

# **Unmanned Aircraft Systems (UAS) - Metro District Bridge Inspection Implementation**

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Collins Engineers, Inc.

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# UNMANNED AIRCRAFT SYSTEMS (UAS) – METRO DISTRICT BRIDGE INSPECTION IMPLEMENTATION

## FINAL REPORT

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# TABLE OF CONTENTS

<b>CHAPTER 1: Introduction .....</b>	<b>1</b>
1.1 Research Background and objectives .....	1
1.1.1 Phase I – Unmanned Aerial Vehicle Bridge Inspection Demonstration Project.....	2
1.1.2 Phase II – Unmanned Aircraft System Bridge Inspection Demonstration Project .....	4
1.1.3 Phase III – Improving the Quality of Bridge Inspection using Unmanned Aircraft Systems (UAS) .....	6
<b>CHAPTER 2: Technology Review.....</b>	<b>9</b>
2.1 Collision Tolerant and Obstacle Avoidance UAS .....	10
2.1.1 Flyability Elios.....	10
2.1.2 Skydio 2.....	12
2.2 Consumer Grade UAS .....	15
2.2.1 Parrot Anafi.....	16
2.3 Commercial Grade UAS .....	18
2.3.1 Intel Falcon 8+ .....	18
2.3.2 Sensefly Albris .....	21
2.4 Terrestrial Imaging Platforms.....	22
2.4.1 Point and Shoot Cameras .....	23
2.4.2 Commercial DSLR Cameras .....	24
2.4.3 Smart Phone Cameras .....	24
2.5 Processing Technologies.....	25
2.5.1 Post Processing Technologies .....	25
2.5.2 Processing and Modeling Accessory Hardware .....	28
2.6 Field Testing and Calibration.....	33
2.6.1 Carver County Survey and AeroPoint Accuracy Report.....	33

2.6.2 UAS Imaging Payload Test .....	34
2.6.3 QR Code Distance Test with Pix4D Tagger.....	44
2.6.4 Computer Processing Comparison .....	47
<b>CHAPTER 3: Bridge List Candidate Selection .....</b>	<b>53</b>
3.1 Selection Criteria .....	53
3.2 Implementation.....	53
<b>CHAPTER 4: UAS Implementation in the Field.....</b>	<b>55</b>
4.1 St. Croix Crossing Bridge .....	55
4.1.1 UAS Operation.....	56
4.1.2 Mission Scope .....	56
4.1.3 Deliverables.....	57
4.1.4 Findings.....	58
4.2 I-394 @ Dunwoody On and Off Ramps.....	59
4.2.1 Mission Scope .....	59
4.2.2 Deliverables.....	62
4.2.3 Findings.....	65
4.3 Blatnik Bridge 9030 Inspection.....	66
4.3.1 Mission Scope .....	68
4.3.2 Deliverables.....	69
4.3.3 Findings.....	70
4.4 SHV Bridge.....	71
4.4.1 Mission Scope .....	71
4.4.2 Deliverables.....	72
4.5 Tettegouche Bridge .....	73
4.5.1 UAS Operation.....	74

4.5.2 Mission Scope .....	74
4.5.3 Deliverables.....	75
4.6 Stone Arch Bridge Inspection and Rehabilitation Project.....	78
4.6.1 Mission Scope .....	79
4.6.2 UAS Implementation Workflow .....	81
4.6.3 Deliverables.....	83
4.6.4 Findings.....	89
4.6.5 Conclusion.....	89
<b>CHAPTER 5: Safety and Operation Manual .....</b>	<b>91</b>
5.1 Overview.....	91
<b>CHAPTER 6: Conclusion and Recommendations .....</b>	<b>92</b>
6.1 Program Application.....	92
6.2 Initial Implementation .....	92
6.3 Sustained Implementation.....	92
<b>REFERENCES.....</b>	<b>93</b>
<b>APPENDIX A Unmanned Aircraft Systems (UAS) Safety and Operation Manual .....</b>	<b>94</b>
<b>APPENDIX B MnDOT Metro District Prioritized UAS Bridge List .....</b>	<b>1</b>

## LIST OF FIGURES

Figure 1-1 Overall Photographs of Four Bridges Selected for Phase I UAS Inspection (Shown clockwise from upper right, Arcola Bridge, Morrison County Pedestrian Truss, Chisago County Bridge, Oronoco Concrete Arch) .....	3
Figure 1-2 Overall Photograph of Four Bridges Selected for Phase II UAS Inspection .....	5
Figure 1-3 Phase III UAS Enhanced Deliverables.....	7
Figure 2-1 Elios Rolling Along an Abutment Bearing Seat. ....	11
Figure 2-2 Elios 2 Protective Frame with Unobstructed View .....	12



Figure 2-3 Elios 2 Dust Proof Lighting Comparison .....	12
Figure 2-4 Skydio 2 .....	13
Figure 2-5 Skydio 2 Generated Point Cloud Environment Between Bridge Beams .....	13
Figure 2-6 Skydio 2 Flying in Tight Area Using Obstacle Avoidance.....	14
Figure 2-7 Skydio Bridge Inspection Video .....	15
Figure 2-8 Image of Concrete Spall from the Anafi Thermal .....	16
Figure 2-9 Parrot Anafi .....	17
Figure 2-10 Intel Falcon 8+ Platform .....	19
Figure 2-11 Intel Falcon 8+ Cockpit Controller.....	19
Figure 2-12 Intel Mission Control Flight Plan.....	20
Figure 2-13 Sensefly Albris TripleView Payload .....	22
Figure 2-14 Point and Shoot Digital Camera for Photogrammetry.....	23
Figure 2-15 DSLR Digital Camera for Photogrammetry .....	24
Figure 2-16 Custom Built Post Processing .....	25
Figure 2-17 Pix4D Cloud Virtual Inspection Tool.....	26
Figure 2-18 Fuji Film AI Crack Detection on Bridge .....	27
Figure 2-19 Fuji Film AI Crack Detection on Bridge Deck Close Up.....	28
Figure 2-20 Propeller AeroPoint UAS Survey Target .....	29
Figure 2-21 QR Code Targets Created and Used in Phase IV .....	29
Figure 2-22 QR Code Targets Adhered to a Stone Face.....	30
Figure 2-23 Pix4D Tagger QR Code Example with Numbering Convention of the Key Points.....	30
Figure 2-24 Pix4D GCP/MTP Editor Screenshot Showing Points Automatically Inserted on a QR Code....	31
Figure 2-25 Microsoft HoloLens 2.....	31
Figure 2-26 Holographic Overall Bridge View from HoloLens 2.....	32
Figure 2-27 Holographic Close Up View of Bridge from HoloLens 2 .....	32

Figure 2-28 Pix4D Ground Control Point Editor Showing a Propeller AeroPoint.....	33
Figure 2-29 Skydio Image Comparisons at Determined Distances.....	35
Figure 2-30 Skydio Resolution Test at Several Angles From 10' .....	39
Figure 2-31 Anafi Resolution Test at Several Angles from 10' .....	40
Figure 2-32 Intel Falcon 8+ Resolution Test at Several Angles From 10' .....	41
Figure 2-33 Skydio’s Full Image at 100 Feet From the Visual Test Board.....	42
Figure 2-34 Anafi’s Full Image at 100 Feet From the Visual Test Board .....	43
Figure 2-35 Intel Falcon 8+’s Full Image at 100 feet From the Visual Test Board.....	44
Figure 2-36 Pix4D Screenshot with the Locations Of Each Shot Taken During the Test .....	45
Figure 2-37 Pix4D Screenshot in Basic GCP/MTP Editor With Notes .....	46
Figure 2-38 Pix4D Tagger Test Results.....	46
Figure 2-39 Chart Showing the Match Percentage Dropping After 80 Feet.....	47
Figure 2-40 Computer 1 Quality Report Step 1.....	48
Figure 2-41 Computer 1 Quality Report Step 2.....	48
Figure 2-42 Computer 2 Quality Report Step 1.....	49
Figure 2-43 Computer 2 Quality Report Step 2.....	49
Figure 2-44 Computer 3 Quality Report Step 1.....	50
Figure 2-45 Computer 3 Quality Report Step 2.....	50
Figure 2-46 Computer 4 Quality Report Step 1.....	51
Figure 2-47 Computer 4 Quality Report Step 2.....	51
Figure 2-48 Computer 5 Quality Report Step 1.....	51
Figure 2-49 Computer 5 Quality Report Step 2.....	52
Figure 2-50 Table of Computing Power Processing Test Results .....	52
Figure 4-1 St. Croix Crossing Bridge.....	55
Figure 4-2 St. Croix Crossing Bridge Inspection Planning Map .....	56

Figure 4-3 St. Croix Crossing Bridge Flight Plans .....	57
Figure 4-4 St. Croix Crossing Bridge 3D Photolog.....	58
Figure 4-5 RC Imaging Platform with GoPro Camera Cluster .....	61
Figure 4-6 Flyability Inspector Screenshot.....	62
Figure 4-7 Bridge 27831A Deck with Measurements in Pix4D Cloud.....	63
Figure 4-8 27831A Deck using the Inspector Mode in Pix4D Cloud .....	63
Figure 4-9 Unfolded Views of a Box Girder Model, Left Being Untraced and the Right Traced .....	64
Figure 4-10 3D Model of a Cell.....	64
Figure 4-11 Thermal Snapshot of a Dunwoody Bridge Deck .....	66
Figure 4-12 Overall Photo of the Blatnik Bridge .....	67
Figure 4-13 Preplanned Flights of the Blatnik Bridge .....	67
Figure 4-14 Aerial View of the Intel Drone Collecting Data .....	68
Figure 4-15 Intel Drone Collecting Data .....	69
Figure 4-16 Pix4D Cloud Data for the Blatnik Bridge.....	70
Figure 4-17 Timber Culvert Imaging Flight Path.....	71
Figure 4-18 Metal Pipe Arch Interior Model Imaging Path .....	72
Figure 4-19 3D Model Snapshot View with Closeups of Areas of Concern .....	73
Figure 4-20 Tettegouche Bridge.....	74
Figure 4-21 Tettegouche Bridge Flight Plan.....	75
Figure 4-22 Tettegouche Bridge Pix4D Cloud Model Virtual Inspector Tool .....	76
Figure 4-23 Tettegouche Bridge Pix4D Cloud Model with Inspection Notes.....	76
Figure 4-24 Tettegouche Bridge Modeled in Context Capture .....	77
Figure 4-25 Tettegouche Bridge Context Capture Model Close Up View.....	77
Figure 4-26 Overall Photo of the Stone Arch Bridge .....	78
Figure 4-27 Preplanned Flights of the Stone Arch Bridge.....	79

Figure 4-28 Video of Falcon 8+ Performing Inspection .....	80
Figure 4-29 Aerial View of the Intel Drone Collecting Data .....	80
Figure 4-30 Intel Drone Collecting Data .....	81
Figure 4-31 Reality Model of Spans 17-22 .....	82
Figure 4-32 Reality Model of Spans 17-22 With Inspection Notes .....	82
Figure 4-33 3D Reality Model of Stone Arch Bridge During River Drawdown .....	83
Figure 4-34 Reality Model Zoomed in to Pier During River Drawdown .....	84
Figure 4-35 2D Orthomosaic Overall .....	85
Figure 4-36 2D Orthomosaic Medium Close Up.....	85
Figure 4-37 2D Orthomosaic Close Up .....	85
Figure 4-38 ContextCapture Insights AI Crack Detection .....	86
Figure 4-39 Virtual Inspection of Bridge Using Microsoft Hololens 2 .....	87
Figure 4-40 Hololens Inspection Defect Notation .....	88
Figure 4-41 Hologram Virtual Inspection Process .....	88

## LIST OF ABBREVIATIONS

3D	Three-Dimensional
UAS	Unmanned Aircraft System
AASHTO	American Association of State Highway and Transportation Officials
ADT	Average Daily Traffic
BSIPM	Bridge and Structure Inspection Program Manual
CFR	Code of Federal Regulations
CoRE	Commonly Recognized Structural Elements
DSLR	Digital Single-Lens Reflex
FAA	Federal Aviation Administration
FC	Fracture Critical
FCM	Fracture Critical Member
FHWA	Federal Highway Administration
FPV	First Person View
GCP	Ground Control Point
GPS	Global Positioning System
HDR	High Dynamic Range
IRT	Infrared Thermography
JSON	JavaScript Object Notation
LOS	Line of Sight
LRFD	Load Resistance Factor Design
MBE	AASHTO Manual for Bridge Evaluation
MNDOT	Minnesota Department of Transportation
MT	Magnetic Particle Testing
MUTCD	Manual of Uniform Traffic Control Devices

MTP	Manual Tie Point
NBI	National Bridge Inventory
NBIS	National Bridge Inspection Standards
NCHRP	National Cooperative Highway Research Program
NDE	Non-destructive Evaluation
NDT	Non-destructive Testing
NHI	National Highway Institute
NRHP	National Register of Historic Places
PPE	Personal Protection Equipment
QR	Quick Response (QR Codes)
SI&A	Structure Inventory and Appraisal
SIMS	Structure Information Management System
TH	Trunk Highway
TL	Team Leader
UBIV	Under Bridge Inspection Vehicle
UT	Ultrasonic Testing
UTG	Ultrasonic Thickness Gage

## EXECUTIVE SUMMARY

Bridges are important components of our transportation system, and maintaining these structures is critical to ensuring the safety of the traveling public. Inspection of bridges is fundamental to programming maintenance and corrective actions necessary to ensure a healthy and functioning transportation system. Bridge safety inspections are mandated by federal law and the collection of inspection data can be costly and time consuming, and inspections contain inherent risks for the bridge inspectors and the traveling public. Unmanned aircraft systems (UAS) have proven to be a low cost and low risk way to supplement bridge inspection efforts in Minnesota. Technology from both a hardware and software standpoint continues to evolve making UAS use more accessible to bridge inspectors and deliverables are increasingly more beneficial.

As part of this research project, UAS was applied to many different bridge inspections for several different purposes. The bridges ranged from small simple structures to large, river-crossing, signature bridges. The data collected was integrated into the bridge inspection reports mostly by including links in the report to cloud-based, web-sharing platforms.

MnDOT has recently purchased a fleet of drones in addition to the Flyability Elios that is already owned by MnDOT Metro District. The purchase and use of these new drones will exponentially increase the implementation of UAS for bridge inspections within MnDOT. This implementation will benefit MnDOT by lowering inspection costs, improving quality, and improving safety for bridges inspectors and the traveling public. It will also provide an additional access tool, supplementing the use of under-bridge inspection vehicles (UBIV) and rope access.

As part of this research project, a risk based prioritized list was developed for Metro District bridges that would benefit from drone inspections based on parameters conducive to UAS use. These parameters included average-daily-traffic (ADT), feature intersected, bridge type, bridge conditions, and a record of previous successful UAS inspections.

The bridges were sorted by score, with higher scores representing bridges that are more likely to benefit from UAS inspection from both a risk and benefit standpoint. Several of the bridges on the list were inspected as part of this project and results can be found in Chapter 4 of this report. This list can be used in the future to help identify bridges that can be inspected by the Elios drone.

Several case studies are presented in this report demonstrating different applications for using UAS for bridge inspections. These efforts are included as part of this report, but all work was part of actual inspection projects. These results are incorporated into the bridge inspection record. UAS technology has developed to the point where field work and post processing has become routine. The efficiencies gained from technological advances and from experience are significant. For example, the Blatnik Bridge located in Duluth was part of the Phase II Research Project, and one week was spent in the field with a team of 6 people who covered only a small portion of the main spans. In this phase of research, a team of two people completed the entire UAS inspection in less than one day and gathered data for the entirety of the main spans.

An *Unmanned Aircraft Systems (UAS) Safety and Operation Manual* was developed as part of this project. The intent of the manual was to provide a risk-based approach in evaluating when to use UAS for bridge inspections based on hardware and software that MnDOT uses for bridge inspections. Also included were best practices to guide UAS operators and help minimize the risk of incidents, which could cause damage to the drone or property or cause injuries. The manual was based on several years of experience with drone use within MnDOT and while risks cannot be completely eliminated, following the manual will minimize risks significantly.

UAS have proven to be a low-cost and low-risk way to supplement bridge inspection efforts in Minnesota. Technology for both hardware and software is evolving making UAS more accessible to bridge inspectors and making the deliverables more beneficial. Artificial intelligence, mixed reality, holograms, autonomous flights, computer vision, and reality modeling are no longer technologies that our industry looks forward to but are available now and have been used by Collins Engineers and MnDOT. This report demonstrates how the technologies work for bridge inspections, giving examples and use cases that demonstrate the benefits.



# CHAPTER 1: INTRODUCTION

## 1.1 RESEARCH BACKGROUND AND OBJECTIVES

Bridges are important components of our transportation system, and maintaining these structures is critical to ensuring the safety of the traveling public and protecting these public investments. Inspection of bridge structures is fundamental to determining the maintenance schedule and corrective actions necessary to establish a healthy and functioning transportation system. The National Bridge Inspection Standards (NBIS) set minimum requirements for bridge inspections including inspector qualifications, inspection intervals and inspection procedures. The NBIS was implemented into federal law in 1968. As of November 2019, the NBIS was in the process of Proposed Rule Changes which would significantly change the NBIS requirements. Proposed changes include an expanded extent of inspection, stating “Any portion of the bridge not visible using standard access methods must be assessed via another method”. Phases I, II and III of this research project presents the cost effectiveness and increased safety of inspection through the use of Unmanned Aircraft Systems (UAS). For the purpose of this report, the terms UAS and drone are used interchangeably. Combined with the rapid development of UAS technology and regulation, UAS implementation has been proven to be an effective, efficient, and safe tool for visual access to components otherwise not visible using standard methods.

The first two phases of the project, Phase I and II, were carried out in the summers of 2015 and 2016, respectively. The primary scope of these phases was to evaluate UAS technology as a tool for bridge inspection. The third phase, Phase III, was carried out in the summer of 2018 and focused on the application of UASs in the inspection of various bridge types and configurations. As this research has progressed, the focus has shifted from if UASs can be used, to where and how can UASs be used. The resulting studies were published by MnDOT’s Research Services. These phases outline the definition of UAS hardware, processing software, FAA regulation, and various methods of implementation.

Due to the success of the research thus far, MnDOT Metro District purchased the [Elios](#) UAS to supplement bridge inspection access where space is confined and optimal lane closures are prohibited, which has been an on-going issue in the District due to traffic volumes. In addition, as of spring 2020 the MnDOT Bridge Office is in the process of developing a statewide UAS program for structure inspection. The program will provide necessary means and methods to both state and local agencies to inspect bridge components using state issued UASs.

Phase IV of the research effort implements drone inspection to the Metro bridge inventory by creating an inspection plan that would identify situations best suited for UAS use, what parameters govern UAS use in bridge inspection, and how UAS could be integrated into standard inspection operations. This project also develops a UAS Safety and Operation Manual specific to bridge inspections and is applicable to the MnDOT Bridge Office Statewide UAS Program.

## 1.1.1 Phase I – Unmanned Aerial Vehicle Bridge Inspection Demonstration Project

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### 1.1.1.1 Scope

This project phase, completed in 2015, demonstrated the use of UAS for bridge inspection, evaluated the technology's effectiveness, and addressed the safety implications for routine bridge inspections.

### 1.1.1.2 Execution

Investigators identified four bridges in Minnesota that represented a variety of bridge types and sizes: an 80-foot local bridge in Chisago County, a medium-sized concrete arch bridge in Oronoco, a large steel truss bridge in Morrison County, and a 2,682-foot long railroad bridge near Stillwater that rises 185 feet above the St. Croix River.

Researchers then reviewed current and proposed FAA rules and regulations pertaining to UAS use for bridge inspection and worked with the MnDOT Office of Aeronautics to acquire necessary authorization for inspections. After reviewing UAS options, investigators selected the [Aeyron Skyranger](#) UAS and contracted a drone pilot to help conduct inspections of each selected bridge. Researchers compared UAS results to recent bridge inspection records. The Skyranger drone is shown in the photos below.



**Figure 1-1 Overall Photographs of Four Bridges Selected for Phase I UAS Inspection (Shown clockwise from upper right, Arcola Bridge, Morrison County Pedestrian Truss, Chisago County Bridge, Oronoco Concrete Arch)**

### 1.1.1.3 Findings

The Aeyron Skyranger UAS provided high-quality detail on the two large bridges, and its zoom lens was effective with the medium-sized concrete arch bridge, allowing viewing and assessment of many bridge element conditions. Smaller bridges with limited clearance underneath proved challenging for the UAS due to loss of GPS signal under concrete decks. As the UAS lost GPS signal, it would then return automatically to its take-off point, or home base. Another barrier of this specific UAS was that the camera mounted underneath the drone could not look up.

Before UAS field work began on any of the selected bridges, detailed investigation and safety plans were prepared for each structure. Site-specific plans addressed safety, potential hazards and how to mitigate them, current FAA rules, and inspection methods. Based on analysis of field work, inspection results, regulations for UAS use, and emerging inspection specific UAS technology, researchers concluded the following:

- UASs can be used for bridge inspection with little risk to inspectors and the public and can reduce safety risks that inspectors currently face. They should be considered a tool in routine inspection and for situations not requiring hands-on inspection, testing, sounding, or cleaning.

They also suit pre-inspection surveys and can identify rope anchor points and other safety needs before hands-on inspection begins.

- UASs provide inspection detail that effectively replicates detail learned through use of snoopers without traffic control at significantly lower costs in equipment and traffic control needs.
- UASs provide both infrared and 3D modeling detail of bridges, effectively identify concrete delamination, gather topographic mapping detail, and efficiently map riverbank conditions upstream and downstream from the bridge sites.
- Inspectors should select a UAS capable of pointing cameras upward and operating without GPS.

## **1.1.2 Phase II – Unmanned Aircraft System Bridge Inspection Demonstration Project**

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### **1.1.2.1 Scope**

This project phase aimed to expand the demonstration to different structure types and size utilizing an inspection specific UAS to further assess the ability of a UAS to be a widespread and accepted inspection tool. Technology and federal regulation were further evaluated to refine the inspection method.

### **1.1.2.2 Execution**

This research phase built on Phase I findings and looked at additional Minnesota bridges including a large steel through arch, a steel high truss, a large corrugated steel culvert, and a movable steel truss. Now having acquired a new inspection specific [Sensefly Albris](#) shown in the images below, the performance was compared to the industry standards of hands-on inspections. Each method was evaluated by focusing on the differences in access methods, data collection, and the ability of the UAS to be used as a tool for interim and special inspections. FAA rules were explored to determine how practical they were regarding UAS bridge inspection applications.

Before UAS field work began on any of the selected bridges, detailed investigation and safety plans were prepared for each structure. Site-specific plans addressed safety, potential hazards and how to mitigate them, current FAA rules, and inspection methods.

Several imaging devices were tested including still image, video and infrared cameras. After the data collection was completed, data was processed through the computer software [Pix4D](#).



**Figure 1-2 Overall Photograph of Four Bridges Selected for Phase II UAS Inspection**

**1.1.2.3 Findings**

Based on the observations in the field from the Phase I and Phase II studies, the following conclusions were made:

- UASs can be used safely and effectively on bridges in challenging conditions.
- UASs can be used in GPS-deprived environments, but piloting skills become more important.
- UASs are more suitable as a tool for inspection of bridges with elements that are difficult to access.
- UASs themselves cannot perform inspections independently and should be used as a tool for qualified and experienced bridge inspectors to view and assess bridge element conditions in accordance with the National Bridge Inspection Standards (NBIS).
- UASs used in conjunction with thermal sensors can be an effective way to detect concrete delaminations and can be done without closing the bridge to traffic.
- The ability to direct cameras 90 degrees upward and the ability to fly without a GPS signal are important features when using this technology as an inspection tool.
- In some types of inspections, UASs have the capability to be used in lieu of an under-bridge inspection vehicle and would provide significant savings. These savings would come in the form

of reduced or eliminated traffic control and reduced use of under bridge inspection vehicles and lifts.

- Safety risks associated with traffic control, working at heights and/or confined spaces, and near traffic could be reduced with the use of UASs.
- UASs can provide important pre-inspection information for planning large-scale inspections. Information such as clearances, rope access anchor points, and general conditions can easily be established with UASs and would aid in the planning of an inspection.
- Utilizing UASs in conjunction with photogrammetry software such as Pix4D can provide a 3D model and point cloud of a bridge and bridge site that is valuable in determining unknown dimensions and provides a high-quality inspection report deliverable.

### **1.1.3 Phase III – Improving the Quality of Bridge Inspection using Unmanned Aircraft Systems (UAS)**

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#### **1.1.3.1 Scope**

This research phase expanded the implementation of inspection specific UASs to include a larger number of bridge types, sizes, and configurations. Specific consideration was made to use UASs in place of and in addition to standard inspection procedures and access methods. UASs enhanced deliverables were incorporated into inspection reports alongside standard deliverables to facilitate in vetting the work product of UASs.

#### **1.1.3.2 Execution**

This research phase implemented UAS technology on 39 bridges including a wide range of bridge size, type, and location. The results of this research effort demonstrated that UAS technology and processing software are effective tools to improve the quality of bridge inspections in addition to improving safety and reducing costs. By applying the technology to a large number of bridges, we were able to understand where the different technologies work the best and were able to determine cost savings and efficiencies gained. Traditional access and reporting methods will continue to be utilized even as UAS technology improves, but they have proven to be another effective tool. This phase identified opportunities to improve the quality of MnDOT inspections, improve safety, and reduce costs.

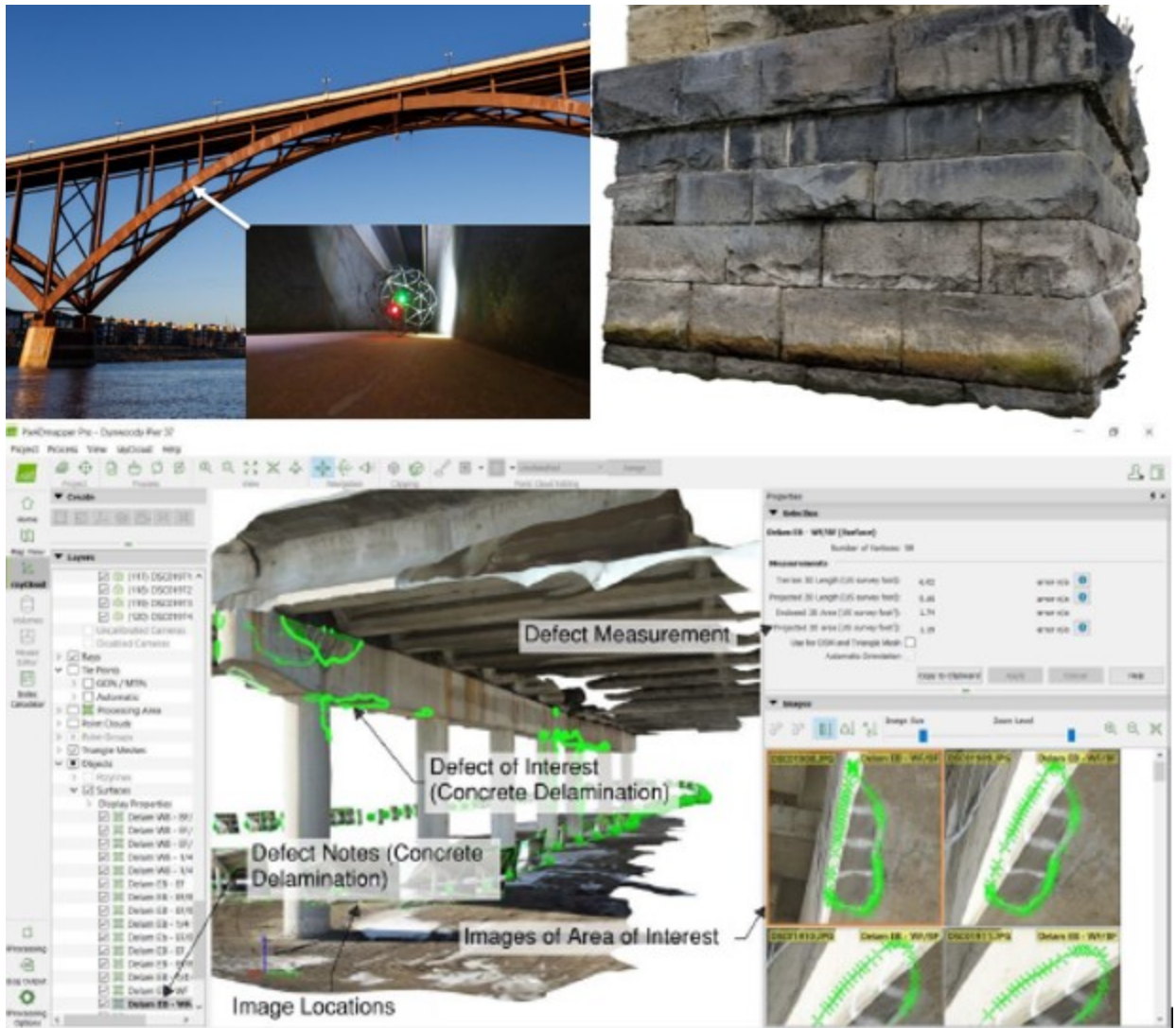


Figure 1-3 Phase III UAS Enhanced Deliverables

### 1.1.3.3 Findings

Phase III findings further concluded that the use of UASs as a bridge inspection tool was a valuable and reliable asset for every bridge inspector to have access to. As identified in Phases I and II, UASs play a specific role in bridge inspection. They should not be considered a replacement for required hands on inspection techniques but, rather to be a supplemental tool for inspectors to use in situations where access or environmental factors would otherwise make standard techniques too risky or not possible. Below are some key findings from Phase III:

- FAA and [MnDOT Office of Aeronautics](#) regulations continue to change. However, they are becoming increasingly more accepting of the technology and its uses. Through the creation of [Part 107](#), [LAANC](#), and other regulation aids, acquiring permission to fly is now a relatively simple process and possible in a significant portion of statewide airspace. Part 107 is the set of

regulations the FAA has in place for commercial drone operators and LAANC is the system in place to exchange data on to provide Low Altitude and Notification Capabilities.

- UAS hardware selection is important to consider with respect to what the desired outcome and deliverable are, along with the operating environment. Commercial grade UASs remain to be the most reliable, safe, and productive UASs on the market but can carry an expensive price tag. Lower cost consumer UASs are adequate for many jobs, however, they are not a substitute for commercial UASs currently.
- UAS hardware is continuing to adapt to its consumer needs to meet the requirements of FAA and professional grade deliverables. Technology advancements such as sense and avoid technology, automated flight capabilities in GPS denied environments, and improved battery life are significant advancements which could make UAS for bridge inspection even more attractive.
- Data captured during UAS missions can be in many different forms and yield different results. Prior to flying, a pilot or engineer should have an idea of the final desired deliverable and ensure data gathered is optimized to create this product.
- Drone use should be considered as part of a risk-based approach to bridge inspection where safety, cost and quality improvements can be realized.
- Safety risks can be reduced for both inspectors and the public. Much of the focus has been on the safety of flying a UAS, but the emphasis should be on reducing the risk of the overall inspection.
- A collision-tolerant UAS should be considered for confined space inspections where access and safety can be improved without a reduction in inspection quality.
- Collision-tolerant UASs should be considered for the inspection of multi-beam bridges, especially when a hands-on inspection is cost prohibitive.
- Field conditions, including weather, and bridge type, location and configurations vary widely. While image quality is important, the focus should be on the inspector's qualifications, experience and ability to determine if the quality of the data is enough to determine with certainty the structural condition of a bridge.
- Many of the same improved deliverable benefits can be realized by utilizing terrestrial photography. Models of specific bridge components such as piers can be generated in combination with a UAS or by themselves.
- UAS technology has advanced rapidly but their benefits are not being realized due to underutilization. Bridge owners should consider their use when considering inspection quality, cost savings and safety.
- Reality modeling of bridges is revolutionizing the documentation and communication of inspection results. Bridge owners should take advantage of this technology to improve their bridge inspection programs.



## CHAPTER 2: TECHNOLOGY REVIEW

UAS technology continues to progress as manufacturers innovate and strive to meet the needs of both commercial and recreational users. The transportation industry is benefiting from these innovations, and the use of UASs for bridge inspections has become more beneficial and accessible in the last five years. Improved sensors, lower costs, better reliability, and collision avoidance are some of the improvements that reduce risks and increase benefits for bridge inspections. The ability to create 3D models provides an additional benefit to inspections.

Recreational use has created a large demand for mid-grade consumer UAS platforms, such as the [Parrot Anafi](#) and [DJI Mavic](#). These mid-grade UAS platforms are incorporating technologies which benefit the inspection specific UAS field; however, they still are not a suitable replacement for commercial UAS platforms for many applications. Image quality continues to be a significant differentiator when selecting a drone for bridge inspections.

There is a wide range of uses and deliverables when using UASs for bridge inspection. The selection of an inspection UAS should be based on the environment and goal of the inspection. For example, using a UAS with collision tolerant frames or active obstacle avoidance is a good choice for situations where close-up imagery is required and in locations that are heavily congested or confined. A consumer grade UAS is generally the best selection for overall terrain modeling, imaging of components that are not easily accessible from the ground but have a clear line-of-sight from a higher elevation, or in 3D model generation of smaller structures (generally under 150 feet long). Commercial UAS platforms are used in the same situations as a consumer grade UAS but, can be used on much larger and more complex bridges that have spans in excess of 4,000 feet. A commercial UAS is also preferred when image quality is important, which is often the case in bridge inspections. Commercial quality drones also improve the ability to create 3D models especially for medium and large bridges. Images that are captured with a commercial UAS are higher quality and allow for either a rapid collection of data or a significantly better deliverable.

While individual images of bridge components are a traditional and important aspect for bridge inspections, the ability to create 3D models of bridges allow for improved deliverables. Bridge inspectors can better document bridge conditions and clearly communicate inspection results to bridge owners with 3D models. Reality modeling software allows for still images and video to be processed and analyzed, regardless of the UAS platform used to collect data in the field. Additional new technologies including automatic ground control targets and QR Codes were explored and implemented in field data acquisition for this project. In addition to reality modeling software, this project also explored the use of software for automatic crack detection which is a promising technology that can improve the ability to both identify and quantify defects. These improvements in reality modeling software continue, making it more accessible and feasible on a wider range of bridge inspections.

The following is a summary of the technologies utilized during Phase IV of this study to provide an outlook of how UAS technology is quickly progressing with improved benefits and the removal of

limitations. The technologies discussed include both new and established UAS platforms, equipment, and software.

## 2.1 COLLISION TOLERANT AND OBSTACLE AVOIDANCE UAS

As part of the Phase II study, it was identified that there are many areas within bridge inspection that are prohibitive for imaging using a larger mapping UAS. Additionally, these are often the same areas that are very difficult, or even impossible, for inspectors to gain visual or tactile access due to environmental hazards and entry restrictions. The concept of a collision-tolerant UAS was explored as part of the Phase II study. The concept of obstacle avoidance was explored as part of the Phase IV study. Other options were considered to access these hard to reach areas with a smaller micro-UAS, larger propeller shrouds, or additional acoustic anti-impact sensors. Some benefits and limitations of both UAS platforms utilized in this phase are listed in the following.

### 2.1.1 Flyability Elios

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MnDOT purchased an [Elios](#) in 2018 for use in confined space and close-quarters bridge inspection. At the time of purchase, the collision-tolerant was the most cost-effective option. Some of the benefits and limitations of the Elios are listed below.

Recognized benefits of an inspection specific collision-tolerant UAS are the following:

- Risk of damage is very low.
- Ability to navigate areas of a bridge that are difficult to access.
- Ability to roll: The protective frame can serve as a rolling device to better control the UAS, save battery life, and maintain a fixed distance from the face of an object. It was ideal for inspecting wide flange beams and concrete deck soffits by rolling the UAS along the top side of the bottom flanges.



**Figure 2-1 Elios Rolling Along an Abutment Bearing Seat.**

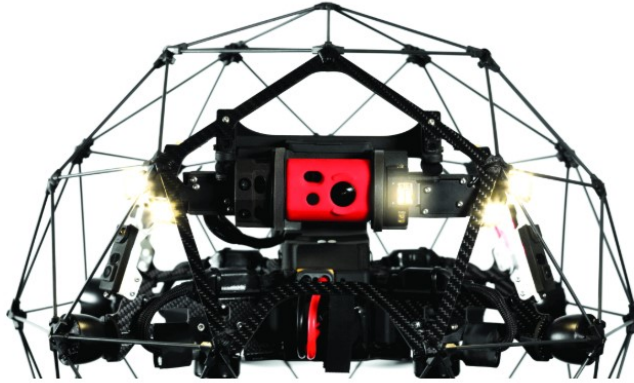
- Set-Up: Due to the simplicity of the equipment and interactive flight type, set up and site assessment are quick processes (typically 5 minutes).
- Lighting: The UAS is equipped with on-board lighting which is required due to fluctuating light/dark environments and proximity to elements.
- Safety for the inspector can be greatly improved by eliminating the need for confined space entries, risks associated with access equipment, and working from heights.
- Safety for the traveling public can be improved by eliminated traffic control and lane closures.

Recognized limitations of an inspection specific collision-tolerant UAS:

- Short battery life: The added weight of the protective frame and equipment reduces the allowable battery size, and thus reduces the battery life. A single battery operating under proper piloting conditions and operation yielded an average of 10 minutes of flight time, which limits its range and coverage. Swapping batteries is a quick process which mitigates this limitation.
- Video Interference: The protective frame is outside the video payload, meaning the frame will always be in the video partially obstructing the view.
- Air Flow and Debris: While operating in confined areas or near object surfaces, the UAS can create air flow eddies which affect the UAS flight. In addition, operating in close proximities to surfaces kicks up dirt and debris which can damage the propellers and interfere with video quality.
- The approximate cost of the Elios 2 drone is \$35,000 USD.

Since Phase II of this study, Flyability has released a second generation of the Elios, the [Elios 2](#). It was not implemented in the field for this study, but many of the new features on the Elios 2 address some of the disadvantages and have new benefits for bridge inspection. Benefits of the Elios 2 include:

- Fixed Frame: The protective frame has been fixed and an opening has been implemented near the camera to create an unobstructed view for the 4K and Thermal cameras. This makes creating 3D models from the UAS possible.



**Figure 2-2 Elios 2 Protective Frame with Unobstructed View**

- Dust Proof Lighting: The Elios 2 contains a 10,000 lumen lighting package that is offset from the camera to help eliminate overexposing images with close proximity to dust and debris.



**Figure 2-3 Elios 2 Dust Proof Lighting Comparison**

- GPS-Free Stability and Obstacle Avoidance: Using cameras and on-board sensors, the Elios 2 creates a virtual bubble around itself to stabilize and avoid contacting surfaces. This also allows a distance lock feature for easier navigation and viewing.

### 2.1.2 Skydio 2

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The Skydio 2 is manufactured by [Skydio](#) as displayed in the image below is the second UAS platform made produced by Skydio. The Skydio 2 was initially marketed to adventure enthusiasts for cinematic use but has also been developed with inspection specific enterprise features includes the ability to use obstacle avoidance with a shorter 1 foot range. The environment in which this UAS was created require autonomous flight through dense wooded areas and AI (Artificial Intelligence) capable of real-time

navigation around obstacles. To accomplish this, the Skydio has a state-of-the-art obstacle avoidance and object sensing technology. Skydio produces and develops UAS in the United States.



Figure 2-4 Skydio 2

The Skydio's autonomy system and navigation camera system consist of a high-speed video processor and an on-board camera payload which create a real-time point cloud of the world around it. The onboard processor can create over 1 million points per second at a rate of 500 iterations per second. The camera payload for navigation consists of 6 cameras in a triangular configuration on the top and bottom. Each camera is a 4k Sony sensor with a super fisheye lens (200-degree field of view) creating a 360-degree coverage of the environment around the camera. The generation of a 3D real-time point cloud in conjunction with AI based autonomous flight makes the Skydio an ideal candidate for flying in close proximity to bridge members, confined space and through bridge trusses.

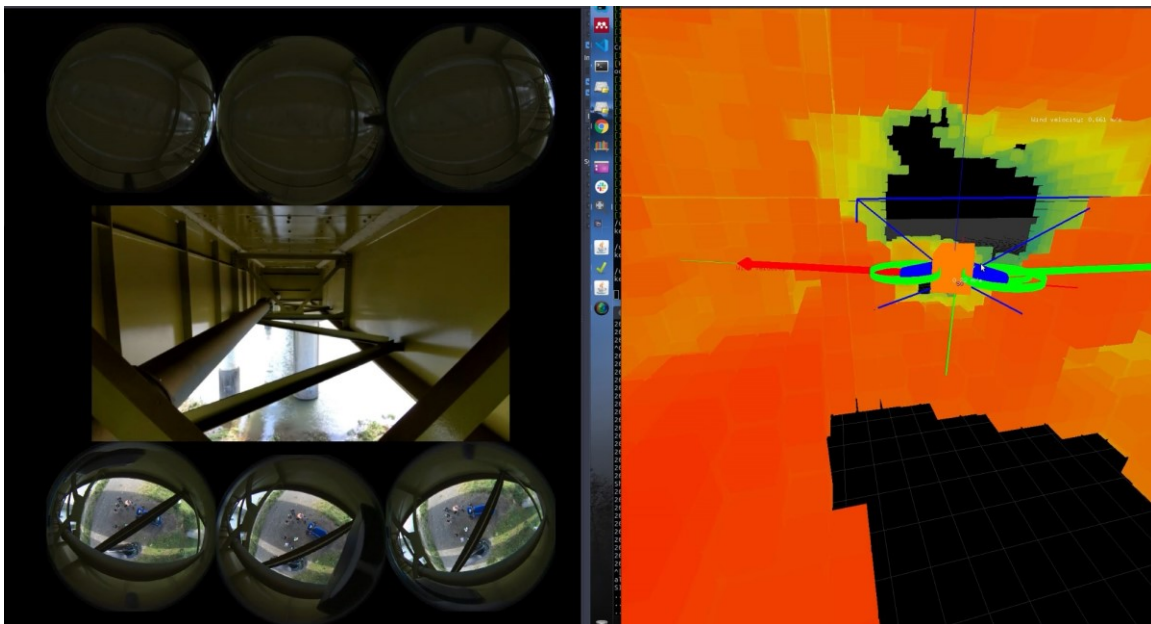


Figure 2-5 Skydio 2 Generated Point Cloud Environment Between Bridge Beams



**Figure 2-6 Skydio 2 Flying in Tight Area Using Obstacle Avoidance**

The onboard primary camera system consists of a Sony 12.3MP sensor capable of 12MP HDR still-photos or 4k HDR video. While this resolution of imagery is on the lower end of professional cameras, the ability to fly within inches of an object or surface makes the 12MP sensor more than adequate for bridge inspection. The following are some benefits and limitations of the Skydio 2 that have been recognized as a result of this study.

Recognized benefits of the Skydio 2:

- Approximate price tag of \$2,500 USD (Jan. 2021) Which includes the hard case and additional batteries. Enterprise features cost and additional \$1,500 per year.
- 3D point cloud-based obstacle avoidance
- Rigid frame construction provides for a more rugged and reliable platform
- 4k HDR video capabilities allows for ideal images in all lighting conditions
- GPS receiver for accurate position data, image tagging, and stabilization
- 23-minute battery life

Recognized limitations of the Skydio 2:

- Low resolution primary camera makes the UAS less than ideal for large area mapping or bridge modeling.
- Six navigation cameras on top and bottom with hemispherical lenses are exposed and has the potential to get scratched if not properly stored or cared for.

