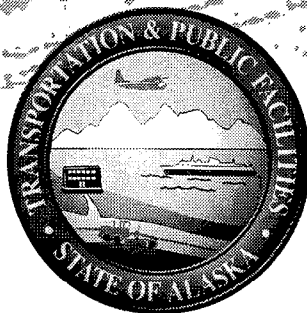
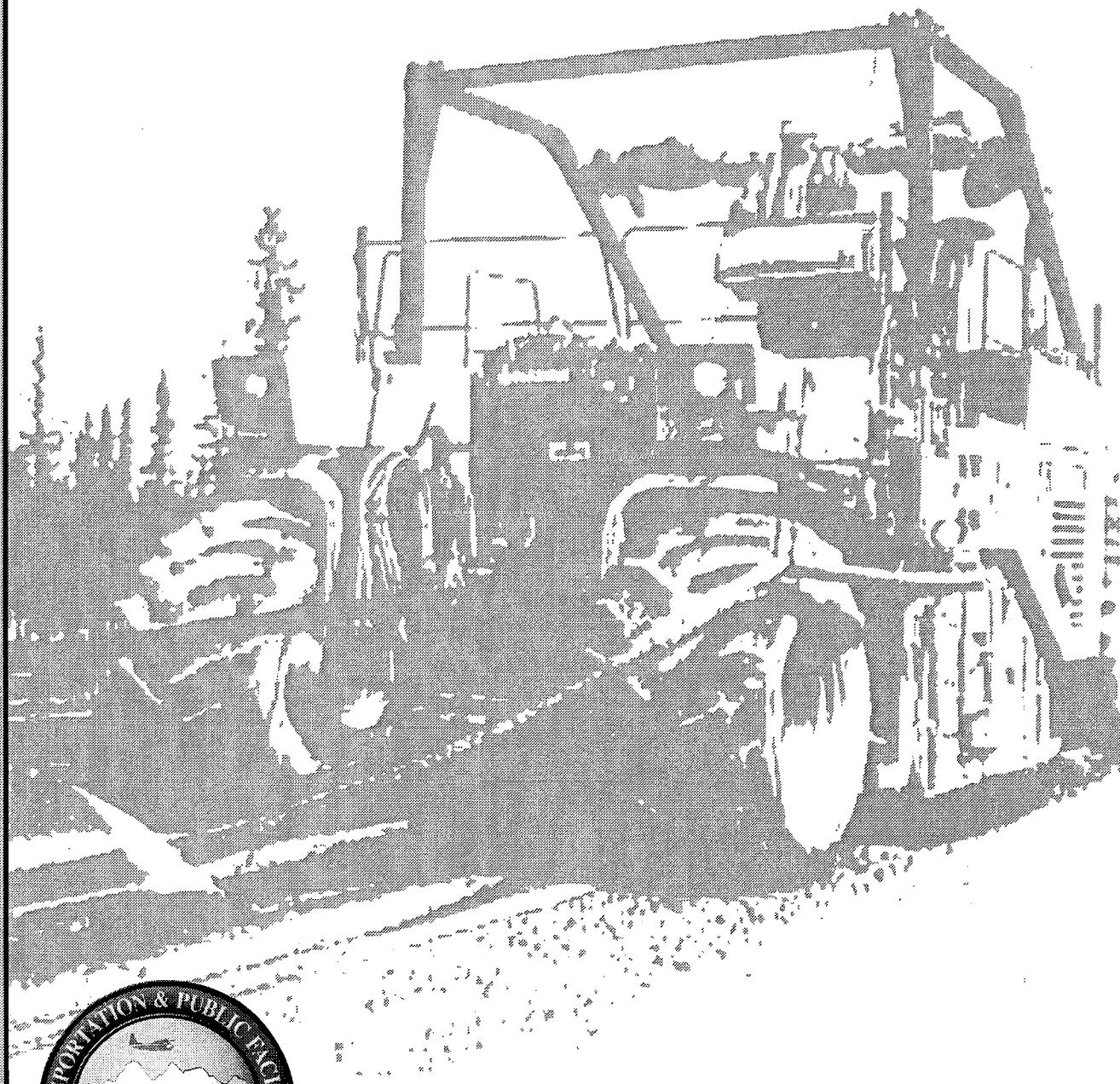


