1394 EB M	2.1	1394 EB M	2.2	1394 EB M	2.1		TH100 SB M	2.2	135W (SS) NB M	1.6
TH62 (I35W) WB M	2.1	TH62 (I35W) WB M	2.2	1494 (135E) WB M	2.1		135W (SS) NB M	2.2	TH36 (I35E) WB M	1.5
TH36 (I694) WB M	2.1	194 (WISC) WB M	2.1	TH62 (I35W) WB M	1.9		TH62 (I35W) WB M	2.1	TH62 (TH55) WB M	1.5
US169 SB M	2.0	1494 (1694) SB M	2.0	TH100 SB M	1.9		194 (WISC) WB M	2.1	I94 (I494) EB M	1.4
1694 (135E) WB M	1.9	TH36 (1694) WB M	2.0	1494 (1694) SB M	1.8		194 (TH101) EB M	2.1	1694 (194) WB M	1.3
1494 (135E) WB M	1.9	194 (1494) EB M	2.0	194 (WISC) WB M	1.8		1694 (135E) WB M	2.1	1494 (135E) WB M	1.3
135W (SS) NB M	1.9	135W (SS) NB M	2.0	135W (SS) NB M	1.7		1394 EB M	2.1	TH100 SB M	1.3
1494 (1694) SB M	1.7	1494 (135E) WB M	2.0	135W (NS) SB M	1.7		TH36 (1694) WB M	2.0	1694 (135E) WB M	1.2
194 (WISC) WB M	1.7	1694 (135E) WB M	2.0	I94 (TH101) EB M	1.7		1494 (194) WB M	2.0	US169 SB M	1.2
135W (NS) SB M	1.7	135W (NS) SB M	1.9	US169 SB M	1.7		US169 SB M	1.9	135W (NS) SB M	1.2
I35E (NS) SB M	1.7	1494 (194) WB M	1.8	TH36 (I694) WB M	1.7		194 (1494) EB M	1.9	194 (WISC) WB M	1.1
1694 (194) WB M	1.5	1694 (194) WB M	1.7	194 (1494) EB M	1.6		1494 (135E) WB M	1.9	I35E (SS) NB M	1.1
1494 (194) WB M	1.5	135E (SS) NB M	1.6	135E (SS) NB M	1.6		1494 (1694) SB M	1.9	TH36 (I694) WB M	1.1
I35E (SS) NB M	1.4	US169 SB M	1.6	TH10 EB M	1.5		135E (SS) NB M	1.8	194 (MPLS) EB M	1.0
TH10 EB M	1.3	194 (TH101) EB M	1.5	I35E (NS) SB M	1.4		1694 (194) WB M	1.7	I35E (NS) SB M	1.0
I94 (TH101) EB M	1.3	135E (NS) SB M	1.5	1694 (194) WB M	1.4		135E (NS) SB M	1.6	1494 (1694) SB M	1.0
I94 (MPLS) EB M	1.2	TH10 EB M	1.4	1494 (194) WB M	1.3		TH10 EB M	1.6	TH10 EB M	1.0
TH52 NB M	1.1	194 (MPLS) EB M	1.3	I94 (MPLS) EB M	1.2	Π	194 (MPLS) EB M	1.5	1494 (194) WB M	1.0
TH610 EB M	0.9	TH52 NB M	1.2	TH52 NB M	1.1		TH52 NB M	1.2	TH610 EB M	0.9
194 (STPL) WB M	0.0	TH610 EB M	0.9	TH610 EB M	1.0		TH610 EB M	1.2	TH52 NB M	0.9

Table 4.2: Route Vulnerability Variation (Morning Routes)

	N / 1 / 1 / 1 / 1 / 1 / 1			
Table 4.3: Route	Vulnerability	/ Variation	(Afternoon Routes)	

2016		2017		2018		2019		2020	
Route	VI								
TH36 (I35W) EB A	4.1	TH36 (I35W) EB A	3.9	TH62 (l494) EB A	3.6	TH36 (I35W) EB A	4.7	TH62 (l494) EB A	2.5
TH62 (1494) EB A	4.0	TH62 (1494) EB A	3.9	TH36 (I35W) EB A	3.2	TH62 (l494) EB A	3.3	I94 (STPL) WB A	2.1
194 (STPL) WB A	3.0	1494 (TH212) EB A	2.6	194 (STPL) WB A	3.0	I94 (STPL) WB A	3.0	TH36 (I35W) EB A	2.0
1494 (TH212) NB A	2.9	TH100 NB A	2.5	194 (MPLS) EB A	2.5	I94 (MPLS) EB A	2.8	I494 (TH212) EB A	1.9
1494 (TH212) EB A	2.7	194 (MPLS) EB A	2.5	1494 (TH212) EB A	2.4	1494 (135E) EB A	2.8	194 (MPLS) EB A	1.9
TH62 (I35W) EB A	2.5	1494 (TH212) NB A	2.4	TH100 NB A	2.0	1694 (TH252) EB A	2.6	1694 (TH252) EB A	1.5
1694 (TH252) EB A	2.5	1694 (TH252) EB A	2.4	1694 (TH252) EB A	2.0	1494 (TH212) EB A	2.4	TH62 (I35W) EB A	1.4
TH100 NB A	2.5	194 (STPL) WB A	2.3	TH62 (I35W) EB A	1.9	TH62 (I35W) EB A	2.4	135W (MPLS) SB A	1.4
194 (MPLS) EB A	2.5	TH62 (I35W) EB A	1.9	1494 (TH212) NB A	1.9	TH100 NB A	2.2	US169 NB A	1.3
US169 NB A	2.2	1394 WB A	1.8	1494 (I35E) EB A	1.8	1494 (TH212) NB A	2.1	194 (1494) WB A	1.3
194 (1494) WB A	1.8	194 (MPLS) WB A	1.7	US169 NB A	1.7	US169 NB A	1.9	TH10 WB A	1.2
I35E (STPL) SB A	1.7	1494 (I35E) EB A	1.7	1394 WB A	1.7	135W (MPLS) NB A	1.8	1394 WB A	1.2
135W (MPLS) NB A	1.7	TH36 (I35E) EB A	1.7	135E (STPL) SB A	1.6	135E (STPL) SB A	1.7	TH100 NB A	1.2
1394 WB A	1.7	US169 NB A	1.7	TH10 WB A	1.6	I35W (MPLS) SB A	1.7	I494 (TH212) NB A	1.2
1494 (I35E) EB A	1.7	TH10 WB A	1.6	135W (MPLS) SB A	1.5	1394 WB A	1.7	TH36 (I35E) EB A	1.2
135W (MPLS) SB A	1.7	135W (MPLS) NB A	1.6	TH36 (I35E) EB A	1.5	TH36 (I35E) EB A	1.6	I35W (MPLS) NB A	1.2
194 (MPLS) WB A	1.6	135E (STPL) SB A	1.6	194 (1494) WB A	1.5	194 (1494) WB A	1.6	135E (STPL) SB A	1.1
135E (STPL) NB A	1.6	135W (MPLS) SB A	1.6	1694 (135E) EB A	1.5	TH10 WB A	1.6	135E (STPL) NB A	1.1
TH36 (I35E) EB A	1.5	194 (1494) WB A	1.5	135W (MPLS) NB A	1.4	1694 (135E) EB A	1.5	194 (STPL) EB A	1.1
TH10 WB A	1.5	194 (STPL) EB A	1.4	194 (STPL) EB A	1.4	194 (STPL) EB A	1.4	1494 (135E) EB A	1.1
194 (STPL) EB A	1.3	1694 (I35E) EB A	1.3	135E (STPL) NB A	1.3	135E (STPL) NB A	1.4	TH610 WB A	1.0
1694 (135E) EB A	1.2	135E (STPL) NB A	1.2	194 (MPLS) WB A	1.3	194 (MPLS) WB A	1.4	1694 (135E) EB A	1.0
TH52 SB A	1.2	TH610 WB A	1.2	TH610 WB A	1.3	TH610 WB A	1.2	194 (MPLS) WB A	0.9
TH610 WB A	1.0	TH52 SB A	1.0	TH52 SB A	1.0	TH52 SB A	1.0	TH52 SB A	0.81