- No onboard lighting for internal confined space inspection
- Any moisture collection on the navigation cameras will prohibit flight

The results of exploring the use of collision-tolerant and obstacle avoidance UASs in bridge inspection were overwhelmingly positive. The relative ease of use and minimal set up make the collision-tolerant and obstacle avoidance UASs a great addition to an inspector's toolbox. With a price tag of approximately \$2,500 USD (Jan. 2021), the Skydio 2 provides many of the benefits of the Elios at a lower cost. The Skydio 2 is certainly a tool for inspectors, which will help image previously difficult to reach or inaccessible bridge components. A video detailing the implementation of the Skydio 2 Drone for Bridge Inspections can be found [here](#):

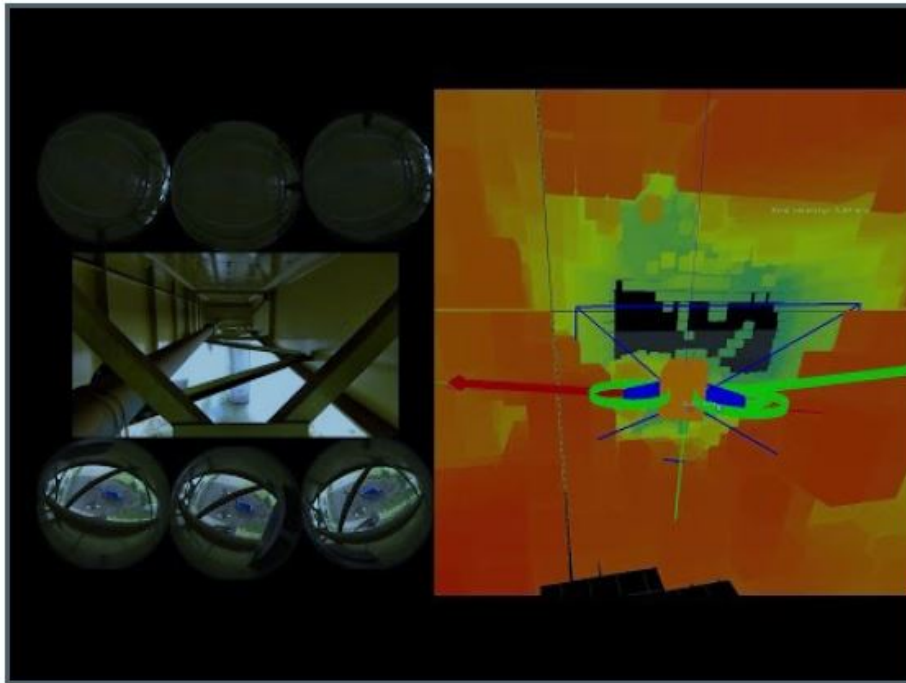


Figure 2-7 Skydio Bridge Inspection Video

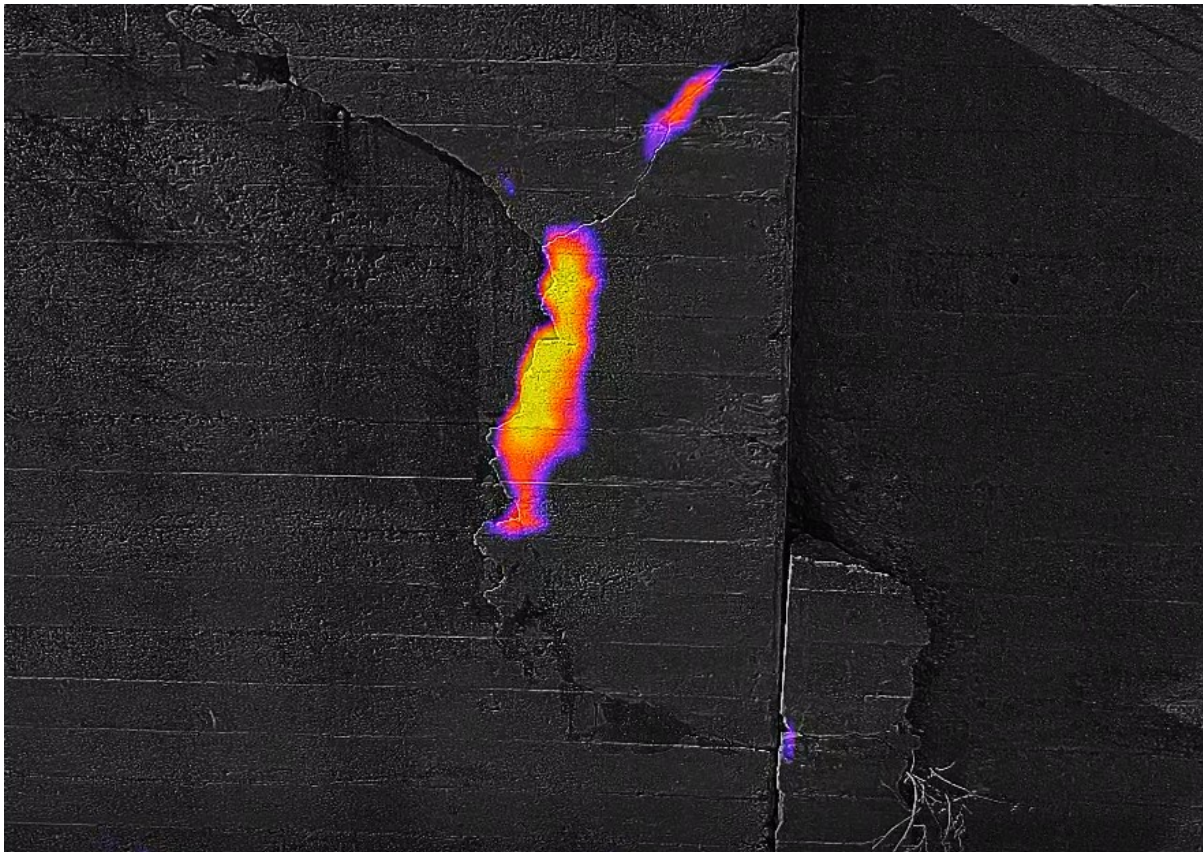
## 2.2 CONSUMER GRADE UAS

Consumer-grade UAS platforms are beneficial due to their ease of use and general wide availability. Many consumer-grade UAS platforms can be purchased at retail stores or online at retail sites such as Amazon. Additionally, consumer-grade UASs are typically small, light, and easily transportable. They typically have a price tag around \$1,000 USD. These attributes make consumer-grade UAS platforms ideal for routine inspection practices and modeling of bridges 150 feet long or less. For this study we used the Parrot Anafi, but many other similar drones exist with similar features such as the DJI Mavic.

### 2.2.1 Parrot Anafi

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The Anafi UAS platform is manufactured by Parrot. Parrot is the parent company of SenseFly who created the Albris and Ebee. Due to the successes of the Albris, Parrot built in many of the essential inspection specific features into the Anafi. One major feature is the ability for the gimbal to pan 180 degrees vertically up or down. The Anafi has a primary camera capable of 21MP wide field, HDR still images, or 4k HDR video. The Anafi also features a 2.8x optical zoom for lossless (non-digital) image magnification and an ISO range of 100 to 3200 for excellent performance in low-light conditions. A thermal version of the Anafi is also on the market which contains an onboard FLIR Lepton 3.5 radiometric thermal camera.



**Figure 2-8 Image of Concrete Spall from the Anafi Thermal**

The Parrot Anafi folds up into a roughly 10in x3 in x3in block which makes it very portable. It can be flown from either a controller using a tablet or phone as the screen or flown from a phone or tablet directly. Several phone apps such as Pix4DCapture and Drone Deploy provide excellent on-site mission planning for near commercial-grade quality of aerial mapping and 3D modeling. The Anafi is not manufactured in China which makes it a candidate for use with government sectors such as military or the Department of Defense.





**Figure 2-9 Parrot Anafi**

The Anafi has onboard acoustic sensors, a GPS receiver, and a propriety digital stabilization technology which all aid in flight stabilization. The UAS is programmed with return to home features and geofence options to provide a safe and reliable working environment. The UAS has a battery loaded weight of 13.6oz and can withstand sustained winds of up to 31mph. These features make the Anafi a safe UAS option and low risk in comparison to other standard methods of bridge inspection. The following are some benefits and limitations of the Anafi that have been recognized as a result of this study:

Recognized benefits of the Anafi:

- 21MP HDR still camera ideal for terrain mapping and modeling
- Small carry size makes easy to deploy in the field
- Easy to set up and to fly
- Lightweight and poses no greater risk to public than a medium size bird
- Low cost compared to commercial UAS platforms
- Small profile provides limited distraction to surrounding public
- Camera gimbal capable of looking vertically up or down

Recognized limitations of the Anafi:

- Limited mission planning software
- Limited use in high wind or moist conditions
- No on-board obstacle avoidance
- Folding frame not as durable as a fixed frame
- Lack of reliability
- Lack of customer support

## 2.3 COMMERCIAL GRADE UAS

Commercial UAS platforms typically vary from consumer UAS platforms by price and quality of product. The two commercial UASs discussed below are the Intel Falcon 8+ and the Sensefly Albris. They both carry a price tag into the 10's of thousands (Feb 2020) and boast high resolution camera payloads with extensive mission planning software. These commercial UAS platforms carry all the benefits of the consumer grade UASs with added safety, reliable mission planning, and execution features. It was realized during Phase IV that environmental factors such as locations (urban vs. rural), distance of flight, and size of bridge govern what UAS platform is best selected. The largest differentiator with commercial UAS is image quality. Commercial UAS typically have image sensors with at least double the resolution of consumer grade drones. Also important is the reliability of commercial UAS and the flight planning software that typically is far more sophisticated than consumer UAS.

### 2.3.1 Intel Falcon 8+

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The Intel Falcon 8+ UAS is a commercial platform featuring best-in-class redundant hardware, GPS, and sensor package with a high-end image payload. The Intel UAS consists of a rigid frame with 8 rotors, cockpit controller, interchangeable imaging payload mounted on a front center gimbal, and uses propriety Intel software for mission planning and post processing. This model can fly under bridge decks, and the camera can look straight up. The battery last for 15 minutes for a typical mission but with the high-resolution sensor this drone can collect a large amount of data per flight. A typical inspection mission can collect 375 42MP images in one flight. This UAS was used extensively throughout Phase IV. Overall, the experience and deliverable created using the Intel Falcon 8+ was the highest quality that has been achieved throughout all four phases of this research project.

The Intel Falcon 8+ features carbon fiber frame construction consisting of 2 rail bars in a V-shape. Each rail houses 4 rotors. At the center of the V is the payload and processing housing. The configuration of rotors allows for flight in wind speeds in excess of 30 mph. Furthermore, the lightweight construction provides best in class weight to payload ratio for excellent flight time. The V-shape provides for an unobstructed field of view from vertical down, 180 degrees to vertical up. The onboard electronic and hardware system features triple redundant flight control with three redundant Internal Measurement Units for quick and reliable data processing of position, altitude and orientation for excellent responsiveness and stability. The 8 rotors, dual batteries, and multiple communication links also offer built-in redundancy for optimal safe operation.



**Figure 2-10 Intel Falcon 8+ Platform**

The cockpit controller is the main control interface with the Falcon 8+ platform. The controller features a built in 8-inch touch screen display for imaging and flight planning and a designated 2-inch system information display for system vitals. The controller has two main joysticks similar to traditional UAS platform controls. However, the right joystick has an additional rotational sensor which makes single hand flight control possible at a set altitude. Toggle switches built into the sides of the controller allow for image payload operation with a single hand. With the innovations of this controller, it is possible for a single pilot to flawlessly fly a manual mission at a given altitude while simultaneously operating and manipulating the imaging payload.



**Figure 2-11 Intel Falcon 8+ Cockpit Controller**

The interchangeable imaging payload of the Falcon 8+ is what sets this UAS apart from other Commercial UAS platforms on the market. Several payloads exist for this platform. Two that stand out specifically are the survey package consisting of a Sony Alpha 7R full frame DSLR camera and the inspection package consisting of a near-infrared and full frame DSLR camera. The Sony Alpha 7R camera

contains a full frame 36MP sensor with photography industry leading processing for excellent imaging in all lighting conditions and optimal image color and sharpness characteristics. The inspection package integrates infrared with full frame photography or high-definition video for real time image overlays and the ability to accurately locate and stamp defects in the field.

Intel Mission Control is a proprietary software suite created by Intel for mission planning for the Falcon 8+. The software features several mission types which can each be flown in series and edited to suit any flight condition. The software also allows for importing digital surface models created from higher altitude initial site flights. This provides a detailed background 3D image to plan flight missions.



**Figure 2-12 Intel Mission Control Flight Plan**

The realized benefits of the Intel Falcon 8+ UAS is the high quality, high precision images that allows for optimal inspection through post processing and a platform that is highly redundant, safe, and resilient to wind, electromagnetic, or various other external influences. Some of the realized limitations of the UAS is the high cost and lack of obstacle avoidance. The overall experience with the Intel Falcon 8+ was very positive, and it proved to be a reliable and durable platform for a best-in-class imaging payload.

Recognized benefits of using the Intel Falcon 8+ UAS:

- Ability to view vertically up and down
- Option of pre-programmed flight or interactive flight (manual flight controls)
- Most advanced flight planning software on the market for complex 3D flights
- High-Resolution high quality images
- Ability to fly without GPS signal
- Up to 15 minutes of usable flight time

- Ability to collect a large amount of data in a short amount of time
- Hardware and software are both resilient and reliable

Recognized limitations of the Falcon 8+ drone:

- Not suited for flight within confined space
- Size of drone and controller makes it difficult to transport easily in the case

### 2.3.2 Sensefly Albris

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This UAS was used in Phase II and III, and then was partially replaced by the use of the Intel Falcon 8+ in Phase IV. Given the overwhelming positive results from Phases II and III, this UAS is still a very suitable and useful tool in inspection. The manufacturer, Sensefly has discontinued the production of the Albris and has halted the progression of their quadcopter development program. Sensefly's parent company, Parrot, has produced the Anafi which implements some of the advantages of the Albris at a much lower cost.

The SenseFly Albris UAS platform was designed for commercial inspection and mapping purposes. This model can fly under bridge decks, and the camera can view straight up. The Albris UAS can be controlled interactively with a controller or autonomously with a pre-programmed flight. Both flight modes utilize a laptop computer to control the UAS. The flight control software contains the UAS's settings, which include a real-time map that displays the UAS's location, live image views, and flight data. The software can also be used to plan and monitor autonomous flights. The UAS is approximately 22 inches by 32 inches by 7 inches and weighs 3.96lbs, allowing for easy handling and transportation. The batteries typically provide up to 215 minutes of flight time when operating under safe manufacturer guidelines. Flight hardware restrictions include wind speeds greater than 22 mph, a range that is over 2.8 miles away, or speeds in excess of 26 mph.

Many of the positive physical specifications and limitations of the Albris have been either duplicated or enhanced in the Parrot Anafi. However, the Albris retains its position in the commercial UAS due to the imaging payload which consists of a TripleView head containing a high-definition video camera, a 38 Mega-Pixel (MP) still camera, and an infrared camera. The 38MP camera is the primary data acquisition tool. The absolute horizontal/vertical accuracy of the UAS is reported at 3ft to 16ft without using ground control points and down to 0.04 in when using ground control points.



**Figure 2-13 Sensefly Albris TripleView Payload**

Recognized benefits of using the Albris UAS:

- Ability to view vertically up and down
- Option of pre-programmed flight or interactive flight (manual flight controls)
- High-Resolution Photogrammetry
- Ability to fly without GPS signal
- Up to 20 minutes of usable flight time
- Distance Lock and Cruise Control
- Onboard LED lighting and camera flash

Recognized limitations of using an inspection-specific mapping and photogrammetry UAS:

- Not suited for flight within confined space
- Set-up is time consuming and requires a laptop, stand, antenna, tripod, and a continuous power source for the laptop.

## **2.4 TERRESTRIAL IMAGING PLATFORMS**

This study focused primarily on the implementation of UAS in bridge inspection, but it was quickly realized that the post processing technology could be an asset to inspectors using traditional terrestrial photography. Terrestrial photography refers to the use of an imaging platform which is held or mounted to something founded on the ground. In certain environmental conditions or regulatory situations, UASs

cannot be safely flown but, the ability to develop a model or 3D photo log still exists with the use of post processing software.

### 2.4.1 Point and Shoot Cameras

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Traditionally, inspectors' toolboxes contain a point and shoot digital camera for capturing defect photos. Using traditional cameras is natural to inspectors and thus, can be implemented with a relatively low additional cost for equipment. Photos taken using any modern digital camera can be an input for post processing software, however the deliverables greatly improved if the camera has the following features:

- GPS Location Enabled: Creates a geo-located image to assist post processing software to order and match images for a model or map.
- High resolution sensor: A camera having a 12MP or greater will allow for a model to be generated and can greatly enhance. In most cases the better resolution, the better the model.
- HDR (High Dynamic Range) Still Imagery: HDR provides excellent photography in all lighting conditions. Often with terrestrial imagery, sky, sun or bridge backgrounds are captured while obscuring the focus on dark elements such as bearing or bridge soffits. HDR imagery assists in consistent image lighting, reducing instances of over or under exposure and saturation.
- Water / Dust / Impact Resistant: Inspection conditions are often in wet, dirty, and tough environments. Waterproof, shockproof, and dustproof camera features provide inspectors a durable and reliable platform with the potential use of underwater imagery for submerged element modeling.



**Figure 2-14 Point and Shoot Digital Camera for Photogrammetry**

### 2.4.2 Commercial DSLR Cameras

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Less traditional to inspectors is DSLR (digital single-lens reflex) cameras. DSLR cameras are commercial grade, high resolution digital cameras. The benefits of these platforms are very high-resolution sensors, typically 36MP or more, which yields images that can display small defects from relatively far distances. The user input settings create better quality images in low light conditions or high-speed photography but requires the user to have a significant understanding of photography. GPS enabled feature are not as common in DSLR cameras and they are much less durable in wet or rough



Figure 2-15 DSLR Digital Camera for Photogrammetry

environments, which limits their use in inspections. A variety of aftermarket lenses such as wide angle or optical zoom options are available for these cameras, but not all are recognized by the post processing software. The use and type of a DSLR camera and lenses should be determined and vetted through post processing software prior to implementing in the field.

### 2.4.3 Smart Phone Cameras

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The largest change and increasing potential in photography platforms is smart phones or tablets. Many phone/tablet producers are making their cameras and image processing focal points in their list of features. The significant ease of use, requiring very little knowledge of traditional photography, makes smart phone or tablet imaging a very reliable deliverable. Additionally, smart phones and tablets continue to have increased resolution sensors which compete with or exceed the ability of point and shoot cameras. Most modern smart phones and tablets have excellent GPS signal and are durable and resilient to wet or tough conditions. Most inspectors carry some version of a smart phone and, in lieu of other technologies, can use these for quick on-site imaging. Some smart phones and tablets are introducing (Feb 2020) LiDAR and other imaging payloads as a supplement to their on-board imaging payload which will further set these data acquisition tools apart from traditional point and shoot cameras.



## 2.5 PROCESSING TECHNOLOGIES

After data is collected in the field in the form of images, GPS tags, ground control and manual tie points the next step is to process that data into deliverables. These deliverables can be in many formats including 3D reality models, 2D orthomosaics and 2D orthoplanes all of which can be geometrically and geospatially correct on the order of sub-centimeter accuracy. These outputs can be used locally or shared with others via cloud platforms. Many software platforms exist to create deliverables and our study is not intended to give recommendations on specific software but, to discuss and compare in general terms the features that are beneficial to bridge engineers. Bridges can be challenging to successfully process because the geometry is complex and mapping the underside of the deck along with the entire bridge takes careful planning and execution in the field. Post processing software is as important as the drone hardware and often gets overlooked in the process. Our team used Pix4D and ContextCapture to process reality models and used Fuji Film Infrastructure Photo Analysis and ContextCapture Insights for artificial intelligence applications.

### 2.5.1 Post Processing Technologies

#### 2.5.1.1 Computer Configurations

Throughout Phases II, III and again, in Phase IV, computing power was foreseen as an issue in processing 3D models efficiently while still delivering a quality product. In several cases where large bridges were imaged using a full frame DSLR camera using preplanned mission software, the raw data files were exceeding 100GB of imagery. On smaller bridges using manual photography and medium resolution cameras, data collected was still typically exceeding several gigabytes. It was quickly realized that to roll out a deliverable in an efficient timeframe, custom built computers would be required to process all the data. In addition to the computer processing being an issue, transfer of files over the internet or FTP sites became very tedious due to size of files and folder. Cloud-based technology and processing was implemented in Phases II through IV and is still a very efficient method of processing; however, from a security, ownership, and user input standpoint, it is still not capable of handling all jobs.



Figure 2-16 Custom Built Post Processing

#### 2.5.1.2 Pix4D Software

Our team used Pix4D for many of our bridge inspections and there are many benefits to this software. It is relatively easy to learn and use. The cloud sharing platform is very good and provides many tools useful for bridge inspectors. Pix4D stores and displays all original high-resolution images and ties them to each pixel created as a result of that image. This can be thought of as a 3D photo log where a user can view an entire bridge model and select in space where they would like a close-up photo view of. There is also a virtual inspection tool on the cloud platform which allows a

user to click on the model and view images of the area they clicked. This feature is very useful to a bridge inspector to be able to find and compare deficiencies over time.

Pix4D software processes the images taken in the field from a handheld camera or UAS in a .jpg or .tif file format and triangulates the photos to form a 3D interactive model. The model can be used to accurately measure distances, areas, and volumes with GPS located images or GCPs. When completed the model can then be uploaded to the cloud, annotated, and sent as a deliverable. The deliverable can be in either 2D or 3D. Pix4D can produce Orthomosaics from a user-defined orthoplane and index maps (Thermal, etc.) in 2D. For 3D output results, it produces 3D PDF, 3D texture mesh, point cloud, and contour lines. Pix4D can struggle with complex 3D models and can create noise in the models especially at bridge railings. This noise can be removed but this is a manual process which add time to the workflow.

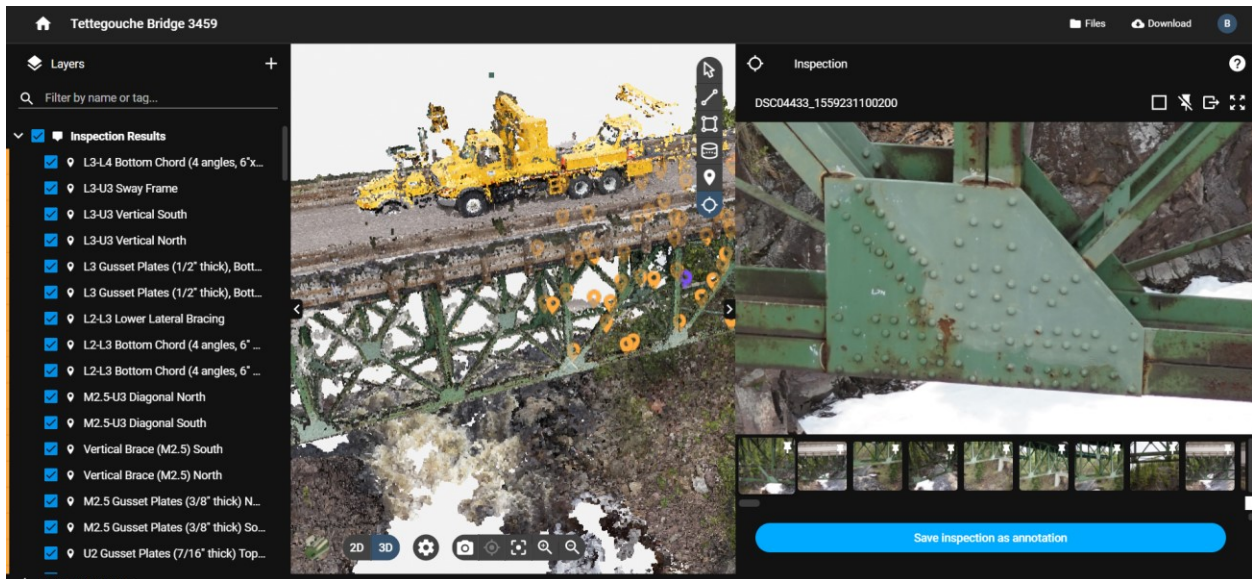


Figure 2-17 Pix4D Cloud Virtual Inspection Tool

### 2.5.1.3 ContextCapture

ContextCapture from Bentley was the other software package we used in our study. ContextCapture can create many deliverables similar to Pix4D and has similar tools. ContextCapture has a slightly longer learning curve but excels at creating very clean reality models of complex structures. Another advantage is that multiple computers can be used to process one project which can speed up the processing significantly. ContextCapture also seamlessly integrates with Microstation CAD software which many DOT's use as their preferred CAD platform. Models can also be shared via the Projectwise software but requires users to register with Bentley to view the models.

Collins Engineers and MnDOT have been working with Bentley to use the ContextCapture models in the Microsoft Hololens 2 which is a significant leap forward and presents a unique and effective way to perform virtual inspections. More information can be found in section 2.5.2.1.

#### 2.5.1.4 FujiFilm Infrastructure Photo Analysis

Collins Engineers and MnDOT have been working with FujiFilm Japan on their upcoming Infrastructure Analysis Software. This artificial intelligence software tool automatically detects and classifies cracks on concrete structures. While the tool can find cracks on any concrete bridge element, it is especially effective and useful for mapping deck cracks as the process has been particularly difficult to do in the field because it is time consuming, inaccurate and has safety concerns since lane closures are often necessary. The software classifies cracks by width and the data can be downloaded in a spreadsheet to extract quantities for the bridge inspection database.

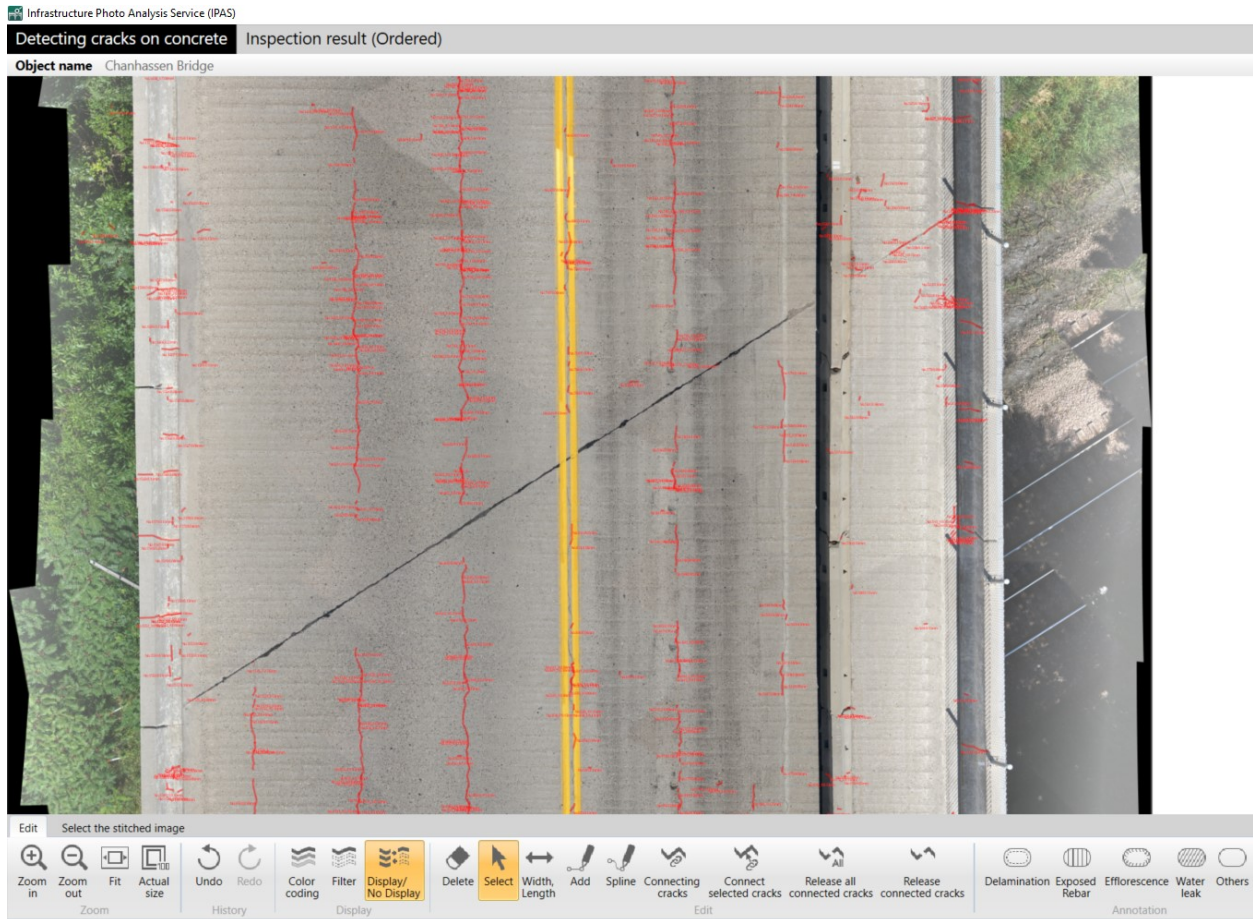
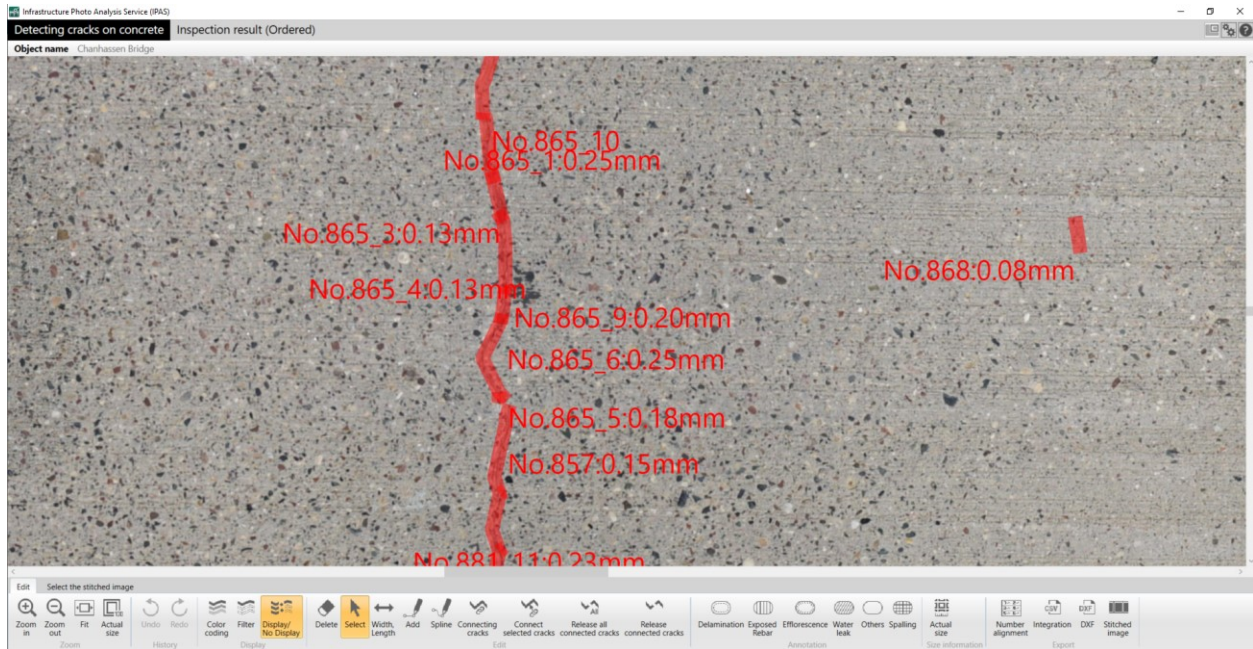


Figure 2-18 Fuji Film AI Crack Detection on Bridge



**Figure 2-19 Fuji Film AI Crack Detection on Bridge Deck Close Up**

## 2.5.2 Processing and Modeling Accessory Hardware

These types of aids consist of products, targets, or other visual aids that are placed in the field prior to UAS flights and are captured in the images to aid in processing. They can be as simple as paint or chalk markings or unique stickers placed on critical faces. These are easy to apply in the field, but require an understanding of the post processing technology to find the best place to create or install marks. Smart technologies such as survey-grade GPS targets or QR codes can be used by inspectors to achieve higher quality results with less user input in post processing. These smart technologies also must be implemented with an understanding of the post processing software to insure they are compatible and efficient.

### 2.5.2.1 Propeller AeroPoints

AeroPoints are drone specific aerial targets that record their own position using GPS, which is processed and corrected to ensure survey grade accuracy. Benefits of this technology is that inspectors do not need traditional base and rover surveyors to mark benchmarks prior to flying and these serve as both the GPS collector and the image target in one package. In Phase IV, our team used the Propeller AeroPoints with great success and reliability on inspections as large as 9-mile-long corridor surveys to one-mile long bridge inspection models. The use of these smart targets greatly increases the accuracy of your survey or model from +/- 3-foot accuracy to sub-inch accuracy.



Figure 2-20 Propeller AeroPoint UAS Survey Target

### 2.5.2.2 QR Codes

QR Codes (Quick Response Codes) is an adaptation of traditional 1-dimensional bar codes. QR codes contain a 2D matrix of pixels which are unique and can contain location or identification information. Placing QR codes on targets in various places during a bridge inspection site prior to imaging can greatly increase the efficiency of processing and quality of the final product. QR code images are publicly available and were implemented during Phase IV. The size of the QR code was examined to determine the optimum target for a camera resolution at a given distance. Application of QR codes to different mediums were explored to test the durability, reflectivity, ease of application, and longevity of each installation. The type of target and method of adherence was proven to be independent of the specific bridge material and environment; however, the application of QR codes was proven to work in nearly any bridge location or condition.

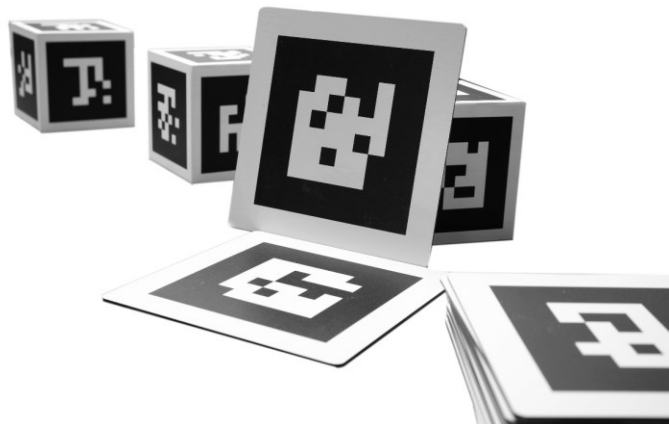


Figure 2-21 QR Code Targets Created and Used in Phase IV

A bridge design maximizes redundancy not only with loads, they also have repetitive components, geometry, and designs. These repetitive structures bring challenges in post-processing software and

often requires a lot of manual input by creating manual tie points in the 3D models. This difficulty only increases with more complex structures like trusses.

Pix4D software, which was the primary post processing software used in Phase IV, stitches images together by producing a point cloud through computer generated key points called Automatic Tie Points (ATPs). This action is done through Pix4D's initial processing step and any intermediate steps of reoptimizing or rematching run by the software.

If the software is uncertain about surfaces or areas, it may produce a model that is incorrect and requires an engineer to enter Manual Tie Points (MTPs) in the model to improve model accuracy and clarity. This method is generally used after the first step is processed where the user can focus on areas of concern. Once an area is identified to improve, the user cycles through the images calibrated within the chosen area to manually mark or adjust key points. This process can be very tedious and time consuming on large structures.

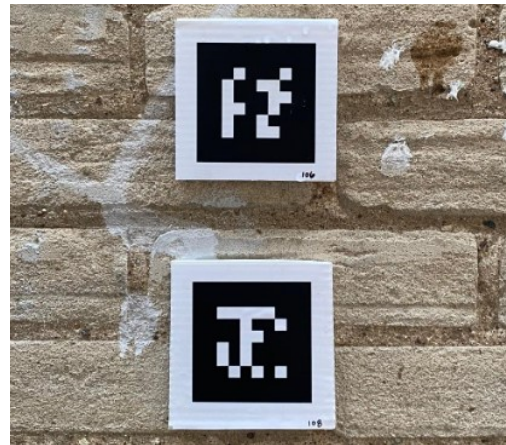


Figure 2-22 QR Code Targets Adhered to a Stone Face

To alleviate some of this engineer manual processing, Pix4D developed a system called Pix4D Tagger, a tool to implement during the acquisition of the data. This is done by placing custom QR codes recognized by the software, scattered around the subject to be imaged for a 3D model. This frontend step generates artificial points to run in the modeling process as MTPs. These points imported into Pix4D provide extra data for the computer to process allowing more accurate initial model and decreasing manual input required to increase accuracy of a model.

Pix4D Tagger utilizes custom QR codes, each with a unique ID from one another. Each tag creates four individual key points on the corners of the square code identified by the software. These tags can be oriented in anyway throughout the area being imaged as the software can identify the unique code and compute the numbering convention. The image shows an example of a Pix4D QR code and its numbering convention of the artificial key points.

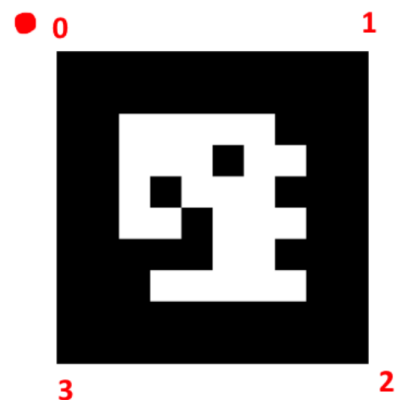
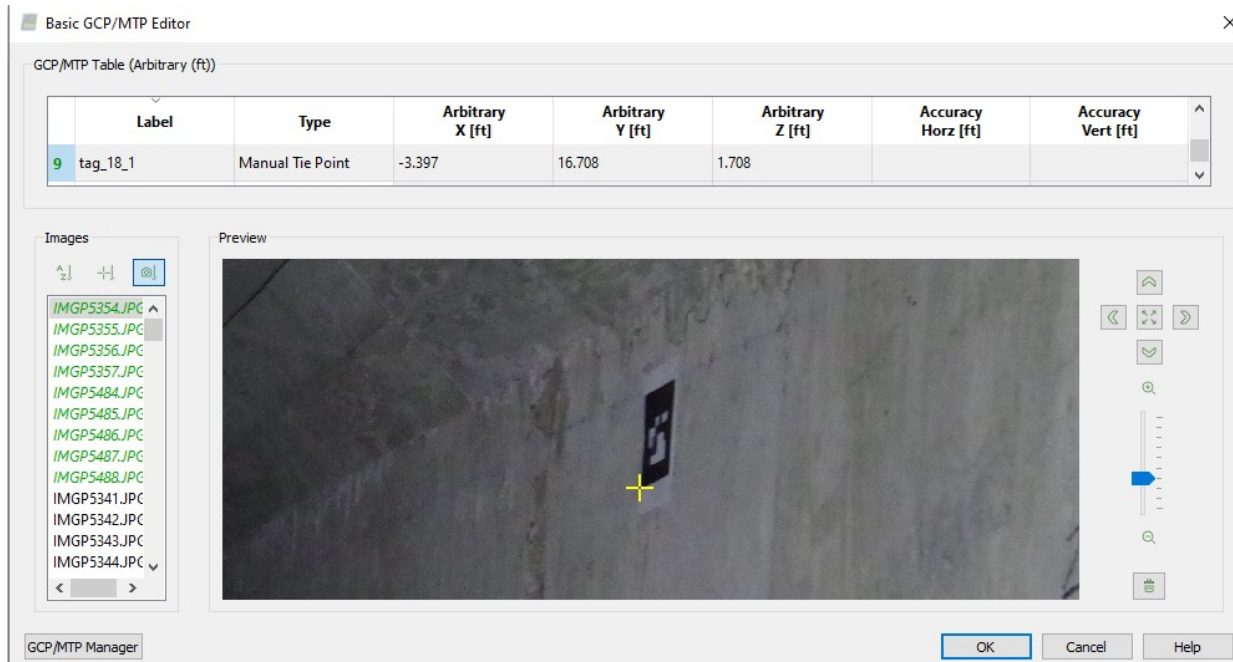


Figure 2-23 Pix4D Tagger QR Code Example with Numbering Convention of the Key Points

After placing the QR codes throughout the area to be imaged in conspicuous locations, the user can proceed as normal with acquiring UAS or Terrestrial Imagery for a model. The user will then import information from these QR codes into Pix4D as MTPs to aid in processing.



**Figure 2-24 Pix4D GCP/MTP Editor Screenshot Showing Points Automatically Inserted on a QR Code.**

Pix4D tagger is a huge benefit for projects which are complex requiring lots of manual input from the user. It provides extra data for the software to compute and ultimately increases its accuracy and decreases the amount of an engineer's time and effort.

### 2.5.2.1 Microsoft Hololens 2

Collins Engineers has been working with Bentley in developing software for the Microsoft HoloLens 2 to be able to perform virtual inspections using mixed reality. A 3D reality model can be loaded on a HoloLens headset and the user can not only view the virtual bridge in a hologram but, can also make measurements, add notes, add and classify defects and denote elements. This data can then be integrated into the bridge inspection report database as condition codes and quantities. Utilizing mixed reality can allow users to populate or review inspection data from their office and allows for the sharing of bridge inspection data with experts in other locations.



**Figure 2-25 Microsoft Hololens 2**



Figure 2-26 Holographic Overall Bridge View from HoloLens 2



Figure 2-27 Holographic Close Up View of Bridge from HoloLens 2



## 2.6 FIELD TESTING AND CALIBRATION

### 2.6.1 Carver County Survey and AeroPoint Accuracy Report

The use of AeroPoints, in conjunction with the Intel Falcon8+ UAS platform, was implemented as part of a 9-mile corridor survey for Carver County in Minnesota to obtain a terrain model. This survey technique proved to have many benefits. It provided additional data than a traditional ‘base and rover’ type survey and was much more efficient and economic than an LiDAR survey. To ensure accuracy and to prove proper calibration of the UAS system using AeroPoints, Carver County surveyors set out 21 Check Points surveyed with traditional base and rover methods. These points were then compared to the final model which utilized 234 individually placed AeroPoints as Ground Control Points. A report was generated which compared the model points to the check point locations and calculated error. These results concluded that with the use of AeroPoints, a UAS survey using a commercial UAS platform can achieve sub-centimeter accuracy of an entire terrain area.