Alaska Soil Stabilization Design Guide

PB2002-108307



**Alaska Department of
Transportation & Public Facilities**

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NTIS

Alaska Soil Stabilization Design Guide

by

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State of Alaska

Department of Transportation and Public Facilities

Research & Technology Transfer

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13. ABSTRACT (Maximum 200 words)

This guide presents information on the types of soil stabilization techniques that have or can be used in the state of Alaska. It covers techniques including asphalt, cement, lime, mechanical, chemical, and other methods. For each method there is a discussion on materials and design considerations, construction issues, and expected performance and costs. The appendices include a glossary of terms, a reading list on prior stabilization used in Alaska, a discussion on the soils in Alaska, and a slide presentation summarizing the highlights of the guide.

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DISCLAIMER

The contents of this report reflect the views of the author, who is responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of Alaska DOT&PF or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.

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1.0 INTRODUCTION

1.1 Background

Soil stabilization is used for a number of activities. These include (1) temporary wearing surfaces, (2) working platforms for construction activities, (3) improving poor subgrade conditions, (4) upgrading marginal base materials, (5) dust control, and (6) recycling old roads containing marginal materials. A wide variety of materials have been used to stabilize soils or marginal aggregates (e.g., asphalt, portland cement, lime and lime/fly ash, chemicals, salt, and other techniques). Selecting the stabilizer type depends on a number of factors, including:

1. gradation,
2. plasticity index (PI),
3. availability and cost of the stabilizer and appropriate construction equipment, and
4. climate.

When correctly designed, stabilization can produce numerous benefits for pavement construction and rehabilitation. However, inappropriate designs and/or construction can lead to premature failures.

Most stabilization of road materials carried out in Alaska has involved the use of asphalt emulsions, portland cement, chemicals, and salt. The reasons for this are:

1. The stabilizing agents are available.
2. Equipment to incorporate the agents is available and proven.
3. The performance of these agents is well documented.
4. Their use is considered cost effective.

Therefore, the major part of this guide is devoted to the use of these stabilizing agents. However, other techniques are also addressed (for example, lime and lime-fly ash, geotextiles, drainage).

We consider the contents of this guide to be best practice for Alaska conditions. However, with the advances that continue to take place with respect to materials, equipment, and techniques, we expect these practices to continually improve.

1.2 Scope

This guide provides practical information to people involved in the design, materials characterization, construction, and maintenance of pavements incorporating stabilized layers. It includes advice on how to select additive types, how to test materials, and construction procedures.

It also provides guidance on the best practices for selecting, designing, and constructing stabilized layers for new road pavements, as well as for maintaining and rehabilitating existing road pavements. This guide brings together stabilization technology research and experience from public agencies, contractors, research organizations, and material suppliers.

1.3 Limitations

The guide is based on information developed both within and outside the state of Alaska. It may not be applicable to all soils and/or environmental conditions found in the state. For more information, refer to the references at the end of each chapter.

2.0 TYPES OF STABILIZATION

2.1 Classification of Stabilization Types

In the past, stabilization has been classified primarily on the type of stabilizer used. Now that mechanistic design procedures are available to help evaluate the performance of the stabilized layer in a pavement structure, the type of stabilization can further be classified in terms of their structural performance.

Following are the three main categories of materials in terms of performance criteria (Table 2.1):

1. **Unbound Materials**—Materials that do not exhibit significant tensile strength and that do resist traffic through a combination of cohesion and interparticle friction, such as natural gravels and fine grained soils.
2. **Modified Materials**—Unbound materials to which small amounts of stabilizing agents are added to
 - correct a material deficiency without causing a significant increase in stiffness,
 - increase the strength, or
 - reduce the moisture or frost susceptibility of fine grained soils.
3. **Bound Materials**—These are produced by adding stabilizing agents to produce a bound material with significant tensile strength. The bound material acts like a beam in the pavement to resist traffic loading. Compared with unbound and modified materials, it has increased structural capacity.