	2016	2017	2018	2019	2020
Level 1	2	2	2	1	7
Level 2	21	23	32	20	34
Level 3	20	20	11	22	5
Level 4	4	3	3	3	2
Level 5	1	0	0	2	0

Figure 4.8: Yearly Variations of Route Vulnerability







3.00E+06

2.50E+06

2.00E+06

1.50E+06

1.00E+06

2016

Vehicle Hours





Figure 4.9: Variations in Traffic-Flow Measures (2016-2019)

2017

2018

2019

4.2.2 MONTHLY AND YEARLY RELIABILITY TRENDS OF INDIVIDUAL DIRECTIONAL ROUTES

As described in the previous section, this study also performed a detailed analysis of individual route's reliability trends under various operating conditions. This section presents a sample output from such an individual-route analysis for the 169 northbound (NB) route. The reliability estimation and analysis results of all other individual routes are included in the Appendix A.

4.2.2.1 Monthly Reliability Trends of US-169 NB Corridor for Afternoon Peak Periods



Figure 4.10: Location of 169 NB Route

4.2.2.2 Effects of Weather conditions on Travel-Time Reliability



Figure 4.11: Monthly Variations of Reliability Measures under Different Weather Conditions (169 NB)

4.2.2.3 Effects of Incidents



Figure 4.12: Monthly Variations of Reliability Measures under Different Incident Conditions (169 NB)

4.2.2.4 Effects of Work Zones



Figure 4.13: Monthly Variations of Reliability Measures under Different Work-Zone Conditions (169 NB)



4.2.2.5 Yearly Variations - Weather Effects

Figure 4.14: Yearly Variations of Reliability Measures under Different Weather Conditions (169 NB)

4.2.3 Incident Effects



Figure 4.15: Yearly Variations of Reliability Measures under Different Incident Conditions (169 NB)



4.2.3.1 Work-Zone Effects

Figure 4.16: Yearly Variations of Reliability Measures under Different Work-Zone Conditions (169 NB)



4.2.3.2 Yearly Variations of Combined (Vulnerability) Index

Figure 4.17: Yearly Variations of Combine Reliability Measures (169 NB)



4.2.3.3 Variations of Traffic-Flow Measures

Figure 4.18: Yearly Variations of Traffic-Flow Measures (169 NB)

4.2.4 Trends Summary for 169 NB corridor

- The planning and travel-rate indices show consistently ascending trend from 2017 until the end of 2019, indicating the congestion level on this route had been rising before the pandemic, as also indicated on the variations on VMT, VHT and DVH.
- The effects of snow were significantly higher than those from rain on this route through years, while incidents had not made much difference on the reliability values.

4.3 BOTTLENECK PRIORITIZATION FOR INDIVIDUAL DIRECTIONAL ROUTES

In this section, the severity of the bottleneck sections within each directional route is analyzed with the travel-time reliability measures estimated from the 2019 weekday data. First, the potential bottleneck sections of the corridor routes were identified by examining the traffic speed patterns during normal weekdays and by considering the geometry of each route. Next, a set of the travel-time reliability measures were estimated for each bottleneck section with the 2019 travel-time data under all conditions and the vulnerability index of each bottleneck section was estimated with the 95th %-ile buffer index and the 95th percentile travel rate of each bottleneck section as described in the previous sections. Finally, the vulnerability index for each bottleneck section was compared with those of other sections in a route and the section with the highest Vulnerability Index value is determined as the most severe bottleneck section for a given route.

The rest of this section presents the vulnerability index estimation results of the potential bottleneck sections in each route along with the geographical locations of each sections. The bottleneck section with the highest vulnerability index value is highlighted as the most vulnerable section in a given route.