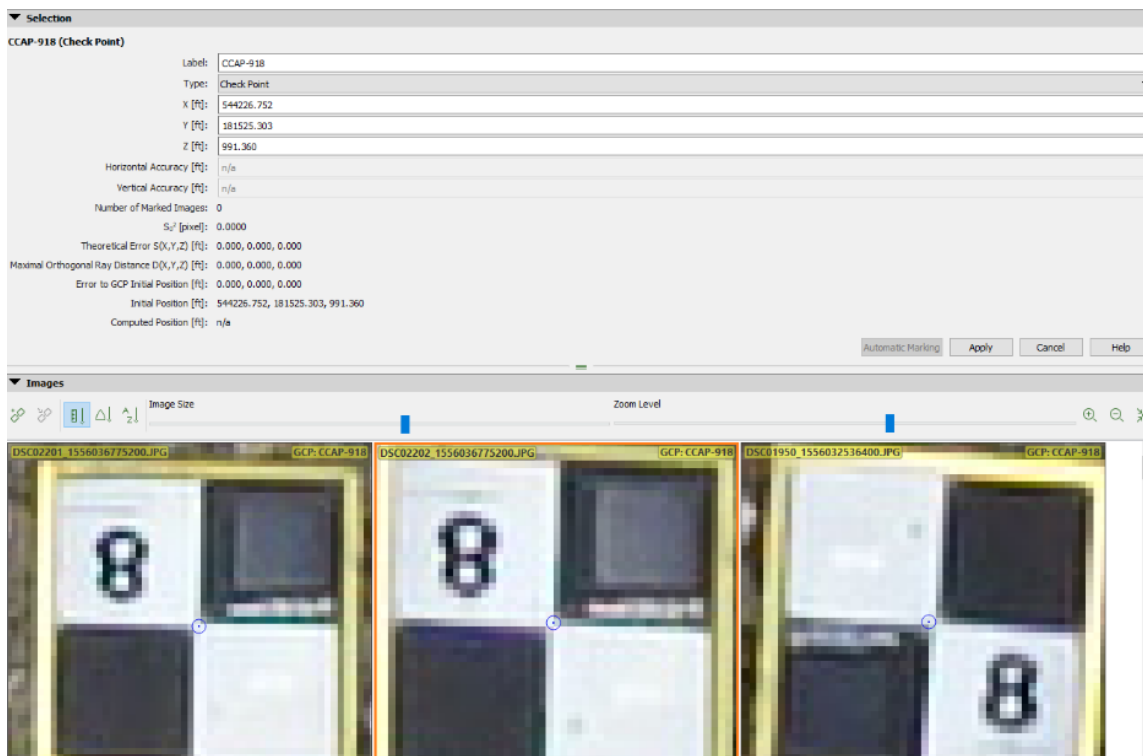


Figure 2-28 Pix4D Ground Control Point Editor Showing a Propeller AeroPoint

## 2.6.2 UAS Imaging Payload Test

UAS systems are the vehicle for data acquisition. For this research, the majority of data acquired was in the format of digital photographs. The quality of each camera payload and resulting photographs were studied as part of this phase and are summarized below. Some considerations include resolution, clarity, contrast, and exposure. Camera attributes were tested and compared on three UAS cameras that were part of the research project. UAS's were the Anafi Parrot, Intel Falcon 8+, and the Skydio 2.

Camera megapixels (MP) is a count of pixels, in millions, the camera can put into a square inch. The greater the data put into an image allows a digital zoom to be clearer and more defined. The importance of this attribute in UAS cameras for inspection directly affects several aspects for producing quality results including, but not limited to, the distance from the structure necessary to take photos, number of photos, and clarity and detection of defects. The three UAS platforms chosen all had different MP counts and sensor size:

- Skydio 2, 12 MP 1/2.3" CMOS
- Anafi Parrot, 21 MP 1/2.4" CMOS
- Intel Falcon 8+, 36 MP Full Frame (35mm) CMOS

### 2.6.2.1 Testing Procedure

Several tests were conducted in a field lighting environment. The tests included an optical test, pixel and resolution test with a field of view comparison. The tests were conducted by imaging the same location at predetermined intervals of 10 feet, 20 feet, 50 feet, 75 feet, and 100 feet. The location imaged included a calibrated visual aid situated 20 feet off the ground. Attached on the visual aid was an optical vision test, crack gauges, ruler tape, and resolution test targets.

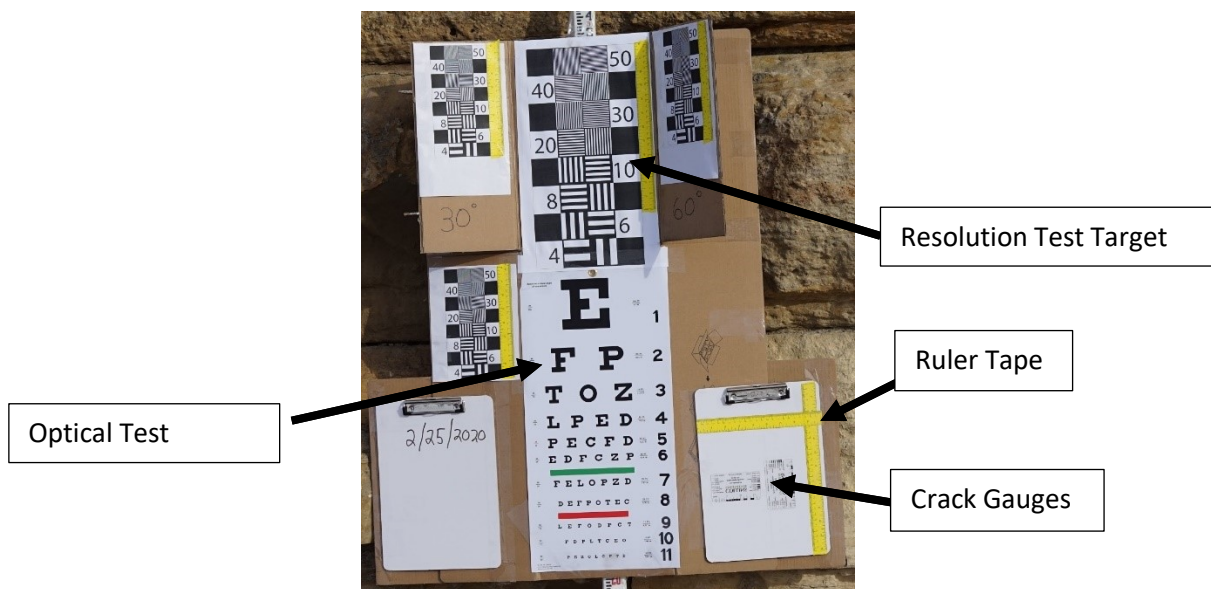
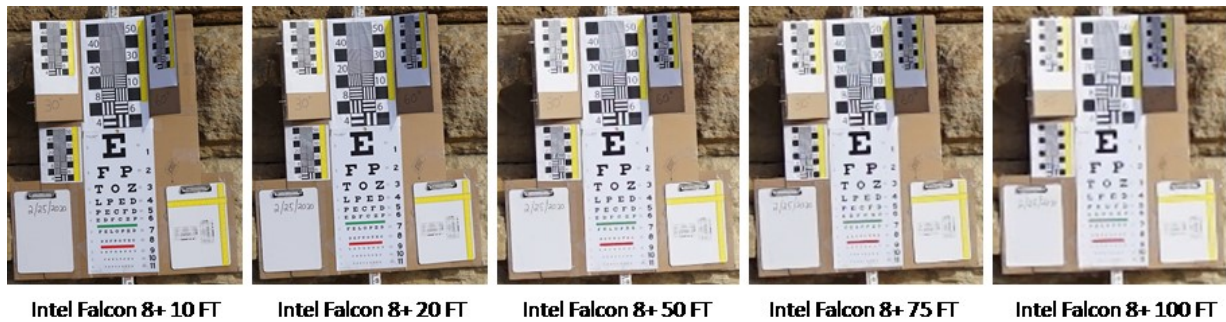


Figure 2-29 Layout of the Visual Aid Used for Testing.

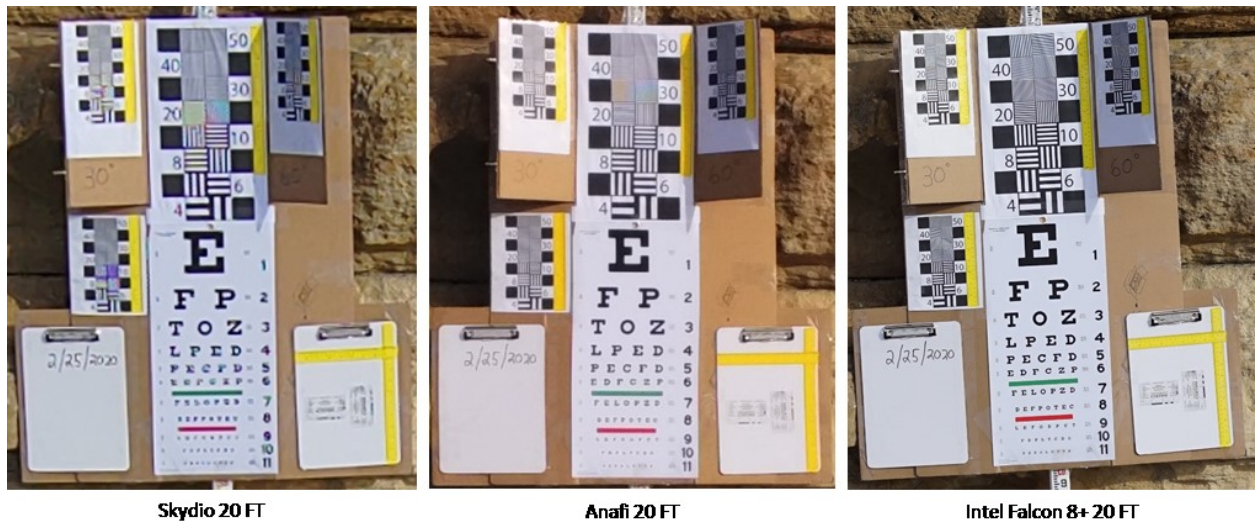


the Skydio under 50 feet. This camera produced images that allowed zooming in digitally from a large distance away and maintained respectable clarity.



**Figure 2-32 Intel Falcon 8+ Image Comparisons at Determined Distances**

The next comparison was between each drone using the parameters modeled after the visual acuity eye exams performed by optometrists. The test compares the actual subject’s vision at 20 feet compared to the average human’s normal vision. A direct comparison was conducted on the photos at the 20-foot distance, shown in the picture below.



**Figure 2-33 Comparison of the Three Cameras at 20 Feet Zoomed In**

When the images were compared, there was a clarity difference in all the camera types. Each camera had a line where the legibility in the optical visual test was no longer clear. The list below summarized the last line that was clearly legible using digital zoom.

- Skydio, line 5 or an eyesight of 20/40
- Anafi, line 7 or an eyesight of 20/25
- Intel, line 9 or an eyesight of 20/18

The fraction designated above represents what the subject would see as legible at 20 feet compared to the standard eyesight. To put the UAS’s imaging payload in perspective the standard human eyesight is

20/20, or the last legible line at 20 feet is line 8. The Intel Falcon 8+ had a relative sharper vision at twenty feet compared to the standard eyesight which would read line 9 at 18 feet.

Lastly, there was a distinct clarity difference related to the different camera payloads. The below image was a direct comparison of line 7 between all three drones highlighting the clarity differences.



Figure 2-34 Comparison of the Three Cameras at 20 Feet for Line 7 of the Optical Vision Test

### 2.6.2.3 Resolution Test Results

The focus of the resolution test was to determine the limitations of each camera in visualizing cracks with several variables. The variables included the distance of the camera compared to the subject, the camera, and angle of subject relative to the imaging payload. This test used the same images captured during the visual test with a focus on the resolution test targets.

There were four resolution test targets on the visual aid with two different size targets. One size target was the larger boxes which corresponded with a two-inch square filled with several lines. The smaller boxes corresponded with a one-inch square filled with several lines. The number of lines in each standardized square was represented by the large typed number to the side. For example, the closeup below shows the number '30' with a smaller box measuring 1"x1"; meaning there were 30 lines in the 1"x1" square or each line being 1/30-inch wide.

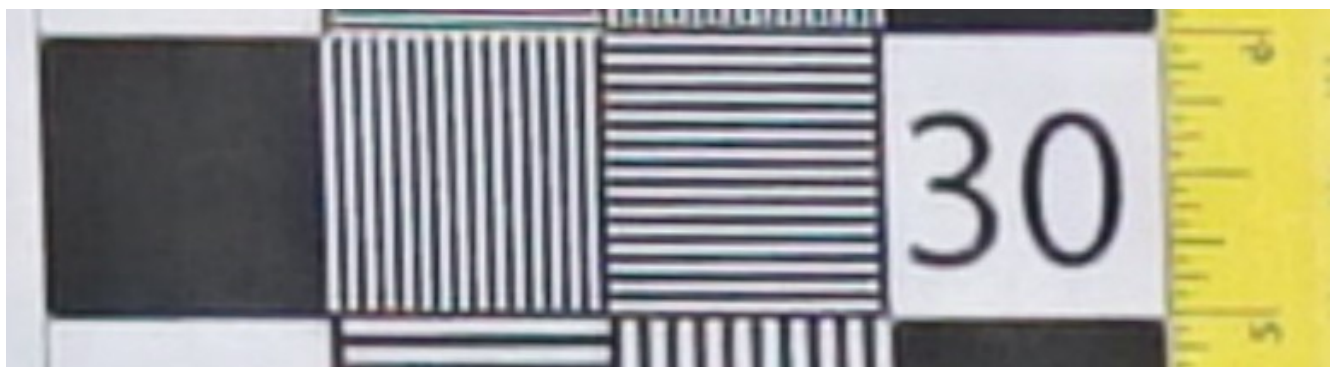
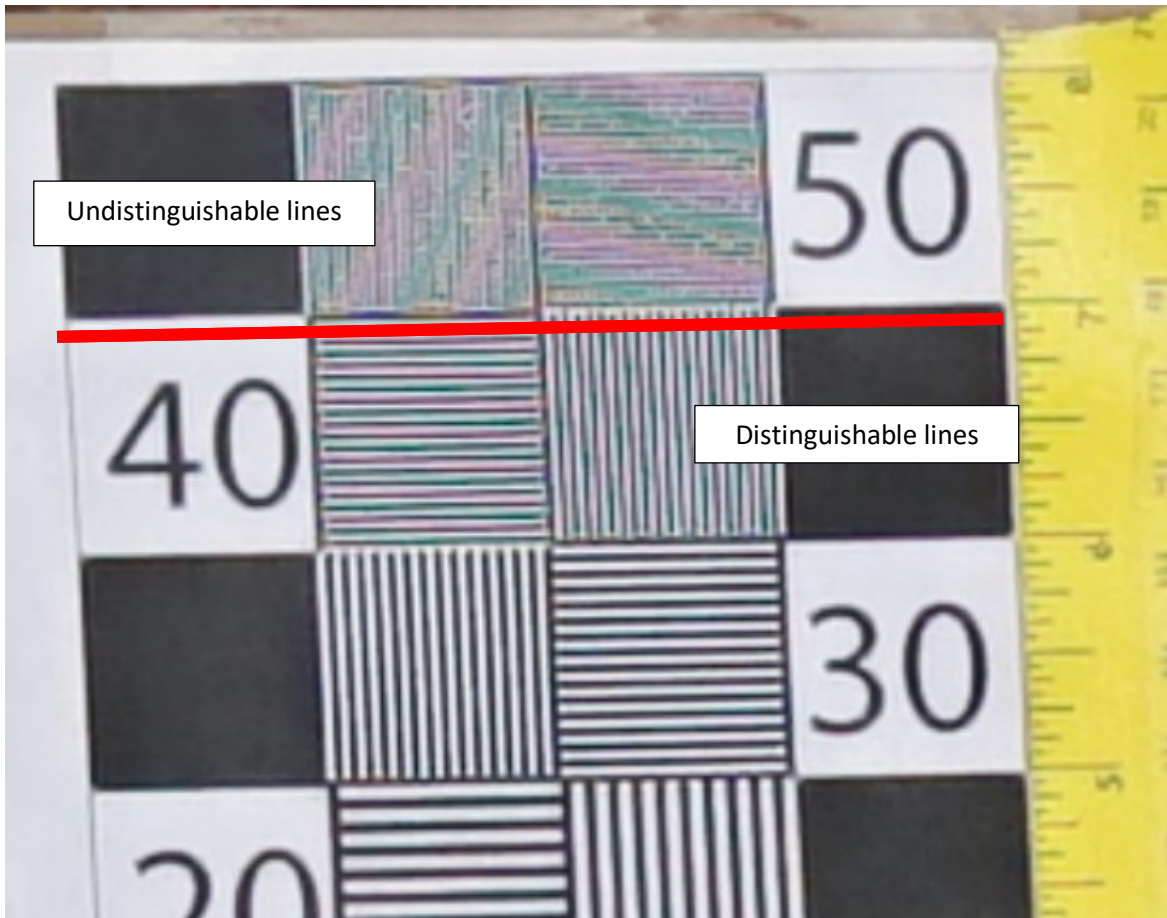


Figure 2-35 Closeup of the Resolution Test Target Which Signified 30 Lines in a 1"x1" Square

The significance of this test provided a quantitative way to discover how large a defect, such as a crack, would theoretically need to be to be discovered by means of a UAS imaging. This was done by finding the largest amount of lines in a square while still being distinguished or legible. The image below is an example between clear lines versus blurred ones.



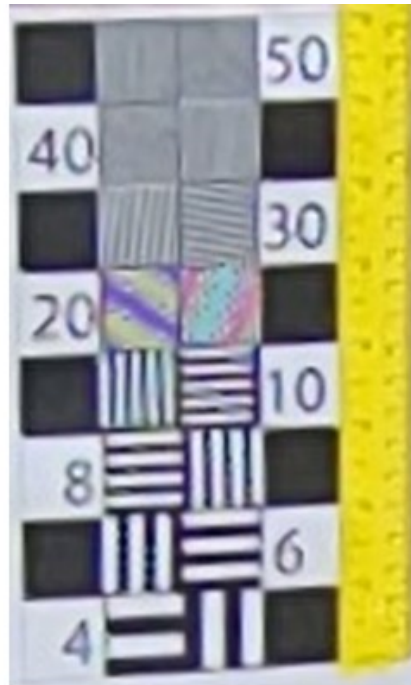
**Figure 2-36 Example Showing the Difference Between Distinguished Lines and Undistinguished Lines (Separated by the Red Line)**

There were three small target grids presented on the visual aid. Each target was oriented to specific angles relative to the direction normal to the camera lens. This simulated conditions where a drone could not provide a normal line of sight to the structure and resulted in a skewed photo angle. The angles chosen were 0°, 30°, & 60°.

The first comparison at 10 feet resulted with the Skydio is shown below. At all skew angles, 20 lines per square were blurry and could not be counted. The largest amount of lines still able to be legible was 10 lines per one inch. This correlated to being able to detect a crack width of 0.1" at 10 feet away at any angle up to 60 degrees.



**Skydio 10 FT at 0 degrees**



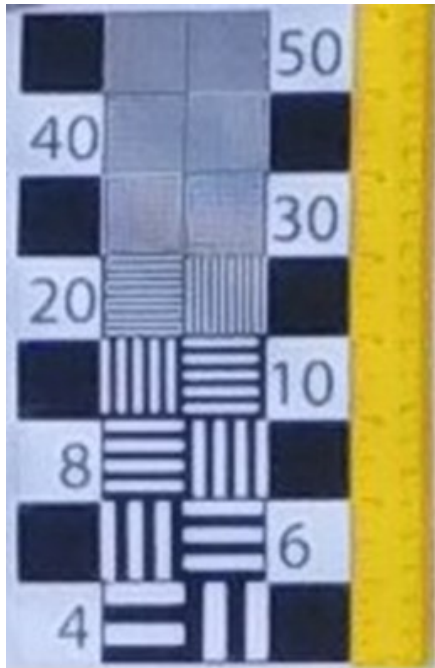
**Skydio 10 FT at 30 degrees**



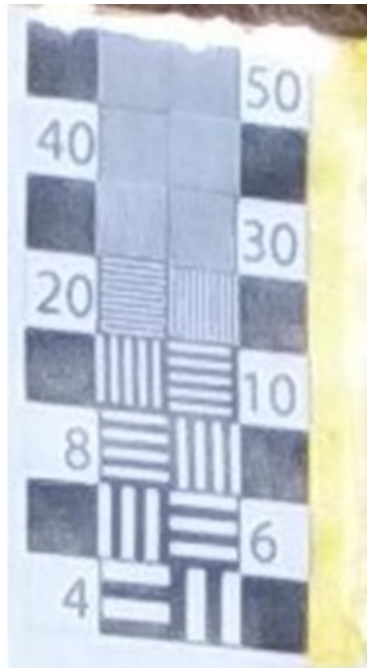
**Skydio 10 FT at 60 degrees**

**Figure 2-30 Skydio Resolution Test at Several Angles From 10'**

Another comparison at 10 feet was performed with the Anafi and is shown below. At all skew angles, 30 lines per square were blurry and could not be counted. The largest amount of lines still able to be legible was 20 lines per one inch. This correlated to being able to detect most crack widths of 0.05" or larger at 10 feet away at any angle up to 30 degrees. At 60 degrees, horizontal lines were still clear at 20 lines per inch with the vertical lines blurred until 10 lines per on inch. Therefore, at a 60-degree skew it would be understood that a horizontal crack of 0.05" or larger would be noticed although, a vertical crack would need to be 0.1" or larger to be noticed.



**Anafi 10 FT at 0 degrees**



**Anafi 10 FT at 30 degrees**



**Anafi 10 FT at 60 degrees**

**Figure 2-31 Anafi Resolution Test at Several Angles from 10'**

The last comparison at 10 feet was performed with the Intel Falcon 8+ which is shown below. At all skew angles, 50 lines per square were blurry and could not be counted. The largest amount of lines still able to be legible was 40 lines per one inch. This correlated to being able to detect most crack widths of 0.025" or larger at 10 feet away at any angle up to 30 degrees. At 60 degrees, horizontal lines were clear up to the 40 lines per inch with the vertical lines blurred above 40 lines per inch while vertical lines only started to be visible at 20 lines per inch. At a 60-degree skew, it would be understood that a horizontal crack of 0.025" or larger would be noticed while vertical cracks would need to be 0.05" or larger to be noticed.



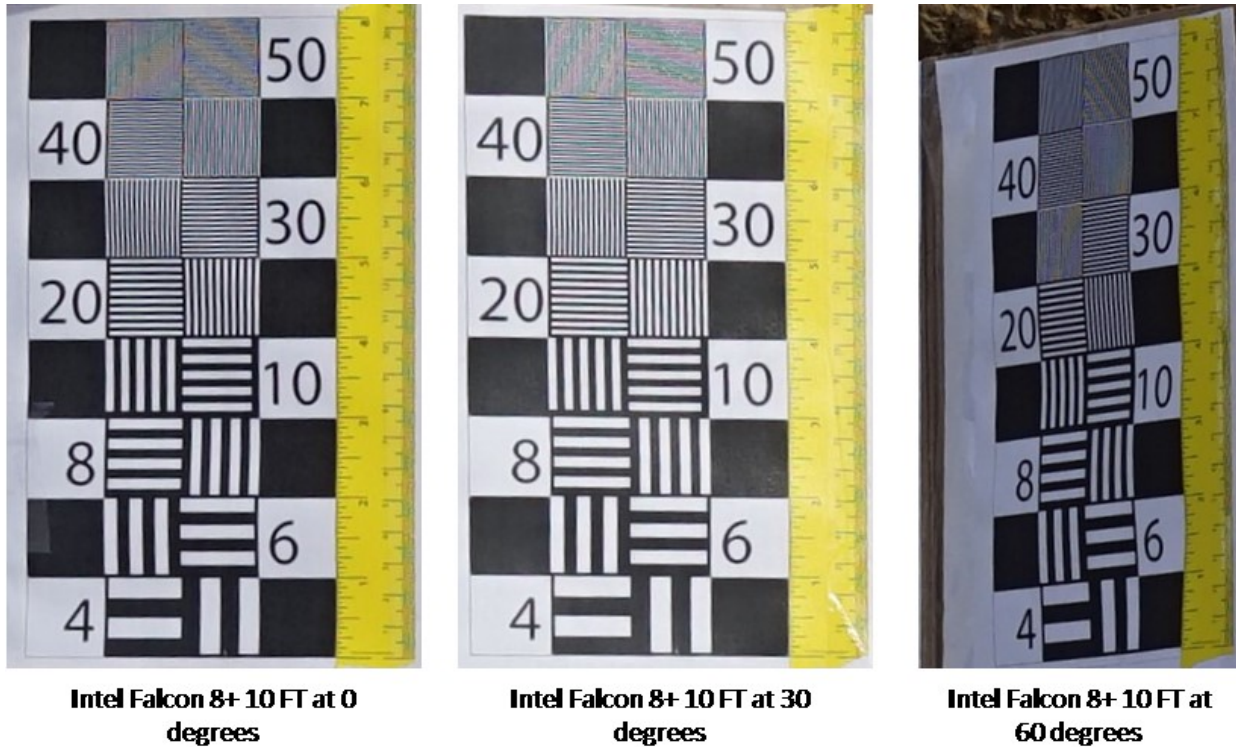


Figure 2-32 Intel Falcon 8+ Resolution Test at Several Angles From 10'

#### 2.6.2.4 Field of View Test Results

The last comparison of the three cameras was the field of view (FOV) from the images. Images were compared at the 100-foot distance of the photos taken. The field of view is primarily a factor of the camera lens. The Skydio and Anafi both have fisheye lenses that create a much larger FOV with a convex lens. When using a fisheye camera, the larger FOV collects on the same sensor, thus the resolution is decreased due to the condensed light waves. A large FOV can be very beneficial when figuring out where an image is in respect to the bridge or other members, but typically produces imagery that is not as clear when reviewing under digital zoom.

The Skydio image shown below has a relatively darker image when compared to the other two cameras. Of all three drones this had the widest shot which included more background and more of the adjacent piers to the target.



**Figure 2-33 Skydio's Full Image at 100 Feet From the Visual Test Board**

The Anafi Parrot also had a wider shot compared to the Intel and is shown below. Colors were the brightest on this camera. Detail was lost within the shadows when compared to the other images by the Intel and Skydio.



Figure 2-34 Anafi's Full Image at 100 Feet From the Visual Test Board

The Intel's camera is shown below, which had a noticeable narrower field of view. Also, colors observed in this were paler in contrast to the other camera images.



**Figure 2-35 Intel Falcon 8+'s Full Image at 100 feet From the Visual Test Board**

### **2.6.3 QR Code Distance Test with Pix4D Tagger**

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While Pix4D Tagger is an invaluable tool in processing, it has limitations. Like most instruments, if it is not used correctly, it may limit its effectiveness. The importance of having this knowledge could help minimize issues in processes by addressing concerns before starting the project. The first concern to be addressed was at what distance from the camera to a tag would render it ineffective. Understanding this dilemma would provide necessary information for the user to maximize their effectiveness out in the field and in the office when processing.

#### **2.6.3.1 Procedure**

A simple test was conducted to evaluate the effectiveness of the QR codes corresponding to its distance away from the camera. The test was performed under conditions at a site that would be considered typical in the field. The Intel Falcon 8+ was chosen to take the photos as it is predominately used on large and complex jobs like the Stone Arch Bridge located in Minneapolis, Minnesota.

The flight plan of the Intel Falcon 8+ was to take a photo starting at an altitude of 20 feet directly over the QR codes and capturing a photo every additional 20 feet up to the altitude of 140 feet. The screenshot below showcases the flight. Each green point represents the location where a photo was taken.

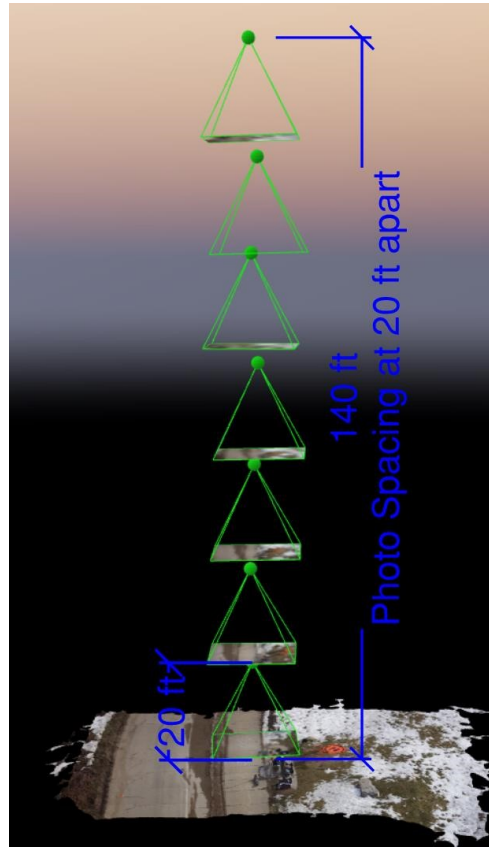


Figure 2-36 Pix4D Screenshot with the Locations Of Each Shot Taken During the Test

After the photos were taken in the field, the images were then modeled using Pix4D with the aid of Pix4D Tagger. After the first and second steps were completed, a visual analysis was done to compare percentage of QR codes lost due to distance and lack of resolution.

### 2.6.3.2 Results

Each tag was examined through the Basic GCP/MTP Editor. Through this editor, images were automatically color-coded notifying if the Pix4D Tagger recognized the QR code for the individual photo. A recognized QR code created four key points corresponding to the four corners of the unique code as described above. The figure below is a screenshot of how it was visually analyzed.

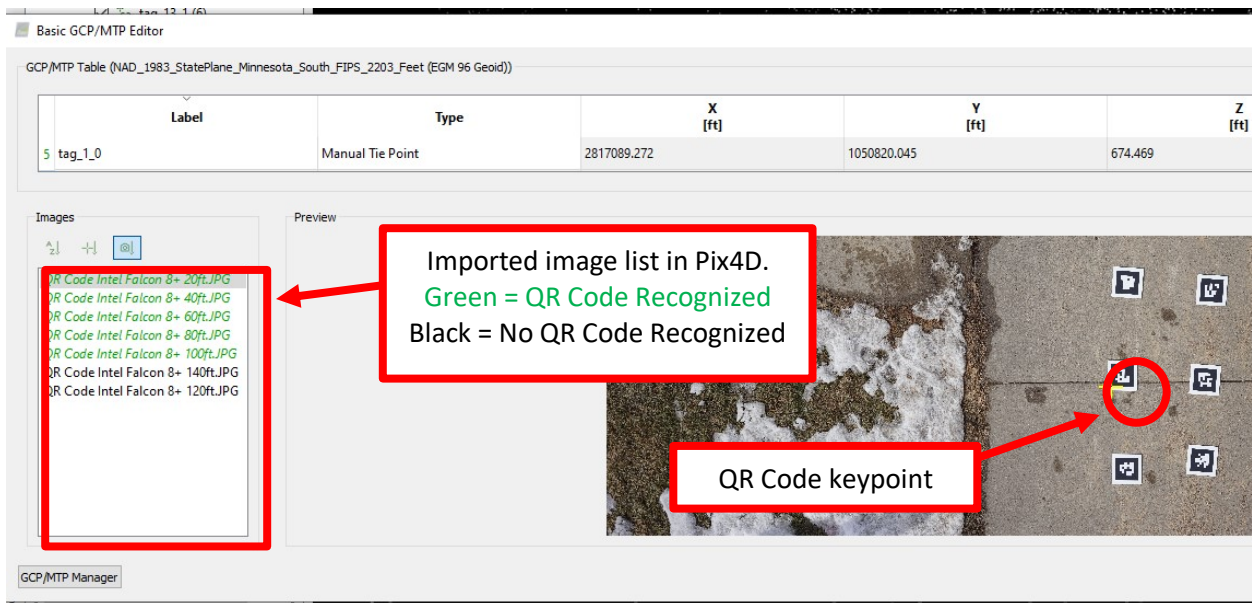


Figure 2-37 Pix4D Screenshot in Basic GCP/MTP Editor With Notes

Each tag was examined and results of each was noted. A summary of the results is shown below. If a QR code tag was recognized, a '1' was used, a '0' represented Pix4D not recognizing the code in the image.

Pix4D Tagger QR Code Recognition Summary							
QR code ID	20 ft	40 ft	60 ft	80 ft	100 ft	120 ft	140 ft
1	1	1	1	1	1	0	0
6	1	1	1	1	0	1	0
8	1	1	1	1	1	0	0
13	1	1	1	1	1	1	0
14	1	1	1	1	1	1	0
24	1	1	1	1	1	0	0
31	1	1	1	1	1	0	0
<b>Percentage of matches per distance</b>	100%	100%	100%	100%	86%	43%	0%

Figure 2-38 Pix4D Tagger Test Results

### 2.6.3.3 Conclusion

In summary, all QR codes were fully effective up until a distance above 80 feet. After 80 feet from the camera to the target, accuracy of recognition sharply dissipated. At 100 feet, the success rate of dropped to 86% while 120 feet resulted in 43% of targets recognized. Any QR code 140 feet away was ineffective and it would be reasonable to assume anything beyond this limit would also be ineffective. The chart below visualizes the results of the matches compared to the distance.

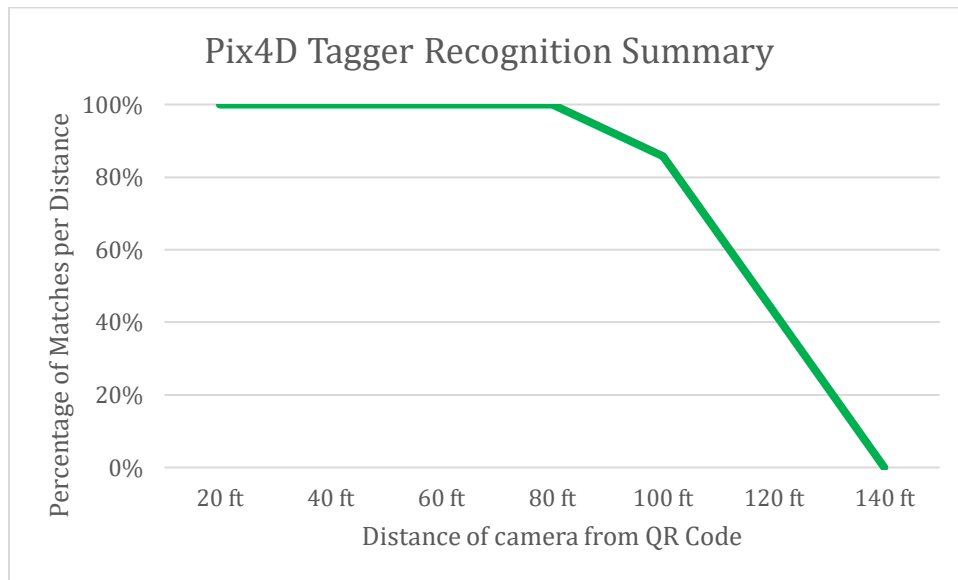


Figure 2-39 Chart Showing the Match Percentage Dropping After 80 Feet

### 2.6.3.4 Limitations

This test was helpful in providing a general understanding of the effectiveness for QR code with Pix4D Tagger relative to the distance of the camera to the QR code. While the test provided valuable insights there are some caveats to consider. These caveats include a variety in imaging devices, a comparison in QR Code sizing and material, angle of the photo relative to the tag, and more photos to compare. Further testing analysis could provide more awareness on how these variables may affect Pix4D Tagger with the focus of creating a superior product by efficient means.

### 2.6.4 Computer Processing Comparison


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Phase IV tested the effectiveness of five different processors at running a model that was previously completed. With a higher photo count, the process will take longer to complete a model, so this test was to show the time required to complete the model using varying levels of processor. This test was completed using the software Pix4D on each computer. The five computer processors compared are as follows in highest to lowest processing power:


- Computer 1: Intel® Core™ i9-9980XE CPU @ 3.00GHz (36 CPUs) 3.00GHz
- Computer 2: Intel® Core™ i9-9900K CPU @ 3.6 GHz (16 CPUs) 3.6GHz

- Computer 3: Intel® Core™ i7-8850H CPU @ 2.60 GHz (12 CPUs) 2.6GHz
- Computer 4: Intel® Core™ i7-6820HK CPU @ 2.7GHz (8 CPUs) 2.7GHz
- Computer 5: Intel® Core™ i7-8650U CPU @ 1.90GHz (8 CPUs) 2.11 GHz

Each processor ran a model in Pix4d of a retaining wall that had 875 images to stitch together. Pix4D has two steps in creating a model. Step 1: Initial Processing and Step 2: Point Cloud Densification. To make each model run as smoothly and efficiently as possible, the model and the photos were saved onto the internal hard drive. This also made sure no failed internet connection interfered with the processing time. The Pix4D software produces a file called “Quality Report” which gives a detailed summary of what occurred to produce the model. The comparisons came from the details of this report. Below are images taken from the Pix4D Quality Report for each processor.

**Summary** 

Project	MhDOT Retaining Wall R-156-003-010 (C)
Processed	2020-03-17 08:43:14
Camera Model Name(s)	ILCE-7R_FE35mmF2.8ZA_35.0_7360x4912 (RGB)
Average Ground Sampling Distance (GSD)	0.23 cm / 0.09 in
Time for Initial Processing (without report)	55m:54s

**Quality Check** 











 <b>Images</b>	median of 92459 keypoints per image	
 <b>Dataset</b>	866 out of 875 images calibrated (98%), all images enabled	
 <b>Camera Optimization</b>	1.02% relative difference between initial and optimized internal camera parameters	
 <b>Matching</b>	median of 14580.3 matches per calibrated image	
 <b>Georeferencing</b>	yes, 10 GCPs (10 3D), mean RMS error = 0.008 ft	

Figure 2-40 Computer 1 Quality Report Step 1


**Processing Options** 

Image Scale	multiscale, 1/2 (Half image size, Default)
Point Density	Optimal
Minimum Number of Matches	3
3D Textured Mesh Generation	yes
3D Textured Mesh Settings:	Resolution: Medium Resolution (default) Color Balancing: no
LOD	Generated: no
Advanced: 3D Textured Mesh Settings	Sample Density Divider: 1
Advanced: Image Groups	group1
Advanced: Use Processing Area	yes
Advanced: Use Annotations	yes
Time for Point Cloud Densification	01h:43m:05s
Time for Point Cloud Classification	NA
Time for 3D Textured Mesh Generation	26m:08s

Figure 2-41 Computer 1 Quality Report Step 2



## Summary



Project	MhDOT Retaining Wall R-156-003-010 (C)
Processed	2020-03-18 16:55:58
Camera Model Name(s)	ILCE-7R_FE35mmF2.8ZA_35.0_7360x4912 (RGB)
Average Ground Sampling Distance (GSD)	0.23 cm / 0.09 in
Time for Initial Processing (without report)	43m:01s

## Quality Check



Images	median of 92360 keypoints per image	
Dataset	863 out of 875 images calibrated (98%), all images enabled	
Camera Optimization	1.03% relative difference between initial and optimized internal camera parameters	
Matching	median of 14648.9 matches per calibrated image	
Georeferencing	yes, 10 GCPs (10 3D), mean RMS error = 0.007 ft	

Figure 2-42 Computer 2 Quality Report Step 1

## Processing Options



Image Scale	multiscale, 1/2 (Half image size, Default)
Point Density	Optimal
Minimum Number of Matches	3
3D Textured Mesh Generation	yes
3D Textured Mesh Settings:	Resolution: Medium Resolution (default) Color Balancing: no
LOD	Generated: no
Advanced: 3D Textured Mesh Settings	Sample Density Divider: 1
Advanced: Image Groups	group1
Advanced: Use Processing Area	yes
Advanced: Use Annotations	yes
Time for Point Cloud Densification	02h:19m:58s
Time for Point Cloud Classification	NA
Time for 3D Textured Mesh Generation	20m:42s

Figure 2-43 Computer 2 Quality Report Step 2

## Summary



Project	MhDOT Retaining Wall R-156-003-010
Processed	2020-03-27 01:04:01
Camera Model Name(s)	ILCE-7R_FE35mmF2.8ZA_35.0_7360x4912 (RGB)
Average Ground Sampling Distance (GSD)	0.23 cm / 0.09 in
Time for Initial Processing (without report)	03h:09m:38s

## Quality Check



<b>Images</b>	median of 92459 keypoints per image	
<b>Dataset</b>	868 out of 875 images calibrated (99%), all images enabled	
<b>Camera Optimization</b>	1.02% relative difference between initial and optimized internal camera parameters	
<b>Matching</b>	median of 14613.6 matches per calibrated image	
<b>Georeferencing</b>	yes, 10 GCPs (10 3D), mean RMS error = 0.007 ft	

Figure 2-44 Computer 3 Quality Report Step 1

## Processing Options



Image Scale	1/2 (Half image size, Default)
Point Density	Optimal
Minimum Number of Matches	3
3D Textured Mesh Generation	yes
3D Textured Mesh Settings:	Resolution: Medium Resolution (default) Color Balancing: no
LOD	Generated: no
Advanced: 3D Textured Mesh Settings	Sample Density Divider: 1
Advanced: Image Groups	group1
Advanced: Use Processing Area	yes
Advanced: Use Annotations	yes
Time for Point Cloud Densification	04h:55m:25s
Time for Point Cloud Classification	NA
Time for 3D Textured Mesh Generation	25m:56s

Figure 2-45 Computer 3 Quality Report Step 2

### Summary



Project	MnDOT Retaining Wall R-156-003-010 (C)
Processed	2020-03-13 16:39:13
Camera Model Name(s)	ILCE-7R_FE35mmF2.8ZA_35.0_7360x4912 (RGB)
Average Ground Sampling Distance (GSD)	0.23 cm / 0.09 in
Time for Initial Processing (without report)	02h:11m:34s

### Quality Check



Images	median of 92439 keypoints per image	
Dataset	866 out of 875 images calibrated (98%), all images enabled	
Camera Optimization	1.02% relative difference between initial and optimized internal camera parameters	
Matching	median of 14641.1 matches per calibrated image	
Georeferencing	yes, 10 GCPs (10 3D), mean RMS error = 0.008 ft	

Figure 2-46 Computer 4 Quality Report Step 1

### Processing Options



Image Scale	multiscale, 1/2 (Half image size, Default)
Point Density	Optimal
Minimum Number of Matches	3
3D Textured Mesh Generation	yes
3D Textured Mesh Settings:	Resolution: Medium Resolution (default) Color Balancing: no
LOD	Generated: no
Advanced: 3D Textured Mesh Settings	Sample Density Divider: 1
Advanced: Image Groups	group1
Advanced: Use Processing Area	yes
Advanced: Use Annotations	yes
Time for Point Cloud Densification	13h:11m:45s
Time for Point Cloud Classification	NA
Time for 3D Textured Mesh Generation	31m:16s

Figure 2-47 Computer 4 Quality Report Step 2

### Summary



Project	MnDOT Retaining Wall R-156-003-010
Processed	2020-03-17 22:30:46
Camera Model Name(s)	ILCE-7R_FE35mmF2.8ZA_35.0_7360x4912 (RGB)
Average Ground Sampling Distance (GSD)	0.23 cm / 0.09 in
Time for Initial Processing (without report)	1d:05h:50m:53s

### Quality Check



Images	median of 92400 keypoints per image	
Dataset	862 out of 875 images calibrated (98%), all images enabled	
Camera Optimization	1.02% relative difference between initial and optimized internal camera parameters	
Matching	median of 14683.1 matches per calibrated image	
Georeferencing	yes, 10 GCPs (10 3D), mean RMS error = 0.007 ft	

Figure 2-48 Computer 5 Quality Report Step 1

**Processing Options**



Image Scale	multiscale, 1/2 (Half image size, Default)
Point Density	Optimal
Minimum Number of Matches	3
3D Textured Mesh Generation	yes
3D Textured Mesh Settings:	Resolution: Medium Resolution (default) Color Balancing: no
LOD	Generated: no
Advanced: 3D Textured Mesh Settings	Sample Density Divider: 1
Advanced: Image Groups	group 1
Advanced: Use Processing Area	yes
Advanced: Use Annotations	yes
Time for Point Cloud Densification	09h:12m:10s
Time for Point Cloud Classification	NA
Time for 3D Textured Mesh Generation	36m:19s

**Figure 2-49 Computer 5 Quality Report Step 2**

From the quality reports above, the following information was compiled to allow an easy comparison between each of the processors. This comparison is shown in the table below:

Computer	1	2	3	4	5
Processor	Intel® Core™ i9-9980XE CPU @ 3.00GHz (36 CPUs) 3.00GHz	Intel® Core™ i9-9900K CPU @ 3.6 GHz (16 CPUs) 3.6GHz	Intel® Core™ i7-8750H CPU @ 2.20 GHz (12 CPUs) 2.2GHz	Intel® Core™ i7-6820HK CPU @ 2.7GHz (8 CPUs) 2.7GHz	Intel® Core™ i7-8650U CPU @ 1.90GHz (8 CPUs) 2.11 GHz
Ram	64GB	64GB	32 GB	64GB	16 GB
System Type	64 bit	64 bit	64 Bit	64 bit	64 bit
Graphics	Nvidia Geforce RTX 2080 Ti	Nvidia Geforce RTX 2080 Ti	Intel® UHD Graphics 630	Nvidia Geforce GTX 1070	Intel® UHD Graphics 620
Onboard Storage	953 GB	930 GB	475 GB	475 GB	475 GB
Step 1 Process Time	55m54s	43m01s	3h09m38s	2h11m34s	1d05h50m53s
Point Cloud Time	1h43m05s	2h19m58s	4h55m25s	13h11m45s	9h12m19s
3D texture Time	26m08s	20m42s	25m56s	31m16s	36m19s
Total Time	3h05m07s	3h23m41s	8h30m59s	15h54m35s	1d14h54m31s
Images Calibrated	866 of 875	863 of 875	868 of 875	866 of 875	862 of 875

**Figure 2-50 Table of Computing Power Processing Test Results**

The processing time correlates to the processing power. The higher the power, the faster the computer was able to create the model. Referring to the table above, we can identify that this model took a maximum of 1 day 14 hours 54 minutes 31 seconds, and a minimum of 3 hours 5 minutes and 7 seconds. The difference in time shows that a higher-powered processor is the optimum configuration when creating models.