Table 2.1. Material Categories and Characteristics

Characteristics	Unbound Materials	Modified Materials	Bound Materials
Materials Types	Crushed rock, natural gravel, granular materials, fine-grained soils.	Unbound materials with small amounts of stabilizing agents incorporated; bitumen-stabilized materials; some bitumen/cement-, lime- and cement-stabilized materials.	Unbound materials stabilized with cementitious or other binders (for example, cement, lime, supplementary cementitious materials, bitumen/cement)
Behavior Characteristics	Development of shear strength through cohesion and internal friction between particles.	Development of shear strength through cohesion and internal friction between particles.	Development of shear strength through particle interlock, chemical bonding, and cohesion. Significant tensile strength.
Distress Modes	Deformation through shear and densification. Disintegration through breakdown of particles and/or material structure.	Deformation through shear and densification. Disintegration through breakdown of particles and/or material structure.	Cracking developed through shrinkage, fatigue, and oversteering. Erosion and pumping in the presence of moisture.
Parameters Required for Structural Design	Modulus Poisson's Ratio Degree of anisotropy	Modulus Poisson's Ratio Degree of anisotropy	Modulus Poisson's Ratio Fatigue characterization
Performance Criteria	Current materials specifications (e.g., strength, grading, plasticity, density). Thickness governed by subgrade strain criteria.	Current materials specifications (e.g., strength, grading, plasticity, density). Thickness governed by subgrade strain criteria.	Fatigue and erosion

For definition of terms such as modulus and Poisson's Ratio, see Appendix A.

2.2 Types of Stabilizing Agents

Engineers use stabilization to enhance materials properties for pavement design procedures or to overcome deficiencies in available materials. Stabilization agents fall into a number of categories:

1. Asphalt—emulsions, cutbacks, and other proprietary products
2. Portland cement—in accordance with AASHTO standards
3. Lime—includes hydrated lime $[\text{Ca}(\text{OH})_2]$ and quicklime $[\text{CaO}]$
4. Blends of the above
 - Asphalt/cement
 - Asphalt/lime
 - Lime/fly ash
 - Cement/lime/fly ash
5. Chemicals—generally proprietary chemicals
6. Salt—generally CaCl_2
7. Others—in Alaska, these include drainage, geotextiles, and mechanical stabilization

Table 2.2 gives a broad indication of the application and effects of the various stabilizing agents.

2.3 Selecting the Correct Stabilizing Agent

2.3.1 Soil Type

Particle size distribution and Atterburg limits are commonly used to gain a preliminary assessment of the type of stabilization required for a particular material. The usual range of suitability of various types is based on the 75 μm sieve (#200) and the plasticity index of the soil. Figure 2.1 provides initial guidance for selecting a stabilizer type.

2.3.2 Climate and Drainage

Climate can have a significant effect on your choice of stabilizer. In wetter areas, where the moisture content of the pavement materials is high, it is important to ensure that the wet strength of the stabilized material is adequate. In these conditions, cementitious binders are usually preferred, although asphalt and asphalt/cement blends would also work. Lime is suitable for

cohesive soils, particularly when used as the initial agent to dry out the material. Lime can also work with silty soils if a pozzolan is added to promote the cementing reaction.

Using emulsions in cold dry climates requires using cement or lime to facilitate moisture removal from the emulsion during the stabilization process. It also promotes strength.

2.3.3 Sampling and Testing

It is essential with all stabilization work that you thoroughly assess all materials and properly evaluate their reactions with a specific admixture in the laboratory before any field work begins. Stabilized materials should be tested to determine their quality and uniformity. Testing requirements are described in the sections of the guide dealing with the relevant methods of stabilization.

2.3.4 Final Selection

After analyzing all available data, you may find there are a number of feasible solutions. The decision is usually based on costs and/or expected performance. You also need to consider the skills, resources, and equipment available in the area, past performance of similar work, and availability of materials and construction equipment.

2.4 References

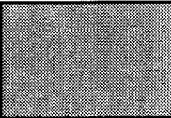
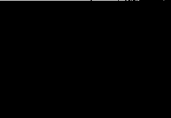
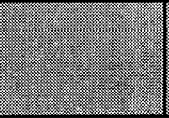
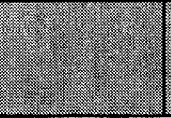
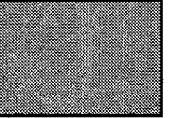
- Alaska DOT&PF, Standard Specification for Highway Construction—Metric Edition, 1998.
- Austrroads, *Guides to Stabilization in Roadworks*, Austrroads, Inc., Sydney, 1998.
- Epps, J.A., W.A. Dunlap, and B.M. Galloway, "Basis for Development of a Soil Stabilization Index System," USAF Contract #F29601-70-C-0008, Texas A & M University, November 1970.
- ERES Consultants, "Soil and Base Stabilization and Associated Drainage Considerations," Vol. I and II, *FHWA-SA-93-004*, Federal Highway Administration, December 1992.
- Transportation Research Board, "Soil Stabilization," *Transportation Research Record 1295*, 1990.