4.3.1 US169 SB Route (Morning Peak): Section with the highest vulnerability index \Rightarrow 1394 to TH7



Figure 4.19: Identification of Bottleneck Sections (169 NB)

4.3.2 US169 NB Route (Afternoon Peak) \Rightarrow 1494 to TH62



Figure 4.20: Identification of Bottleneck Sections (169 SB)



4.3.3 TH610 EB Morning \Rightarrow US169 to TH47



4.3.4 TH610 WB Afternoon \Rightarrow US169 to 194



Figure 4.22: Identification of Bottleneck Sections (TH610 WB)

4.3.5 TH100 SB Morning \Rightarrow *1694 to 36th*



Figure 4.23: Identification of Bottleneck Sections (TH100 SB)

4.3.6 TH100 NB Afternoon \Rightarrow 1494 to 1394



Figure 4.24: Identification of Bottleneck Sections (TH100 NB)

4.3.7 TH62 EB Afternoon \Rightarrow TH100 to US169



Figure 4.25: Identification of Bottleneck Sections (TH62 EB)

4.3.8 TH62 WB Morning \Rightarrow *I35W to TH100*



Figure 4.26: Identification of Bottleneck Sections (TH62 WB)

4.3.9 TH52 SB Afternoon \Rightarrow 194 to 1494





4.3.10 TH52 NB Morning \Rightarrow 1494 to 194



Figure 4.28: Identification of Bottleneck Sections (TH52 NB)




Figure 4.29: Identification of Bottleneck Sections (TH36 EB)

4.3.12 TH36 WB Morning \Rightarrow 135E to 135W



Figure 4.30: Identification of Bottleneck Sections (TH36 WB)



4.3.13 I-494 (TH212 to I35E) EB Afternoon \Rightarrow MN100 to I35W

Figure 4.31: Identification of Bottleneck Sections (I-494 S1 EB)

4.3.14 I-494 (I35E to TH212) WB Morning \Rightarrow *I35W to MN100*



Figure 4.32: Identification of Bottleneck Sections (I-494 S1 WB)



4.3.15 I-494 (I35E to I94) EB Afternoon \Rightarrow I35E to MN52

Figure 4.33: Identification of Bottleneck Sections (I-494 S2 EB)

4.3.16 I-494 (I94 to I35E) WB Morning \Rightarrow *194 to MN61*



Figure 4.34: Identification of Bottleneck Sections (I-494 S2 WB)



4.3.17 I-494 (TH212 to I694) NB Afternoon \Rightarrow US169 to I394



4.3.18 I-494 (I694 to TH212) SB Morning \Rightarrow MN55 to I394







4.3.19 I-394 EB Morning \Rightarrow TH100 to MPLS



4.3.20 I-394 WB Afternoon \Rightarrow MPLS to MN100



Figure 4.38: Identification of Bottleneck Sections (I-394 S1 WB)



4.3.21 I-94 (I494 to MPLS) EB Morning \Rightarrow TH252 to I394

Figure 4.39: Identification of Bottleneck Sections (I-94 S1 EB)

4.3.22 I-94 (MPLS to I494) WB Afternoon \Rightarrow *1394 to TH252*



Figure 4.40: Identification of Bottleneck Sections (I-94 S1 WB)



4.3.23 I-94 (MPLS to STPL) EB Afternoon \Rightarrow 1394 to 135W



4.3.24 I-94 (STPL to MPLS) WB Afternoon \Rightarrow *135E to 135W*



Figure 4.42: Identification of Bottleneck Sections (I-94 S2 WB)



4.3.25 I-94 (MPLS to STPL) EB Morning \Rightarrow 1394 to 135W

Figure 4.43: Identification of Bottleneck Sections (I-94 S3 EB)

4.3.26 I-94 (STPL to MPLS) WB Morning \Rightarrow *135E to 135W*



Figure 4.44: Identification of Bottleneck Sections (I-94 S3 WB)



4.3.27 I-94 (STPL to WISC) EB Afternoon \Rightarrow 135E to 1694



4.3.28 I-94 (WISC to STPL) WB Morning \Rightarrow 1694 to 135E







4.3.29 I-35W (SS to MPLS) NB Morning \Rightarrow *135E to River Bridge*

Figure 4.47: Identification of Bottleneck Sections (I-35W S1 NB)



4.3.30 I-35W (MPLS to SS) SB Afternoon \Rightarrow 1494 to River Bridge

Figure 4.48: Identification of Bottleneck Sections (I-35W S1 SB)



4.3.31 I-35W (MPLS to NS) NB Afternoon \Rightarrow MPLS to 1694

Figure 4.49: Identification of Bottleneck Sections (I-35W S2 NB)



4.3.32 I-35W (NS to MPLS) SB Morning \Rightarrow NS to 1694

Figure 4.50: Identification of Bottleneck Sections (I-35W S2 SB)

4.3.33 I-35E (SS to STPL) NB Morning \Rightarrow 1494 to Ayd Mill Rd



Figure 4.51: Identification of Bottleneck Sections (I-35E S1 NB)



4.3.34 I-35E (STPL to SS) SB Afternoon \Rightarrow STPL to Ayd Mill Rd

Figure 4.52: Identification of Bottleneck Sections (I-35E S1 SB)



4.3.35 I-35E (STPL to NS) NB Afternoon \Rightarrow STPL to MN36

Figure 4.53: Identification of Bottleneck Sections (I-35E S2 NB)



4.3.36 I-35E (NS to STPL) SB Morning \Rightarrow 1694 to MN36

Figure 4.54: Identification of Bottleneck Sections (I-35E S2 SB)



4.3.37 I-694 (I94 to I35E) WB Morning \Rightarrow TH36 to I35E

Figure 4.55: Identification of Bottleneck Sections (I-694 S1 WB)

4.3.38 I-694 (I-35E to I-94) EB Afternoon \Rightarrow *135E to TH36*



Figure 4.56: Identification of Bottleneck Sections (I-694 S1 EB)



4.3.39 I-694 (I35E to TH252) WB Morning \Rightarrow I35W to MN65



4.3.40 I-694 (TH252 to I35E) EB Afternoon \Rightarrow MN65 to I35W



Figure 4.58: Identification of Bottleneck Sections (I-694 S2 EB)