## CHAPTER 3: BRIDGE LIST CANDIDATE SELECTION

### 3.1 SELECTION CRITERIA

A bridge list was developed for bridges that would benefit from drone inspections based on parameters conducive to UAS inspections. All bridges in the MnDOT Metro District were ranked based on the following five factors:

#### Feature Intersected

Bridges over waterways are more conducive to UAS applications because the drone is less likely to fly over traffic. Bridges over waterways score 20 points and bridges over roadways is 0 points; however, new proposed rulemaking by the FAA will allow more flights over traffic. An additional score has been calculated to show how the scores and rankings will change when this new rule is in place. This new score ignores the feature intersected. The new rulemaking will dramatically increase the number of bridges eligible for a UAS inspection implementation.

#### Average Daily Traffic (ADT)

Bridges with ADT of less than 5000 score 20 points, ADT of less than 20000 score 10 points and ADT over 20000 score 0 points.

#### Bridge Type

With a focus on utilizing the Elios drone for confined space, bridges that could be inspected with the Elios score 20 points, and other bridges score 0 points.

#### Previous Successful UAS Inspection

Bridges that have previously been successfully inspected utilizing UAS score 25 points, and other bridges score 0 points.

#### National Bridge Inspection (NBI) Ratings

The use of UAS gives inspectors the ability to collect more data that is higher quality. This score awards 25 points for an average NBI of less than 6, an average NBI of between 7 and 8 receives 10 points, and an NBI of 9 scores 0 points.

### 3.2 IMPLEMENTATION

The bridges were sorted by score with higher scores representing bridges that would more likely benefit from UAS inspection, both from a risk and benefit standpoint. Several of the bridges on the list were inspected as part of this project and results can be found in Chapter 4 of this report. This list can be used in the future to help identify bridges that can be inspected by the Elios drone or the new drones

purchased by MnDOT which will be coming online soon.. The final sorted bridge list can be found in Appendix B.

## CHAPTER 4: UAS IMPLEMENTATION IN THE FIELD

Several case studies are presented below demonstrating different applications for utilizing UAS on bridge inspections. These efforts were included as part of this study, as well as part of inspection projects. The UAS results were incorporated into the bridge inspection record.

### 4.1 ST. CROIX CROSSING BRIDGE

This case study consists of the results of an UAS bridge modeling and defect mapping of St. Croix Crossing bridge number 82045, TH 36 over St. Croix River in Oak Park Heights, Minnesota. The section of bridge modeled for this phase was unit 3. Unit 3 is a 6-span, extradosed cable-stayed, post-tensioned concrete box girder unit spanning a total length of 3,365 feet between Piers 7 and 13. The unit is comprised of parallel precast box girder segments and carries the two lanes and shoulders of west and eastbound TH 36 and a 12-foot wide barrier-separated trail on the north side of the bridge. Transverse post-tensioning is located at each deck level with cable-stay anchorage. Loads are transferred from the interior box girder webs to the cable-stays. A catwalk is present along the full length of the unit along the bridge centerline. The typical unit width is 98'-6" out-to-out with a depth of 18' throughout.



Figure 4-1 St. Croix Crossing Bridge

### 4.1.1 UAS Operation

A two-person crew, consisting of a professional engineer-pilot and a field engineer conducted the UAS inspection. The pilot was certified as a FAA Remote Pilot. The inspection was conducted using a registered UAS and several AeroPoints. The UAS was an Intel Falcon 8+ equipped with a Sony Alpha 7R camera and the majority of flight missions were preplanned using Intel Mission Control software.

During the inspection, the pilot flew the UAS and had direct line of sight and headset communication with the other crew members who also maintained line-of-sight of the UAS. The photos were taken in four sections: Fascia, Deck, Underside, and Piers to achieve a high-resolution 3D model of the bridge. Deck and fascia sections had a preplanned mission to allow for an accurate overlap of each photo of approximately 85 percent. Underside and pier section photos were taken using manual flight control. All photos were taken in a systematic overlapping manner to ensure total photo coverage of the bridge.

### 4.1.2 Mission Scope

UAS was utilized for two main purposes for this bridge inspection: for planning the inspection and for executing the actual inspection.

Two weeks prior to the inspection, the entire bridge was flown from an altitude of 100 feet and 1200 images were collected. This data was post processed in Pix4D to create a model and a map. Both were used to plan the inspection and provided a visual tool to communicate this plan with a large team of inspectors. The inspection plan map delineated the substructure and span numbers, meeting areas, muster and manhole access locations. The plan was used in the field from phones and tablets to monitor progress and to orient inspectors to their location on the bridge.

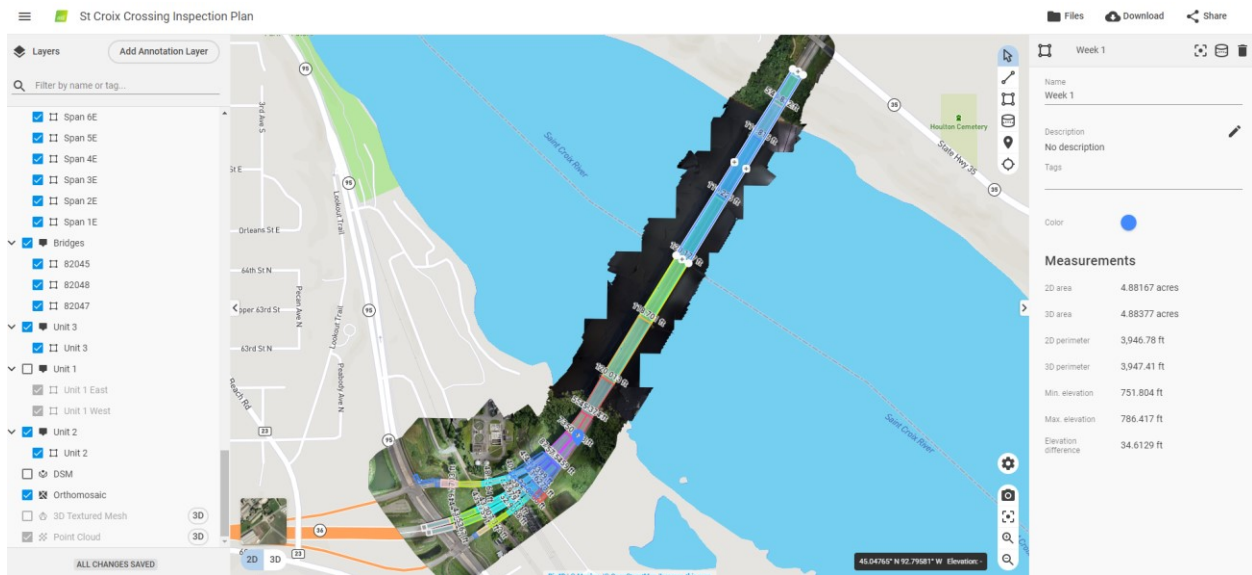


Figure 4-2 St. Croix Crossing Bridge Inspection Planning Map



Mapping missions were performed of all the main spans of the bridge using the Falcon 8+ UAS to explore the potential of UAS imaging including:

- the ability to map the entire large-scale bridge
- the difference in quality of a UAS and terrestrial inspections
- the applicability of a 3D model in assisting an inspection team during an in-depth inspection with areas of difficult accessibility

The Falcon 8+ efficiently captured a series photographs to use to map the bridge deck and to create a post-inspection tool to aid in repair plans, quantity estimation and historical documentation. The last in-depth inspection that was conducted was used as a control to compare the outcome of the UAS mapped post-inspected bridge. A total of 5,670 images were taken and collected with autonomous flights preprogrammed with Intel’s Mission Control Software. Images were collected of the underside of the bridge by placing the Intel drone on the boat and manually triggering photos as the boat was driven around underneath the bridge.

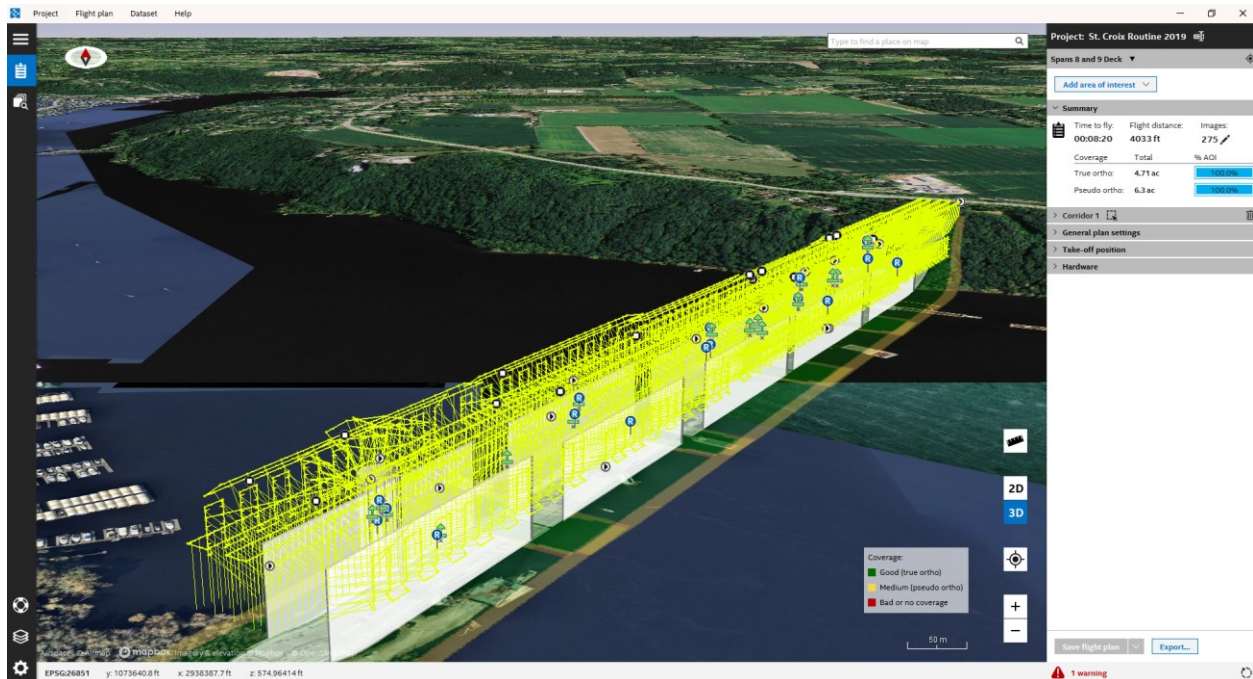


Figure 4-3 St. Croix Crossing Bridge Flight Plans

#### 4.1.3 Deliverables

A 3D Model with a High-Resolution Photograph Log was processed from the flights. All photographs taken during the Mapping Missions were processed in the Pix4D software. The software processed

photographs to form a 3D point cloud for digital viewing. The point cloud was then processed by the same software to generate a 3D mesh of the bridge.

An inspector was able to use the Pix4D program to navigate the 3D model and select areas of interest. The software provided the inspector with all still images containing the selected area of interest which can then be viewed in high resolution. Additionally, the inspector could place annotations and measurements areas into the model for design plans, further detailed inspection, or construction documents.

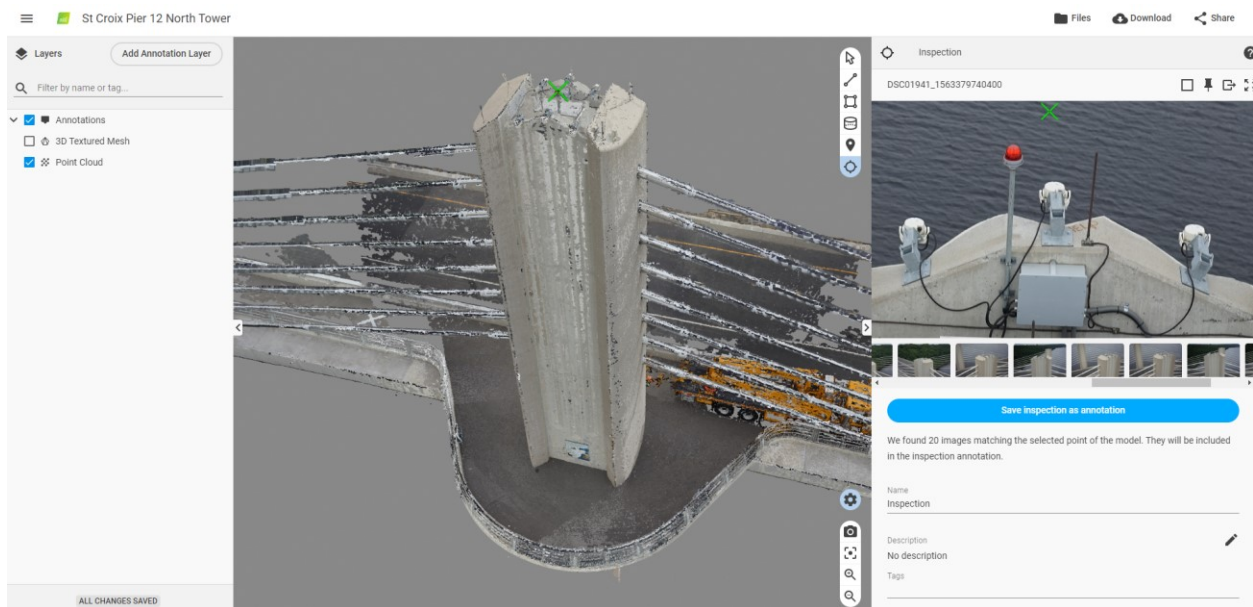


Figure 4-4 St. Croix Crossing Bridge 3D Photolog

#### 4.1.4 Findings

##### 4.1.4.1 Opportunities

The execution of the Mapping and Photogrammetry Mission was a success and provided great insight into the potential of UAS as a tool during in-depth inspections. The set-up, flight, and data generation went as planned. The Falcon 8+ UAS performed well and demonstrated to be a cost effective and efficient way to inspect areas of a bridge that were difficult to access. The use of UAS would reduce the high cost of traditional access equipment, produce an accurate account of defect quantities at the time of inspection, and the data gathered was substantial enough to create the planned deliverables. The deliverables demonstrated that significant improvements can be made in inspection documentation utilizing photogrammetry techniques. Defect measurement was more accurate and easier to share with the bridge owner.

#### 4.1.4.2 Limitations

While the Falcon 8+ has a high-resolution camera onboard, some photos were too blurry to process. This issue was due to UAS movement and slow camera shutter speeds. The quality and settings of the camera on the UAS dictate how the pilot should fly a mission and the pilot needs an intimate understanding of the operating environment and UAS settings. If the UAS has a lower quality camera, it will need to be flown closer to the bridge and require more photos to produce the same model. Flight time is an issue that must be factored in and planned for. For the Falcon 8+, there are two batteries that will allow a maximum flight time of roughly eighteen minutes. After the images have been taken and are put into a modeling software, the number of photos and the processing power of the computer drives the time it takes to process the bridge. After the initial process is completed, the model has the potential of needing manual editing to produce an accurate reality model.

#### 4.1.4.3 Conclusion

The use of UAS for the bridge safety inspection of the St. Croix Bridge was invaluable to creating a higher quality deliverable, saving time and resources, and creating a safer working environment. Although not all elements or components can or should be inspected with the drone, the use of a UAS under preplanned missions could replace the need for certain aerial access platforms, rope access requirements, or other hazardous or high-risk operations.

## 4.2 I-394 @ DUNWOODY ON AND OFF RAMPS

This field inspection implementation consisted of UAS imaging and terrestrial radio-controlled imaging in the routine inspection, element condition assessment, and defect mapping of several interior boxes and exterior elements at bridge number 27831C and 27831D, I 394 over Dunwoody Blvd. in Minneapolis, Minnesota. The bridges were inspected, and a study was conducted of the RC/UAS Inspection for the Minnesota Department of Transportation (MnDOT) in June-November 2019 as part of Phase IV.

Bridge 27831C is a westbound I-394 on ramp from I-94 near downtown Minneapolis. This structure is comprised of five prestressed concrete beam spans and four concrete box girder spans supported by reinforced concrete caps with reinforced concrete columns.

Bridge 27831D is an eastbound I-394 off ramp to I-94 near downtown Minneapolis. The structure is comprised of five prestressed concrete beam spans and four concrete box girder spans supported by reinforced concrete caps with reinforced concrete columns.

### 4.2.1 Mission Scope

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A mapping mission using the Parrot Anafi was identified to explore the potential of UAS photography to map concrete deck wearing surface defects. Primary goals of the missions included:

- Gather information in the form of photographs in a quick, efficient process that could later be used to map the deck wearing surface deficiencies
- Create a post-inspection tool to aid in repair plans, quantity estimation and historical documentation
- Eliminate the need for lane closures
- Explore the effectiveness of thermal imaging

Most of the Dunwoody bridge's 'On and Off' ramps were selected for imaging. The Parrot Anafi was flown using manual flight missions parallel to the ramp on an offset of approximately 30 feet from the outside railing with an elevation of the same distance above ground level. These parallel runs were done only on one side of each ramp without being directly over any traffic.

A high definition video mission was also conducted of limited access areas using the collision tolerant Elios UAS. Primary goals of this mission included:

- Determine the quality and applicability of using a Collision-Tolerant UAS to capture images
- Assess the condition of bridge pier cap topsides, end diaphragms and bearing elements which typically are difficult and costly to access and not easily viewed during a routine inspection
- Test the UASs' flight quality in open-air environments

The mission consisted of several interactive flights using only pilot controls. The use of the Elios UAS focused on these hard to access locations. An inspector then reviewed the video footage for defects and findings.

A two-person crew, consisting of a professional engineer-pilot and a field engineer conducted the UAS inspection. The pilot was certified as a FAA Remote Pilot. During the inspection the pilot worked with a mobile controller and the other crew member maintained a line-of-sight with the UAS. Flights that were conducted under the bridge deck and in the box girders hindered the GPS connection, meaning the flight was conducted in interactive GPS denied mode. The bridge was located in Class G Airspace therefore, no FAA waiver or additional authorization was necessary.

Several additional missions using terrestrial cameras mounted on a Remote Control (RC) platform were identified for this bridge to explore the potential of terrestrial photography to map concrete box girder interior defects. The target goal of the mission was to explore the difference in quality between UASs and terrestrial photography in confined spaces. The imaging of all boxes for bridge numbers 27831C and 27831D were selected for imaging using terrestrial means and cell DE1 from Bridge 27831C was selected for UAS means. The Flyability Elios was selected for the UAS method. The small confined space had limited effectiveness using the UAS due to the high dust levels. The terrestrial method utilized four GoPro Hero 7 High-Resolution Cameras mounted in a cluster on the RC device and orientated in different directions (up, down, left, and right). Images were taken simultaneously from all cameras at calibrated distances from the RC device. The cameras were equipped with a 12 Megapixel still camera and a fisheye lens. During the inspection, the driver worked with a mobile controller while the other crew members monitored hazards and maintained communication in the confined space for the occupant(s). The entirety of terrestrial imaging was conducted inside the box girder spans. The number of images ranged from 100-350 per box depending on the size of the individual cells.



**Figure 4-5 RC Imaging Platform with GoPro Camera Cluster**

## 4.2.2 Deliverables

The use of UASs provided adequate visuals of areas with limited access such as the elements above the pier caps. This eliminated the need to utilize expensive access equipment during the routine inspection. The videos were then analyzed with Flyability Inspector software to highlight any points of interests in the video and provide heading orientation and altitude relative to launch location. The image below highlights the Flyability Inspector program:

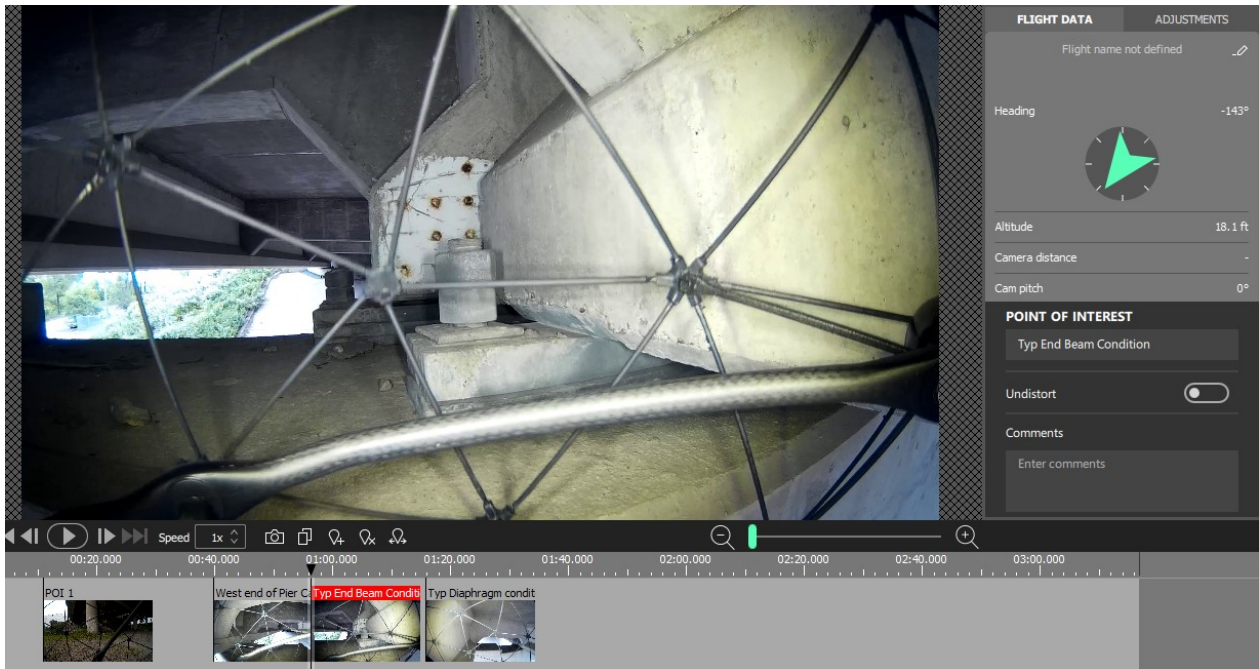
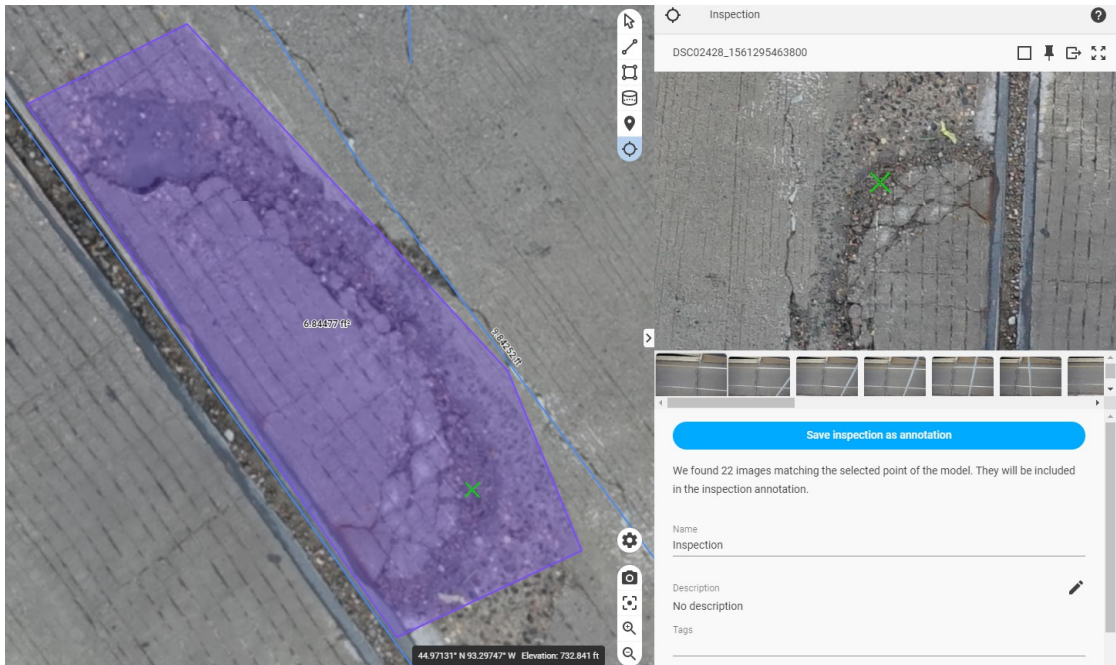


Figure 4-6 Flyability Inspector Screenshot.

A second use of UASs provided inspectors the ability to produce an orthomosaic map of the deck with the Pix4D program. These models were uploaded to the cloud and the inspector placed annotations and measurements on any deficiencies found in the model. If there was a specific area of interest, the inspector could click the area, and the software would provide all the still images affiliated with its annotations showing the original pictures of the area of interest. Refer to Figure 4-7 and Figure 4-8 for an example of a deck with inspection notes and annotations.



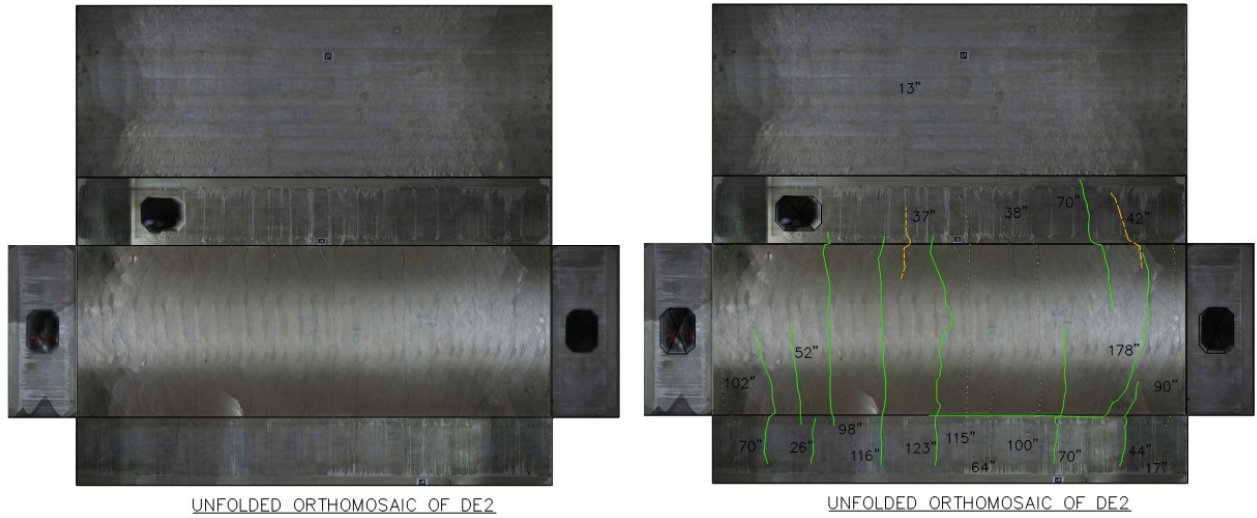
Figure 4-7 Bridge 27831A Deck with Measurements in Pix4D Cloud



A  
3D

Figure 4-8 27831A Deck using the Inspector Mode in Pix4D Cloud

Additional models were created in Pix4D software for all interior boxes from the terrestrial imaging missions. These models provided a visually representation the current condition of the boxes within a virtual interactive environment, allowing for future comparisons. The models could be simplified further into 2D orthomosaics where deficiencies could easily be traced electronically, shown below. Some limitations included time to figure out a proper process for correct imaging and increased processing time required to refine or rectify the final product for some models.



**Figure 4-9 Unfolded Views of a Box Girder Model, Left Being Untraced and the Right Traced**



**Figure 4-10 3D Model of a Cell**



## 4.2.3 Findings

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### 4.2.3.1 Advantages

The execution of the Mapping and UAS Mission was successful and provided great insight into the potential for UAS as a tool during routine inspections. The set-up, flight, and data generation went as planned and was substantial enough to create the planned deliverables. The deliverables demonstrated that significant improvements were made to the inspection documentation. Defect measurements were more accurate, simple to gather, and easier to share with the bridge owner.

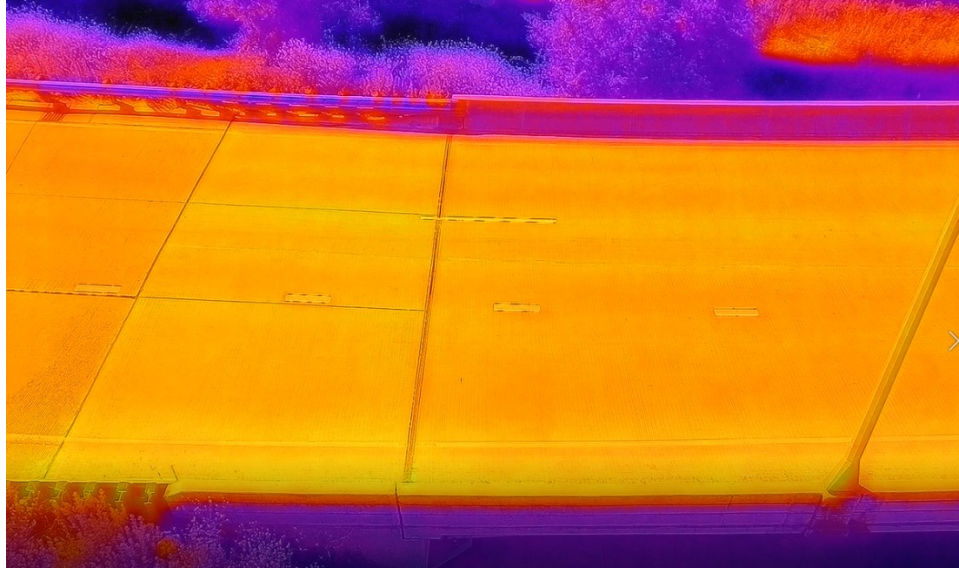
The Elios collision tolerant drone performed well and demonstrated a cost effective and efficient way to inspect bearings and top of pier caps. These elements were typically not accessed during routine inspections before due to the high cost of access equipment, and the ability to get access equipment into these locations without expensive lane closures.

Photogrammetry methods using terrestrial devices proved to be beneficial for field work as it eliminated much of the necessary field documentation which translated to less time for engineers to spend inside confined space. This method also helped for post-inspection evaluation giving the inspector an effective way to accurately translate defects and measurements for future monitoring.

### 4.2.3.2 Limitations

While the collision tolerant drone video is not as reliable as a hands-on inspection, most of the components near the top of the pier are not being accessed at close range due to the high cost of inspection equipment and the ability to get access equipment into place easily. The collision tolerant drone also had limited effectiveness in the confined spaces of the boxes because of the high amount of dust that has accumulated there in combination with very tight vertical clearances.

The thermal implementation of the deck was difficult to showcase any potential areas of delaminations. Further site investigation would be needed to confirm any areas that were of concern for confidence in accurate information. Refer to the image below for a snapshot of the deck under the thermal camera. Additional exploration of the thermal capabilities and settings would need to be explored to implement this technology on a large scale with engineering confidence.



**Figure 4-11 Thermal Snapshot of a Dunwoody Bridge Deck**

The terrestrial method inside the box girders was not able to navigate through diaphragms without aid from the driver. The lack of light also proved to be troublesome which required lighting from 2400 Lumen portable lights lengthening set-up time for each cell. Since the box girder bridges were split into 48 cells per bridge, set-up times had a large impact in labor hours.

#### **4.3 BLATNIK BRIDGE 9030 INSPECTION**

This implementation consisted of unmanned aircraft system (UAS) inspection and element condition assessment of the Blatnik Bridge in conjunction with the routine and fracture critical inspection performed by Collins Engineers for WisDOT and MnDOT. The primary purpose was to utilize UASs to provide an inspection 3D model of the bridge with images georeferenced for documentation of the bridge conditions.

The Blatnik Bridge crosses Saint Louis Bay between Duluth, Minnesota and Superior, Wisconsin. The bridge is a steel through arch truss bridge and was originally constructed in 1961 and widened in 1992.



Figure 4-12 Overall Photo of the Blatnik Bridge

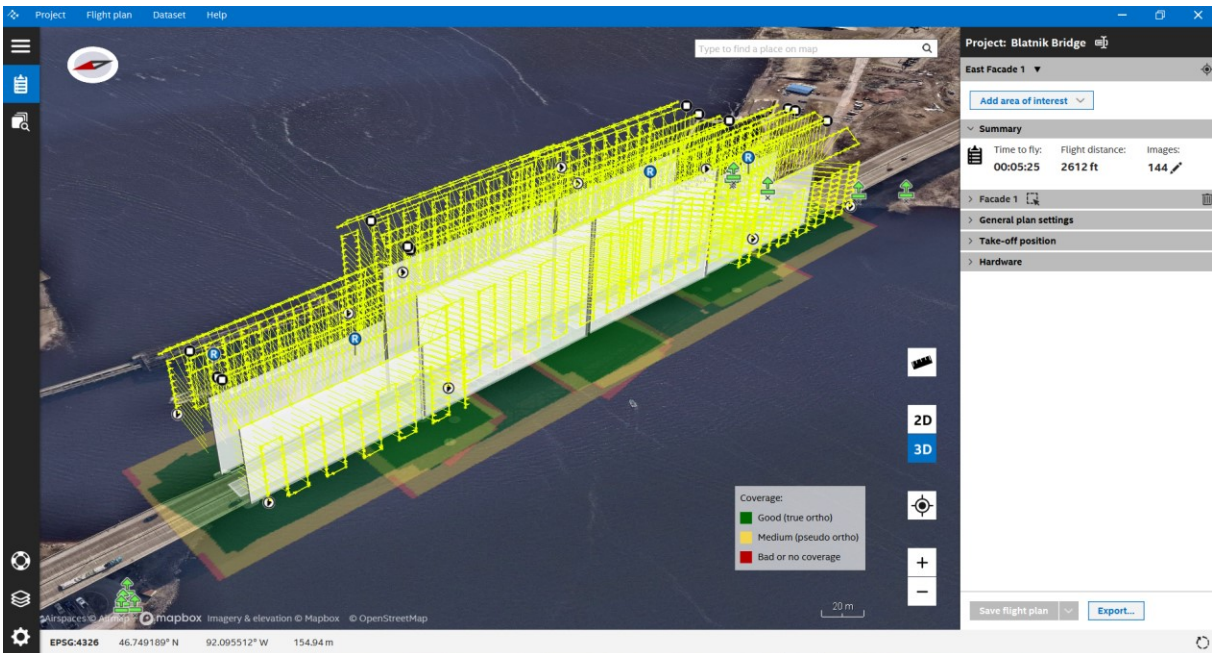


Figure 4-13 Preplanned Flights of the Blatnik Bridge

### 4.3.1 Mission Scope

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Collins Engineers performed the routine and fracture critical inspection of the Blatnik Bridge as part of the FHWA bridge safety inspection requirements. As part of this project, the team utilized UAS to collect bridge inspection data. The UAS was not intended to replace the arm's length inspection but to supplement the inspection and communicate inspection results.

The UAS inspection was conducted using the Intel Falcon 8+ equipped with a Sony Alpha 7R camera. The missions were preplanned using Intel's Mission Control software. During the inspection, the pilot worked with a mobile controller and the other crew member-maintained line-of-sight with the UAS. In order to achieve a high-resolution 3D model of the bridge, flights were conducted manually using a pre-planned flight to allow for a more accurate overlap of each photo of approximately 85 percent. Flights were performed from the corners underneath the bridge. AeroPoint automatic ground control targets were placed on the bridge deck and on the piers near the waterline to geolocate the deliverables. The bottom of the bridge was covered by using a boat to drive patterns under the bridge while images were triggered manually. The entire data collection effort took less than 8 hours with a crew comprised of two bridge engineers.



Figure 4-14 Aerial View of the Intel Drone Collecting Data



**Figure 4-15 Intel Drone Collecting Data**

### **4.3.2 Deliverables**

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A 3D Model was generated in Pix4D software of the truss spans of the bridge. The models were generated locally on a high-end desktop computer and uploaded to the cloud using Pix4D's cloud platform. The models provided a way to communicate visually the current condition with a virtual interactive environment and provide means for future comparisons. The virtual inspection tool from Pix4D allowed the user to click on an area of interest and see the corresponding high-definition images of that area sorted by shortest distance to the object. Inspection notes can be added which are geolocated and linked to a specific image. The model was also used to annotate measurements of the bridge in addition to bridge labels and truss panel points.

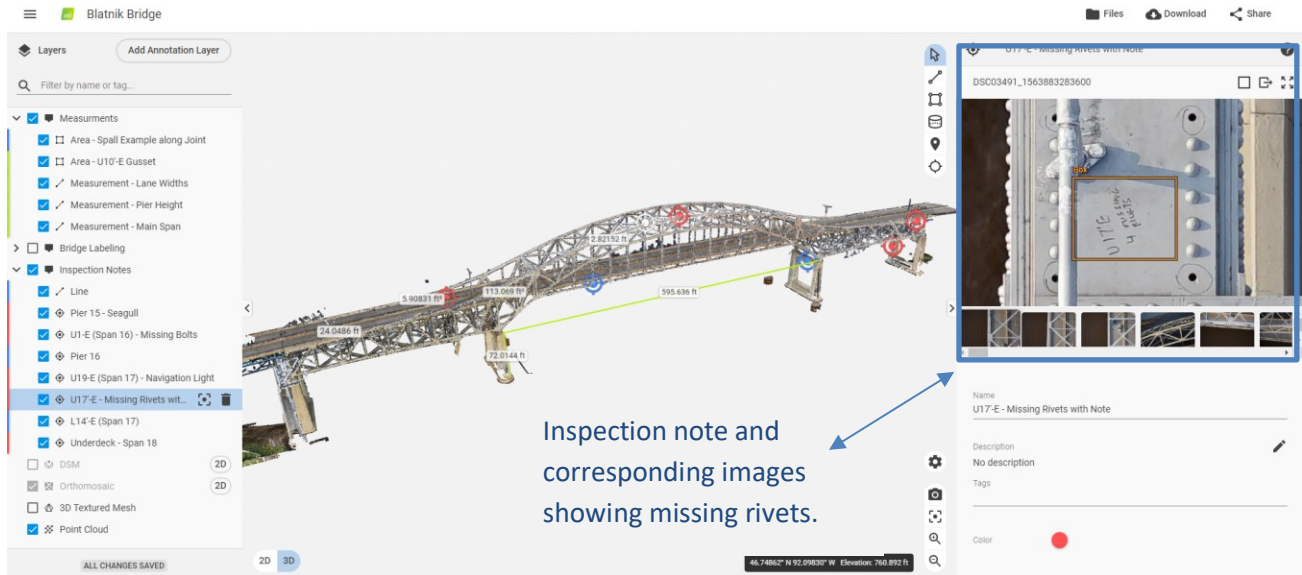


Figure 4-16 Pix4D Cloud Data for the Blatnik Bridge

### 4.3.3 Findings

#### 4.3.3.1 Advantages

The execution of the modeling with the Intel Falcon 8+ in combination with Pix4D software was successful and provided a very useful way to document the conditions of the bridge. The set-up, flight, and data generation went as planned and was substantial enough to create the planned deliverables. The deliverables demonstrated that significant improvements were made to the inspection documentation. Defect measurement was accurate, simple to gather, and easier to share with the bridge owner.

The preplanned missions from the Intel Software can be used in the future to collect the same data from the same locations and will offer a direct comparison of any deterioration between inspection cycles.

#### 4.3.3.2 Limitations

Minor limitations were presented when acquiring the data in the field. One limitation was that a large bridge can be difficult to model. Some portions of the bridge model were not initially complete and manual tie points were needed to accurately depict the bridge. QR codes should be used on future inspections to eliminate the need to manually add tie points to save time in processing. These codes could potentially be installed on the bridge for future inspections.

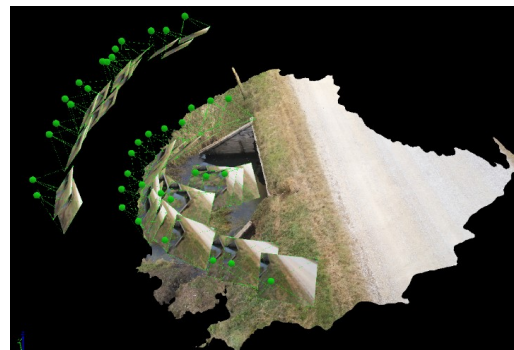
## 4.4 SHV BRIDGE

This UAS implementation consists as a supplement to a special hauling vehicle (SHV) condition, load rating assessments, and defect mapping of culverts in Minnesota. The primary purpose during the inspection was to document and record actual conditions efficiently, rather than the typical, time consuming technique of documenting via paper and pencil. A variety of bridges included in this UAS field work were selected across the state of Minnesota. Structures were culverts consisting of a flexible corrugated metal pipe arch on top of short masonry walls and timber box culverts.

A two-person crew, consisting of a professional engineer-pilot and a field engineer, conducted the UAS inspection. The pilot was certified as a FAA Remote Pilot. The inspection was conducted using a Parrot Anafi Thermal UAS. During the inspection, the pilot worked with a mobile controller and the other crew members maintained line-of-sight with the UAS. The bridges were located in class G airspace therefore, no FAA waiver or additional authorization was necessary. In addition to the UAS, terrestrial imagery was used to capture photographs inside the confined corridor of the interior of the culverts. The camera utilized was an Olympus Tough TG-6 Waterproof Digital Camera.

### 4.4.1 Mission Scope

A mission using the Parrot Anafi was identified to implement UAS photography to image an isolated area of the timber culvert. The target goal of the mission was to gather information in the form of photographs in a quick, efficient process. These could later be used to document and measure existing deficiencies and have a post-inspection tool to aid in repair plans, quantity estimation and historical documentation. This mission eliminated the need for a full-scale model as it was only an isolated area. The inlet area of the culture was selected for imaging due to the washout and erosion present there. The Parrot Anafi was flown using manual flight in a half circle around the inlet on two different altitudes and distance from the culvert. The flight was conducted without being directly over any traffic.