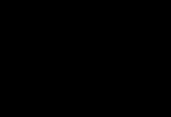
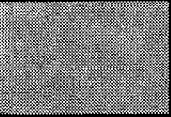
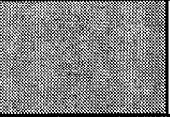
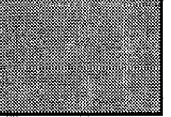
Table 2.2. Application of Different Stabilizing Agents

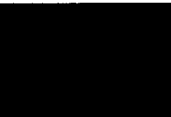
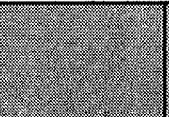
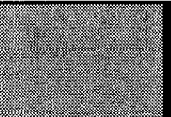
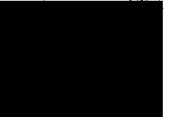
Stabilization Agent	Process	Effects	Applicable Soil Types
Cement	Cementitious inter-particle bonds are developed.	- Low additive content (<2%): decreases susceptibility to moisture changes, resulting in modified or bound materials. - High additive content: increases modulus and tensile strength significantly, resulting in bound materials.	Not limited apart from deleterious components (organics, sulphates, etc., which retard cement reactions). Suitable for granular soils but inefficient in predominantly one-sized materials and heavy clays.
Lime (including hydrated lime and quicklime)	Cementitious inter-particle bonds are developed but rate of development is slow compared to cement. Reactions are temperature dependent and require natural pozzolan to be present. If natural pozzolan is not present, a blended binder that includes pozzolan can be used.	Improves handling properties of cohesive materials. - Low additive content (<2%): decreases susceptibility to moisture changes, and improves strength, resulting in modified or bound materials. - High additive content: increases modulus and tensile strength, resulting in bound materials.	Suitable for cohesive soils. Requires clay components in the soil that will react with lime (i.e., contain natural pozzolan). Organic materials will retard reactions.
Blended slow-setting binders (for example, slag/lime, fly ash/lime, and slag/lime/fly ash blends)	Lime and pozzolan modifies particle size distribution and develops cementitious bonds.	Generally similar to cement but rate of gain of strength similar to lime. Also improves workability. Generally reduces shrinkage cracking problems.	Same as for cement stabilization. Can be used where soils are not reactive to lime.
Bitumen (including foamed and high impact bitumen, cutback bitumen, and bitumen emulsion)	Agglomeration of fine particles.	Decreases permeability and improves cohesive strength. Decreases moisture sensitivity by coating fines.	Applicable to granular materials with low cohesion and low plasticity.
Bitumen/cement blends	Agglomeration of fine particles with some cementitious bonding.	Decreases permeability and improves strength. Cement aids in providing early strength.	Applicable to granular materials with low cohesion and plasticity.
Mechanical stabilization	Mixing two or more materials to achieve planned particle size distribution.	Some changes to soil strength, permeability, volume stability, and compactibility. Materials remain granular.	Poorly graded soils, granular soils with a deficiency in some size(s) of the particle size distribution.
Miscellaneous chemicals	Agglomeration of fine particles and/or chemical bonding (see trade literature).	Typically increased dry strength, changes in permeability and volume stability.	Typically poorly graded soils.