4.4 SUMMARY

This chapter summarized the analysis results for the travel-time reliability trends of a total of 23 corridors (48 directional routes) in the metro freeway network for the period of 2016 to 2020. First, a set of the travel-time reliability and traffic-flow measures were estimated using TeTRES with the monthly and yearly data collected in the previous chapter. Further, a new vulnerability index combining the 95th percentile buffer index and the 95th percentile travel rate was used to assess the overall reliability status and variation trends of each route from 2016 to 2020 under all conditions. The resulting vulnerability index value for each route was also applied to identify the most vulnerable routes from 2016 to 2020 for both morning and afternoon peak periods. The yearly vulnerability values of individual directional routes in the metro freeway network indicate an improving trend from 2016 through 2018, while the increased construction activities in 2019 made reliability indices of most routes worse than those in 2018. These trends were also noted in the traffic-flow measures, i.e., the total vehicle-milestraveled of all the directional routes were decreased in 2019 from 2018, while the total delayed-vehiclehours were increased in 2019 for both morning and afternoon routes. The travel-time reliability estimates under different operating conditions indicate that the weather, in particular, snow is the most important factor affecting the travel-time reliability, while the quantified effects of snow on specific reliability measures can vary depending on the amount and frequency of precipitation on a given route. Finally, a set of potential bottleneck sections were identified for each directional route by examining the normal weekday congestion patterns and the specific geometry conditions of a given route. For each potential bottleneck section of a directional route, the vulnerability index was estimated with the 2019 data under all conditions and the bottleneck section with the highest vulnerability index was identified as the priority bottleneck section for a given route. Future research can extend the current work by normalizing the vulnerability indices of the bottleneck sections in each route and determine the network-wide priorities of the existing bottleneck sections of all directional routes.

CHAPTER 5: PRELIMINARY EVALUATION OF FREEWAY CORRIDOR OPERATIONAL RESILIENCE

5.1 INTRODUCTION

A reliable and resilient freeway network, which can absorb, recover and adapt to various operating conditions, is of critical importance in sustaining the way of life and economic vitality of the Twin Cities metro area. While substantial progress has been made in estimating travel-time reliability on transportation networks, quantifying operational resiliency on a corridor level is still in its infancy.

To be sure, most research efforts to date on the resilience of transportation systems have focused on estimating the capability of a given transportation network in dealing with extreme events, such as natural disasters or large-scale incidents. In the early work by Murray-Tuite [3], a set of multiple metrics were estimated using simulation to quantify the resilience of a transportation network in four dimensions, i.e., adaptability, mobility, safety and recovery. Specifically, DYNASMART-P was used to estimate the effects of user equilibrium and system optimum traffic assignments on each of those four areas in a test network. Cimellro et al. [4] presented an approach to define the resilience of a transportation system as a time-variant functionality of a given system during adverse conditions. A conceptually similar approach was also proposed by Devannadham et al. [5], who defined the resilience of a transportation system as the ratio of the system's recovery at a given time to the loss that system suffered because of the adverse condition in subject. The use of graph theory to quantify the networkwide resilience was proposed by Leu et al. [6], and also by Berche et al. [7]. In both approaches, the structural deficiencies of a given network, e.g., network-connectivity and spatial distributions, were modeled with a graph theory, and applied to quantify the network-wide resilience. The incorporation of traffic-flow theory in formulating resilience has been tried by Kim et al. [8], who defined the resilience of a given traffic network as the sum of the areas between the network-wide fundamental diagrams under normal and emergence operations. The application of a network simulation model was also proposed by Dorbritz [9], who simulated node removals and estimated their operational impacts in measuring the resilience of a given network. A recent study by Calvert and Snelder [10] proposed a Link Performance Index for Resilience based on assumed values of the capacity and critical density for individual links. A similar effort to quantify the resilience of individual roadway sections was also presented by Khaghani [11], who proposed a resilience score for an individual section as a weighted average of multipledimensional measures on the speed-variation patterns, such as loss, recovery rates and event duration time.

As noted above, most existing research efforts regarding resiliency have tried to quantify network or community-wide resilience by integrating the individual metrics in multiple dimensions, i.e., organizational, and social aspects of resilience with financial and technical ones. While these approaches tried to capture multi-dimensional aspects of resilience and their complex interactions, the major difficulties of such approaches include the quality and availability of the data required for estimating

proposed resilience measures. As a result, no-widely accepted measure of resilience is currently available for transportation systems. Recently some researchers tried to quantify the operational resilience of roadways using traffic data, their efforts to date have been limited to individual roadway sections and the development of a corridor-wide resilience measure reflecting the interactions of various bottleneck types has not been addressed in their work.

Developing corridor-specific resilience measures that can be estimated with the field data available from the current detection system is of critical importance for an effective allocation of limited resources to competing corridors, thus for improving the resilience of a given roadway network. In this chapter, a preliminary study to formulate and estimate operational resilience of a freeway corridor is conducted using 3 corridors in the metro freeway network as the sample corridors. The proposed corridor-wide operational resilience index (CORI) from this study is designed to capture the capability of a given directional corridor, i.e., route, in coping with the common traffic disturbances on a day-to-day operation, such as incidents and demand variations. The rest of the chapter summarizes the collected data, formulation, and estimation of the corridor-wide operational resilience measures with the traffic and geometry data from the sample corridors.

5.2 MODELING AND ESTIMATION OF OPERATIONAL RESILIENCE OF SAMPLE CORRIDORS

5.2.1 Sample Corridors and Data Collection

Figure 5.1 shows the locations of the 3 sample freeway corridors, i.e., I-494 Northbound/Southbound (NB/SB), TH 100 (NB/SB) and US 169 (NB/SB), used in this study to formulate and estimate the operational resilience of each corridor. As can be seen from the figure, these 3 corridors were in parallel and each of them could be used as an alternative to others. It needs to be noted that the corridor-wide operational resilience measure to be developed in this study is focusing on the capability of each individual corridor in dealing with the traffic disturbances on its own roadway, while the effects of the potential interaction among adjacent corridors can be reflected on the traffic demand entering/exiting to/from each corridor.