**Figure 4-17 Timber Culvert Imaging Flight Path**

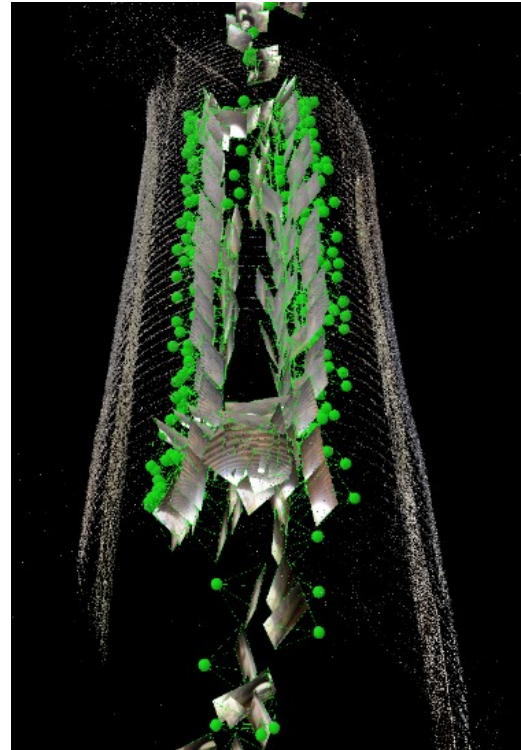
An additional mission was performed using a terrestrial camera to explore the opportunity to model and document deflection of metal pipe culverts. The target goal of the mission was to explore the quality in terrestrial photography in a confined corridor. The imaging of the corrugated metal pipe arch bridge, number 88883 was selected for imaging using terrestrial means. The method used a point and shoot camera that represented most field photography devices used during general inspections. To begin, images were taken of both inlet and outlet with adequate overlap of the subject. Then photos were strategically spaced out from the same angle as the inspector walked through the barrel. The process of walking through while taking pictures was repeated several times, back and forth with the camera at different angles. The number of images totaled 257.

#### 4.4.2 Deliverables

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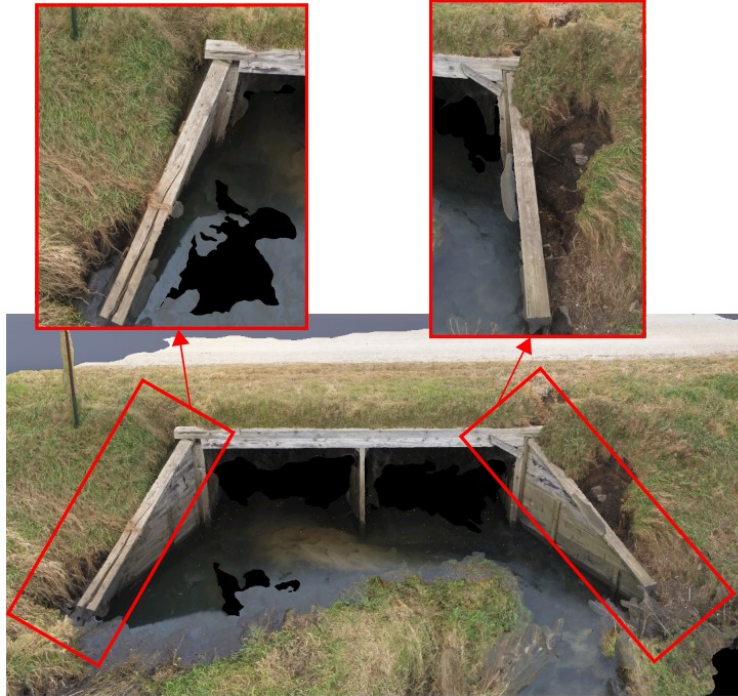
A 3D Model was processed in Pix4D software for the SHV bridge inspections presented above. The models created a virtual interactive environment with the ability to visually inspect current conditions, and allow for future comparisons, measurements, and documentation of condition assessment and load rating. The models also could be broken down into 2D orthomosaics where deficiencies could be easily traced and measured electronically.

The use of UASs provided adequate visuals for areas of concern efficiently with the ability for the inspector to obtain multiple photos that would reflect the best view of the deficiency as possible. The washout of Bridge 92460 was modeled at the inlet side of the culvert with the Anafi UAS. This gave insight on being more efficient with this tool by only implementing modeling on specific areas of concern. The image below represents the overall elevation view of the timber structure.



**Figure 4-18 Metal Pipe Arch Interior Model Imaging Path**





**Figure 4-19 3D Model Snapshot View with Closeups of Areas of Concern**

A 3D model was made of the metal pipe arch to document and measure the amount of deformation and bulging at the center of the culvert length. This is typically a difficult task to perform since the ends of a culvert are at different elevations and the deformation has no baseline to be relative to. Using this 3D model an extremely accurate measurement of the change in curvature and any flattening could be assessed and used in the load rating of the structure.

#### **4.5 TETTEGOUCHE BRIDGE**

This case study consists of the results of an UAS bridge modeling and inspection of the Tettegouche State Park Bridge. This bridge is 383 feet long and includes a 140-foot steel deck truss main span over the Baptism River and 4 steel girder approach spans. The UAS inspection was performed in conjunction with a fracture critical inspection performed by MnDOT.



**Figure 4-20 Tettegouche Bridge**

#### **4.5.1 UAS Operation**

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A two-person crew, consisting of a professional engineer-pilot and a field engineer, conducted the UAS inspection. The pilot was certified as a FAA Remote Pilot. The inspection was conducted using a registered UAS and AeroPoint ground control targets. The UAS was an Intel Falcon 8+ equipped with a Sony Alpha 7R camera and the majority of flight missions were preplanned using Intel Mission Control software.

During the inspection, the pilot flew the UAS and had direct line of sight. The photos were taken in four sections: Fascia, Deck, and Underside to achieve a high-resolution 3D model of the bridge. Deck and fascia sections had a preplanned mission to allow for an accurate overlap of each photo of approximately 85 percent. Underside photos were taken using manual flight control. 1,229 photos were taken in a systematic overlapping manner to ensure total photo coverage of the bridge.

#### **4.5.2 Mission Scope**

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A UAS was utilized to collect inspection information and to create a 3D reality model of the bridge to aid in organizing inspection results. The bridge was processed in both Pix4D and Context Capture to compare the quality of the models. The bridge was also uploaded to Pix4D cloud platform and bridge elements and defects were recorded directly on the model.



Figure 4-21 Tettegouche Bridge Flight Plan

### 4.5.3 Deliverables

A 3D Model with a High-Resolution Photograph Log was processed from the flights. All photographs taken during the Mapping Missions were processed in Pix4D software. The software processed the photographs to form a 3D point cloud for digital viewing. The point cloud was then processed by the same software to generate a 3D mesh of the bridge. The bridge was also processed in Context Capture and the resulting model was cleaner with less noise and modeling artifacts.

An inspector was able to use the Pix4D program to navigate the 3D model and select areas of interest. The software provides the inspector with still images containing the selected area of interest which can then be viewed in high resolution. Additionally, the inspector can place annotations or measurements in the model for repair plans and further detailed inspection.

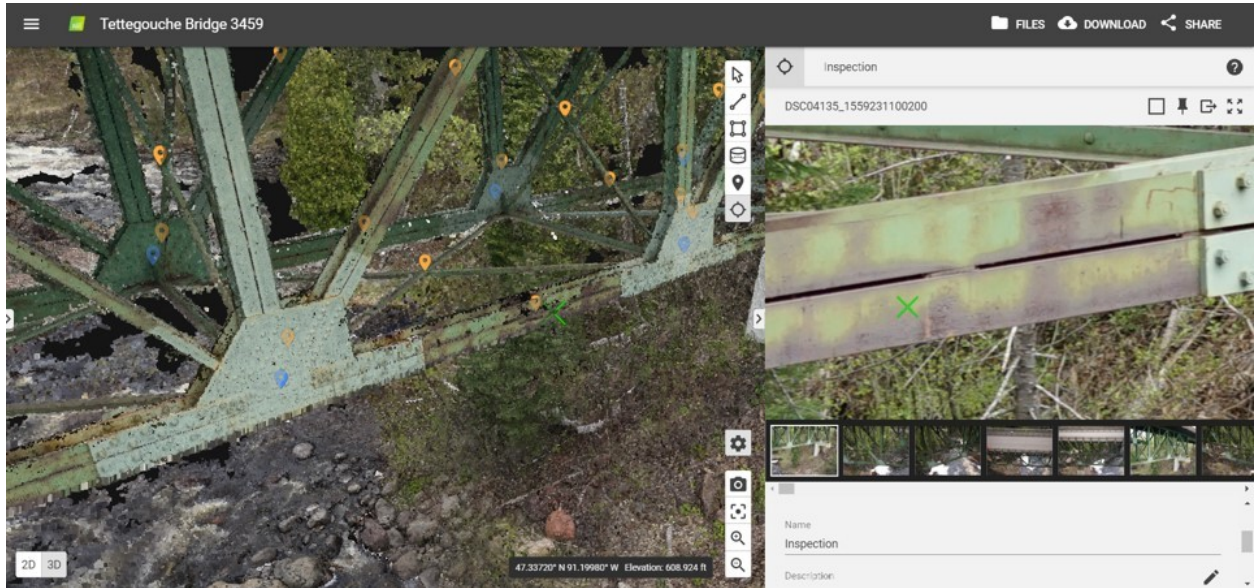


Figure 4-22 Tettegouche Bridge Pix4D Cloud Model Virtual Inspector Tool

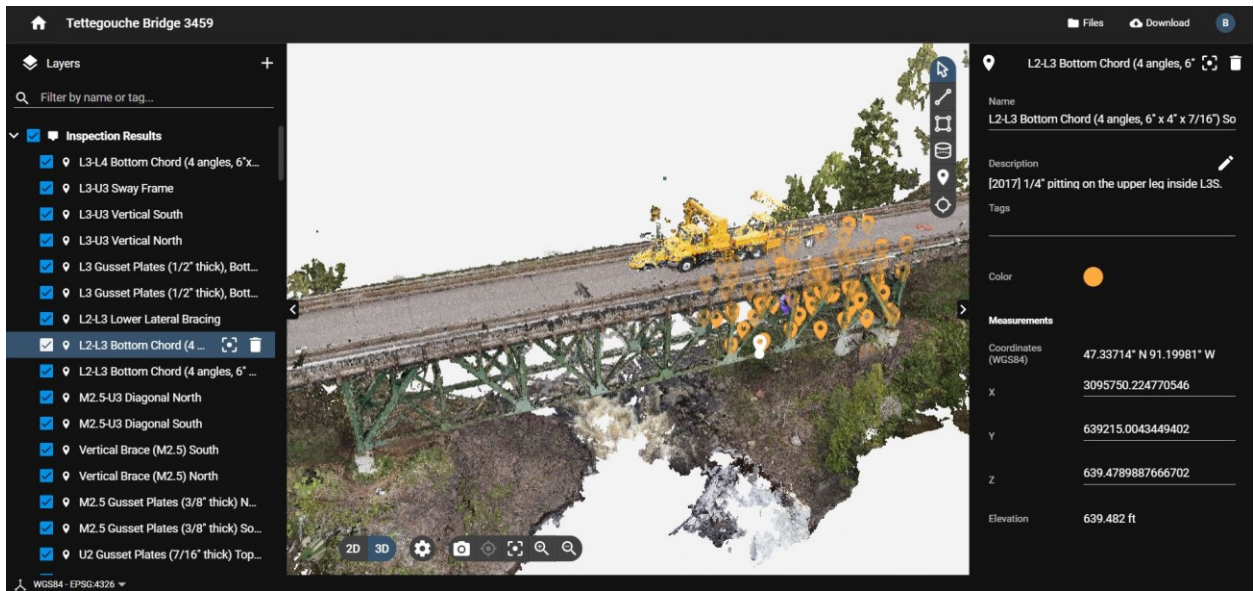


Figure 4-23 Tettegouche Bridge Pix4D Cloud Model with Inspection Notes



Figure 4-24 Tettegouche Bridge Modeled in Context Capture



Figure 4-25 Tettegouche Bridge Context Capture Model Close Up View

## 4.6 STONE ARCH BRIDGE INSPECTION AND REHABILITATION PROJECT

This implementation consisted of unmanned aircraft system (UAS) inspection and element condition assessment of the Stone Arch Bridge in conjunction with a rehabilitation project currently underway by Collins Engineers and LHB for MnDOT. The primary purpose was to utilize UASs to provide an 3D inspection model with georeferenced images to document bridge conditions and for use as data and a deliverable for the rehabilitation project.

The Stone Arch Bridge is located in downtown Minneapolis and crosses the Mississippi River. The bridge is a 2100-foot-long, 22 span masonry stone arch bridge built by James J. Hill for his Great Northern Railroad in 1883. The bridge served rail traffic until the early 1990's when it was converted to a pedestrian bridge. The Stone Arch Bridge is a historically significant bridge and is a landmark for the City of Minneapolis and the State of Minnesota.



Figure 4-26 Overall Photo of the Stone Arch Bridge

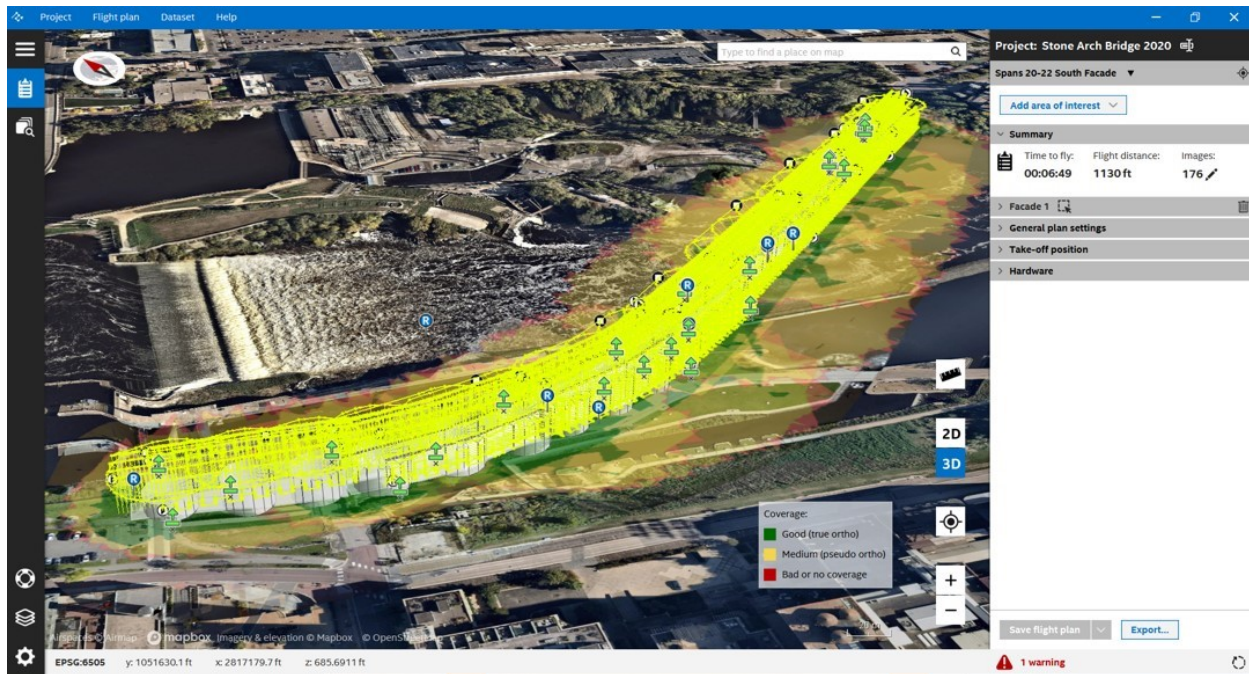


Figure 4-27 Preplanned Flights of the Stone Arch Bridge

#### 4.6.1 Mission Scope

Collins Engineers performed the inspection of the Stone Arch Bridge as part of a rehabilitation project for the Minnesota Department of Transportation. The project included a detailed inspection of the bridge in anticipation of masonry repairs. Detailed information on the condition of the masonry was required to develop repair plans and to calculate repair quantities. As part of this project, the team utilized UAS to collect bridge data. The UAS did not replace the arm’s length inspection but supplemented the inspection and provided an improved way to communicate the inspection results.

The UAS data collection process was split into two separate efforts. The first effort collected data for the entire bridge and the second effort recollected the same data for the river spans to take advantage of a river drawdown that exposed areas of the bridge that are normally underwater.

The UAS inspection was conducted using the Intel Falcon 8+ equipped with a Sony RX1R camera. The missions were preplanned using Intel’s Mission Control software. During the inspection, the pilot operated the drone with the mobile cockpit controller and the other crew members maintained line-of-sight with the UAS. In order to achieve a high-resolution 3D model of the bridge, most flights were conducted autonomously using pre-planned flights to allow for a more accurate overlap of each photo of approximately 85 percent. Flights were performed from areas near the bottom of the bridge with some flight performed from the bridge deck. When flights were flown from the bridge deck additional team members were present to direct pedestrians for safety. AeroPoint automatic ground control targets were placed on the bridge deck and on the ground near the bottom of the bridge to geolocate the deliverables. The images of the underside of the arches was collected by using a boat to drive

patterns under the bridge while images were triggered manually. The entire data collection effort took less than 4 days with a crew comprised of two to three bridge engineers. A video of the drone in flight collecting data can be viewed [here](#):



Figure 4-28 Video of Falcon 8+ Performing Inspection



Figure 4-29 Aerial View of the Intel Drone Collecting Data





**Figure 4-30 Intel Drone Collecting Data**

#### **4.6.2 UAS Implementation Workflow**

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This unique project required a workflow to incorporate the benefits of reality modeling and hands on inspection. The project workflow as it relates to UAS implementation is as follows:

1. UAS Field Data Capture
2. Digital Twin Creation
3. Field Inspection Utilizing Reality Models
4. Rehabilitation Design and Plans
5. Construction

Before any inspection was performed, the bridge was flown in its entirety and reality models were created of sections of the bridge in anticipation of a hands-on field inspection. The models were created with Context Capture and were uploaded to Projectwise for cloud sharing and editing in the field. Bridge engineers used iPad tablet computers with an internet connection to view the models and input inspection notes directly on the model during the hands on inspection. This method, the reality model and digital access while in the field was found to be advantageous. It allowed for greater accuracy than traditional paper and pencil note taking, and efficiencies were gained from not having to locate each stone or determining which stone on the drawing the inspector was looking at as this information was easily and accurately accessible. With all notes going directly onto the model and there was no

confusion about what the correct stone is. Ideally, inspection information could be prepopulated in the office and verified in the field but, the project schedule did not allow that in this case.



**Figure 4-31 Reality Model of Spans 17-22**



**Figure 4-32 Reality Model of Spans 17-22 With Inspection Notes**

Another advantage of creating and tying inspection notes to the reality model is that all of the data is available as a .json file and each note is geospatially located. The notes can be easily transferred to the construction drawings without having to manually copy notes into CAD, which is tedious and a timely process.

## 4.6.3 Deliverables

### 4.6.3.1 3D Reality Models and 2D Orthomosaics

3D Reality Models and 2D orthomosaics were generated in ContextCapture software as discussed previously. The models were generated locally on high-end desktop computers and uploaded to Projectwise. The models provided a way to communicate visually the current condition with a virtual interactive environment and provide means for future comparisons. The reality models are being used locally in Microstation to develop repair plans and are also available on Projectwise for all team member to view.

The following figure is a model of spans 1-12 that was created from the data collected during the river drawdown. This model represents a unique opportunity to capture data and display what the bridge looks like for the portions of the bridge that is normally underwater. The river basin is drawn down approximately every 12 years so, this data will be valuable for many years to come.



Figure 4-33 3D Reality Model of Stone Arch Bridge During River Drawdown



Figure 4-34 Reality Model Zoomed in to Pier During River Drawdown



Figure 4-35 2D Orthomosaic Overall



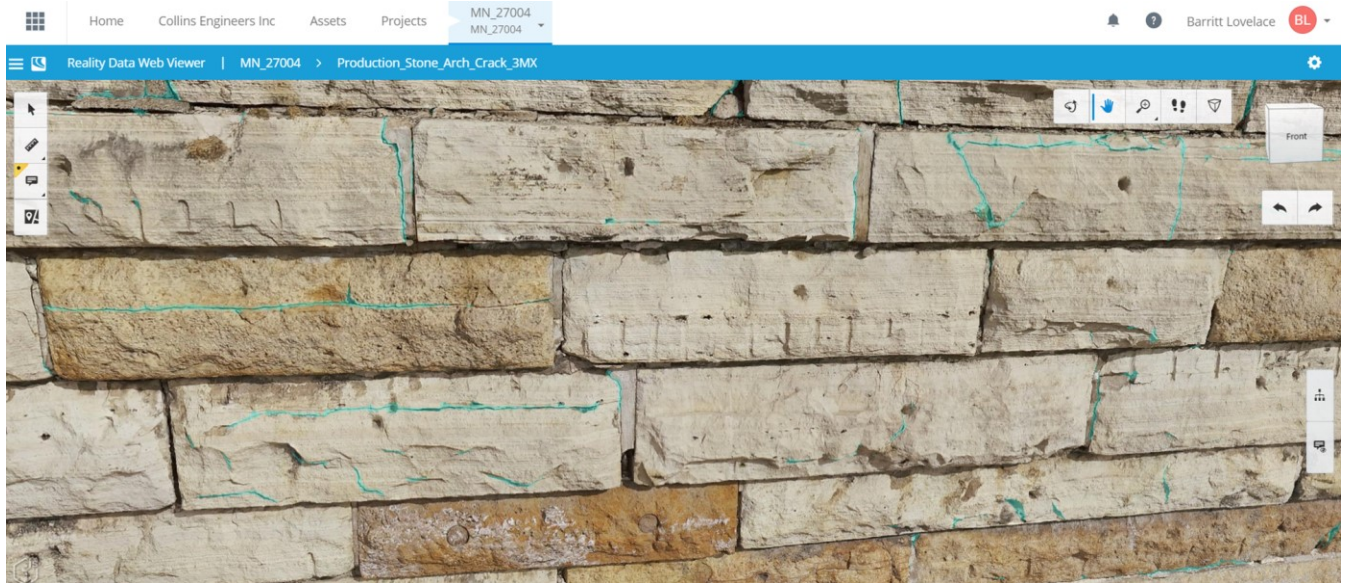
Figure 4-36 2D Orthomosaic Medium Close Up



Figure 4-37 2D Orthomosaic Close Up

#### 4.6.3.2 Artificial Intelligence

The models of the Stone Arch Bridge were analyzed using artificial intelligence and machine learning to find cracks on the masonry. ContextCapture Insights software was used to find and delineate the cracks automatically. While the algorithms are not perfect, they were impressive and this technology gives engineers a quick way to find and quantify cracking which is difficult to complete in the field using hand tools. Cracks are typically very irregular in their nature which makes it challenging to measure and document accurately.



**Figure 4-38 ContextCapture Insights AI Crack Detection**

#### 4.6.3.3 Holographic Virtual Inspections

Another deliverable that we created with Bentley Inc. is the ability to view and annotate the bridge model using the Microsoft Hololens 2. This unique method allows team members to perform virtual inspections of the bridge from their office.



Figure 4-39 Virtual Inspection of Bridge Using Microsoft Hololens 2

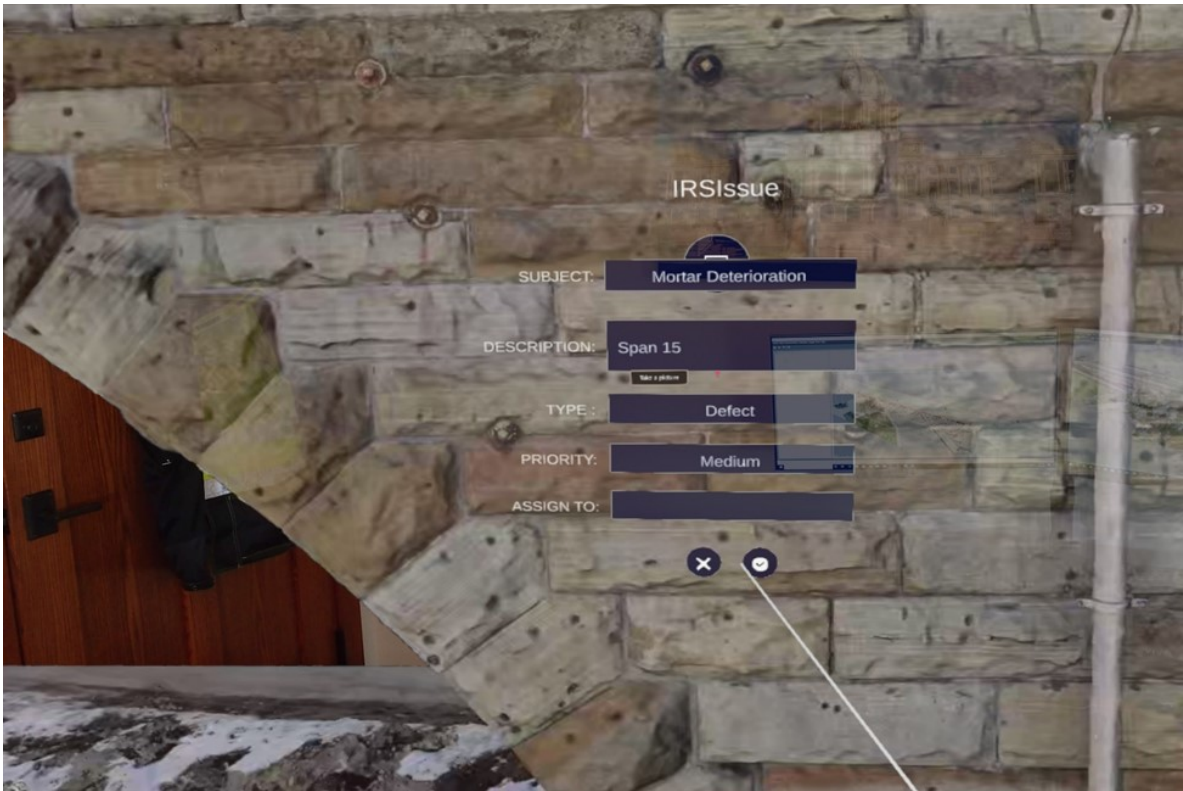


Figure 4-40 Hololens Inspection Defect Notation

More information on the holographic virtual inspection process can be found [here](#):



Figure 4-41 Hologram Virtual Inspection Process



## 4.6.4 Findings

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### 4.6.4.1 Advantages

The execution of the modeling with the Intel Falcon 8+ in combination with ContextCapture Software was successful and provided a very useful way to document and communicate the conditions of the bridge. The set-up, flight, and data generation went as planned and was substantial enough to create the planned deliverables. The deliverables demonstrated that significant improvements were made to the inspection documentation workflow. Defect measurement was accurate, simple to gather, and easier to share with the project team members.

The preplanned missions from the Intel Software can be used in the future to collect the same data from the same locations and will offer a direct comparison of any deterioration between inspection cycles.

### 4.6.4.2 Limitations

Minor limitations were presented when acquiring the data in the field. One limitation was that a large bridge can be difficult to model. The bridge is a very popular and busy pedestrian bridge so, our field teams had to be very careful when flying the drone and worked earlier in the day when there were fewer pedestrians. Another minor limitation was working with the large volume of data can be computer and network intensive but utilizing high-end processing computers allowed for reasonable workflows.

## 4.6.5 Conclusion

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### 4.6.5.1 Advantages

The execution of the 3D modeling mission was successful and provided some insight into the potential for UAS as a tool during load ratings for measurements and deficiency mapping. The set-up, flight, and data generation went as planned and was substantial enough to create the planned deliverables. The deliverable demonstrated improvements which can be made to the inspection documentation. Defects could be measured, simple to gather, and easier to share with the bridge owner. This could be a powerful tool to be more transparent from the field to the office in any areas the inspector feels may influence the performance of structures.

Photogrammetry methods using terrestrial devices proved to be beneficial for field work as it provided the owner a more comprehensive outlook of the structures. This method also helped for post-inspection evaluation giving the inspector an effective way to accurately translate defects and measurements for future monitoring.

#### 4.6.5.2 Limitations

The value of the models related to load ratings included in this report seemed to be limited to the documentation of defects and global measurements. Taking the photos and processing them did add extra time to perform and small critical measurements such as steel thickness, member dimensions, and wearing surface thickness still should be measured by hand in the field.

## CHAPTER 5: SAFETY AND OPERATION MANUAL

### 5.1 OVERVIEW

An Unmanned Aircraft Systems (UAS) Safety and Operation Manual was developed as part of this project. The manual can be found in Appendix A. The intent of the manual is to provide a risk-based approach to deciding when to take advantage UAS for bridge inspections. Also included are best practices for when UAS is used to help minimize the risk of incidents which could cause damage to the drone, property, or injuries. The manual is based on five years of drone use within MnDOT and while risks cannot be eliminated, the application of the best practices will minimize these risks significantly.

## CHAPTER 6: CONCLUSION AND RECOMMENDATIONS

### 6.1 PROGRAM APPLICATION

Unmanned Aircraft Systems have proven to be a low cost and low risk way to supplement bridge inspection efforts in Minnesota. Technology from both hardware and software continue to evolve making it more accessible to bridge inspectors and making the deliverables more beneficial and accessible.

### 6.2 INITIAL IMPLEMENTATION

As part of this project, UAS was applied to many bridge inspections using different applications. The bridges ranged from small simple structures to major signature river crossing bridges. The data collected was integrated into the inspection reports mostly by including links in the report to cloud-based, web sharing platforms.

### 6.3 SUSTAINED IMPLEMENTATION

MnDOT is has recently purchased a fleet of drones in addition to the Flyability Elios that is already owned by MnDOT Metro District. The purchase and use of these new drones will exponentially increase the implementation of UAS for bridge inspections within MnDOT. This report and the Safety and Operation Manual Bridge UAS Operations Manual should provide a framework for a successful implementation. This implementation will benefit MnDOT by lowering inspection costs, increasing safety for bridge inspectors and the traveling public and provide an additional access tool in tandem with the use of snoopers and rope access. Perhaps even more significant, the utilization of UASs and reality models provides not only an increased amount of data but also, an improved accuracy of the data. These enhancements will allow engineers to make data driven decisions to better manage these assets throughout their lifecycle.

## REFERENCES

Federal Highway Administration, (2004) *National Bridge Inspection Standards*, 23 CFR 650, FHWA, Washington, DC.

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Minnesota Department of Transportation (2020), *Bridge and Structure Inspection Program Manual*, MnDOT Office of Bridges and Structures, Oakdale, MN

Federal Aviation Administration (2020). Internet. Unmanned Aircraft Systems, Retrieved from <https://www.faa.gov/uas/>.

**APPENDIX A**  
**UNMANNED AIRCRAFT SYSTEMS (UAS) SAFETY AND OPERATION**  
**MANUAL**

STATE OF MINNESOTA

## Bridge and Structure

# Inspection Program Manual



## Chapter U

# UAS SAFETY AND OPERATIONS MANUAL

## TABLE OF CONTENTS

<b>U.1 OVERVIEW</b> .....	<b>1</b>
U.1.1 UAS USE PHILOSOPHY .....	1
U.1.1.1 Safety Improvements.....	1
U.1.1.2 Reduced Costs .....	1
U.1.1.3 Improve Data/Deliverables .....	1
<b>U.2 ABBREVIATIONS</b> .....	<b>2</b>
<b>U.3 UAS OPERATIONS</b> .....	<b>2</b>
U.3.1 RULES AND REGULATIONS .....	2
U.3.1.1 Federal Aviation Administration .....	2
U.3.1.2 MnDOT Requirements.....	3
U.3.1.3 Qualifications.....	4
U.3.3 EQUIPMENT .....	4
U.3.3.1 Battery Safety .....	5
U.3.3.2 Drone Airframe and Propellers .....	5
U.3.5 PRIVACY .....	6
U.3.6 ROLES AND RESPONSIBILITIES .....	6
U.3.6.1 Inspection Team .....	6
U.3.7 INSPECTION PLANNING .....	6
U.3.8 FIELD OPERATIONS.....	7
U.3.8.1 Signs .....	8
U.3.8.3 PPE.....	9
U.3.8.6 Risks and Mitigations.....	10
U.3.9 RESULTS AND DELIVERABLES .....	13

## LIST OF FIGURES

Figure 1- 1 View of Flyability Elios under a bridge.....	5
Figure 1- 2 Screenshot of flight planning software demonstrating a flight plan for a bridge.....	7
Figure 1- 3 Drone inspection ahead sign at the Stone Arch Bridge in Minneapolis.....	9
Figure 1- 4 UAS Inspection staff wearing PPE at the St. Croix Bridge.....	10
Figure 1- 5 Risk chart showing the likelihood of failure .....	11
Figure 1- 6 Map showing inspection plan notes for the St. Croix Bridge Inspection. ....	13
Figure 1- 7 A close-up view of a hard to reach area at the top the pier on the St. Croix Bridge .	14
Figure 1- 8 Bridge Deck Surveys.....	14
Figure 1- 9 Routine Inspection 3D Models .....	15
Figure 1- 10 Inspection Results Annotation .....	15



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Figure 1- 11 Individual Bridge Element Modeling .....	16
Figure 1- 12 Confined Space Inspection .....	16

## **U.1 OVERVIEW**

An Unmanned Aircraft System (UAS) is defined by the Federal Aviation Administration (FAA) as an aircraft operated without the possibility of direct human intervention from within the aircraft. Unmanned aircraft are commonly referred to as drones, and the names can be used interchangeably. The use of UASs to aid in bridge inspection should be considered as a tool for a qualified Team Leader when a hands-on inspection is not necessary or required. UASs are controlled either autonomously or with the use of a remote control by a pilot from the ground. Current technologies for commercial use include both fixed wing and rotor aircraft, although for bridge safety inspections rotary aircraft are more suitable. A wide range of imaging technologies including still, video, and infrared sensors can be utilized. On-site or in-office image processing can be used to produce inspection deliverables. UASs themselves cannot perform inspections independently but can be used as a tool for bridge inspectors to view and assess bridge element conditions in accordance with the National Bridge Inspection Standards.

This chapter is not intended to be a complete training manual on the use of UAS for bridge inspection but is intended to establish best practices and guidelines for a risk based approach to improve inspection outcomes. The owner or engineer may have to implement additional requirements that exceed those outlined in this chapter based on specific site conditions and engineering judgment or when presented with unusual circumstances.

### **U.1.1 UAS USE PHILOSOPHY**

Drones have proven to be a useful tool to provide alternative methods of access and to improve the quality of data for a bridge inspection. Bridge inspectors should consider incorporating the use of drones into a bridge inspection with the following guidance:

#### **U.1.1.1 Safety Improvements**

Drones should be considered when safety improvements can be realized for inspectors and/or the traveling public. Risks can be reduced by eliminating or reducing the need for:

- Personnel Working Near Traffic
- Traffic Control
- Personnel Working from Heights

Drones themselves can introduce their own risks which should be weighed against safety improvements. As an example the risk and consequence of a collision with a pedestrian should be weighed against the risk and consequence of a traffic accident resulting from a lane closure.

#### **U.1.1.2 Reduced Costs**

Drone use for bridge inspection has shown cost savings when traffic control and snooper use can be reduced. These cost savings should be considered especially when the quality of the inspection remains the same or improves.

#### **U.1.1.3 Improve Data/Deliverables**

Drones can improve the quality and quantity of data collected during an inspection and their use should be considered when these improvements can be realized.

## **U.2 ABBREVIATIONS**

AMSL – Above Mean Sea Level  
AGL – Above Ground Level  
ATO – Above Take Off  
ATC – Air Traffic Control  
BVLOS – Beyond Visual Line of Sight  
COA – Certificate of Waiver or Authorization  
CFR – Code of Federal Regulations  
PIC – Pilot in Command  
UAS – Unmanned Aircraft System  
sUAS – Small Unmanned Aircraft System  
FPV – First Person View  
GPS – Global Positioning System  
NAS – National Airspace System  
NOTAM – Notice to Airmen  
TFR – Temporary Flight Restrictions  
FAA – Federal Aviation Administration  
VO – Visual Observer  
LAANC – Low Altitude Authorization and Notification Capability

## **U.3 UAS OPERATIONS**

The following sections describe the recommended operating procedures and considerations when using UAS for bridge inspections.

### **U.3.1 RULES AND REGULATIONS**

#### **U.3.1.1 Federal Aviation Administration**

The Federal Aviation Administration (FAA) of the United States is a national authority with powers to regulate all aspects of civil aviation. These include the use of UAS for commercial purposes. All bridge inspections that utilize UAS are required to follow the FAA's UAS requirements.

UAS operations are allowed with a Certificate of Waiver or Authorization (COA) or under the FAA's Part 107. Part 107 applies to drones weighing less than 55 pounds, operated within the visual line of sight of the remote pilot in command, and flown during daylight hours. The remote

pilot in command must have a Remote Pilot Certification from the FAA which can be obtained by passing an aeronautical knowledge test. With direct supervision from a licensed remote pilot, anyone over the age of 16 can legally operate a drone for commercial purposes. Each UAS must be registered with the FAA. Operations in Class G airspace are allowed without air traffic control permission (ATC), however operations in Class B, C, D and E airspace need air traffic control approval. A basic summary of the requirements is included below.

<b>Pilot Requirements</b>	<ul style="list-style-type: none"> <li>Must have Remote Pilot Airman Certificate</li> <li>Must be 16 years old</li> <li>Must pass TSA vetting</li> </ul>
<b>Aircraft Requirements</b>	<ul style="list-style-type: none"> <li>Must be less than 55 lbs.</li> <li>Must be registered if over 0.55 lbs. (online)</li> <li>Must undergo pre-flight check to ensure UAS is in condition for safe operation</li> </ul>
<b>Location Requirements</b>	<ul style="list-style-type: none"> <li>Class G airspace can be flown under part 107 without an FAA waiver.</li> <li>Classes B, C, D, and E airspace can be flown with an FAA waiver</li> </ul>
<b>Operating Rules</b>	<ul style="list-style-type: none"> <li>Must keep the aircraft in sight (visual line-of-sight)</li> <li>Must fly under 400 feet</li> <li>Must fly during the day</li> <li>Must fly at or below 100 mph</li> <li>Must yield right of way to manned aircraft</li> <li>Must NOT fly over people</li> <li>Must NOT fly from a moving vehicle</li> </ul>
<b>Legal or Regulatory Basis</b>	<ul style="list-style-type: none"> <li>Title 14 of the Code of Federal Regulation (14 CFR) Part 107</li> </ul>

More information on Part 107 can be found on the FAA website.  
[https://www.faa.gov/uas/getting\\_started/fly\\_for\\_work\\_business/](https://www.faa.gov/uas/getting_started/fly_for_work_business/)

### U.3.1.2 MnDOT Requirements

The offices of Aeronautics and Chief Counsel provide assistance to districts and offices that are pursuing or contracting for UAS services. The Aeronautics Office has an official policy for the

use of UAS on MnDOT projects. The policy and procedures is detailed at the following website: <http://www.dot.state.mn.us/policy/operations/op006.html>

For UAS operation, MnDOT employees must:

- Obtain a blanket public Certificate of Waiver or Authorization (COA) that permits flights in Class G airspace at or below 400 feet, or
- Perform operations that adhere to 14 CFR Part 107 (“Part 107” operations).
- Create a safety and operations plan that addresses all aspects of the intended mission.

When contracting for UAS services, the contractor must adhere to the requirements of Part 107. MnDOT will review Section 333 Exemption and COA of third parties, and these contractors will be required to license the vehicle and obtain a commercial operator’s license from the MnDOT Office of Aeronautics as required by Minnesota Statutes §360.521 - Minnesota Statutes §360.675.

### **U.3.1.3 Qualifications**

FAA and MnDOT rules and regulations should be considered a minimum. The pilot in command should be experienced and confident in each application and in each environment in which it is used. The pilot and team members should be well versed in the particular drone models operating procedures and limitations. Each mission should be evaluated for risks and carefully planned to mitigate for those risks.

### **U.3.3 EQUIPMENT**

UAS equipment is available that is specific to inspections with features that are important when performing bridge inspections. It is recommended to employ a UAS specifically designed for the particular mission. Consideration should be given to the robustness and reliability of the drone to ensure safety and quality results. While technologies and capabilities differ, the most common inspection UASs share these general features:

- Powered by rechargeable batteries
- Controlled either autonomously or with a remote-control device
- Contain 4 to 8 rotors
- Ability to use GPS to track location
- Contain fail safes such as return to home technology
- Includes a camera with both video and/or still image capabilities
- Thermal sensors
- Proximity sensors and awareness
- Ability to preprogram autonomous missions

- Ability to fly under bridge decks in a GPS denied environment and within confined spaces.
- Ability to look up to view the underside of a bridge deck

### **U.3.3.1 Battery Safety**

Lithium Ion Batteries require special care to ensure they perform reliably. The following guidelines will help ensure batteries perform as designed. Specific care tips can also be found in each drones manual.

- Never fully discharge a battery.
- Do not store batteries fully charged.
- Only charge batteries in areas free from flammable materials.
- Never charge batteries unattended.
- Never store batteries in extreme temperatures.
- Protect batteries from punctures or impact damage.

### **U.3.3.2 Drone Airframe and Propellers**

Any UAS used should have a preflight inspection performed to ensure the equipment is operating properly. Special attention should be paid to critical parts including propellers and should be replaced according to manufacturer recommendations. Any discrepancies during flight should be logged and addressed.

Drones should be inspected for damage prior to mobilizing for and inspection and prior to each individual flight. A visual and tactile inspection of the drone propellers should be performed to identify damage. Propellers with any sign of damage or cracking should be replaced prior to use. Logs should be kept detailing when maintenance and repair activities have occurred.



Figure 1- 1 View of Flyability Elios under a bridge

### **U.3.5 PRIVACY**

Most bridge inspections are performed in areas where the public does not have a reasonable expectation of privacy. However, the privacy of the public should be respected and flying over private property should be avoided when possible.

### **U.3.6 ROLES AND RESPONSIBILITIES**

#### **U.3.6.1 Inspection Team**

The UAS operator is required by the FAA to have a Remote Pilot Certification. In addition, the operator should be very familiar with the UAS and have studied the owner's manual and received training on the operation of the UAS before attempting to fly near a bridge. Similar to manned aircraft, the crew should not operate with a medical condition that could interfere with safety. Generally, the minimum size of a crew should be two people, one to operate the aircraft and one to act as a spotter. It is recommended that the operator also be a qualified bridge inspector and at a minimum, the bridge inspector should always be on site directing the inspection.

Bridge inspections that utilize UAS should not deviate from FHWA and MnDOT requirements for Bridge Inspection Qualifications. The team leader should remain in control of the inspection efforts and if possible, the PIC should be a qualified inspector.

- Pilot in Command (PIC) – This person holds a remote pilot certification and has the final authority for flight operations. The PIC is responsible for planning and safe operation of the UAS during the inspection. Ideally this person is also a qualified bridge inspector. The PIC should only communicate with the VO during flights and distractions should be minimized.
- Visual Observer (VO) – This person assists the PIC to help identify air traffic, obstacles, vehicles and pedestrians. The VO should be in direct communication of the PIC throughout each flight.
- Support – Additional field staff may be required to monitor pedestrians and the public or to provide additional VO support in congested areas.

### **U.3.7 INSPECTION PLANNING**

Flights should carefully planned before mobilization in the field. Equipment necessary for the inspection should be listed and compiled. Autonomous flight plans should be prepared and any site-specific obstacles or challenges should be identified and addressed. Weather should be monitored and airspace restrictions should be checked. If airspace waivers are required they should be obtained from the FAA prior to mobilization. Temporary Flight Restrictions and Notices to Airmen should also be checked just prior to mobilizing for the inspection. Once mobilized in the field flight planning should be reviewed and adjusted based on site specific conditions.