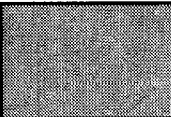
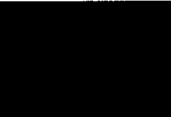
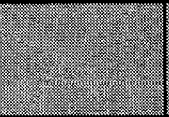
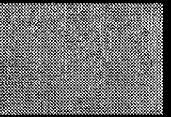
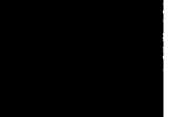
	More than 25% Passing 75 μ m			Less than 25% Passing 75 μ m		
Plasticity Index	PI $\leq$ 10	10 $\leq$ PI $\leq$ 20	PI $\geq$ 20	PI $\leq$ 6 (PI $\times$ % passing 0.075 mm $\leq$ 60)	PI $\leq$ 10	PI $\geq$ 10

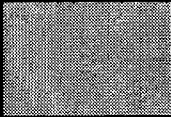
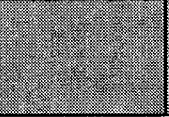
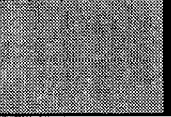
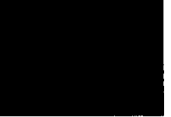
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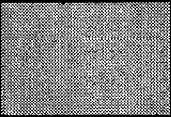
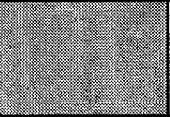
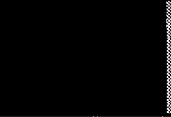
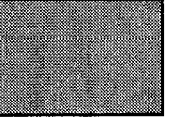
Cement and Cementitious Blends						
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Lime						
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Bitumen						
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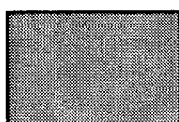
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Granular						
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Miscellaneous Blends						
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Key

Usually
suitable



Doubtful



Usually not
suitable



Should be taken as a broad guideline only.

Note: The above forms of stabilization may be used in combination, for example, using lime stabilization to dry out material and reduce their plasticity, making them suitable for other methods of stabilization.

Figure 2.1. Guide to Selecting a Method of Stabilization

3. DESIGN PHILOSOPHY

3.1 Design Considerations

The three main aspects influencing the successful design and use of stabilized pavement materials include:

- the mix design for the stabilized materials,
- the structural design of the pavements into which the stabilized materials will be incorporated, and
- construction of the stabilized layer.

These aspects are interrelated, since stabilized material performance depends on the thickness and composition of the pavement in which it is used, while the structural design process depends on the characteristics of the stabilized pavement material. Both are also affected by the quality of the construction process.

3.2 Materials Mix Design

The materials design aspects of stabilized materials require the engineer to investigate and know about both the pavement material to be stabilized and the stabilizing agents available. Sections 4–10 of this guide describe the desirable materials characteristics and assessment requirements.

Important characteristics include:

- strength (compressive and shear),
- durability,
- shrinkage characteristics,
- setting and curing characteristics,
- moisture susceptibility,
- erodability,
- stiffness,
- fatigue performance (where applicable), and
- variability.

3.3 Structural Design

You cannot design stabilized materials without considering the composition and structural design of the pavement into which they will be incorporated. The performance of pavements that include stabilized layers will depend on many factors, including

- subgrade strength,

- thickness and stiffness of the stabilized and other pavement layers (including the wearing surface),
- design traffic, and
- environmental conditions—temperature and moisture conditions and provision for pavement drainage.

Use the flexible pavement design procedures recommended by Alaska DOT&PF or the material supplier to design pavements incorporating stabilized layers.

In Alaska DOT&PF's mechanistic design procedure, stabilized materials are characterized by their:

- stiffness/modulus,
- Poisson's ratio, and
- fatigue performance criteria.

These can be obtained either by testing or by estimation and guidance. Figure 3.1 illustrates the mechanistic design system used in Alaska.

Stabilized materials for pavements fall into two broad categories:

- unbound materials (includes modified materials), and
- bound materials.

3.3.1 Unbound Materials

Granular stabilized materials, modified materials, and some asphalt-bound materials are considered as unbound granular materials for structural design purposes. There are no performance criteria available for these types of materials that can be checked during the structural design process. Unbound materials gain their load-spreading ability from a combination of internal friction and cohesion and are assumed to perform satisfactorily if they meet their respective materials specification requirements.