First, the peak-period, traffic-flow data for each directional route of those 3 corridors, i.e., two routes per corridor, were collected by using TICAS, Traffic Information Condition Analysis System developed at University of Minnesota Duluth, for the weekdays, i.e., Tuesdays, Wednesdays, and Thursdays, of a two-month period from September to October 2019. They include route-wide total entering volume ($V_{E,t}$) and delayed-vehicle-hours (DVH_t) for every 5-minute interval. Figure 5.1 also shows the locations of the three sample corridors used in this study along with the detector stations on the northbound directional route on each corridor. Further, all the accident data during the same periods were also collected in cooperation with the Regional Transportation Management Center, MnDOT. Figure 5.2.2 shows a screenshot of an example incident data extracted from the MnDOT incident database for the sample

corridors. As shown in Figure 5.2, the collected data includes incident location, type and time duration for each incident on each corridor. In addition, the weather data for the study period were also extracted from the TeTRES, Travel Time Reliability Estimation System, database for the sample corridors. In this study, only the traffic data under dry-weather condition were used for formulating the operational resilience of each directional route.



Figure 5.1: Locations of Sample Freeway Corridors and Detector Stations on Northbound Routes

Cdts	Udts	Xdts	Number of Lan	Event Type	Sub Event Type	Classification	X Street1	X Street2		
2019-09-03	15:3 2019-09-03 10	6:0 2019-09-03 16:0	0	TRAFFIC MGM1	OCCUPIED STA	STALL	NB 100 HWY N	W 50TH ST		
2019-09-03	16:2 2019-09-03 1	6:2 2019-09-03 17:1	0	PROPERTY DA	HIT AND RUN	CRASH	EB 494 I E	NB 100 HWY N		
2019-09-04	16:2 2019-09-04 10	6:2 2019-09-04 17:1	0	PROPERTY DA	MAGE CRASH	CRASH	NB 100 HWY N	CEDAR LAKE R	DS	
2019-09-04	17:0 2019-09-04 1	7:0 2019-09-04 18:3	0	TRAFFIC MGM1	UNOCCUPIED S	STALL	NB 100 HWY N	394 HOV I		
2019-09-05	18:1 2019-09-05 18	8:1 2019-09-05 19:1	0	PROPERTY DA	MAGE CRASH	CRASH	NB 100 HWY N	394 HOV I		
2019-09-11	06:0 2019-09-11 06	5:0 2019-09-11 19:3	0	TRAFFIC MGM1	UNOCCUPIED S	STALL	SB 81 CR	NB 100 HWY N	TO SB 81 CR RM	P
2019-09-11	17:1 2019-09-11 17	7:1 2019-09-11 17:5	0	TRAFFIC MGM1	UNOCCUPIED S	STALL	DULUTH ST TO	DULUTH ST		
2019-09-11	17:2 2019-09-11 17	7:3 2019-09-11 18:4	0	TRAFFIC MGM1	OCCUPIED STA	STALL	NB 100 HWY N	MINNETONKA E	BLVD	
2019-09-12	15:1 2019-09-12 1	5:1 2019-09-12 15:5	0	PROPERTY DA	MAGE CRASH	CRASH	NB 100 HWY N	10 CR		
2019-09-17	17:4 2019-09-17 1	7:4 2019-09-17 19:3	0	PERSONAL INJ	URY CRASH	CRASH	NB 100 HWY N	MINNETONKA E	BLVD	
2019-09-19	15:3 2019-09-19 1	5:3 2019-09-19 18:2	0	TRAFFIC MGM1	OCCUPIED STA	STALL	NB 100 HWY TO	36TH AVE N TO	NB 100 RMP	
2019-09-25	15:2 2019-09-25 1	5:2 2019-09-25 17:1	0	STALLED VEHIC	CLE- BLOCKING	STALL	NB 100 HWY N	MINNETONKA E	BLVD	
2019-09-25	16:1 2019-09-25 1	6:1 2019-09-25 17:3	0	PROPERTY DA	MAGE CRASH	CRASH	NB 100 HWY N	394 HOV I		
2019-09-25	17:2 2019-09-25 1	7:2 2019-09-25 18:1	0	STALLED VEHIC	CLE NOT BLOCK	STALL	NB 100 HWY N	DULUTH ST		
2019-09-25	18:1 2019-09-25 18	8:1 2019-09-26 12:1	0	STALLED VEHIC	CLE NOT BLOCK	STALL	NB 100 HWY N	394 HOV I		
2019-10-01	13:4 2019-10-01 13	3:4 2019-10-03 09:0	0	TRAFFIC MGM1	UNOCCUPIED S	STALL	NB 100 HWY N	BROOKLYN BL	D TO NB 100 HW	VY RMP
2019-10-01	17:4 2019-10-01 10	8:2 2019-10-01 18:2	0	TRAFFIC MGM1	LAW ENFORCE	LAW ENFORCE	NB 100 HWY N	W 50TH ST		
2019-10-08	17:2 2019-10-08 1	7:2 2019-10-08 18:1	0	PROPERTY DAI	MAGE CRASH	CRASH	NB 100 HWY N	EB 7 HWY		
2019-10-09	14:2 2019-10-09 14	4:2 2019-10-09 15:3	0	NON-MOTORIZE	ED VEHICLE ON	PEDESTRIAN	NB 100 HWY N	BROOKLYN BL	/D	
2019-10-09	14:5 2019-10-09 14	4:5 2019-10-09 15:3	0	PEDESTRIAN O	N FREEWAY	PEDESTRIAN	NB 100 HWY N	W 77TH ST		
2019-10-15	15:4 2019-10-15 1	6:0 2019-10-15 17:1	0	TRAFFIC MGM1	OCCUPIED STA	STALL	NB 100 HWY N	10 CR		
2019-10-16	15:4 2019-10-16 1	5:4 2019-10-16 15:5	0	MOTORIST ASS	SIST	STALL	LILAC DR N	EB 55 HWY TO	NB 100 HWY RM	Р
2019-10-16	15:4 2019-10-16 1	5:4 2019-10-16 16:5	0	TRAFFIC MGM1	OCCUPIED STA	STALL	NB 100 HWY N	55 HWY		
2019-10-23	17:4 2019-10-23 1	7:5 2019-10-23 18:2	0	TOW ASSIST		STALL	NB 100 HWY N	394 HOV I		
2019-10-24	10:5 2019-10-24 10	0:5 2019-10-25 07:5	0	STALLED VEHIC	CLE NOT BLOCK	STALL	NB 100 HWY N	LAKE BREEZE	AVE	
2019-10-29	14:1 2019-10-29 14	4:1 2019-10-29 17:1	0	TRAFFIC MGM1	OCCUPIED STA	STALL	W BROADWAY	42ND AVE N		
2019-10-30	17:1 2019-10-30 1	7:1 2019-10-30 18:2	0	PROPERTY DA	HIT AND RUN	CRASH	WB 62 HWY	NB 100 HWY N		