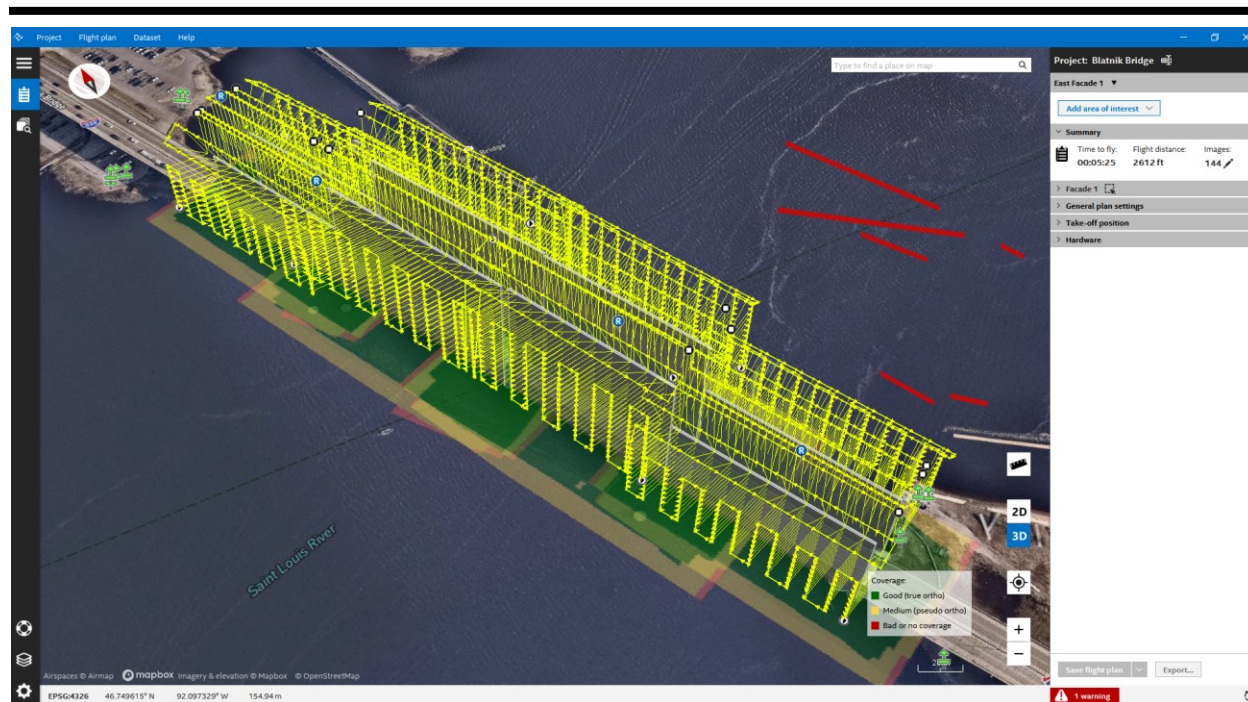


Figure 1- 2 Screenshot of flight planning software demonstrating a flight plan for a bridge.

### Automated Bridge Flight Plan Example

A safety plan should be prepared that addresses site safety and the proper qualifications of personnel and proper use of the UAS. The safety plan should address the following:

- Purpose of the effort
- Field team personnel
- Site location
- Structure description
- Any site-specific hazards
- FAA airspace class and waiver status if required
- Any privacy concerns

All personnel should be equipped with full personal protective equipment including eye protection and hard hats. The operations area should be delineated with cones, signs, and markers. If operations include the possibility of drivers seeing the drone within close proximity.

### U.3.8 FIELD OPERATIONS

The PIC has the ultimate authority over the flight operations and the responsibility to determine if flights can be safely perform considering weather and site conditions. Checklists should be



utilized to ensure safe flights. While each specific UAS will have its own specific checklist the following list includes general checklist items that should be included.

### Flight Checklist

- Remote Pilot Certificate
- UAS registered
- Updated firmware/apps
- Airframe Inspected
- Batteries charged
- Weather conditions checked
- NOTAMs/TFRs
- Identify Site Hazards and Obstacles
- Preflight Risk Assessment
- Airspace authorization
- Site plan/flight plan prepared
- Part 107 Compliance
- Verify GPS and Control Links
- Verify home point set and return to home parameters verified
- Scope/Deliverables Defined
- PPE
- Landing Area Delineated
- Return to home location and procedures understood and set
- Safety Briefing Performed
- Crew Mobile Devices off or in Airplane Mode

### **U.3.8.1 Signs**

Drone Inspection Ahead signage should be placed so drivers are not distracted by a UAS. An onsite safety briefing should be performed before work begins on the site each day



Figure 1- 3 Drone inspection ahead sign at the Stone Arch Bridge in Minneapolis.

### **U.3.8.3 PPE**

Personal Protective Equipment should include the following as needed based on site conditions:

- Reflective Clothing or Vests
- Safety Glasses
- Cut Proof Gloves
- Hearing Protection
- Respirators or Masks
- Skin Protection



Figure 1- 4 UAS Inspection staff wearing PPE at the St. Croix Bridge.

### U.3.8.6 Risks and Mitigations

UAS can be flown safely when risks are properly mitigated. Risk is managed by evaluating the likelihood of failure and the consequence of failure. An example would include hitting a manned aircraft during a bridge inspection would be a low likelihood of failure but a high consequence of failure. Alternatively hitting a tree with the drone would be a higher likelihood of failure but a low consequence of failure. We can manage risk by monitoring, managing, mitigating or re-

evaluating. The following chart can be used to assess risks. No operation is without any risk but as we increase the like

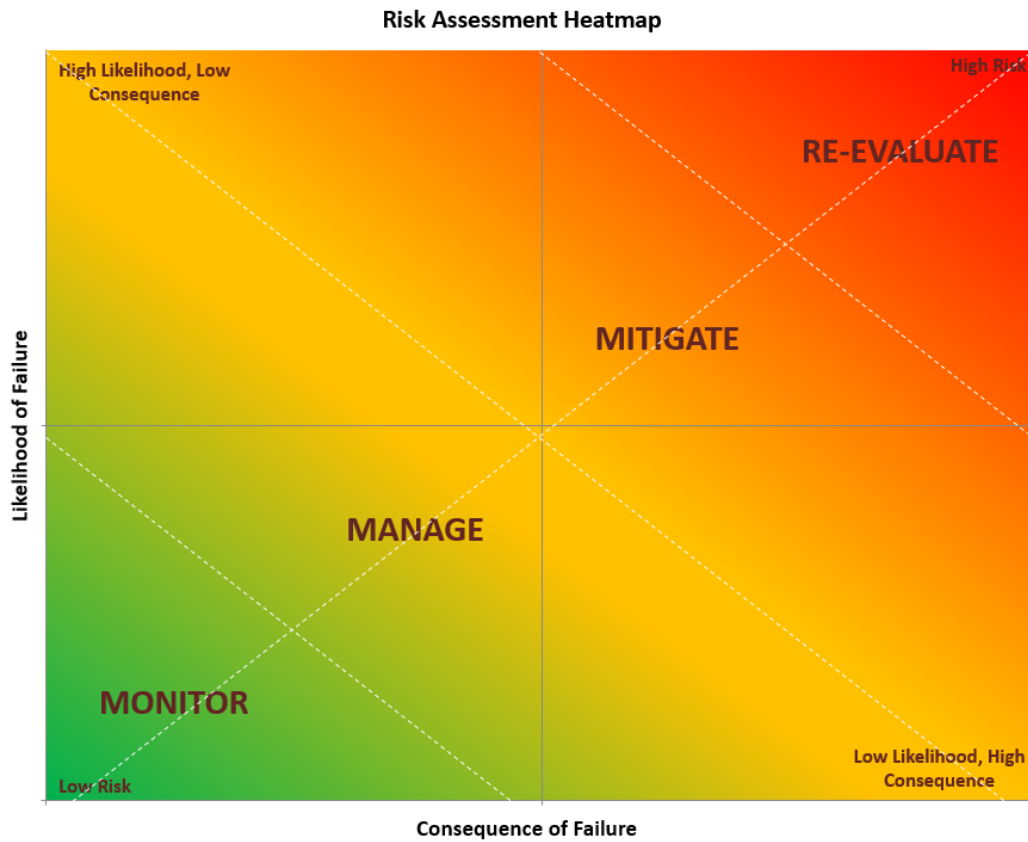


Figure 1- 5 Risk chart showing the likelihood of failure

The following is a list of risks associated with flying drones for inspection and some mitigation strategies that can be employed to limit the risks.

Risk: Collision with Manned Aircraft

Mitigations: Redundant Control Systems  
Additional Visual Observer(s)  
Limit size and weight of UAS  
Limit ceiling of drone flights to lower altitude  
Utilize Pilots with Higher Experience Level

Risk: Collision with Pedestrians or Vehicles

Mitigations: Limit Size and Weight of UAS  
Increase Horizontal Distance from Public  
Redundant Battery Systems  
Clearly Delineate Landing Area  
Shut off Motors if Crash Imminent  
Preplanned Ditch Procedures and Locations

Risk: Collision with Structure Being Inspected

Mitigations: Use Collision Tolerant Drone or Propeller Guards  
Utilize UAS Collision Avoidance Technology  
Additional Visual Observer(s)  
Increase Stand Off Distance

Risk: Propeller Injuries

Mitigations: PPE  
Cut Proof Gloves  
Proper Clothing  
Shut off Motors if Crash Imminent  
Preplanned Ditch Procedures and Locations  
Limit Size and Stiffness of Propellers

Risk: Loss of Signal

Mitigations: Monitor Signal Strength

- Utilize Return to Home Function
- Limit Distance to Operator
- Use Reliable Drones

Another important factor when evaluating risk when performing UAS missions is the risk associated with the alternative method of access. Flying a UAS to obtain inspection information may be less risky than closing a lane and using an under bridge inspection vehicle and so the mission and so any risk from UAS is less than the traditional method and therefore UAS should be utilized. Additionally, the value of the UAS mission should be considered and a more valuable missions can tolerate more risk. For instance, bridge safety inspection information collection should tolerate more risk that just taking an overall bridge photo for promotional purposes.

### U.3.9 RESULTS AND DELIVERABLES

Each inspection utilizing UAS should consider the scope and goal of the inspection. A wide variety of deliverables can be created to improve the inspection documentation. UAS can be utilized for bridge and structure inspection in a wide range of applications including the following:

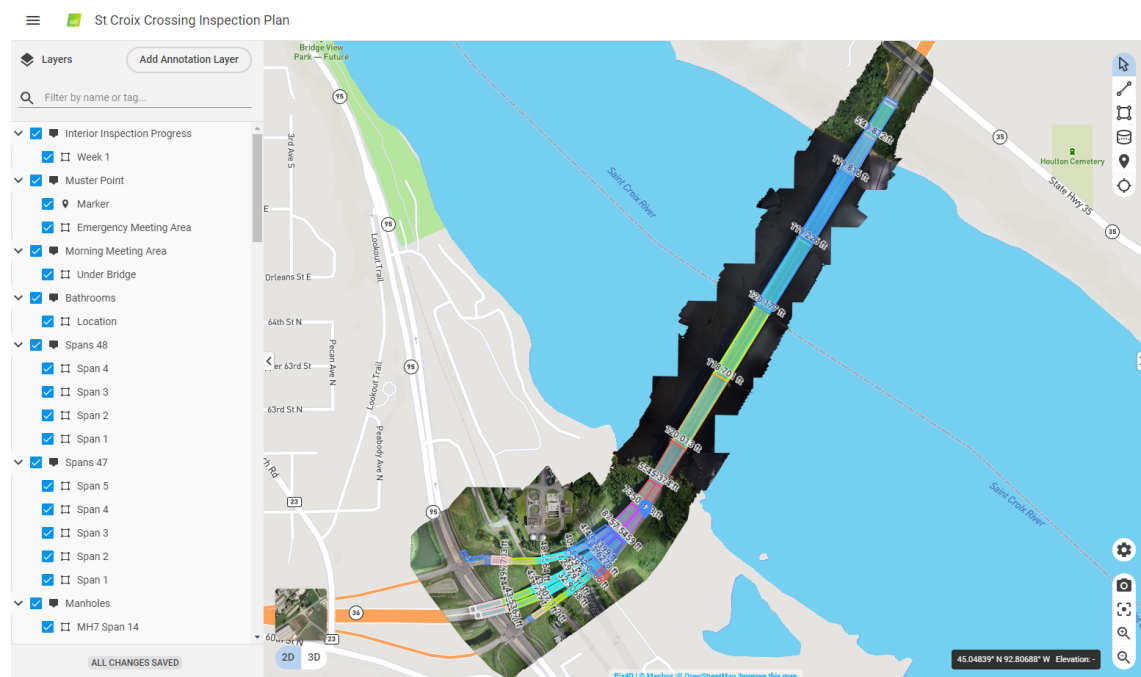


Figure 1- 6 Map showing inspection plan notes for the St. Croix Bridge Inspection.

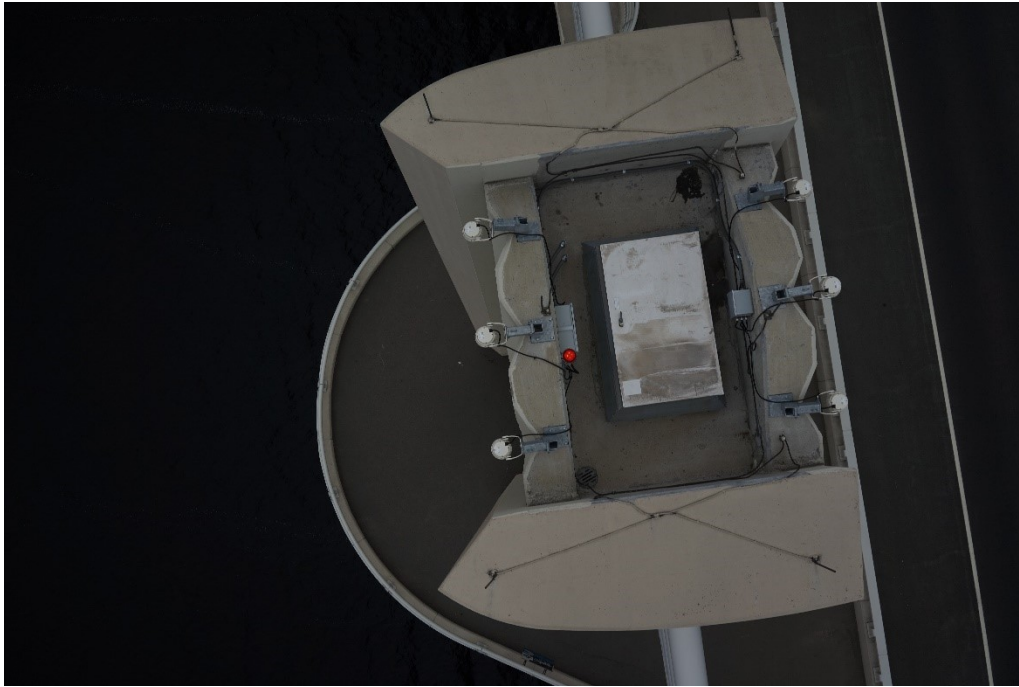


Figure 1- 7 A close-up view of a hard to reach area at the top the pier on the St. Croix Bridge

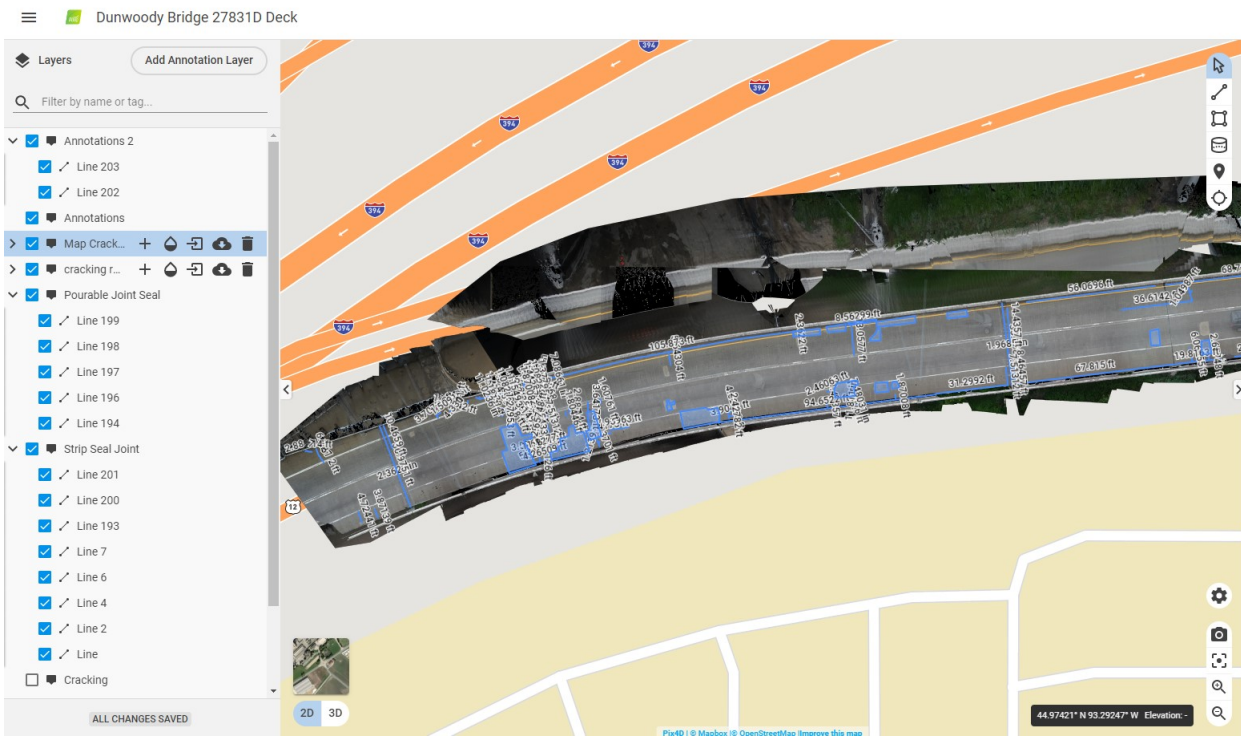


Figure 1- 8 Bridge Deck Surveys



Figure 1- 9 Routine Inspection 3D Models



Figure 1- 10 Inspection Results Annotation



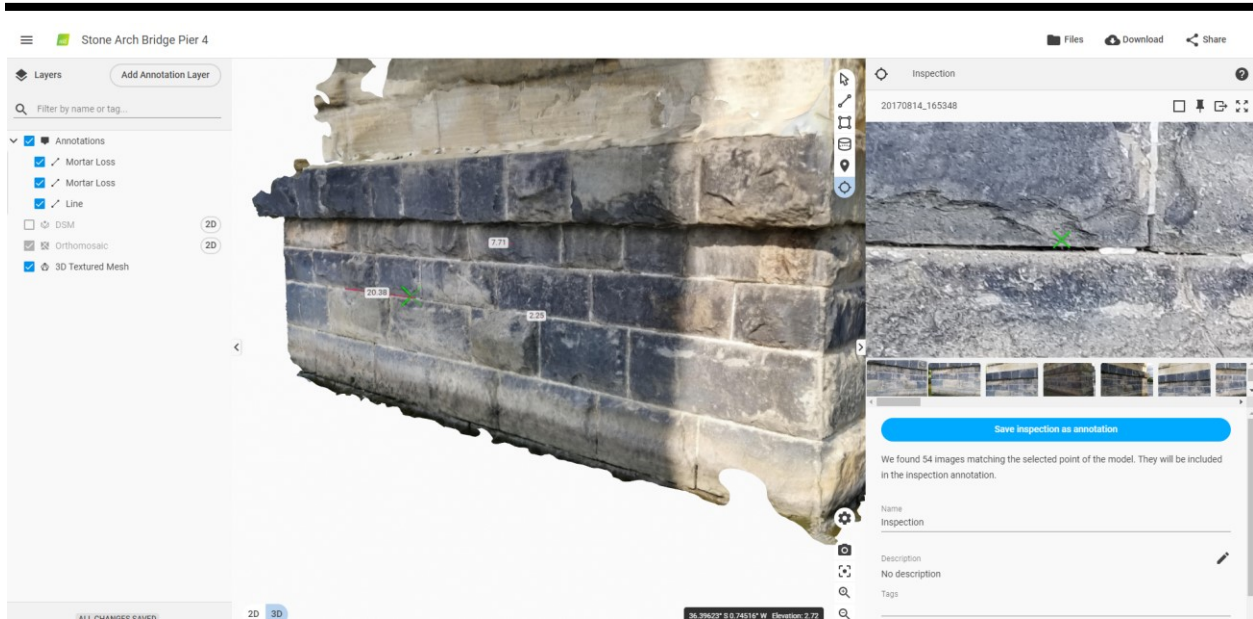


Figure 1- 11 Individual Bridge Element Modeling

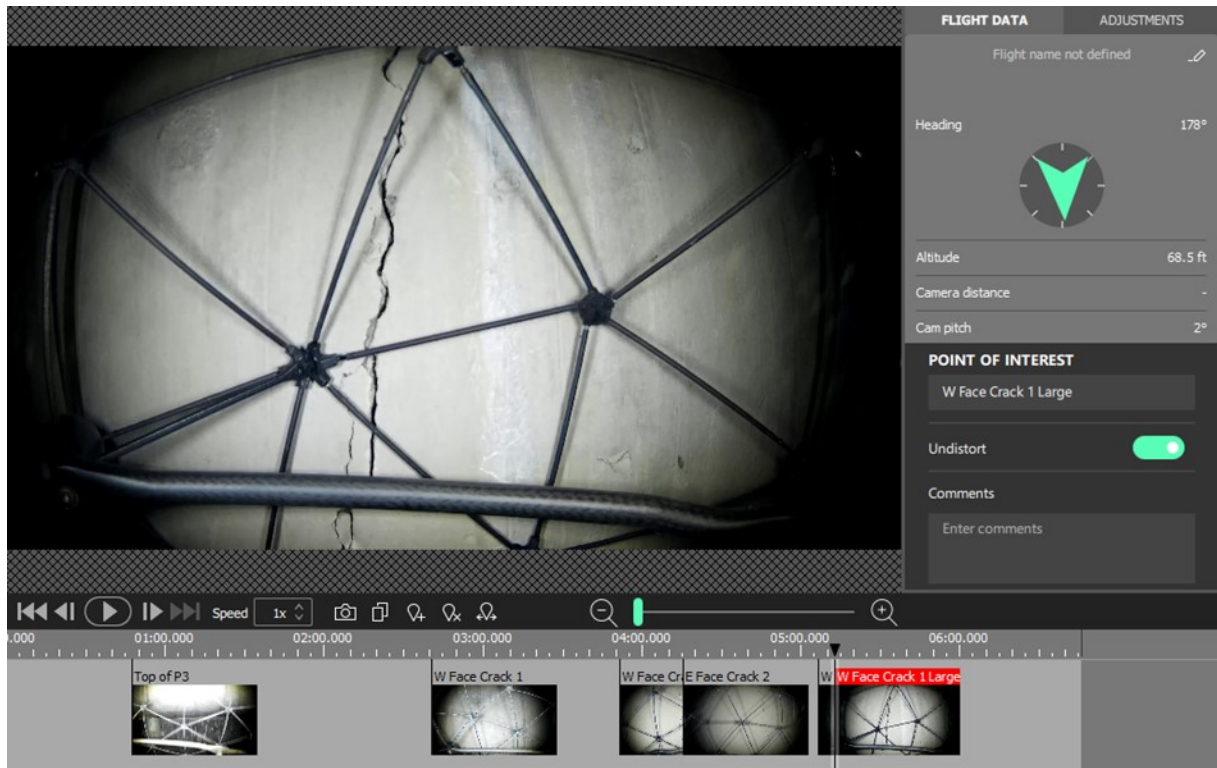


Figure 1- 12 Confined Space Inspection

**APPENDIX B**

**MNDOT METRO DISTRICT PRIORITIZED UAS BRIDGE LIST**

Asset Name	Latitude	Longitude	Airport	Schedule	No Flights over			Bridge Type/ Interior/Approach	Previous Success/ Knowledge	Bridge Condition	NBI 27: Year Built	NBI 6A: Feature/Intersecting/ Narrative	NBI 7: Facility/Category or Structure	NBI 29: Average Daily Traffic (ADT)	NBI 26: Functional Classification	NBI 16: Latitude Degree	NBI 16: Latitude Minute	NBI 16: Longitude Degree	NBI 16: Longitude Minute	NBI 17: Longitude Degree	NBI 17: Longitude Minute	Main Span Detail	MN Main Span Length	NBI 5E: Main Span	NBI 5S: Superstructure	NBI 60: Substructure	
					Traffic: (80)	Total Score	Flights over Traffic: Total Score (100)																				
27004	44.9807444	-93.2533028	Normal	48	90	20	20	20	25	25	1883	Mississippi River	FDCTY AT ANTHONY	1		50.68	-93	15	11.89	E	4	7	6	4			
27001	44.9494222	-93.2818389	Normal	24	70	20	0	10	25	15	1996	LAKE ST	TH 55	40623		44	56	54.32	-93	14	17.48		7	6	7		
82090	44.9326161	-93.0403318	Normal	24	80	20	0	10	25	15	1986	STREETS, MISS RIVER	TH 149	14000		44	54	52.74	-93	14	16.60	V	4	6	7		
82045	45.020194	-92.7850806	Normal	48	80	20	10	10	25	15	2017	ST CROOK RIVER THES-UPRR	TH 36	18400		44	42	31.27	-92	47	06.29		7	8	7		
82047	45.0306611	-92.7197556	Normal	24	65	65	20	10	0	15	2016	UP RR	TH 35 WB EXT RAMP	1		44	45	02	13.06	-92	47	30.52		7	8	8	
82048	45.0306611	-92.7197556	Normal	24	65	65	20	10	0	15	2016	UP RR	TH 35 WB ON RAMP	1		44	45	02	13.06	-92	47	29.86		7	8	7	
10001	44.7678	-93.7800694	Normal	48	55	55	20	20	0	0	15	1974	TCW RR, CSAH 36	TH 47	4225	02	44	42	04.08	-93	04	24.5		1	7	6	7
10002	44.767972	-93.7902389	Normal	48	55	55	20	20	0	0	15	1974	TCW RR, CSAH 36	US 212 EB	4250	02	44	42	03.35	-93	04	24.86		1	6	7	6
10004	44.813227	-93.5414417	Normal	24	55	55	20	24	0	0	15	2003	Bluff Creek	TH 16	15	14	44	48	47.62	-93	01	10.2		7	8	7	
10002	44.86275	-93.5741806	Normal	24	45	45	20	0	10	0	15	2000	Bluff Creek Trail	TH 5	14500	06	44	51	47.05	-93	12	12	10		6	6	6
10004	44.8627778	-93.5389	Normal	24	45	45	20	0	10	0	15	2000	Riley Creek Trail	TH 5	14500	06	44	51	46.00	-93	12	12	10		6	6	6
10007	44.8028028	-93.8487871	Normal	24	55	55	20	24	0	0	15	2001	County Ditch 10	TH 25	1850	06	44	48	13.69	-93	08	31.69		13	8	7	7
13005	45.5123233	-92.8536694	Normal	24	55	55	20	0	0	15	1994	SUNRISE RIVER	TH 95	3202	06	45	30	47.64	-92	51	13.21		1	8	8	7	
14001	44.634878	-93.1380903	Normal	24	55	55	20	0	0	15	1996	STREAM	TH 13 SIDE RD	1		44	39	17.56	-93	8	12.87		13	8	7	7	
14002	44.6449111	-92.7746094	Normal	24	55	55	20	0	0	15	2004	STREAM	OTEROO TRM (S 316)	1		44	38	43.48	-92	46	20.89		13	8	7	7	
14004	44.5723289	-92.8732694	Normal	24	55	55	20	0	0	15	2005	DITCH	TH 20 SIDE ROAD	40	09	44	34	20.06	-92	52	23.77		13	8	7	7	
14007	44.5423289	-93.1367028	Normal	24	55	55	20	0	0	15	2017	DITCH	TH 18	4950	06	44	32	35.57	-93	08	12.13		13	8	7	7	
21002	44.952708	-93.84765	Normal	48	65	65	0	10	0	15	1977	TH 100	Pod at 26th St	1		44	57	20.55	-93	27	6	7	6	7			
21003	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	1985	BASSETT CREEK	TH 55 WB on ramp	2000	14	44	59	02.14	-93	28	10.18		13	8	7	7	
21006	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	1985	BASSETT CREEK	TH 55 EB off ramp	4500	14	44	58	38.88	-93	24	17.46		13	8	7	7	
21009	44.9923444	-93.6178	Normal	48	55	55	20	0	0	15	2007	CSH 62	US 12, BNSF RR	1		49	32.44	-93	27	4	09.36		1	7	7	6	
22002	45.1230889	-93.1831934	Normal	48	45	65	0	20	0	15	1980	TH 610	Pedestrian	1		45	07	23.12	-93	19	09.55		7	7	7	7	
22004	44.9820222	-93.2930928	Normal	48	50	55	20	0	0	15	1985	CP RAIL (Frontage Rd)	TH 55 N Frontage	4700	19	44	58	60.08	-93	21	21.23		7	7	7	7	
22008	44.9820222	-93.2930928	Normal	24	55	55	20	0	0	15	2015	194, RUSH CREEK	TH 55 ON RAMP	520	19	44	58	40.20	-93	29	54.48		1	8	8	8	
22009	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2000	UP RR, RCRS DRIVE	TH 100 WEST FRONT	1		44	59	16.77	-93	20	57.66		1	7	8	8	
22011	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2000	UP RR, RCRS DRIVE	TH 100 ON RAMP	1		44	59	16.77	-93	20	56.67		1	7	8	7	
22012	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2003	TH 12, BNSF RR	LUCIE LINE TRAIL	1		44	58	49.17	-93	33	04.43		3	7	5	5	
22017	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2004	TH 12, BNSF RR	BROWN ROAD	4400	17	44	59	07.80	-93	34	25.84		1	7	8	7	
22014	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2004	TH 12, BNSF RR	WILLOW BRIDGE	4100	17	44	59	07.40	-93	33	20.44		1	7	8	7	
22015	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2004	TH 12, BNSF RR	OLD CRYSTAL BAY RD	2750	17	44	59	07.80	-93	33	39.29		1	8	8	7	
22018	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2004	TH 12, BNSF RR	TRAIL A	1		44	58	57.57	-93	35	30.87		1	8	7	5	
22008	44.9839278	-93.403725	Normal	24	55	55	20	0	0	15	2008	TRAIL A	LEGION LAKE	1		44	53	50.34	-93	33	50.17		1	8	8	7	
22009	44.9790306	-93.2451556	Normal	48	45	45	20	0	10	0	15	2008	MSS R, W R FWY, RD, RR	135W SB	70000	11	44	58	44.51	-93	14	42.56		7	7	7	7
22010	44.9790306	-93.2451556	Normal	24	45	45	20	0	10	0	15	2008	MSS R, W R FWY, RD, RR	135W SB	70000	11	44	58	44.51	-93	14	40.55		7	7	8	7
22011	44.7919111	-93.406425	Normal	48	45	45	20	0	10	0	15	2008	BNSF RR	UPRR 102 EB RAMP	2000	12	47	44	20.88	-93	27	30.88		7	8	8	7
22017	45.0337194	-93.2869194	Normal	48	45	45	20	0	10	0	15	1989	Shingle Creek	194	124000	11	45	2	1.78	-93	17	12.91		7	7	7	7
22018	44.9618833	-93.3449639	Normal	48	55	55	20	0	0	15	1989	BNSF RR, CITY STREETS	TH 100 WB FRONTAGE	4225	12	44	57	42.39	-93	20	41.87		1	7	7	7	
22019	44.9618833	-93.3449639	Normal	48	55	55	20	0	0	15	1989	BNSF RR, CITY STREETS	TH 100 WB FRONTAGE	4225	12	44	57	42.39	-93	20	41.87		1	7	7	7	
22021	44.9839278	-93.2763806	Normal	48	55	55	20	0	0	15	1982	PED PATH	TH 100 WB FRONTAGE	124000	11	45	2	11.59	-93	02	19.2		1	7	7	7	
22023	44.9839278	-93.2763806	Normal	48	55	55	20	0	0	15	1982	BNSF RR		1		44	59	01.49	-93	16	34.97		1	7	6	8	
22024	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22025	44.9732389	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22026	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22027	44.9732389	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22028	44.9732389	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22029	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22030	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22031	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22032	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22033	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22034	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22035	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22036	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22037	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd	1394 WB on ramp	3300	11	44	58	18.01	-93	17	50.18		1	6	6	6		
22038	44.9716694	-93.2917222	Normal	48	60	80	0	25	15	1967	Dunwoody Blvd																

02162	45.1756639	-93.3026806	48	35	55	0	20	0	0	15	1998	US 10	Creek Meadows Dr	3500	19	45	10	32.39	-93	18	09.65	1	7	7	7
02167	45.087671	-93.26825	24	35	55	0	20	0	0	15	2017	1 604	MAIN ST TRAIL	1	14	45	4	7.54	-93	16	5.97	6	8	8	8
02169	45.2147961	-93.04312	24	35	55	0	20	0	0	15	2017	1 350	CSAH 12	3050	12	45	12	03.67	-93	12	05.17	1	6	6	6
02182	45.1965	-93.0798222	24	35	55	0	20	0	0	15	2015	1 33E	80TH ST	1130	08	45	11	47.36	-93	1	4	6	6	6	
02183	45.0648858	-93.2710444	24	35	55	0	20	0	0	15	1987	1 604	BNSF RR		07	45	04	16	-93	1	7	7	7	7	
02184	45.2645	-93.415356	24	45	45	0	20	0	0	15	2006	1 206	TRAIL	10000	16	45	15	55.10	-93	18	47	8	8	8	8
02186	45.2994917	-93.4102639	24	45	45	0	20	10	0	15	2000	1 47	Ford Brook	6900	06	45	17	58.17	-93	24	36.95	13	N	N	N
02187	45.311233	-93.4530917	24	45	45	0	20	10	0	15	2000	1 47	Ford Brook	5300	06	45	18	40.44	-93	24	21.57	13	N	N	N
02188	45.091556	-93.43335	24	35	55	0	20	0	0	15	2001	1 65	TH 165	3400	14	45	5	36.71	-93	15	16.65	1	6	6	6
02189	45.1547	-93.1695639	24	35	35	0	20	0	0	15	2004	1 33W	CD DTCH S1-62	12000	11	45	9	16.92	-93	10	10.43	13	N	N	N
02190	44.83727	-93.0679744	24	45	45	0	20	10	0	15	2012	1 62	LK MINNETONKA LKT TRL	15000	16	44	44	35.01	-93	37	20.97	1	8	8	8
02191	44.837075	-93.062122	24	45	55	0	20	10	0	15	2012	1 62	THS ARDELVA LAK BVD	19100	16	44	51	14.47	-93	29	13.47	1	8	8	8
02192	44.8592306	-93.5417028	48	45	45	0	20	10	0	15	1984	1 5 B	TH 5 B	19750	16	44	51	33.23	-93	12	30.13	1	6	7	6
02193	44.8592372	-93.4133261	48	45	45	0	20	10	0	15	1990	1 5 B	TH 5 B	19750	16	44	51	33.23	-93	12	30.13	1	6	7	6
02194	44.794461	-93.020444	24	45	45	0	20	10	0	15	2004	1 41	Chin Lake Pavilion	12500	14	44	44	39.64	-93	14	1	4	4	6	6
02195	44.7827611	-93.5991278	24	45	45	0	20	0	0	15	2007	1 41	Minnesota River	12500	14	44	46	37.94	-93	15	54.66	1	7	8	7
02196	44.7910389	-93.413444	24	35	55	0	20	0	0	15	2008	1 212	CNTY RD 140	750	07	44	47	27.74	-93	37	54.64	1	7	7	7
02197	44.7772611	-93.6425133	24	35	55	0	20	0	0	15	2008	1 212	CSAH 11	1200	16	44	46	38.14	-93	38	33.12	1	8	8	8
02198	44.8008833	-93.6297611	24	45	45	0	20	10	0	15	2008	1 212	WBB	7650	14	44	48	02.10	-93	37	47.14	1	8	8	8
02199	44.8206964	-93.0291211	24	45	45	0	20	0	0	15	2008	1 212	CR	7650	14	44	48	01.89	-93	37	45.88	1	8	8	8
02200	44.8136056	-93.6124639	24	35	55	0	20	0	0	15	2007	1 212	BAVARIA ROAD	4800	17	44	48	48.87	-93	36	44.87	1	7	8	7
02201	44.8146883	-93.6071339	48	45	45	0	20	10	0	15	2007	1 212	WBB	15000	14	44	48	52.86	-93	36	26.33	1	7	7	7
02202	44.8146883	-93.6071339	48	45	45	0	20	10	0	15	2007	1 212	CR	15000	14	44	48	52.86	-93	36	26.33	1	7	7	7
02203	44.8174417	-93.59675	48	45	45	0	20	10	0	15	2007	1 212	HUNDETRMARK CREEK	15000	14	44	49	02.79	-93	35	48.3	1	7	7	8
02204	44.8172389	-93.5969944	48	45	45	0	20	10	0	15	2007	1 212	HUNDETRMARK CREEK	15000	14	44	49	02.79	-93	35	48.3	1	7	7	8
02205	44.8215283	-93.56295	48	35	55	0	20	0	0	15	2007	1 212	BLUFF CRK DR	1	14	44	49	31.39	-93	35	47.74	1	7	7	7
02206	44.8215194	-93.56295	24	35	55	0	20	0	0	15	2007	1 212	BLUFF CRK DR	1	14	44	49	31.39	-93	35	47.74	1	7	7	7
02207	44.8199722	-93.5809972	24	35	55	0	20	0	0	15	2007	1 212	BLUFF CRK DR	1	14	44	49	31.39	-93	35	47.74	1	7	7	7
02208	44.82051	-93.579936	24	35	55	0	20	0	0	15	2007	1 212	BLUFF CREEK(MAIN BRANCH)	29000	14	44	49	48.54	-93	37	48.54	1	7	8	8
02209	44.828861	-93.5576861	24	35	45	0	20	0	0	15	2007	1 212	BLUFF CREEK(MAIN BRANCH)	29000	14	44	49	48.54	-93	37	48.54	1	7	8	8
02210	44.8121111	-93.6176	24	45	45	0	20	10	0	15	2007	1 212	KOHNEN CREEK	11500	14	44	48	43.60	-93	37	03.36	1	8	8	8
02211	44.8128228	-93.6176228	24	45	45	0	20	10	0	15	2007	1 212	KOHNEN CREEK	11500	14	44	48	43.60	-93	37	03.36	1	8	8	8
02212	44.8605628	-93.5274417	48	35	55	0	20	0	0	15	1995	1 5	TH 5	1	44	51	38.35	-93	11	38.79	1	7	7	7	
02213	44.8809056	-93.616	24	45	45	0	20	10	0	15	2007	1 5	LAKE VIRGINIA TRAIL	14000	14	44	52	51.26	-93	17	53.76	13	N	N	N
02214	44.8808133	-93.538868	24	45	45	0	20	10	0	15	2012	1 5	PED UNDERPASS	9000	14	44	50	12.60	-93	19	19.07	1	8	7	7
02215	44.8115528	-93.6021278	24	45	45	0	20	10	0	15	2001	1 41	CHESAPEAKE	16500	16	44	48	41.39	-93	36	07.46	13	N	N	N
02216	44.8114833	-93.701212	24	45	45	0	20	10	0	15	2004	1 41	TH 5	16600	16	44	51	05.34	-93	36	17.87	13	N	N	N
02217	44.8597528	-93.6658778	24	35	35	0	20	0	0	15	2011	1 41	PED TRAIL	24700	16	44	51	35.11	-93	39	57.16	13	N	N	N
02218	44.8774694	-93.624044	24	45	45	0	20	10	0	15	2012	1 41	PED TRAIL	14000	16	44	52	38.89	-93	35	13.36	13	N	N	N
02219	44.8587858	-93.8071361	24	45	45	0	20	10	0	15	2015	1 41	PEDESTRIAN	17000	16	44	50	08.73	-93	41	08.73	13	N	N	N
02220	45.3908086	-92.8191139	24	45	45	0	20	10	0	15	2005	1 4	CENTER LAKE CHANNEL	14360	08	45	23	24.29	-93	47	08.81	26	6	8	7
13016	45.4510028	-92.9989806	48	35	55	0	20	0	0	15	2006	1 35	CSAH 17	1300	02	45	27	03.61	-92	59	21.77	1	7	7	7
13017	45.4517878	-92.9922389	48	35	55	0	20	0	0	15	1967	1 35	CSAH 10	360	08	44	24	36.50	-92	59	14.44	1	7	7	7
13018	45.6155056	-92.9923861	48	45	65	0	20	0	0	25	1967	1 35	CSAH 9	255	08	45	36	55.82	-92	59	32.59	1	6	6	5
13019	45.7197306	-92.9922139	48	35	55	0	20	0	0	15	1968	1 35	CSAH 3	190	08	45	43	0.23	-92	59	31.97	1	6	6	7
13020	45.6705956	-92.9923861	48	35	55	0	20	0	0	15	1970	1 35	CSAH 2	400	08	44	49	20.00	-92	59	35.44	1	6	6	5
13021	45.3751222	-92.8854917	24	45	45	0	20	10	0	15	2003	1 5	PED 8	16500	02	45	22	30.44	-92	53	07.77	13	N	N	N
13022	44.7468833	-92.815111	24	25	45	0	10	0	0	15	2013	1 61	MISS RZND STN LOOP RD	29500	14	44	44	48.60	-92	51	11.20	24	6	8	6
13023	44.6012611	-92.8913511	24	35	55	0	20	0	0	15	1962	1 5	US 52	4250	06	44	54	6.54	-92	48	36.28	1	7	7	7
13024	44.8197694	-93.241222	48	35	55	0	20	0	0	15	1978	1 5	UP RR	47000	12	44	49	11.17	-93	18	26.84	1	7	6	6
13025	44.8097939	-93.061611	48	35	55	0	20	0	0	15	1972	1 5	WENTWORTH AVE	890	12	44	53	52.67	-93	03	58.18	1	7	7	7
13026	44.821806	-93.061611	48	35	55	0	20	0	0	15	1992	1 5	Barrae Ave CSAH 73	490	16	49	30.65	-93	18	26.84	1	7	6	6	
13027	44.9175528	-93.0618194	35	55	0	20	0	0	0	15	1973	1 5	PED @ Lewis St	1	44	55	03.39	-93	03	49.75	1	7	7	7	
13028	44.6612056	-93.0274917	48	45	45	0	20	10	0	15	1978	1 5	Vermillion River	31750	02	44	39	41.42	-93	0	26.97	1	6	6	6
13029	44.7249139	-92.812472	48	45	45	0	20	10	0	15	1978	1 5	VERMILION RIVER	31750	14	44	39	41.42	-93	0	26.97	1	6	6	6
13030	44.796	-93.0818472	48	35	35	0	20	0	0	15	1978	1 5	UP RR	US 52 SB	2	23000	14	44	47	47.76	-93	1	7	7	6
13031	44.8015417	-93.0824978	48	35	35	0	20	0	0	15	1978	1 5	UP RR	US 52 NB	2	23000	14	44	47	47.76	-93	1	7	7	6
13032	44.8844528	-93.079803	48	35	55	0	20	0	0	15	1990	1 5	EBN STREET	1500	19	44	48	6.99	-93	2	21.94	1	7	7	7
13033	44.																								