Subgrade materials are also considered, for design purposes, as unbound granular materials. If subgrades are stabilized to form a working platform or to increase their bearing capacity,

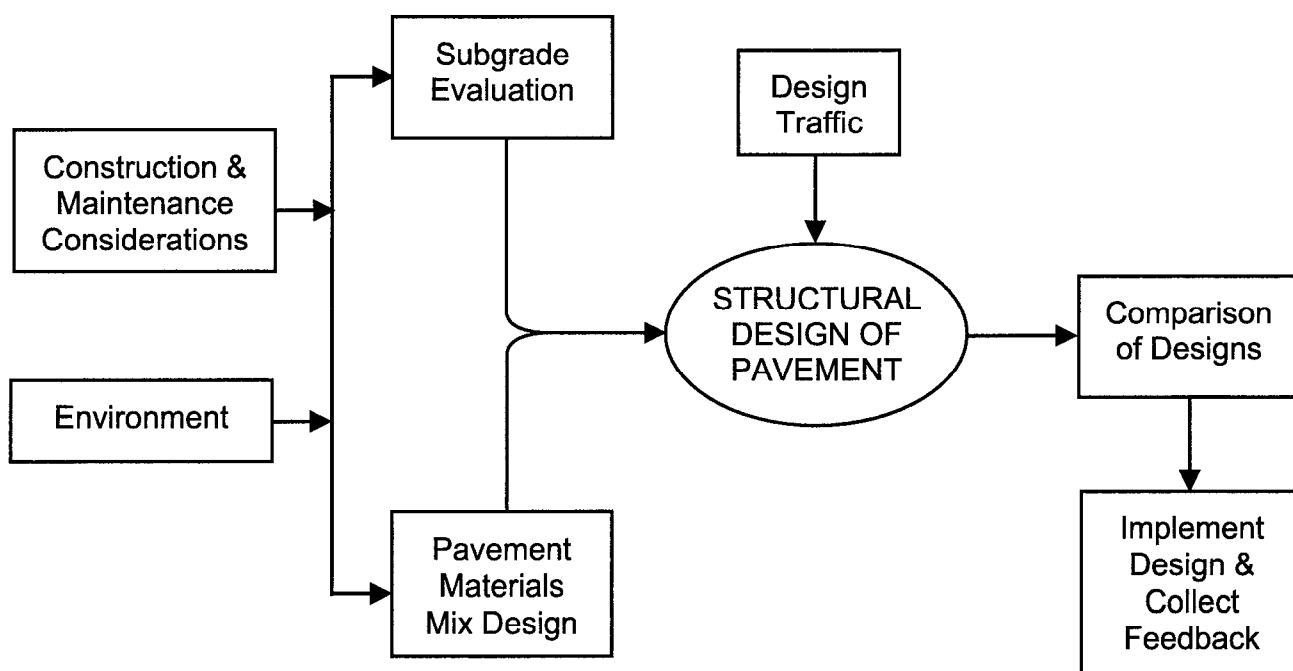


Figure 3.1. Structural Design System for Pavements with Stabilized Layers

then they are treated, for design purposes, as subbase layers. Unbound materials are considered to be anisotropic and their stiffness is stress dependent (that is, it varies depending on where it is located within the pavement structure).

3.3.2 Bound Materials

Bound materials can be classified as either cementitiously bound materials or asphalt-bound materials as described in the following sections.

3.3.2.1 Cementitiously Bound Material. Materials bound with sufficient amounts of cementitious binders to achieve significant tensile strength are considered cemented materials and should meet a fatigue performance criterion for structural design. For design purposes, cementitiously bound materials are considered to be isotropic and their stiffness is not stress dependent.

The fatigue behavior of cemented materials can be described by a relationship that takes the form

$$N = \left[\frac{K}{\sigma} \right]^a \quad (3.1)$$

where N is the number of stress repetitions to failure, K and a are constants dependent on

binder content, and σ is the tensile stress in the cemented material. For cemented materials, the value of the exponent a in the above equation is usually about 12, which means that a small change in the tensile stress in the cemented material will result in a large change in the fatigue performance of the material.

In other words, small changes in layer thickness (as a result of poor construction control or application of construction tolerances), density, or uniformity can lead to major deficiencies in long-term performance. This is why you should use conservative values of the materials characteristics of cemented materials for structural design.

Bonding between layers is another critical aspect of the design of pavements incorporating cemented layers. The Alaska DOT&PF mechanistic design procedure assumes there is full bonding between all layers. If cemented pavement courses are constructed in more than one layer, it is vital that these layers bond together to act as a single structural layer. The possible effect of not achieving bonding between stabilized layers is illustrated in Figure 3.2.