Figure 5.2: Sample Incident Data

5.2.2 Collection of Geometry Data for Each Directional Route

A set of the geometry data for each directional route was also collected in this task to study the potential relationship between the operational resilience and the geometry of each corridor. The selected data for each directional route includes the followings:

- Route length (Linear distance between upstream and downstream boundary stations)
- Number of exit and entrance ramps on each route
- Lengths of weaving sections on each route
 - The length of each weaving section is measured from the Google Earth map as the distance from the merge to diverge gore points, i.e., pavement markings.
- Length of each mainline section with same number of through lanes
 - Auxiliary lanes are not considered as through lanes.

Table 5.1 shows the raw geometric data collected for each directional route of the sample corridors.

Table 5.1: Geometric data of Directional Routes

	Length (mi)	Miles of Weaving Sections (number of)	# of Exit Ramps	# of Entrance Ramps
1494 NB A	14.4	0.17 (2)	11	12
1494 SB M	15	0.12 (2)	12	11
US169 NB A	14.5	1.91 (14)	24	26
US169 SB M	14	1.82 (12)	24	24
TH100 NB A	13.7	0.97 (6)	18	19
TH100 SB M	14.4	0.95 (6)	20	22

5.2.3 Formulation and Estimation of Operational Resilience for Selected Corridors with Traffic-Flow Data

Figures 5.3 and 5.4 show the Delayed-Vehicle-Hours (DVH_t) through time for each directional route of the sample corridors during the weekday peak periods. The values of DVH_t are estimated every 5-minute for an entire route with the traffic data from each route using the following formula:

 $DVH_t = \Sigma [(TT_{i,t} - FF_TT_i) * K_{i,t} * L_i]$ for all segment i in a given route,

where, DVH_t = Total Delayed-vehicle-hours during t for a given route,

TT_{i,t} = Estimated travel time of segment i during t,

FF_TT_i = Free-Flow travel time of segment i

K_{i,t} = *Traffic density of segment i during t,*

 L_i = Length of segment *i*.



Figure 5.3: Delayed-Vehicle-Hour (DVHt) and Total Entering Volume (VE,t) Variations through Time



Figure 5.4: Delayed-Vehicle-Hour (DVHt) and Total Entering Volume (VE,t) Variations through Time

As implied in the above definition, DVH_t indicates the amount of congestion during t interval and the DVH_t – time variation pattern directly reflects the congestion start-expansion-recovery process of a given route in responding to the various types/levels of traffic disturbances, e.g., changes in traffic demand and incidents. Figure 5.3 also includes the variations of the total route-wide entering volume, $V_{E,t}$, through time for each directional route during the weekday peak periods. The values of $V_{E,t}$ are calculated every 5 minutes with the traffic-volume data from the detector stations on each route as follows:

 $V_{E,t}$ = (Upstream Boundary Station Volume)_t + Σ (All Entrance Ramp Volumes)_t

The analysis of the DVHt variation patterns of the sample directional routes indicates:

- The DVH_{t,max}, i.e., the maximum value of DVH_t during a peak period, and the time duration to reach DVH_{t,max}, reflect the 'Resistance' and 'Adaptation' capabilities of a given route in coping with the congestion caused by the traffic disturbances, i.e., incidents, weather or traffic demand changes.
- The congestion 'Recovery' patterns, i.e., the slope of the 'DVH_t Recovery' lines, i.e., the lines from DVH_{t,max} to Zero DVH in the DVH_t-time graphs, exhibit remarkable similarities for a same route, while the slopes of those recovery lines vary from route to route. This could imply each route has its own 'Recovery' capability from congestion.

Figure 5.5 illustrates the above observations with the DVH variations on multiple days at the I-494 Northbound routes. As shown in this figure, the DVH recovery lines on different days indicate similar slopes, while the time duration and pattern to reach the maximum DVH value on each day varies depending on the demand and incident patterns.



Figure 5.5: I-494 Northbound DVH Variation Patterns

Based on the above observations, in this study, the *Operational Resilience of a freeway corridor* is defined as "*the capability of a freeway corridor traffic system to absorb and adapt to various levels of*

traffic disturbances and recover from congestion with maximum efficiency under given operating conditions." Figure 5.6 shows the schematic diagram, where the congestion duration/scope of a freeway corridor is the direct output from the operational resilience, which is the core capability of a given corridor in resisting and recovering from congestion caused by various external factors.