27111	44.8927194	93.1841056	unmap	48	45	65	0	20	0	0	25	1971	TH S WB ON RAMP	CHAPEL ROAD	500	19	44	53	33.79	-93	11	02.78	1	5	6	6
27112	45.1222522	93.1031078	unmap	24	35	55	0	10	10	0	15	1998	TH E10	TH S22 NB ON RAMP	9000	14	45	07	21.08	-93	18	36.64	7	7	7	7
27113	45.1232972	93.3061816	unmap	24	35	55	0	10	10	0	15	1998	TH E10	TH S22 NB ON RAMP	9000	14	45	07	21.08	-93	18	36.64	7	7	7	7
27114	45.1200472	93.3080583	unmap	24	35	55	0	10	10	0	15	1998	TH S22 NB ON RAMP	TH S23 SB	23500	14	45	07	29.01	-93	18	29.01	7	7	8	8
27115	45.1202522	93.4049139	unmap	48	35	55	0	20	0	0	15	1997	TH E10	Regent Avenue	1850	19	45	07	20.29	-93	17	20.29	1	7	7	6
27116	45.1382	93.4649417	unmap	24	35	55	0	20	0	0	15	2016	TH E10	ELM CREEK BLVD	194	14	45	08	17.52	-93	17	17.52	1	6	6	6
27117	45.1316417	93.4424222	unmap	48	35	55	0	20	0	0	15	2005	TH E10	TH S15 S	940	19	45	07	53.91	-93	26	32.72	1	6	6	7
27118	45.0035889	93.4473484	unmap	24	35	55	0	20	0	0	15	2014	BNSF RR	BNSF RR	1	44	50	00	12.00	-93	14	30.44	5	6	8	8
27119	44.94	93.3426594	unmap	48	35	55	20	0	0	0	15	1984	BNSF CP RAIL	TH S15 S	23000	12	44	58	02.40	-93	18	18.57	6	6	6	6
27120	45.1295139	93.2878444	unmap	24	35	55	0	20	0	0	15	1985	Mississippi River	TH E10 WB	43500	14	45	07	46.25	-93	17	46.25	1	6	7	7
27121	45.118015	93.3426594	unmap	24	35	55	0	20	0	0	15	2010	TH E10	HIMMELCK LANE	1	19	45	07	45.34	-93	17	45.34	1	6	7	7
27122	45.1217604	93.3000134	unmap	48	35	55	0	20	0	0	15	1985	TH E10	West River Road	2850	17	45	07	39.61	-93	17	39.61	1	6	6	6
27123	45.1377056	93.4022028	unmap	24	35	55	20	0	0	0	15	2017	FERNBROOK LANE	TH E10 WB	23500	11	45	08	15.74	-93	27	43.93	1	8	8	8
27124	45.1375506	93.4022028	unmap	24	35	55	20	0	0	0	15	2017	FERNBROOK LANE	TH E10 WB	23500	11	45	08	15.74	-93	27	43.93	1	8	8	8
27125	45.1318141	93.4242222	unmap	24	35	55	0	20	0	0	15	TH101	FERNBROOK LANE	TH E10 WB	23500	11	45	08	15.20	-93	27	43.96	1	8	8	8
27126	45.1304444	93.4120722	unmap	24	35	55	0	20	0	0	15	TH101	ZACHARY LANE	TH E10	16	45	07	33.51	-93	17	33.51	1	8	8	8	
27127	45.1305667	93.4071813	unmap	24	35	55	0	20	0	0	15	2010	TH E10	REVERE LANE	1	17	45	07	43.46	-93	24	43.46	1	8	8	8
27128	45.1292889	93.2885917	unmap	24	35	55	20	0	0	0	15	1999	Mississippi River	TH E10 EB	43500	12	45	07	45.43	-93	17	45.43	1	7	8	7
27129	44.9001694	93.5404028	unmap	24	35	55	0	20	0	0	15	2001	RECREATION TR TROLLEY	TH 7 WB Connection	1	16	44	54	05.83	-93	33	37.45	1	8	8	8
27130	44.9007083	93.5404028	unmap	48	35	55	0	20	0	0	15	2000	CP RR, TH E10	TH 100	30000	12	44	58	50.55	-93	27	53.05	1	7	7	7
27131	44.9500611	93.4006917	unmap	24	35	55	0	20	0	0	15	1999	BNSF RR	US 169 SB	48000	12	44	57	02.27	-93	24	02.49	1	8	8	7
27132	44.9960472	93.4384778	unmap	24	45	45	20	10	0	0	15	1998	UP RR	TH 55 EB	18500	14	44	59	4.72	-93	26	18.52	1	6	8	7
27133	44.9879933	93.3494265	unmap	24	35	55	0	20	0	0	15	2000	UP RR, WIDE DRIVE	TH 100	80000	12	44	59	16.74	-93	27	57.63	1	7	8	7
27134	45.0423194	93.3303139	unmap	24	35	55	0	20	0	0	15	2003	TH 100	FRANCE AVENUE	4850	17	45	02	32.35	-93	19	49.13	1	7	8	7
27135	45.0442278	93.3289217	unmap	48	35	55	0	20	0	0	15	2002	TH 100	CP Railroad	1	45	02	39.22	-93	19	41.85	1	7	6	7	
27136	45.0320389	93.3497409	unmap	24	35	55	0	20	0	0	15	2000	TH 100	RED AT 39th Ave	1	45	01	34.10	-93	20	50.93	1	8	8	6	
27137	45.0349083	93.3445972	unmap	24	35	55	0	20	0	0	15	2001	TH 100, RAMPS	BNSF RR	1	45	02	05.67	-93	20	40.55	1	8	8	7	
27138	45.0389833	93.3491933	unmap	24	35	55	0	20	0	0	15	2001	Two Lake Channel	TH 100	71000	12	45	02	20.16	-93	20	20.88	1	7	8	7
27139	44.9017194	93.1403975	unmap	24	35	55	0	20	0	0	15	2010	TH 17	DAKOTA STATE REG TR	1	54	44	06.19	-93	21	31.11	1	8	8	8	
27140	45.1955583	93.5521189	unmap	24	35	55	20	0	0	0	15	2010	DIAMOND LAKE ROAD	194 OFF RAMP	47000	12	45	11	58.41	-93	33	57.23	1	8	8	7
27141	44.9448917	93.3481444	unmap	24	35	55	0	20	0	0	15	2015	TH 100	CAMST, P RAMP	1	44	56	27.21	-93	20	53.32	5	8	8	7	
27142	44.9410281	93.3481444	unmap	24	35	55	0	20	0	0	15	2015	TH 100	CEAR LAKE TRAIL	1	56	44	27.89	-93	56	53.24	1	8	8	8	
27143	49.009667	93.2750167	unmap	24	35	55	20	0	0	0	15	2008	MINNEHAWA PFWY AND CREEK	135V SB	85500	11	44	54	12.28	-93	16	30.06	1	8	8	7
27144	44.9091975	93.2745972	unmap	24	35	55	20	0	0	0	15	2008	MINNEHAWA PFWY; CREEK	135V NB	85500	11	44	54	13.03	-93	16	28.55	1	8	8	7
27145	44.9501389	93.4004344	unmap	48	35	55	0	20	0	0	15	1963	BNSF RR	BNSF RR	48000	12	57	4	5.90	-93	47	3.90	1	7	7	6
27146	44.8889556	93.3389194	unmap	48	35	55	0	20	0	0	15	1963	TH 62, W 64TH ST	PEDESTRIAN	1	44	53	13.04	-93	20	20.11	1	7	6	7	
27147	44.8984417	93.2114861	unmap	48	45	65	0	20	0	0	25	1966	TH 62	TH 62	1550	17	44	53	54.39	-93	12	41.55	1	6	6	5
27148	44.8984417	93.2114861	unmap	48	45	65	0	20	0	0	25	1966	TH 62	TH 62	1550	17	44	53	54.39	-93	12	41.55	1	6	6	5
27149	44.8984417	93.2114861	unmap	48	45	65	0	20	0	0	25	1966	BLOOMINGTON AVE N	UP RAMP	1	44	53	17.20	-93	44	30.28	1	6	6	6	
27150	44.9918778	93.4007722	unmap	48	35	55	20	0	0	0	15	1968	UP RR	US 169	86000	12	44	59	30.58	-93	24	02.78	1	6	6	6
27151	45.0494472	93.4008261	unmap	48	35	55	0	20	0	0	15	1969	US 169, E W FRONTAGE RD	CP RAIL	1	45	02	56.21	-93	24	03.19	1	6	6	6	
27152	45.0150058	93.4012	unmap	48	45	45	20	10	0	0	15	1976	US 169 S	US 169 S	4550	14	44	45	48.61	-93	54	48.61	1	6	6	6
27153	44.891889	93.4348089	unmap	48	45	45	20	10	0	0	15	1986	Nine Mile Creek	TH 62 WB	14750	12	44	53	31.88	-93	26	02.72	8	5	7	7
27154	44.891889	93.4348089	unmap	48	45	45	20	10	0	0	15	1986	TH 62	TH 62	14750	12	44	53	31.88	-93	26	02.72	8	5	7	7
27155	44.921194	93.4016722	unmap	48	35	55	0	20	0	0	15	1978	EXCELSIOR, 3RD ST	US 169	83000	12	44	55	23.23	-93	24	06.02	1	6	7	6
27156	44.9838361	93.3036444	unmap	24	35	55	0	20	0	0	15	1978	MN 55	TH 100, 3RD ST	1	44	59	52.40	-93	22	51.40	1	7	8	7	
27157	45.0021275	93.3045428	unmap	24	35	55	0	20	0	0	15	1980	TH 100, 3RD ST RAMP	TH 100, 3RD ST RAMP	1	44	53	41.83	-93	21	41.83	1	7	8	7	
27158	44.788625	93.3987583	unmap	48	35	55	20	0	0	0	15	1993	Minnesota River	US 169 NB	31500	12	44	47	55.05	-93	23	55.53	1	7	7	5
27159	44.7907944	93.3910083	unmap	48	35	55	20	0	0	0	15	1993	Minnesota River	US 169 SB	31500	12	44	47	55.66	-93	23	56.79	1	7	7	5
27160	44.7907944	93.3910083	unmap	48	35	55	20	0	0	0	15	1993	US 169 NB ON RAMP	US 169 NB ON RAMP	2000	14	47	25.75	-93	24	25.75	1	7	7	4	
27161	44.8053361	93.3886444	unmap	48	35	55	0	20	0	0	15	1993	Ramp 108th Street	US 169 SB on ramp	500	12	44	48	19.21	-93	40	55.12	1	7	7	7
27162	44.7910722	93.405375	unmap	48	35	55	0	20	0	0	15	1993	SB 169 S EB TH 13	US 169 SB on ramp	1000	12	44	47	27.86	-93	24	19.85	1	7	7	7
27163	44.7911813	93.4054612	unmap	48	35	55	0	20	0	0	15	1993	County 101 WB	US 169 SB ON RAMP	1	44	47	28.26	-93	24	28.26	1	7	7	7	
27164	45.0561972	93.3111639	unmap	48	35	55	0	20	0	0	15	1983	TH 100	Pedestrian Bridge	1	45	03	47.39	-93	18	47.39	1	7	6	6	
27165	45.1092417	93.3102694	unmap	24	35	55	0	20	0	0	15	2003	TH 202	TH 202 @ 8th Ave	1	45	06	23.31	-93	18	23.31	1	7	6	6	
27166	44.9792922	93.2817813	unmap	48	45	45	0	20	0	0	15	1985	LINDSEY	LINDSEY	3000	16	44	53	33.32	-93	24	33.32	1	7	8	8
27167	44.9746222	93.2812806	unmap	48	35	55	0	20	0																	

27867	45.10825	-93.4665917	amdp	48	35	35	20	0	0	0	15	1969	Rice Lake Channel	I 94 WB	56000	11	45	06	29.70	-93	27	59.73	1	7	6	7	
27868	45.1001556	-93.4675983	amdp	48	35	35	20	0	0	0	15	1969	Rice Lake Channel	I 94 EB	56000	11	45	06	29.30	-93	28	0.33	1	7	6	7	
27869	45.1100444	-93.4748651	amdp	48	35	35	20	0	0	0	15	1969	Elm Creek	I 94 WB	56000	11	45	06	37.76	-93	28	57.76	1	7	6	7	
27870	45.1159167	-93.4779611	amdp	48	35	35	20	0	0	0	15	1969	Elm Creek	I 94 EB	56000	11	45	06	27.36	-93	28	39.94	1	7	6	7	
27885	44.98265	-93.5022809	amdp	48	35	55	20	0	0	0	15	1973	135W, NB OFF RAMP	PED @ SWANBER ST	1	14	44	48	37.87	-92	14	17.60	1	6	7	8	
27887	44.984667	-93.49611	amdp	48	35	55	20	0	0	0	15	1973	135W, OFF-ON RAMP	PED @ 5TH ST E	1	14	47	59	38.03	-93	14	31.66	8	6	7	8	
27890	44.9879111	-93.2371306	amdp	48	35	55	20	0	0	0	15	1969	135W, RAMP, FRONT PDS	BNSF RR	1	44	59	16	48	-93	14	1.63	1	7	6	7	
27892	44.9872083	-93.2122861	amdp	48	35	55	20	0	0	0	15	1994	134 EB OFF RAMP	I 94 EB OFF RAMP	4000	11	44	48	10.45	-93	13	16.48	1	7	6	7	
27893	44.9901819	-93.217772	amdp	48	35	55	20	0	0	0	15	1973	135W, NB OFF RAMP	BNSF RR	1	44	59	16	48	-93	13	32.88	1	7	6	7	
27915	44.8456306	-93.3984306	amdp	48	35	35	20	0	0	0	15	1995	South Anderson Lakes	US 169 NB	40000	14	44	58	00.29	-93	23	14.35	1	7	7	8	
27916	44.8458111	-93.3987278	amdp	48	35	35	20	0	0	0	15	1995	South Anderson Lakes	US 169 SB	40000	14	44	58	00.03	-93	23	35.42	1	7	7	8	
27917	44.8452389	-93.3941417	amdp	48	35	35	20	0	0	0	15	1995	North Anderson Lakes	US 169 NB	40000	14	44	58	00.11	-93	23	55.42	1	7	7	8	
27918	44.8527333	-93.3963167	amdp	48	35	35	20	0	0	0	15	1995	North Anderson Lakes	US 169 SB	40000	14	44	58	00.34	-93	23	64.74	1	7	7	8	
27924	45.0080206	-93.3793078	amdp	48	35	55	20	0	0	0	15	1994	FSD LAKE RD	FSD LAKE RD	1	44	51	30.39	-93	26	48.23	1	6	7	8		
27925	45.0080206	-93.3949277	amdp	48	35	55	20	0	0	0	15	2003	TH 100, I RET RD	PED (BARRETT CRK)	1	45	00	11.23	-93	26	49.47	1	7	7	8		
27926	45.1629139	-93.3915028	amdp	48	35	55	20	0	0	0	15	2004	US 169	PIED	1	45	09	46.49	-93	23	57.47	1	7	7	8		
27927	45.0215861	-93.4007906	amdp	24	35	55	20	0	0	0	15	2008	US 169	PED/BIKE	1	45	01	18.79	-93	25	02.81	1	8	7	8		
27928	44.8589833	-93.2981333	amdp	24	35	55	20	0	0	0	15	2004	135W, HUMBOLDT, BLOOM	79TH 80TH ST	1600	17	44	51	12.34	-93	17	53.28	1	7	8	7	
27929	45.0377389	-93.3406411	amdp	24	35	55	20	0	0	0	15	2001	FROM CSAN B1	TH 100 NB ON RAMP	1000	19	44	52	15.86	-93	20	25.66	1	8	7	7	
27930	44.9020094	-93.3412833	amdp	24	35	45	20	0	0	0	15	2002	SR MILC CREEK	TH 7	15400	14	54	44	03.67	-93	24	03.66	1	7	8	7	
27938	45.041472	-93.331222	amdp	24	35	55	20	0	0	0	15	2003	DNB WETLAND FRANCIE AVE	NB TH 100 OFF RMP	1000	12	45	02	29.21	-93	19	52.4	1	8	7	7	
27910	45.138878	-93.4478389	amdp	24	35	55	20	0	0	0	15	2007	ELM CREEK	TH 810 E	75000	18	45	08	1.78	-93	26	52.22	1	8	8	8	
27912	45.1334937	-93.4407927	amdp	24	35	55	20	0	0	0	15	2005	WET CREEK	PED/STRAW H HGO	1	45	08	07.77	-93	28	55.67	1	7	8	8		
27918	45.1060139	-93.3953083	amdp	24	35	55	20	0	0	0	15	2008	CSAN B1, RAMP, BNSF RR	US 169 SB	58000	12	45	06	21.65	-93	23	36.63	1	8	8	8	
27922	45.1090361	-93.3983694	amdp	24	35	55	20	0	0	0	15	2008	CSAN 109, CSAN 81	RAMP (69 SB TO 81)	1	12	45	06	32.53	-93	23	36.85	1	7	8	8	
27928	44.45259	-93.2866444	amdp	48	35	55	20	0	0	0	15	2011	1494, RAMPS	WASHINGTON AVE	1	44	51	32.48	-93	23	55.12	1	7	8	7		
27930	44.8500444	-93.4893222	amdp	24	35	55	20	0	0	0	15	2006	US 212	PED/BIKE	1	44	51	0.16	-93	29	38.12	5	8	8	7		
27931	44.8478639	-93.5166439	amdp	24	35	35	20	0	0	0	15	2006	RILEY CREEK	US 212 WB	36500	14	44	50	53.21	-93	30	99.27	1	8	8	8	
27932	44.84765	-93.5169322	amdp	24	35	35	20	0	0	0	15	2006	RILEY CREEK	US 212 EB	36500	14	50	54	51.54	-93	30	58.8	1	8	7	8	
27937	44.9525283	-93.4596333	amdp	24	35	35	20	0	0	0	15	1998	BNSF RR, STONE RD	I 494 SB	45000	11	44	57	09.57	-93	27	34.68	1	7	8	7	
27938	44.9525283	-93.4599722	amdp	24	35	35	20	0	0	0	15	1998	BNSF RR, STONE RD	I 494 NB	45000	11	44	57	09.59	-93	27	33.74	1	7	8	7	
27939	45.0239317	-93.4236667	amdp	24	35	55	20	0	0	0	15	2006	1494	US 212	91000	15	44	51	42.21	-93	31	29.60	1	6	7	7	
27928	44.9658833	-93.2496806	amdp	24	45	45	20	10	0	0	15	2000	LRT, FRONT ROAD	I 94 EB ON RAMP	13000	11	44	57	15.85	-93	14	58.85	1	7	8	7	
27944	44.8597978	-93.2428861	amdp	24	35	55	20	0	0	0	15	2003	I 494, ON RAMP	CP RAIL	1	44	51	14.48	-93	21	45.67	5	8	8	7		
27945	44.8601869	-93.2827444	amdp	24	35	55	20	0	0	0	15	2002	1494	WB USH LAKE ROAD	3950	17	51	44	36.68	-93	14	36.68	1	8	7	8	
27943	45.0782056	-93.3738133	amdp	24	35	35	20	0	0	0	15	2001	CSAN B1, BNSF RR	I 94 WB	49000	11	45	04	21.22	-93	22	25.80	1	7	8	7	
27944	45.0779944	-93.3737944	amdp	24	35	35	20	0	0	0	15	2001	CSAN B1, BNSF RR	I 94 EB	49000	11	45	04	40.78	-93	22	25.66	1	7	8	8	
27951	44.8987389	-93.4457028	amdp	24	35	35	20	0	0	0	15	2005	CP RAIL	I 494 SB	46000	11	53	44	55.46	-93	25	45.86	1	7	8	8	
27952	44.8988028	-93.4454417	amdp	24	35	35	20	0	0	0	15	2005	CP RAIL	I 494 NB	46000	11	44	53	55.69	-93	26	43.59	1	7	8	8	
27953	44.9023056	-93.4471444	amdp	24	35	35	20	0	0	0	15	2005	HENN CD TRAIL	I 494 SB	82000	14	44	54	08.20	-93	26	49.72	1	7	8	7	
27954	44.9023194	-93.4469619	amdp	24	35	35	20	0	0	0	15	2005	HENN CD TRAIL	I 494 NB	82000	14	44	54	09.07	-93	26	49.14	1	7	8	7	
27957	44.9114278	-93.4492111	amdp	24	35	55	20	0	0	0	15	2005	I 494	PED AT MARWOOD LN	1	44	54	41.14	-93	26	57.16	5	7	8	8		
27958	44.9346361	-93.4534889	amdp	24	35	55	20	0	0	0	15	2005	I 494	ORCHARD ROAD	1	19	44	56	04.69	-93	27	19.76	1	6	8	8	
27965	44.8894147	-93.2958189	amdp	24	35	55	20	0	0	0	15	2007	135W	MIN 62	6000	12	44	53	6009	-93	23	20.39	1	8	7	8	
27967	44.8904722	-93.2957694	amdp	24	35	55	20	0	0	0	15	2007	MM 121	MIN 62 WB	1	12	44	53	25.70	-93	17	44.77	1	8	8	7	
27968	44.8897528	-93.2938528	amdp	24	35	55	20	0	0	0	15	2007	135W, MM 121, RAMP	MIN 62 EB	1	12	44	53	23.11	-93	17	37.87	1	7	8	7	
27969	44.8909222	-93.29292	amdp	24	35	55	20	0	0	0	15	2007	TH 121	RAMP (121 TO E20)	1	12	44	53	26.24	-93	17	36.34	1	8	7	7	
27970	44.8905639	-93.2884444	amdp	24	35	55	20	0	0	0	15	2007	LYNDALE AVE SO	MIN 62 WB	1	12	44	53	26.03	-93	17	26.03	1	8	8	8	
27971	44.8890928	-93.2882322	amdp	24	35	55	20	0	0	0	15	2007	LYNDALE AVE SO	MIN 62 EB	1	12	44	53	23.65	-93	17	17.96	1	8	8	8	
27974	44.8909289	-93.28321	amdp	24	35	55	20	0	0	0	15	2007	CP RAIL	I 94	156000	14	44	53	24.23	-93	17	18.50	1	8	7	8	
27976	44.8909056	-93.2758278	amdp	24	25	45	0	10	0	0	15	2007	I 35W, NICOLLT AVE S	TH 62 WB	49000	12	44	53	23.96	-93	16	32.98	7	7	8	7	
27978	44.8899917	-93.2782056	amdp	24	35	55	20	0	0	0	15	2008	NICOLLT AVE SO	MIN 62 EB	1	12	44	53	27.97	-93	16	41.18	1	7	8	7	
27979	44.8907667	-93.2745417	amdp	24	35	55	20	0	0	0	15	2008	I 35W, MIN 62 WB	RAMP (150W TO 62EB)	28	14	44	26.76	-93	14	26.76	1	8	7	7		
27980	44.894275	-93.2753972	amdp	24	35	55	20	0	0	0	15	2008	I 94	60 S (150S 271)	RAMP (150WB TO 62)	1	11	44	53	39.39	-93	16	31.43	1	8	8	7
27984	44.8501583	-93.3189972	amdp	24	35	55	20	0	0	0	15	2011	I 494, US 169, MARTY RD	1494WB TO US169WB	1	11	44	53	11.29	-93	23	38.03	1	7	8	7	
27996	44.8627833	-93.3615628	amdp	24	35	55	20	0	0	0	15	2012	WASHINGTON AVE	1494WB TO US169WB	1	35	11	44	51								

62002	44.9401111	93.1152861	amvb	48	35	55	0	20	0	0	0	15	1985	13SE, SB ON RAMP	GRAND AVE EB	632	19	44	56	24.4	-93	06	55.03	1	5	6	7	
62004	44.9442056	93.1078028	amvb	48	35	55	0	20	0	0	0	15	1987	13SE THOMPSON ST	PED AT WALNUT ST	1	44	56	20.4	-93	06	28.09	1	6	6	6		
62005	44.9517831	93.1154121	amvb	48	35	55	0	20	0	0	0	15	2009	194, EB ON RAMP	GRIGGS ST	1	44	57	06.42	-93	06	06.42	1	8	6	7		
62016	44.9581833	93.1199372	amvb	48	35	55	0	20	0	0	0	15	1965	194, EB ON RAMP	CP RAIL	1	44	57	33.06	-93	11	33.06	1	7	6	5		
62021	45.0101294	93.2024937	amvb	48	35	55	0	20	0	0	0	15	1970	TH 280 SB	TH 280 SB OFF RAMP	1	2325	16	45	00	26.79	-92	12	08.97	1	6	7	7
62022	45.0314939	93.0316618	amvb	48	35	55	0	20	0	0	0	15	1966	194	RECREATION TRAIL	2	1966	45	00	14.69	-93	08	10.19	1	6	6	6	
62033	45.1046417	93.0550025	amvb	48	35	55	0	20	0	0	0	15	1967	13SE	Co Rd H2(CSAH 5)	4550	16	45	06	16.71	-93	03	07.73	1	6	7	6	
62034	44.9490967	93.2021472	amvb	48	35	55	0	20	0	0	0	15	1966	TH 280, RAMPS	MC RAIL	1	45	57	39.46	-93	12	07.71	5	6	6	5		
62044	44.9610472	93.2002722	amvb	48	35	55	0	20	0	0	0	15	1966	TH 280, RAMPS	WABASH AVE	3200	19	44	57	07.46	-93	08	07.46	1	6	6	5	
62045	44.9528056	93.1024944	amvb	48	35	55	0	20	0	0	0	15	1966	194	PRIOR AVE	3700	17	44	57	10.20	-93	10	16.98	1	7	6	6	
62046	44.9513939	93.1176533	amvb	48	35	65	0	20	0	0	0	15	1967	194	CP RAIL	1	44	57	35.70	-93	10	35.70	1	5	6	5		
62047	44.9513939	93.1176533	amvb	48	35	65	0	20	0	0	0	15	1966	194	UHFRR	1	44	57	06.98	-93	10	06.98	1	5	6	5		
62049	44.9518306	93.1155556	amvb	48	35	65	0	20	0	0	0	25	1966	194	PED AT ALDINE	1	44	57	05.59	-93	10	17.60	1	5	6	6		
62050	44.9462233	93.1100055	amvb	48	35	55	0	20	0	0	0	15	1975	MET COUNCIL LASEMENT	NW LOCK 1316 E SB	1	11	58	34.18	-93	11	24.18	1	7	7	6		
62051	44.9517639	93.0573133	amvb	48	35	55	0	20	0	0	0	15	2013	Earl Street	Earl Street	2200	17	44	26.54	-93	03	06.57	1	6	6	7		
62061	44.9295833	93.1438222	amvb	48	35	55	0	20	0	0	0	15	1971	Ayd Mill Rd	13SE SB OFF RAMP	4000	11	44	55	4.50	-93	08	37.76	1	7	7	7	
62062	44.9519544	93.0416167	amvb	48	35	55	0	20	0	0	0	15	2010	194	PED AT CHARLOTTE	1	44	57	06.30	-93	08	06.30	1	8	8	7		
62063	44.9519056	93.0625833	amvb	48	35	55	0	20	0	0	0	15	1973	194, HUDSON, PACIFIC	PED AT MAPLE	1	44	57	06.86	-93	03	45.30	1	6	6	6		
62072	44.9218417	93.1484444	amvb	48	35	55	0	20	0	0	0	15	1984	13SE	PED AT Bayard Ave	1	44	55	18.63	-93	08	41.44	1	7	7	7		
62074	44.85467	93.07323	amvb	48	35	45	0	20	0	0	0	15	1989	BNSF RR, STUYVEN CONN	194 WB OFF RAMP	5750	11	44	57	41.52	-93	04	41.52	1	7	6	6	
62083	44.9484111	93.1053472	amvb	48	35	55	0	20	0	0	0	15	1988	194 connection	Mulberry St	50	19	44	56	54.28	-93	06	18.89	1	7	7	7	
62089	44.9514111	93.0983772	amvb	48	35	55	0	20	0	0	0	15	1991	13SE, NB OFF RAMP	CEDAR ST	4900	17	44	57	05.08	-93	05	54.14	1	7	6	6	
62090	45.0646061	93.2055722	amvb	24	35	55	0	20	0	0	0	15	2005	BNSF RR, STUYVEN CONN	135W, 135W/SB ON RAMP	1	17	03	45	52.87	-93	11	10.85	1	8	8	8	
62092	44.9517361	93.1207056	amvb	24	35	55	0	20	0	0	0	15	2016	194	PED at MacLubin St	1	44	57	06.25	-93	07	14.54	1	8	8	8		
62094	44.948885	93.1032417	amvb	48	35	55	0	20	0	0	0	15	1988	13SE, NB OFF RAMP	10TH STREET	4000	16	44	56	53.86	-93	06	08.43	1	7	6	6	
62095	44.9090889	93.0878389	amvb	48	35	45	0	20	0	0	0	15	2006	BNSF RR	BANKERS BUILDING	2100	44	53	37.56	-93	00	28.22	1	7	8	8		
62097	45.0347833	93.0893194	amvb	24	35	35	20	0	0	0	0	15	2006	13SE, 1694, BNSF RR	135 E NB OFF RAMP	23000	11	45	02	05.22	-93	05	21.19	1	7	8	6	
62098	45.0370722	93.0909139	amvb	24	35	35	20	0	0	0	0	15	2006	BNSF RR	1694 WB	47000	11	45	02	05.46	-93	05	27.29	1	7	8	6	
62099	45.0376167	93.0978389	amvb	24	35	35	20	0	0	0	0	15	2006	1694, RAMPS	US 6	2000	11	45	02	15.42	-93	05	28.32	1	8	8	6	
62101	44.9046439	93.1391361	amvb	24	35	35	20	0	0	0	0	15	2001	13SE	MISSISSIPPI, UP RR	87000	11	44	54	16.07	-93	08	20.89	1	7	8	7	
62114	44.9038906	93.2014972	amvb	48	35	35	20	0	0	0	0	15	2006	BNSF RR	1694 EB	47000	11	45	02	13.13	-93	05	29.39	1	8	7	8	
62115	44.9612361	93.0910017	amvb	48	35	35	20	0	0	0	0	15	2014	BNSF RR, MINNEAPOLIS AVE	135 NB	47000	11	57	48.21	-93	07	24.78	1	7	7	7		
62121	44.9615083	93.0909194	amvb	24	35	35	20	0	0	0	0	15	2014	BNSF RR, MINNEAPOLIS AVE	135 SB	69000	11	44	57	48.63	-93	05	27.31	1	8	8	8	
62124	44.966725	93.0890806	amvb	24	35	35	20	0	0	0	0	15	2014	CAVIGAS ST, BNSF RR	135 NB	69000	11	44	58	0.21	-93	11	7	8	7			
62125	44.9667256	93.0890806	amvb	24	35	35	20	0	0	0	0	15	2015	BNSF RR, CAVIGAS ST	135 NB	69000	11	44	58	0.32	-93	08	29.96	1	8	8	7	
62127	45.0155556	93.0927222	amvb	24	45	45	20	0	0	0	0	15	2008	GATEWAY TRAIL	MARGARET ST	5806	19	45	00	56	-92	59	32.18	13	N	N	N	
62029	44.9088094	93.142975	amvb	48	35	45	20	0	0	0	0	15	2001	Ped Trail	13SE	77000	19	44	54	11.93	-93	11	13	N	N	N		
62030	44.9581833	93.0828218	amvb	48	35	45	20	0	0	0	0	15	2012	GATEWAY TRAIL	MARGARET AVE	19602	16	44	58	36.63	-93	11	11.63	13	N	N	N	
62064	45.0430566	93.2054222	amvb	48	45	45	20	0	0	0	0	15	1955	ST CROIX RIVER	US 8 Taylors Falls	9993	02	45	24	06.86	-92	39	01.52	1	7	6	6	
62092	45.0502278	93.115105	amvb	48	45	45	20	0	0	0	0	15	1958	1694, CO PD E	CP RAIL	1	45	24	0.82	-93	06	54.18	6	7	6	6		
62062	45.0201694	93.2041344	amvb	48	45	45	20	0	0	0	0	15	1957	135 W	US 6	11500	16	44	59	45.03	-93	49	45.03	1	7	6	7	
62092	44.9962222	93.0658861	amvb	48	55	55	20	0	0	0	0	25	1951	PED-BIKE TRAIL	US 61	15100	16	44	59	46.40	-93	05	57.55	9	6	6	5	
62094	44.9997978	93.0478861	amvb	48	45	45	20	0	0	0	0	15	1952	UP RR	TH 55 WB	17000	14	44	59	45.52	-93	26	16.75	1	7	6	7	
62062	44.9987861	93.3888083	amvb	24	35	35	20	0	0	0	0	15	1951	BASSETT CREEK	TH 55 EB	31000	15	44	59	31.00	-93	04	05.93	1	7	8	8	
62092	45.3785667	93.2361361	amvb	24	35	35	20	0	0	0	0	15	1951	Cedar Creek	TH 65	21500	14	45	22	42.84	-93	14	10.09	13	N	N	N	
62094	44.6721611	93.6350778	amvb	48	35	35	20	0	0	0	0	15	1954	SAND CREEK	US 169	22400	12	44	40	19.78	-93	38	06.28	1	7	7	7	
62092	44.6721611	93.6350778	amvb	48	35	35	20	0	0	0	0	15	1955	UP RR	US 169	22400	12	44	40	25.52	-93	38	06.28	1	7	7	7	
62094	45.2327528	93.234925	amvb	48	45	45	20	0	0	0	0	15	1953	COON CREEK	TH 65 SB	16250	14	45	13	57.91	-93	14	05.73	1	6	6	6	
62064	44.6687083	93.2446139	amvb	48	45	45	20	0	0	0	0	15	1953	SAND CREEK	TH 282	8000	06	44	40	07.35	-93	38	04.61	1	6	6	6	
62064	44.9714717	93.0216889	amvb	48	45	45	20	0	0	0	0	15	1959	BNSF RR, ENERGY DR	TH 282	47000	14	44	58	37.73	-93	08	09.68	1	6	6	7	
62068	45.0425667	93.5223611	amvb	24	45	45	20	0	0	0	0	15	1940	Robert Creek	TH 55	31050	12	45	2	33.24	-93	11	04.25	13	N	N	N	
62068	44.8014667	93.7072111	amvb	24	45	45	20	0	0	0	0	15	1955	ELM CREEK	US 169	14600	12	44	36	03.28	-93	40	26.56	13	N	N	N	
62068	44.8014667	93.6478722	amvb	24	45	45	20	0	0	0	0	15	1900	ROBERT CREEK	DITCH	31500	12	44	36	01.94	-93	40	21.84	13	N	N	N	
20006	44.785725	93.4017639	amvb	48	35	35	20	0	0	0	0	15	1991	UP RR	TH 901B EB	25000	14	44	47	08.61	-93	14	06.12	1	6	7	7	
20008	44.7842	93.4027528	amvb	48	35	55	0	20	0	0	0	15	1994	TH 169R	Stagmoosh Rd	950	14	44	47	03.12	-93	24	31.51	1				





13001	45.3907139	92.8512028	24	25	45	0	10	0	0	15	2012	CHANNEL	US 8	16500	02	45	23	26.57	-92	51	11.53	1	7	8	8	
13001	45.3364806	93.0040917	48	25	45	0	10	0	0	15	1967	1 35	US 61	13800	05	45	20	11.33	-93	00	16.89	1	6	7	6	
13002	45.3908861	92.994417	48	25	45	0	10	0	0	15	1968	CSAH 34	CSAH 34	10000	07	45	23	4.79	-93	03	48.79	1	6	7	6	
13008	45.6842083	92.9898222	48	25	45	0	10	0	0	15	1968	1 35	CSAH 1	7100	06	45	41	23.36	-92	59	23.96	1	7	6	7	
13009	45.5101917	92.9613566	24	15	35	0	0	0	0	15	2008	TH 95	TH 95	20800	05	45	30	36.69	-99	59	35.36	1	8	8	8	
13005	44.7490472	93.0510278	24	15	35	0	0	0	0	15	2017	CSAH 42	US 52 NB	29000	02	44	44	20.57	-93	04	20.57	1	8	8	8	
13006	44.7390111	93.0347778	24	15	35	0	0	0	0	15	2017	US 52 NB	US 52 NB	29000	02	44	44	20.44	-93	02	05.02	1	8	8	8	
13016	44.8139611	93.0238028	48	35	45	20	0	0	0	15	1978	UPR R	TH 77 NB	47000	12	44	44	11.86	-93	13	25.69	1	7	5	7	
13016	44.8849611	93.0636366	48	15	35	0	0	0	0	15	1973	SOUTH VIEW BLVD	US 52 NB	30500	12	11	44	53	06.86	-93	04	48.41	1	7	6	7
13016	44.8850833	93.0633917	48	15	35	0	0	0	0	15	1973	SOUTH VIEW BLVD	US 52 NB	30500	12	44	44	11.30	-93	03	03.33	1	7	6	7	
13016	44.8101511	93.0653889	48	15	35	0	0	0	0	15	1978	CONCORD BLVD	US 52	Thompson Ave (CR 6)	10600	19	44	44	58.44	-93	54	56.12	1	7	6	7
13022	44.89795	93.0269028	48	15	35	0	0	0	0	25	1972	US 1972	US 1972	9500	12	11	44	53	52.62	-93	03	52.62	1	6	7	7
13021	44.9123583	93.0642556	48	25	45	0	10	0	0	15	1972	US 52	Butler Ave (CR 4)	12200	17	44	54	44.49	-93	03	51.12	1	7	6	5	
13022	44.8479232	93.0269028	48	25	45	0	10	0	0	15	1993	CSAH 38	US 52	12300	16	44	54	22.32	-93	11	24.74	1	6	7	5	
13023	44.8239318	93.0527306	48	25	45	0	10	0	0	15	1992	TH 55 NB	TH 55 NB	8200	14	44	49	27.83	-93	03	27.83	1	6	6	5	
13026	44.7726904	93.0348778	24	25	45	0	0	0	0	15	2010	CSAH 5	TH 13	8100	16	44	46	22.53	-93	18	17.56	1	8	8	7	
13027	44.8775167	93.0205133	48	25	45	0	10	0	0	15	1984	TH 13	TH 13 NB	9000	16	44	52	39.26	-93	04	49.92	1	7	6	7	
13028	44.877225	93.0805278	48	25	45	0	10	0	0	15	1984	TH 3	TH 10 EB	9000	16	44	52	38.01	-93	04	49.90	1	7	7	7	
13031	44.8831617	93.0996111	48	25	45	0	10	0	0	15	1991	US 52	80TH ST E	11300	16	44	50	03.01	-93	03	34.6	1	7	7	7	
13032	44.8650583	93.0399583	48	25	45	0	10	0	0	15	1988	US 52	Upper 50th St	12800	17	44	51	56.37	-93	03	35.85	1	7	7	6	
13034	44.8337806	93.0865889	48	25	45	0	10	0	0	15	1994	TH 55	TH 3	7000	12	44	50	01.61	-93	05	11.72	1	7	6	7	
13037	44.8159944	93.0443639	48	15	35	0	0	0	0	15	1994	CONCORD BLVD	US 52 NB	24750	14	44	48	57.58	-93	02	39.71	1	6	7	6	
13037	44.8159944	93.0443639	48	15	35	0	0	0	0	15	1994	CONCORD BLVD	US 52 NB	24750	14	44	48	56.92	-93	02	41.11	1	6	7	6	
13038	44.8047556	93.2210556	48	35	45	0	10	0	0	25	1978	TH 77	CSAH 30	17400	16	44	48	17.12	-93	13	15.8	1	6	6	5	
13038	44.8153111	93.2212611	48	25	45	0	10	0	0	15	1978	SB TH 77	TH 13 SB	12500	16	44	48	15.14	-93	13	15.14	1	7	6	6	
13038	44.7902066	93.2212611	48	25	45	0	10	0	0	15	1978	TH 77	CSAH 32	28000	14	44	47	25.01	-93	13	17.09	1	6	6	5	
13076	44.9036833	93.13385	48	25	45	0	10	0	0	15	1983	Lilydale Road	TH 13	8850	17	44	54	13.26	-93	8	1.86	1	7	7	7	
13077	44.9103556	93.1238111	48	25	45	0	10	0	0	15	1979	HAPPY HOLLOW	TH 13	9400	17	44	54	37.28	-93	7	25.72	8	7	7	7	
13078	44.7559917	93.2178617	48	25	45	0	10	0	0	15	1985	CSAH 38	TH 37, 38 OFF RAMP	16400	16	45	44	34.61	-93	45	3.86	1	7	6	5	
13084	44.7658664	93.2175861	48	25	45	0	10	0	0	15	1983	TH 77	TH 77	5400	19	44	44	57.13	-93	13	03.31	1	6	7	5	
13087	44.8833194	93.1089917	48	15	35	0	0	0	0	15	1993	SIBLEY MEMORIAL BIKE	TH 55 WB	21250	12	44	52	59.95	-93	10	11.61	1	7	7	7	
13088	44.8820712	93.1200917	48	15	35	0	0	0	0	15	1993	SIBLEY MEMORIAL BIKE	TH 55 WB	21250	12	44	52	59.96	-93	10	12.25	1	6	7	7	
13088	44.8823583	93.1652056	48	25	45	0	10	0	0	15	1993	MM 62 (OLD 110)	TH 55 WB	13000	14	44	52	16.49	-93	11	54.74	1	7	7	7	
13092	44.7824083	93.2861556	48	25	45	0	10	0	0	15	1984	13SW	CSAH 38	17400	16	44	45	08.67	-93	17	10.16	1	6	6	6	
13092	44.7474741	93.2829817	48	15	35	0	0	0	0	15	1989	13SW	CSAH 42	9000	14	44	44	44.67	-93	44	44.67	1	6	6	6	
13093	44.7145056	93.2835694	48	25	45	0	0	0	0	15	1995	1 35	CSAH 46	33000	16	44	42	52.22	-93	17	0.85	1	7	7	6	
13097	44.6978444	93.238375	48	25	45	0	0	0	0	25	1965	CSAH 30	135 SB	18500	16	44	41	52.24	-93	17	18.15	1	6	6	5	
13098	44.6978489	93.2831818	48	25	45	0	0	0	0	15	1965	CSAH 38	135 NB	18500	16	44	41	51.68	-93	17	17.47	1	6	6	5	
13098	44.7398944	93.2830583	48	25	45	0	10	0	0	15	1964	13SW	CSAH 46	19500	11	44	44	23.62	-93	16	59.01	1	7	7	6	
13098	44.8274588	93.17025	48	25	45	0	10	0	0	15	2012	1 35E, RAMPS	DUCKWOOD DR	5700	12	44	49	38.85	-93	10	12.90	1	7	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.56	-93	44	36.56	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11	44	44	36.04	-93	44	32.32	1	6	7	6	
13099	44.7474889	93.2747741	48	25	45	0	10	0	0	15	1978	CSAH 42	CSAH 42	13500	11											