Figure 5.6: Conceptual Relationship between Operational Resilience and External Factors

The conceptual relationship shown in Figure 5.6 implies that each corridor has its own operational resilience level, which can be directly reflected by the corridor-wide delay measures. Further, the resilience level of each corridor depends on the geometric structure and operating strategies of a given corridor. In this study, a Corridor-wide Operational Resilience Index (CORI) is formulated and estimated for each directional route with the traffic-flow data available from the current detection system on freeways as follows:

$$CORI_{i} = \frac{\Sigma_{t}(DVH_{t}*A_{t})}{(\Sigma_{t}V_{E,t})*\sigma}$$

where,

 $CORI_i = Operational Resilience Index of Corridor i$ $DVH_t = Corridor$ -wide delayed-vehicle-hours during t, $A_t = Proportion of weighted average number of lanes during t,$ $V_{E,t} = Corridor$ -wide total entering volume during t,

 σ = Standard deviation of $V_{E,t}$ during a peak period.

As noted from the above formulation, the proposed CORI tries to quantify the capability of a given directional corridor in minimizing the traffic delay under given variations in corridor-wide traffic demand and geometric conditions, i.e., number of available through lanes through time. Therefore, a smaller CORI index indicates a stronger resilience level of a given corridor. It needs to be noted that, in the proposed formulation of CORI, the effects of incidents on the traffic flow are reflected in A_t, the proportion of the corridor-wide available lanes during t, which is determined as follows:

 $A_{t} = \frac{Average \ Weighted \ Number \ of \ Through \ Lanes \ - \ (Number \ of \ Lanes \ Blocked)_{t}}{Average \ Weighted \ Number \ of \ Through \ Lanes}$

In the above formulation, the 'number of blocked lanes' during t depends on the type/scope of an individual incident, however, in the current incident database from MnDOT, very few incidents have the blocked-lane data, while the data regarding the type and duration of each incident are available. Since most incidents affect the speed of the traffic flow going through an incident location, in this study, due to the time limitation, the following values in Table 5.2 are assumed to be the number of the blocked lanes for each type of incident. It needs to be noted that these values should be refined with the field data in the future study.

Incident Classification	Number of Lanes Blocked
STALL	0.3
PEDESTRIAN	0.6
CRASH	0.9
SPINOUT	0.6
DEBRIS	0.6
WRONG WAY DRIVER	1.2
JUMPER	0.6
LAW ENFORCEMENT	0.6
MAINTENANCE	0.6
FIRE	1.2
ANIMAL	0.6
MEDICAL	1.2
SLUMPER	0.9
MNPASS MAINTENANCE	0.6
CLASSIFICATION NOT LISTED	0.3

Table 5.2: Assumed Number of Blocked Lanes

In addition, the 'Average weighted number of through lanes' of a given corridor, a surrogate measure of the corridor-wide through capacity, is determined as shown in Figure 5.7:



Figure 5.7: Process to Determine Average Number of Through Lanes

In the above formulation, the roadway sections located downstream have larger weights than those upstream, therefore, the effects of downstream bottleneck sections on the corridor-wide through traffic can be captured more effectively. It also needs to be noted that the auxiliary or acceleration/deceleration lanes are not considered as through lanes in this formulation.

5.2.3.1 Estimation of Corridor-wide Operational Resilience for Sample Directional Routes

The above definition of the corridor-wide operational resilience is applied to the sample corridors and the daily CORI values of each directional route is calculated with the traffic data from a two-month period, September – October 2019. In this preliminary study, the specific effects of weather are not directly addressed, i.e., the traffic data from only dry weather conditions were used to estimate the daily CORI values of the sample directional routes. Figure 5.2.7 includes the daily estimates of CORI for each directional route under the dry weather conditions, i.e., no precipitation, during September – October 2019. It needs to be noted that, according to the NOAA weather data, there were rains in the mornings of Sept 11 and Oct 22, while no rain was reported in the afternoons of those two days. Therefore, the CORI values of all the southbound routes with morning peak periods are not determined for those two days. Table 5.2.3 includes the average and standard deviation values of the daily CORI during the test period and Figure 5.2.8 shows the graphical comparison of the average CORI for each sample route.

As noted in Figures 5.8 and 5.9, the CORI estimates of those routes with the morning-peak periods show relatively stable day-to-day variations, while the afternoon peak-period routes show more fluctuations than those from the morning-peak routes. The t-test results indicate that the CORI values of the sample routes are significantly different at 95% confidence level. While the further study needs to address these differences in CORI between the northbound and the southbound routes, the initial assessments indicate the northbound routes appear to have more favorable geometric structures, as discussed in the following section with the geometric friction modeling, compared to the southbound routes. Furthermore, the afternoon traffic-demand patterns also show more variations than those with the morning periods. It can also be noted that more accurate incident data regarding the blocked number of lanes and time durations for each incident will improve the accuracy of the estimation results.



Figure 5.8: Daily Estimates of CORI for the Sample Directional Routes

Table 5.3: Average CORI for Each Route

Route	1494 NB A	1494 SB M	US169 NB A	US169 SB M	TH100 NB A	TH100 SB M
CORI	0.0302	0.0244	0.0486	0.0220	0.0375	0.0261
Standard Dev.	0.0095	0.0067	0.0197	0.0042	0.0139	0.0092





5.3 EFFECTS OF CORRIDOR-WIDE GEOMETRIC FEATURES ON OPERATIONAL RESILIENCE

Finally, the potential relationship between the corridor-wide operational resilience and the geometric structure of each corridor is studied in this task. In this study, the 'strength' of the corridor-wide geometric structure in terms of handling the traffic flows going through a given corridor is quantified with the following geometry-based parameters:

- Number of Exit Ramps per mile (G1),
- Number of Entrance Ramps per mile (G2),
- Proportion of the total non-weaving section length (G3) = 1 [Σ (lengths of all weaving sections) / route length],
- Average number of through lanes weighted with the distance from upstream boundary (G4) as defined before.

Using the above parameters, the 'strength' of the geometric structure of a given corridor in terms of facilitating through traffic movements, G, is formulated as follows:

$$G = \frac{G1 * G3 * G4}{G2}$$

In the above formula, the numerator reflects the combined effects of the geometric features facilitating the through movements within a given corridor, while the denominator quantifies the potential frictions caused by the entrance volumes to the through traffic in a given corridor. In this study, the G factor is named as the geometric-friction factor, i.e., the higher G value, the less geometric friction in a given directional route. Table 5.4 includes the values of each parameter and the resulting G values for all the sample directional routes. As shown in this table, the southbound routes show a higher G value, i.e., less geometric friction, than the northbound route in a same corridor. Figure 5.10 shows the relationship between the CORI, corridor-wide operational resilience index, and the geometric-friction parameter, G, of each route. As expected, the routes with the higher G values, i.e., less geometric friction, generally show stronger operational resilience.

Table 5.4: Geometric Friction Parameters and Estimation Results

	1494 NB	I494 SB	US169 NB	US169 SB	TH100 NB	TH100 SB
Total Route Length (mi)	14.4	15	14.5	14	13.7	14.4
G1 (Exit ramps/mi)	0.764	0.800	1.655	1.714	1.314	1.389
G2 (Entrance ramps/mi)		0.733	1.793	1.714	1.387	1.528
G3c (Proportion of non-weaving length)	0.988	0.992	0.868	0.870	0.929	0.934
G4 (Avg weighted # of through lanes)	3.426	3.411	2.520	2.476	3.033	3.344
G	3.104	3.691	2.020	2.155	2.669	2.840



Figure 5.10: Operational Resilience vs Geometric Friction

5.4 SUMMARY

This chapter summarized the results from the preliminary study to quantify the operational resilience of a freeway corridor. A total of 6 directional routes in 3 corridors were selected as the sample routes and, for each route, a set of the detailed geometric and traffic-flow data, including incidents and weather information, were collected for a two-month period from September to October in 2019. Using the collected traffic and incident data, the congestion start/recovery process of each route were analyzed and a model to quantify the corridor-wide operational resilience, CORI, of a given corridor was formulated and applied to the sample directional routes for the weekday peak periods under dry weather condition. The resulting CORI estimates of the sample directional routes indicate that the southbound routes show consistently stronger resilience with stable day-to-day variations than the northbound route in a same corridor. Further, the average resilience values of all the sample routes are significant different at 95% confidence level. Finally, the potential relationship between the operational resilience and the geometric structure of each sample route was analyzed by quantifying the geometric friction of each directional route in facilitating corridor-wide through traffic flows. The quantification of the geometric friction is based on the geometry data easily measurable from the field. The resulting geometric friction parameters of the sample routes show clear correlation with the operational resilience of each route, i.e., for the sample routes selected in this study, the southbound route in a given corridor has less geometric friction with stronger operational resilience than the northbound route in a same corridor.

The corridor-wide operational resilience and geometric friction measures developed and tested in this study shows the possibility of applying the proposed measures for prioritizing freeway corridors with accurate understanding of the main sources of traffic congestion and travel-time reliability issues. Future study needs include the expanded testing of the proposed resilience and geometric-friction models to other corridors in the metro network with the detailed information on the blocked lanes during incidents.

CHAPTER 6: CONCLUSIONS – RESEARCH BENEFITS/IMPLEMENTATION/FUTURE STUDY NEEDS

6.1 RESEARCH BENEFITS

6.1.1 Decrease Engineering/Administration Costs

The enhanced TeTRES and its newly expanded database, populated with the historical data from the metro freeway network, can significantly reduce the time and effort of MnDOT staff in preparing data, calculating reliability measures and analyzing estimation results. The detailed quantification of time and cost savings in MnDOT through TeTRES would depend on the types and/or scopes of potential MnDOT projects and their specific needs for estimating travel-time reliability and traffic-flow measures.

6.1.2 Operation and Maintenance Savings

As described in this report, the enhanced TeTRES with the additional traffic-flow MOE module can estimate both travel-time reliability and traffic-flow measures under different operating conditions for given corridors and periods. This indicates TeTRES can be applicable for an efficient identification of the major factors affecting the performance of various operational strategies for selected corridors. Such a capability can lead to the development of effective operation and maintenance strategies for each corridor, i.e., tailored strategies addressing the specific issues of given corridors, thus saving the operational and maintenance costs for the metro freeway network.

6.1.3 Reduce Risk

The identification of the vulnerable bottleneck sections in the major freeway corridors, which resulted from this study, can be used as the basis for developing optimal resource-allocation plans, which could maximize the cost-effectiveness of improving the metro freeway network, thus minimizing risks.

6.1.4 Reduce Road-User Cost

The identification of vulnerable freeway sections can also contribute to the improvement of the daily operations of freeway traffic systems, e.g., effective routing of FIRST trucks during peak periods and advanced driver guidance with time-of-day reliability information for pre-defined corridors. Such improved operations of freeway systems could decrease the unpredictability in corridor travel times, and, thus, reduce road-user cost in the metro freeway network.

6.2 IMPLEMENTATION STEPS

To facilitate the realization of the benefits that resulted from this study, the following steps are recommended to be taken in cooperation with the Regional Traffic Management Center (RTMC), MnDOT:

- Technical assistance in installing the final version of TeTRES at the RTMC, MnDOT, and conducting customized workshops for MnDOT staff to facilitate the adaptation of TeTRES for each office operations.
- Technical assistance to MnDOT offices in estimating and applying reliability and traffic-flow measures with TeTRES for selected corridors.
- Technical assistance to MnDOT offices in applying TeTRES for formulating and conducting before/after studies to quantify the changes in traffic-system performances in terms of travel-time reliability and traffic-flow measures of effectiveness.

6.3 FUTURE STUDY NEEDS

Developing a reliable and resilient freeway network requires the continuous assessment of the corridorwide reliability trends and operational resilience on an ongoing basis. Future research needs include 1) continuous population of the TeTRES database with detailed data on incident and work-zone information, including the lane status affected by each incident and work zone, 2) analysis of the integrated effects of operational and geometric-changes at selected corridors on travel-time reliability and traffic-flow measures, 3) refinements of the corridor-wide operational-resilience and the geometricfriction indices, developed in this study, with data from additional corridors with diverse geometric features. Finally, an institutional support for ongoing system enhancements needs to be explored to continuously improve the operations and maintenance of the TeTRES software and the databases associated with TeTRES.

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