27805	45.0670194	93.2861556	48	15	35	0	0	0	0	15	1964	TH 252 SB	194	26500	11	45	04	01.27	-93	17	10.16	1	7	7	7	7
27807	45.0511556	93.2615556	48	25	45	0	0	0	0	15	1979	SB Ave	94	6500	17	45	03	04.16	-93	17	06.56	1	6	6	6	6
27808	45.0499528	93.2946628	48	15	35	0	0	0	0	15	1982	194, BUS RAMP, ON RAMP	87th Ave N	17	45	02	07.12	-93	17	38.23	1	7	7	7	7	
27811	45.0293333	93.2845167	48	25	45	0	0	0	0	15	1977	194	Dowling Ave	17	45	01	26.16	-93	17	04.26	1	6	7	7	7	
27812	45.0111111	93.2832578	48	25	45	0	0	0	0	15	1977	194	Lowry Ave	17	45	00	47.20	-93	17	07.28	1	6	7	7	7	
27815	44.9991722	93.28278	48	15	35	0	0	0	0	15	1985	BROADWAY	US 52&A	16	45	08	57.02	-93	16	57.02	1	6	7	6	7	
27816	44.9858844	93.27795	48	55	55	20	10	0	0	25	1982	RR, STREETS TO 1/4	US 92&A NB	16	44	59	29.20	-93	16	45.66	1	5	5	5	5	
27817	44.9978417	93.2835311	48	25	45	0	0	0	0	15	1980	SB off ramp	194 SB on ramp	16	44	59	42.21	-93	16	04.61	1	6	7	6	7	
27818	44.9821972	93.2716722	48	15	35	0	0	0	0	15	1980	194 NB on ramp	NE off ramp	11	44	59	46.35	-93	11	04.35	1	6	6	6	6	
27820	44.964917	93.2818888	48	15	35	0	0	0	0	15	1989	CSAH 1	194	16	44	51	13.82	-93	14	15.82	1	7	6	7	6	
27821	44.9710389	93.46025	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194	11	44	58	17.90	-93	11	36.90	1	6	7	7	7	
27822	44.9710389	93.46025	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194 SB	11	44	58	15.90	-93	11	37.82	1	7	7	7	7	
27831	44.9726444	93.2845028	48	50	70	0	0	0	25	25	1987	Dunwoody Blvd	1394	147000	11	44	58	21.52	-93	11	40.21	1	6	5	5	5
27832	44.964917	93.2818888	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194	11	44	58	15.90	-93	11	37.82	1	7	7	7	7	
27833	44.964917	93.2818888	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194 SB	11	44	58	15.90	-93	11	37.82	1	7	7	7	7	
27834	44.964917	93.2818888	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194 SB	11	44	58	15.90	-93	11	37.82	1	7	7	7	7	
27835	44.964917	93.2818888	48	15	35	0	0	0	0	15	1986	1384 COLLECTOR RDS	194 SB	11	44	58	15.90	-93	11	37.82	1	7	7	7	7	
27840	44.9630313	93.2809917	48	15	35	0	0	0	0	25	1967	TH 65, RAMPS	15TH ST E	16	44	58	04.92	-93	16	10.89	1	6	6	6	6	
27845	44.9613389	93.2558861	48	25	45	0	0	0	0	15	1967	194 WB on ramp	194 WB	16	44	57	58.1	-93	16	20.11	1	6	6	6	6	
27849	44.9676083	93.2837833	48	35	55	0	0	10	0	25	1967	135W, RAMPS	TH 55 EB (ETH ST)	14	44	58	03.89	-93	14	13.82	1	5	7	5	5	
27850	44.9664194	93.2548444	48	35	55	0	0	10	0	25	1967	135W, SB ON RAMP	194 WB ON RAMP	17	44	57	59.11	-93	17	17.44	1	6	6	6	6	
27851	44.9657139	93.2609722	24	25	45	0	0	10	0	25	1967	135W, 194, ramp	PARK AVE	16	44	57	56.57	-93	16	54.55	1	7	6	6	6	
27853	44.9656861	93.2625667	48	25	45	0	0	10	0	25	1967	135W, 194	CHICAGO AVE	17	44	57	56.47	-93	17	45.24	1	6	6	6	6	
27854	44.9658806	93.2818778	48	35	55	0	0	10	0	25	1965	135W, 194, WB ON RAMP	11 AVE	17	44	57	57.17	-93	17	29.44	1	6	6	6	6	
27855	44.9659667	93.2816778	48	35	55	0	0	10	0	25	1967	194 WB ON RAMP	TH 55	17	44	57	55.58	-93	17	6.84	1	7	7	7	7	
27860	44.9669833	93.2729033	48	25	45	0	0	10	0	15	1994	194, RAMPS	194 EB ON RAMP	11	44	58	01.14	-93	11	26.07	1	7	6	6	6	
27863	44.9657889	93.2478111	48	15	35	0	0	0	0	15	1966	Cedar Ave	194	11	44	57	56.84	-93	14	50.14	1	7	7	5	5	
27865	44.9660389	93.2431661	48	25	45	0	0	0	0	15	1966	20th Ave S	194	11	44	57	52.70	-93	14	39.07	1	5	5	5	5	
27873	44.9671361	93.2540917	48	25	45	0	0	10	0	15	1970	135W, NB OFF RAMP	TH 55 SB OFF RAMP	11	44	58	01.69	-93	11	14.73	1	6	7	7	7	
27874	44.9674772	93.2529361	48	25	45	0	0	10	0	15	1970	135W, NB off ramp	TH 55 NB on ramp	11	44	58	03.89	-93	11	10.57	1	6	6	6	6	
27875	44.9680718	93.2333389	48	25	45	0	0	10	0	15	1967	135W, RAMPS	TH 55 WB OFF ST	14	44	58	05.44	-93	14	12.74	1	6	6	6	6	
27876	44.9689833	93.2528444	48	25	45	0	0	10	0	15	1968	135W, RAMP, COLL RDS	6TH ST/ EB 84	11	44	58	11.40	-93	11	10.24	1	7	6	6	6	
27877	44.9703333	93.2816472	48	25	45	0	0	10	0	15	1969	135W, COLL RDS	5TH ST OFF WB	11	44	58	13.20	-93	11	09.53	1	7	6	6	6	
27879	44.9712661	93.2812444	48	15	35	0	0	0	0	15	1970	3rd Street	117000	11	44	58	19.69	-93	11	04.12	1	6	6	6	6	
27878	44.9722861	93.2815544	48	25	45	0	0	10	0	15	1970	3rd Street	135W SB Coll Rd	10000	11	44	58	20.23	-93	11	05.74	1	5	6	6	6
27880	44.9718361	93.282004	48	25	45	0	0	10	0	15	1970	U of M West Bank	135W NB Coll Rd	10000	11	44	58	18.61	-93	11	07.77	1	7	7	7	7
27881	44.9718361	93.282004	48	25	45	0	0	10	0	15	1967	135W	Washburn Ave	16	44	58	20.60	-93	16	26.60	1	6	6	6	6	
27883	45.000472	93.2864417	48	35	55	0	0	10	0	25	1972	135W, NB OFF RAMP	JOHNSON ST NE	8500	16	45	0	2.33	-93	14	11.91	1	6	6	6	6
27884	44.9998444	93.2878033	48	25	45	0	0	10	0	25	1972	135W, SB OFF RAMP	BROADWAY ST NE	19400	16	44	59	55.30	-93	14	16.11	1	6	6	6	6
27885	45.0014917	93.2815488	48	25	45	0	0	10	0	15	1972	135W	JOHNSON ST CONNECTION	8500	16	45	0	5.37	-93	16	7.26	1	6	6	6	6
27886	45.070722	93.2868778	48	25	45	0	0	10	0	15	1988	TH 252 SB OFF RAMP	1694 WB OFF RAMP	14200	11	45	04	12.78	-93	11	17.78	9	6	7	7	7
27887	45.098406	93.286025	48	15	35	0	0	0	0	15	1988	TH 252, SB OFF RAMP	1694	142000	11	45	4	10.13	-93	17	9.69	1	6	7	7	7
27882	44.829233	93.2879778	48	15	35	0	0	0	0	15	1985	Frank Ave S	1484	5	44	54	32.13	-93	5	42.13	1	6	7	7	7	
27893	45.0013167	93.2269278	48	15	35	0	0	0	0	25	1971	135W	Stinson Blvd	97000	11	45	00	04.74	-93	11	36.94	1	6	6	6	6
27897	45.004389	93.212722	48	25	45	0	0	0	0	25	1970	Industrial Blvd	135W	132000	11	45	00	14.90	-93	11	44.18	1	6	6	6	6
27899	45.0063661	93.2078472	48	25	45	0	0	0	0	15	1970	MCV	108000	11	45	00	21.7	-93	11	45.35	1	6	6	6	6	
27906	45.0915389	93.4462139	48	25	45	0	0	10	0	15	1969	1484 NB	TH 48 WB TO SB 1484	16800	11	45	05	29.54	-93	26	46.37	1	7	7	7	7
27907	45.0916667	93.4461361	48	15	35	0	0	0	0	15	1969	194 EB	TH 48 NB TO 94 WB	38000	11	45	05	26.00	-93	26	46.09	1	6	7	7	7
27908	45.0927917	93.4072122	48	15	35	0	0	0	0	15	1978	194, 1484 EB	SHINGLE CREEK PKWY	42400	14	45	04	15.37	-93	14	15.44	1	5	6	6	6
27913	45.083611	93.3002389	48	25	45	0	0	10	0	15	1980	194, 1484 EB, RAMPS	TH 100 SB ON RAMP	5250	12	45	04	09.57	-93	12	0.86	1	5	7	7	7
27914	45.083667	93.299972	48	25	45	0	0	10	0	15	1980	194, 1484 EB, RAMPS	TH 100 NB ON RAMP	5250	12	45	04	07.92	-93	12	0.79	1	6	7	6	6
27915	45.071794	93.3093233	48	25	45	0	0	0	0	15	1964	Shingle Creek	194	4	45	04	18.46	-93	4	18.46	1	6	7	6	6	
27916	45.1848556	93.5214833	48	15	35	0	0	0	0	15	1969	194	TH 101	47000	12	45	11	08.93	-93	12	07.44	1	7	6	6	6
27917	45.1827664	93.5124528	48	35	55	0	0	0	0	25	1969	194	Brookline Ln	5600	16	45	09	49.57	-93	16	17.23	1	6	6	6	6
27918	45.172556	93.528265	48	35	55	0	0	10	0	25	1969	194	CSAH 6	800	16	45	09	20.12	-93	16	36.07	1	6	6	6	6
27919	44.9646222	93.2815817	48	25	45	0	0	10	0	15	1965	194	Franklin Ave	6000	16	44	57	52.64	-93	16	56.94	1	7	6	6	6
27920	45.0638417	93.325556	48	25	45	0	0	10	0	15	1980	1694 EB on ramp	1694	3200	11	45	04	09.63	-93	11	18.09	1	6	7	6	6
27921	44.9628213	93.2884472	48	25	45	0	0	10	0	15</																

62014	44.9734389	93.14670444	48	45	45	20	0	0	0	0	0	25	1965	BNF RFL SERVICE RD	TH S1	42000	16	44	58	24.38	-93	10	01.36	1	6	5	6
62015	44.9317583	93.0215472	48	35	35	20	0	0	0	0	0	15	1965	UP RR, EATON ST	LAFAYETTE (US 52)	74000	12	44	55	54.23	-93	04	17.57	1	7	5	6
62016	44.9397786	93.0274778	48	35	45	0	0	0	0	0	0	15	1967	Phase B (CSAH 60)	US 52 (Lafayette)	74000	12	44	56	31.03	-93	04	21.03	1	7	6	6
62017	45.0103417	93.1765028	48	25	45	0	0	0	0	0	0	15	1969	Fairview Avenue	TH 36 WB	41000	12	45	00	35.41	-93	10	35.41	1	5	7	5
62018	45.0100278	93.1794	48	25	45	0	0	0	0	0	0	25	1969	Fairview Avenue	TH 36 SE	41000	10	45	00	36.10	-93	10	36.04	1	6	5	5
62019	44.9646028	93.2023472	48	25	45	0	0	0	0	0	0	15	1970	Franklin Ave	TH 380 S	9100	12	45	00	52.57	-93	04	52.57	6	7	6	6
62020	44.9645917	93.2020333	48	25	45	0	0	0	0	0	0	15	1970	Franklin Ave	TH 380 NB	9100	16	44	57	02.32	-93	12	07.32	1	6	7	5
62021	45.0102917	93.1367	48	25	45	0	0	0	0	0	0	15	1969	Hawkins Ave North	TH 36	14000	10	45	00	37.05	-93	09	34.64	1	7	6	5
62022	45.0002278	93.1448683	48	24	25	45	0	0	0	0	0	15	2015	TH S1 (SMELUNG AVE)	CO RD 1 (US 60)	12000	16	45	00	0.82	-93	04	11.12	1	7	7	5
62023	44.9278611	93.0697556	48	15	35	0	0	0	0	0	0	15	1972	US 52 (Lafayette)	US 52 (Lafayette)	61000	12	44	55	38.50	-93	04	44.12	1	6	7	5
62024	44.9918917	93.2051139	48	25	45	0	0	0	0	0	0	15	2009	LARPERTEUR AVE	TH 280 S	11500	12	45	58	20.09	-93	12	20.09	1	6	7	5
62025	45.0603556	93.1802394	48	24	25	45	0	0	0	0	0	15	2012	1694 WB	TH S1	9100	14	45	03	48.08	-93	09	48.08	1	8	8	8
62026	45.0615639	93.1829137	48	15	35	0	0	0	0	0	0	15	2012	1694 RAMP	TH S1	31500	16	45	03	41.63	-93	09	29.85	1	8	8	8
62027	45.0915961	93.1881339	48	24	25	45	0	0	0	0	0	15	2012	135W 5th St NB RAMP	US 10 NB	12000	16	45	05	16.89	-93	11	17.89	1	8	8	8
62028	45.0600822	93.1879667	48	24	15	35	0	0	0	0	0	15	2012	135W NB Off RAMP	US 10 EB	23500	16	45	00	33.04	-93	08	33.04	1	6	8	8
62029	45.0117641	93.0459056	48	25	45	0	0	10	0	0	0	15	2013	TH 36	ENGLISH STREET	6200	17	45	00	42.26	-93	02	42.26	1	8	8	8
62030	45.0117694	93.0501067	48	25	45	0	0	0	0	0	0	25	1970	US 61	59000	12	45	00	42.37	-93	00	42.37	6	5	6	5	
62031	45.07925	93.1766139	48	15	35	0	0	0	0	0	0	15	2013	CSAH 96	US10 EB	23500	16	45	04	45.20	-93	10	35.81	1	8	8	7
62032	45.0792361	93.1768225	48	15	35	0	0	0	0	0	0	15	2013	CSAH 96	US 10 WB	23500	16	45	04	45.35	-93	10	34.41	1	8	8	8
62033	45.0596056	93.2019722	48	15	35	0	0	0	0	0	0	15	1999	Dale St CSAH 53	TH 36 WB	40000	12	45	00	34.85	-93	00	34.21	1	8	8	8
62034	45.0051139	93.1262056	48	15	35	0	0	0	0	0	0	15	1999	Dale St CSAH 53	TH 36 EB	40000	12	45	00	34.25	-93	07	34.34	1	7	8	7
62035	45.0122556	93.0800417	48	15	35	0	0	0	0	0	0	15	2007	MCNIGHT RD (CSAH 68)	MN 36 WB	47000	14	45	00	44.12	-93	00	18.15	1	8	8	8
62036	45.0120917	93.0902026	48	15	35	0	0	0	0	0	0	15	2007	MCNIGHT RD (CSAH 68)	MN 36 EB	47000	14	45	00	42.53	-93	00	18.29	1	7	8	8
62037	45.0147222	93.1875611	48	25	45	0	10	0	0	0	0	15	1986	135W W FRONTAGE RD	CO RD R2(CSAH 24)	7100	16	45	00	53.00	-93	11	15.22	1	6	6	6
62038	44.9771472	93.0886472	48	25	45	0	0	0	0	0	0	15	2012	135E	MARYLAND AVE	28500	16	44	58	37.73	-93	05	19.13	1	7	8	8
62039	45.0076444	93.2019972	48	25	45	0	0	0	0	0	0	15	2011	135E	RICE ST	18300	16	45	00	27.52	-93	06	27.52	1	8	8	8
62040	45.0076111	93.1048278	48	25	45	0	0	0	0	0	0	15	2011	TH 36	TH 36 EB RAMP	7100	12	45	00	27.40	-93	03	17.38	1	8	8	7
62041	44.9918883	93.0892722	48	24	25	45	0	10	0	0	0	15	2014	135E	LARPERTEUR AV	13500	16	44	59	20.69	-93	05	21.88	1	8	8	8
62042	44.9527956	93.2000556	48	25	45	0	0	0	0	0	0	15	1989	135E NB off ramp	135E NB OFF RAMP	1000	12	45	00	26.9	-93	05	124.3	1	7	7	6
62043	44.9522694	93.0865472	48	35	55	0	0	0	0	0	0	15	1987	194 EB OFF RAMP	EAST 7TH STREET	19000	16	44	57	08.17	-93	05	11.57	1	6	6	5
62044	44.9519389	93.0852222	48	25	45	0	10	0	0	0	0	15	1990	94 EB Off Ramp	94 EB Off Ramp	6000	12	44	57	06.98	-93	05	06.8	1	7	7	6
62045	44.9518086	93.0851639	48	25	45	0	0	0	0	0	0	15	1990	194 WB off ramp	194 WB Off Ramp	6000	11	44	57	06.03	-93	04	06.03	1	7	7	6
62046	44.9522889	93.0851333	48	25	45	0	0	0	0	0	0	15	1990	194 WB	Off Ramp to TH S2	194 WB	11	44	57	08.60	-93	05	07.56	1	7	7	7
62047	44.9522944	93.0879944	48	15	35	0	0	0	0	0	0	15	1987	194	US 61(Northwest Blvd)	21000	16	44	57	08.26	-93	04	11.26	1	6	6	6
62048	44.9532521	93.1023722	48	25	45	0	0	0	0	0	0	15	1990	194EB	OFF RAMP TO US 52	12000	16	44	57	09.29	-93	04	10.29	1	7	7	6
62049	45.0667806	93.1697472	48	15	35	0	0	0	0	0	0	15	2013	CSAH 76	1694 WB	39500	11	45	04	04.1	-93	10	11.09	1	8	8	7
62050	45.0664778	93.1879222	48	15	35	0	0	0	0	0	0	15	2012	CSAH 76	1694 EB	39500	11	45	03	09.32	-93	10	11.00	1	7	8	7
62051	45.0667611	93.1851278	48	15	35	0	0	0	0	0	0	15	2013	US 10 NB	1694 WB	39500	14	45	03	58.80	-93	05	58.80	1	8	8	7
62052	45.0666639	93.1659694	48	15	35	0	0	0	0	0	0	15	2012	US 10 SB	1694 EB	39500	14	45	03	57.49	-93	09	57.49	1	8	8	7
62053	45.0104722	93.1465	48	15	35	0	0	0	0	0	0	15	2016	LEXINGTON AVE(CSAH 51)	US 10	84000	11	45	00	37.70	-93	08	47.40	1	8	7	8
62054	45.0054222	93.1882828	48	15	35	0	0	0	0	0	0	15	2016	150W	CNTY RD H	85000	14	45	05	43.52	-93	08	43.52	1	6	6	6
62055	45.0104722	93.1465	48	15	35	0	0	0	0	0	0	15	1965	135E NB	LEXINGTON AVE(CSAH 51)	84000	14	45	00	47.40	-93	08	47.40	1	8	7	8
62056	44.9413389	93.1212444	48	25	45	0	0	0	0	0	0	15	1985	135E	Ramey-Grand Ave	14700	16	44	56	28.82	-93	06	43.72	1	5	6	7
62057	44.9603694	93.2011111	48	25	45	0	0	0	0	0	0	15	1967	TH 280 NB, SR OFF RAMP	US 14	85000	12	44	57	07.16	-93	08	37.16	1	7	7	6
62058	44.9579389	93.0906528	48	15	35	0	0	0	0	0	0	15	1989	UNIVERSITY AVENUE	135E NB	77000	11	44	57	28.58	-93	05	28.58	1	7	7	6
62059	44.957925	93.0906611	48	15	35	0	0	0	0	0	0	15	1989	University Avenue	135E NB on ramp	10000	12	44	57	28.53	-93	05	26.02	1	7	7	7
62060	44.9593194	93.2001417	48	25	45	0	0	0	0	0	0	15	1967	TH 380 SB	TH 380 S	1500	11	44	57	33.55	-93	04	33.55	1	7	6	5
62061	44.9594194	93.2001111	48	15	65	0	20	0	0	0	0	25	1965	194 EB ON RAMP	PELLHAM BLVD	4350	17	44	57	33.91	-93	12	0.40	1	5	5	5
62062	44.9488333	93.101875	48	15	65	0	0	0	0	0	0	15	1989	194 EB	1694	48000	14	44	56	59.40	-93	06	06.75	1	7	7	7
62063	45.0061472	93.1131394	48	15	65	0	0	0	0	0	0	15	1992	135E SB ON RAMP	McKENZIE ST	61000	14	45	02	21.33	-93	04	21.33	1	7	7	7
62064	45.0596194	93.1471778	48	15	35	0	0	0	0	0	0	15	1986	1694	LEXINGTON (CSAH51)	23200	16	45	02	49.84	-93	08	49.84	1	6	6	7
62065	45.0374667	93.1072222	48	15	35	0	0	0	0	0	0	15	1990	WHITE BEAR AVE (CSAH 65)	1694 WB	18000	14	45	02	14.88	-93	01	03.80	1	7	7	7
62066	45.0317111	93.10277	48	15	35	0	0	0	0	0	0	15	1969	WHITE BEAR AVE (CSAH 65)	1694 EB	18000	14	45	02	13.96	-93	02	13.96	1	7	7	7
62067	45.0343	92.9848361	48	35	55	0	10	0	0	0	0	25	1989	1694	TH 120	13000	16	45	02	03.48	-92	59	05.41	1	6	6	5
62068	45.0645467	93.2022111	48																								

70014	44.7780639	-93.5053722	48	15	35	0	0	0	0	15	1993	US 169	CSAH 17	24800	16	44	46	41.03	-93	30	19.34	1	7	7	7	
70011	44.6125261	-93.7708899	48	25	45	0	0	0	0	15	2016	US 169	ENTERPRISE DR	5000	19	44	36	45.13	-93	46	12.32	1	8	8	8	
70010	44.7811981	-93.4797919	48	15	35	0	0	0	0	15	1994	MSA 131	US 169 NB	20500	14	44	46	52.17	-93	46	21.17	1	7	7	7	
70038	44.7818383	-93.4769889	48	15	35	0	0	0	0	15	1994	MSA 131	US 169 SB	20500	19	44	46	52.38	-93	14	37.16	1	7	7	7	
70039	44.7825967	-93.4690056	48	15	35	0	0	0	0	15	1993	CSAH 83	US 169 NB	20250	14	44	46	50.08	-93	28	10.58	1	7	7	7	
70040	44.7821339	-93.4690056	48	15	35	0	0	0	0	15	1993	CSAH 83	US 169 SB	20250	14	44	46	50.97	-93	44	10.83	1	7	7	5	
70043	44.6226778	-93.7491715	48	25	45	0	0	0	0	15	2005	US 169, RAMPS	TH 25 NB	5700	07	44	37	21.64	-93	44	57.03	1	7	6	6	
70044	44.6226361	-93.7489611	48	25	45	0	0	0	0	15	2005	US 169, RAMPS	TH 25 SB	5700	07	44	37	22.21	-93	44	56.46	1	7	6	6	
70045	44.7889474	-93.4034810	48	15	35	0	0	0	0	15	1993	CSAH 101	US 169 SB OFF RAMP	5000	14	44	37	24.43	-93	24	24.43	1	7	7	7	
70019	44.7890889	-93.4064639	48	15	35	0	0	0	0	15	1993	TH 801B	US 169 NB	23500	14	44	47	20.72	-93	24	23.27	1	7	7	7	
70020	44.7878425	-93.4062071	48	15	35	0	0	0	0	15	1993	TH 801B	US 169 SB	23500	14	44	47	21.93	-93	24	24.18	1	7	7	7	
70021	44.7879417	-93.4064664	48	15	35	0	0	0	0	15	1993	Country 101 EB (Savage)	US 169 NB	17750	14	44	47	16.59	-93	14	16.59	1	7	7	7	
70022	44.7808089	-93.4086306	48	15	45	0	0	0	0	15	1993	US 169 SB	17750	14	44	47	17.12	-93	24	31.07	1	7	7	7		
70023	44.7878761	-93.4041110	48	25	45	0	0	0	0	15	1994	TH 169R NB	US 169 NB	17750	14	44	47	01.54	-93	25	16.63	1	7	7	7	
70024	44.7878833	-93.42195	48	25	45	0	0	0	0	15	1994	TH 169R WB	US 169 WB	17750	14	44	47	01.98	-93	14	17.92	1	7	7	7	
70027	44.78595	-93.4133917	48	25	45	0	0	0	0	15	1994	CSAH 18	US 169 EB	17750	14	44	47	9.42	-93	24	48.21	1	7	7	7	
70028	44.7852056	-93.4134056	48	25	45	0	0	0	0	15	1994	CSAH 18	US 169 WB	17750	14	44	47	10.24	-93	14	48.26	1	7	7	7	
70029	44.7839083	-93.4129778	48	25	45	0	0	0	0	15	1994	TH 169R	CSAH 18 NB	11400	16	44	47	02.07	-93	24	46.72	1	7	7	6	
70030	44.7877778	-93.4133306	48	25	45	0	0	0	0	15	1994	TH 169R	CSAH 18 SB	11400	16	44	47	02.60	-93	24	47.99	1	7	7	6	
70031	44.5732394	-93.2802978	48	25	55	0	0	0	0	15	1963	CSAH 2	35	0000	06	44	34	23.59	-93	37	54.26	1	6	6	5	
7201	44.8869417	-93.289975	48	25	45	0	0	0	0	25	1962	TH 62	France Avenue	26000	12	44	53	12.99	-93	19	44.31	1	6	6	5	
7205	44.8888611	-93.3337083	48	15	35	0	0	0	0	0	0	0	VALLEY VIEW RD	TH 62 EB	45000	12	44	53	12.70	-93	20	20.35	1	6	6	6
7206	44.820775	-93.2805978	48	25	45	0	0	0	0	15	1962	TH 62	Penn Ave (CSAH 23)	14700	16	44	53	26.79	-93	37	31.21	1	7	7	5	
7209	44.8909444	-93.2678444	48	25	45	0	0	0	0	0	0	0	TH 62		16300	16	44	53	26.50	-93	16	04.24	1	6	6	6
82001	45.2889556	-92.9693556	48	15	35	0	0	0	0	15	1969	2nd St NW	US 8	21000	14	45	17	20.24	-92	59	20.24	1	6	6	6	
82002	45.2889778	-92.9693556	48	15	35	0	0	0	0	15	1969	TH 61	CS 8	21000	14	45	17	20.32	-92	59	02.32	1	7	7	6	
82003	44.8152111	-92.9344651	48	25	45	0	0	0	0	15	1982	Jamaica Avenue	US 61 SB	11250	14	44	48	54.76	-92	56	04.15	1	7	7	7	
82004	45.0381417	-92.8642478	48	15	35	0	0	0	0	15	1982	Jamaica Avenue	US 61 NB	11250	14	44	48	55.35	-92	56	03.57	1	7	6	7	
82005	44.8329167	-92.9645083	48	15	35	0	0	0	0	15	1997	TH 36	20800	15	44	52	01.11	-92	57	47.14	1	6	6	6		
82014	44.8329167	-92.9645083	48	15	35	0	0	0	0	15	1982	US 61	CSAH 39, R0th St	21500	16	44	49	58.14	-92	57	52.23	1	6	7	5	
82015	45.0355083	-92.9650083	48	15	35	0	0	0	0	15	2013	HILTON TR	TH 36 EB	22500	16	44	02	07.83	-92	57	01.83	1	8	8	8	
82016	44.8590056	-92.9900119	48	15	45	0	0	0	0	15	2005	US 61	CSAH 23	10700	15	44	51	02.18	-92	59	24.05	1	7	7	5	
82019	44.8714472	-92.9997944	48	25	45	0	0	0	0	15	2003	HASTINGS AVENUE	GLEN ROAD	9000	19	44	52	17.98	-92	59	59.26	1	7	8	8	
82020	44.8714944	-93.0022833	48	25	45	0	0	0	0	15	2003	GLEN ROAD	GLEN ROAD	5000	19	44	52	17.38	-92	0	1.02	1	6	8	7	
82028	44.8829972	-93.0026889	48	25	45	0	0	0	0	15	2003	RAMP TH 61 SB	RAMP TH 61 SB	1400	14	53	16.79	-93	53	06.89	1	7	8	7		
82034	44.8876333	-93.0049583	48	25	45	0	0	0	0	15	2003	TH 61	RAMP TH 61 SB	5000	14	53	15.85	-93	00	17.85	1	7	8	6		
82035	44.8878583	-93.0049583	48	25	45	0	0	0	0	15	2003	TH 61 SB	1494 WB RAMP	6000	14	53	16.29	-93	0	10.77	1	7	7	7		
82036	44.8878722	-93.0049583	48	25	45	0	0	0	0	15	2003	TH 61 SB ON RAMP	TH 61 SB ON RAMP	6000	14	53	13.46	-93	00	13.46	1	7	7	7		
82043	45.0356222	-92.7974556	48	25	45	0	0	0	0	15	2013	TH 36, RAMPS	CSAH 23	8100	17	45	02	08.24	-92	47	50.84	1	8	8	8	
82044	44.9162111	-92.7975	48	25	45	0	0	0	0	15	1994	1494	Lake Road	15000	16	44	54	58.36	-92	58	46.20	1	6	7	6	
82045	44.9359506	-92.8406000	48	15	35	0	0	0	0	15	2003	1494	TAMM ROAD	12000	16	44	52	11.42	-92	59	58.80	1	6	6	6	
82047	45.2794444	-93.0033806	48	15	35	0	0	0	0	15	2011	135	W BRDGAARD AVE	26000	16	40	15	46.00	-93	16	24.00	1	6	8	8	
82049	44.9794972	-92.9358725	48	25	45	0	0	0	0	15	1967	1694	15th Street North	9900	14	48	58	13.79	-92	57	31.41	1	6	6	7	
82054	44.9827861	-92.9655219	48	15	35	0	0	0	0	15	1967	1694	CS 8	3900	11	44	59	15.92	-92	59	49.02	1	6	6	6	
82057	44.9976684	-92.9594889	48	15	35	0	0	0	0	15	1967	TH 5	1694 SB	33500	11	44	59	51.61	-92	57	34.16	1	8	7	6	
82058	44.9975056	-92.9591917	48	15	35	0	0	0	0	15	1967	1694 NB	33500	11	44	59	51.02	-92	57	33.09	1	8	7	7		
82060	45.0020889	-92.9591917	48	15	35	0	0	0	0	15	1967	SOUTH ST N	1694 SB	33500	11	44	59	15.56	-92	57	31.67	1	7	7	7	
82061	45.0297611	-92.9621394	48	15	35	0	0	0	0	15	1967	TH 36	1694 SB	33500	11	44	59	17.57	-92	57	44.35	1	7	6	5	
82062	45.0298889	-92.9619556	48	15	35	0	0	0	0	15	1967	TH 36	1694 NB	33500	11	44	01	47.6	-92	57	43.04	1	7	6	7	
82063	45.2888444	-93.0030119	48	15	35	0	0	0	0	15	1967	1694	US 15	10500	16	44	02	17.17	-92	59	18.84	1	7	7	7	
82068	44.9632556	-92.9583444	48	25	45	0	0	0	0	15	1967	1694	CSAH 10	16200	16	44	57	47.72	-92	57	30.04	1	6	7	7	
82069	44.9496278	-92.9345167	48	25	45	0	0	0	0	15	1983	194	CSAH 13 SB	16200	16	44	56	55.06	-92	56	04.26	1	7	7	6	
82074	44.9496278	-92.9345167	48	25	45	0	0	0	0	15	1983	194	CSAH 13 NB	16200	16	44	56	54.94	-92	56	04.26	1	7	7	6	
82084	44.9467778	-92.8624806	48	25	45	0	0	0	0	15	1983	194	TH 95	12200	06	44	56	55.24	-92	51	44.93	1	6	7	5	
82089	44.9620639	-92.7733828	48	25	45	0	0	0	0	25	1982	TH 95	TH 95	38500	07	44	57	43.43	-92	46	24.07	1	6	6	6	
82100	44.9617417	-92.7733828	48	25	45	0	0	0	0	25	1982	TH 95	TH 95	38500	07	44	57	43.43	-92	46	24.07	1	6	6	6	
82102	44.9485806	-92.9342889	48	15	35	0	0	0	0	15	1983	194	CSAH 13 NB	16200	16	44	56	54.89	-92	56	03.44	1	6	7	6	
82107	44.8481167	-92.9083917	48	15	35	0	0	0	0	15	2003	1694	MAXWELL AVE	43000	11	44	03	30.21	-93	00	30.21	1				

9605	45.1157139	-93.1884833	48	15	35	0	0	0	15	1968	TH I 35W	US 10	7800	12	45	06	56.57	-93	11	18.54	1	6	6	6	
9607	45.128194	-93.188389	48	25	45	0	0	0	15	1968	I 35W	I 35W ON RP	8500	11	45	07	41.95	-93	11	18.74	1	7	7	7	
9609	44.921667	-93.029717	24	15	35	0	0	0	15	1986	BNE PATH	US 10	3950	14	44	56	06.60	-93	02	06.88	13	N	N	N	
9616	44.928839	-93.274361	48	25	45	0	0	0	15	1964	I 35W	East 42nd St	5500	16	44	55	36.62	-93	05	28.69	1	6	6	6	
9618	44.928129	-93.274889	24	25	45	0	0	0	15	1961	I 35W	E 80th St	9900	17	44	46	05.87	-93	16	28.89	1	7	7	7	
9620	44.937361	-93.274881	48	25	45	0	0	0	15	1965	I 35W	EB I 35th St	10000	17	44	56	10.85	-93	16	28.87	1	6	7	6	
9621	44.939525	-93.274694	48	25	45	0	0	0	15	1965	I 35W	WB I 35th St	17100	17	44	56	22.29	-93	16	28.87	1	7	6	5	
9623	44.957825	-93.011125	48	15	35	0	0	0	15	1963	University Avenue	I 35 SB	77000	11	57	44	28.17	-93	05	28.95	1	6	6	7	
9624	45.2009147	-93.3864833	48	45	65	20	0	0	15	1962	Rum Row	US 10	6000	12	45	12	17.70	-93	11	17.34	1	6	5	4	
9713	45.2049111	-93.3897939	48	25	45	0	0	0	15	1964	US 10	US 10	82000	14	45	12	17.68	-93	23	23.15	1	6	6	5	
9715	45.2049111	-93.3897939	48	25	45	0	0	0	15	1963	US 10	US 10	67000	12	45	12	17.68	-93	23	23.15	1	6	6	5	
9716	45.2049111	-93.3897939	48	25	45	0	0	0	15	1965	US 10	US 10	12000	12	45	12	16.30	-93	23	23.15	1	6	6	5	
9717	45.2044139	-93.373328	48	25	45	0	0	0	15	1965	CSAH 7 7th Avenue	US 10	67000	12	45	12	16.30	-93	23	23.15	1	6	6	5	
9724	45.1339778	-93.270425	48	25	45	0	0	0	15	1966	BNFS PRIVATE ROAD	CSAH 10	87000	10	45	12	24.07	-93	11	13.33	1	6	6	6	
9725	45.1339778	-93.270425	48	25	45	0	0	0	15	1966	CSAH 10	CSAH 10	11000	16	45	08	09.48	-93	11	13.33	1	6	6	6	
9726	45.1339778	-93.270425	48	25	45	0	0	0	15	1966	CSAH 10	CSAH 10	11000	16	45	08	09.48	-93	11	13.33	1	6	6	6	
9727	44.9118	-92.964833	48	15	35	0	0	0	15	1960	CSAH 25 (CENTURY AVE)	I 494 WB	32000	11	44	54	42.48	-92	59	03.78	1	7	6	6	
9728	44.91125	-92.964833	48	15	35	0	0	0	15	1960	I 494 EB	I 494 EB	32000	11	44	54	40.50	-92	59	03.78	1	7	6	6	
9729	44.775322	-93.2883694	48	35	55	0	10	0	25	1959	I 35W	TH 13 SB	18000	13	44	46	31.16	-93	17	18.13	1	6	6	5	
9905	44.9506222	-93.1618444	48	45	65	0	10	10	0	25	1966	12TH STREET EAST	I 94 WB ON RAMP	9400	11	44	57	02.24	-93	06	06.84	7	5	5	4
9906	44.9506222	-93.1618444	48	45	65	0	10	10	0	25	1966	RAMP 101 12TH	I 94 WB	9400	11	44	57	02.26	-93	06	06.84	7	5	5	4
9910	45.19275	-93.0729	48	25	45	0	10	0	15	1967	I 35W	CSAH 14	6400	14	45	11	33.90	-93	04	22.44	1	6	7	5	
9911	45.1681222	-93.1428972	48	25	45	0	10	0	15	1967	I 35W	CR 53	5000	17	45	10	05.24	-93	08	34.43	1	6	7	6	
9913	45.0687139	-93.282178	48	15	35	0	0	0	15	1964	I 694	MAIN ST AVE	27000	16	45	04	07.37	-93	16	05.26	1	6	6	7	
9919	45.0644861	-93.2184306	48	25	45	0	0	0	15	1964	I 694	Silver Lk Rd	27700	16	45	03	52.15	-93	13	06.35	1	6	6	5	
9921	44.901825	-92.9944444	48	15	35	0	0	0	15	1960	CSAH 43 (CARVER AVE)	I 494 WB	32000	11	44	54	06.57	-92	59	40.00	1	6	6	6	
9922	44.9017417	-92.9941256	48	15	35	0	0	0	15	1960	CSAH 43 (CARVER AVE)	I 494 EB	32000	11	44	54	06.27	-92	59	38.96	1	6	6	6	
9923	44.9647861	-93.2288972	48	45	65	20	0	0	25	1962	Franklin Terrace	I 94	167000	11	44	57	53.23	-93	13	44.03	9	5	5	6	
9924	45.1522472	-93.274475	48	25	45	0	0	0	25	1996	US 10, RAMP, MN 47	FOLEY BLVD	20100	11	44	09	08.09	-93	16	28.11	1	7	4	6	
9925	45.0687444	-93.2633611	48	15	35	0	0	0	15	1968	MH 47	194	123500	11	04	45	07.48	-93	15	48.01	1	6	6	6	
9929	44.8085556	-93.625375	48	35	55	0	10	0	15	2007	US 212	ENGLER BLVD	8800	16	44	48	21.08	-93	37	31.35	1	8	5	8	
9936	44.8148556	-93.213083	48	35	55	0	10	0	15	1978	NH 77	TH 13 NB	12500	16	44	48	53.48	-93	13	16.71	1	6	5	5	
9937	44.8716389	-93.077451	48	25	45	0	0	0	15	1985	I 494 WB ON RAMP	TH 13 SB	10750	16	44	53	30.59	-93	12	16.89	1	6	6	6	
9938	44.87465	-93.060856	48	25	45	0	0	0	15	1984	I 494 COLL ROADS	US 52 SB	30000	12	44	52	28.74	-93	03	39.08	1	6	5	5	
9939	44.8747667	-93.0609511	48	25	45	0	0	0	25	1984	I 494 COLL ROADS	US 52 NB	30000	12	44	52	29.16	-93	03	39.02	1	6	5	5	
9940	44.7809198	-93.2485972	48	35	55	0	0	0	15	1969	I 35W	Burnsville Parkway	11400	16	44	46	11.71	-93	14	18.39	1	6	6	6	
9945	44.8761333	-93.0307306	48	25	45	0	0	0	25	1980	TH 156 (CONCOURSE STREET)	I 494	85000	11	44	52	34.08	-93	01	50.63	1	5	5	4	
9946	44.8763333	-93.0424583	48	35	55	0	10	0	25	1983	I 494	7th Avenue South	11000	16	44	52	28.32	-93	01	32.85	1	6	5	5	
9949	44.8744472	-93.039935	48	35	55	0	10	0	15	1983	I 494	5th Avenue South	8600	19	44	52	28.63	-93	02	23.73	1	6	5	5	
9950	45.0509639	-93.3221972	48	15	35	0	0	0	15	1963	TH 100	Brooklyn Blvd	22300	16	44	53	03.47	-93	19	19.91	1	7	5	5	
9951	44.8916333	-93.1808861	48	35	55	0	0	0	25	1968	TH 5 WB ON RAMP	TH 55 EB OFF RAMP	11000	12	44	53	29.88	-93	11	17.11	1	5	5	5	
9952	45.1309661	-93.4933333	48	25	45	0	10	0	15	2005	TH 52B ON RAMP	TH 52B ON RAMP	15400	16	45	07	50.11	-93	26	21.6	1	7	5	7	
9953	44.9544917	-93.4007861	48	35	55	0	10	0	25	1964	US 169	Cedar Lake Rd	14700	16	44	57	16.17	-93	24	02.83	1	5	5	5	
9954	44.8975861	-93.227694	48	35	55	0	10	0	25	1964	TH 62	34th Ave S	6300	19	44	53	51.18	-93	13	22.69	1	6	5	5	
9955	45.0509366	-93.4008778	48	25	45	0	0	0	25	1973	US 169	Bass Lake Road	26100	13	45	03	28.91	-93	24	03.16	1	6	5	5	
9956	45.0683778	-93.4010917	48	35	55	0	10	0	25	1968	US 169	E3rd Ave N	17400	16	44	57	09.76	-93	24	03.93	1	5	5	5	
9957	45.0220477	-93.4007806	48	35	55	0	10	0	25	1969	US 169	86th Ave S	15300	17	45	01	19.35	-93	24	02.81	1	5	5	5	
9958	45.023325	-93.4008311	48	25	45	0	0	0	25	1972	US 169	Rockford Road	12400	16	45	03	19.97	-93	24	02.92	1	5	5	5	
9959	44.9700167	-93.3088611	48	35	55	0	10	0	25	1986	I 394 I 394R	PENN AVE S	10300	16	44	58	11.90	-93	18	12.06	1	5	5	5	
9960	44.9792861	-93.237811	48	25	45	0	0	0	25	1969	Lyndale Avenue SB	I 94 EB on ramp	21400	11	44	58	21.49	-93	17	16.12	1	6	5	6	
9961	44.9658	-93.279417	48	35	55	0	10	0	15	1964	I 94	LASKALE AVE	7100	17	44	57	06.88	-93	16	45.63	1	7	5	5	
9962	44.96625	-93.2778167	48	35	55	0	10	0	25	1966	I 94	Nicollet Ave	8600	16	44	57	58.50	-93	16	40.14	1	5	5	5	
9963	44.9607417	-93.247636	48	35	55	0	10	0	25	1967	I 35W I 94 WB	PORTLAND AVE	11300	16	44	57	56.67	-93	16	03.56	1	7	5	5	
9965	44.9609464	-93.242861	48	25	45	0	0	0	25	1967	TH 5L RAMPS	I 94	113000	11	44	57	57.94	-93	15	08.23	1	6	5	5	
9968	44.9823083	-93.2431	48	25	45	0	0	0	25	1967	I 35W	UNIVERSITY AVE	22300	16	44	58	36.40	-93	14	36.40	10	5	5	7	
9969	44.9823083	-93.2431	48	25	45	0	0	0	25	1967	I 35W	4TH ST SE	21900	16	44	58	39.75	-93	14	33.74	10	4	5	7	
9970	45.0020778	-93.2459172	48	25	45	0	0	0	25	1972	I 35W NB	I 35W NB	48000	11	0	7.48	-93	14	8.06	1	6	5	6		
9971	45.1045139	-93.4599444	48	25	45	0	0	0	25	1969	I 94	Weaver Lake Rd	38900	16	45	06	16.25	-93	27	35.80	1	6	5	5	
9972	45.0284778	-93.453056	48	25	45	0	0	0	25	1965	I 694	Rockford Road	27000	16	45	07	42.52	-93	09	09.88	1	5	5	5	
9973	45.1245917	-93.4834417	48	25	45	0	10	0	15	2001	I 94 RAMP	US 169	19600	16	45	07	28.53	-93	29	01.83	1	6	5</		