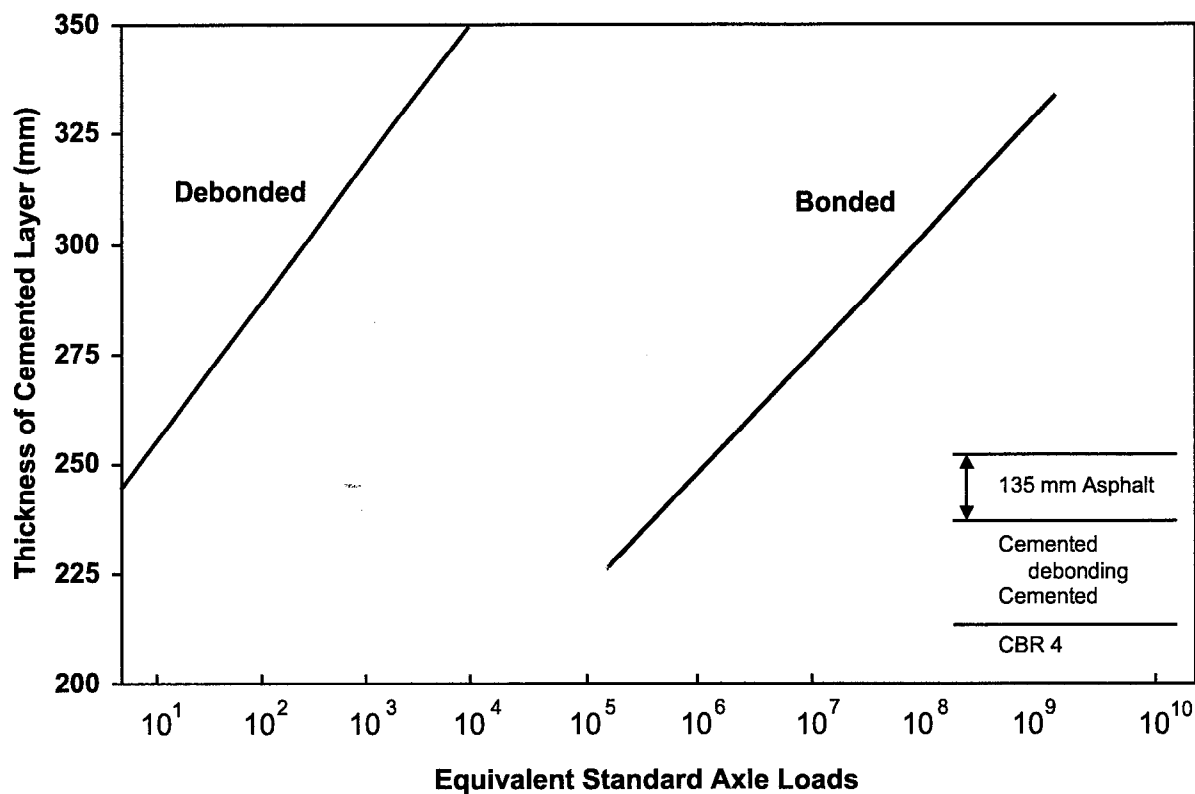


Figure 3.2. An Illustration of the Loss of Performance that Results from Debonding of Cemented Layers (Based on Mechanistic Modeling)

The structural design process results in design layer thicknesses for each pavement layer. For stabilized layers, you should consider the design thickness as a minimum construction thickness because of their sensitivity to curing, density, and uniformity. Apply appropriate construction tolerances to ensure this design thickness is achieved in the field.

3.3.2.2 Asphalt-Bound Materials. These are a combination of asphalt and aggregates that are mixed together, spread, and compacted to form a pavement layer. Some materials that are stabilized with asphalt, usually also with supplementary additives, behave structurally in a similar way to asphalt. This means that they can be classified for structural design purposes in the same way as asphalt. This characterization requires knowledge of stiffness, which may be dependent on temperature and rate of loading, Poisson's ratio, and a fatigue performance relationship in the same form as previously described for cemented materials.

3.3.3 Assessment of Existing Pavements for Recycling

Pavement recycling can also be considered a form of stabilization. Asphalt emulsions are generally used in this type of stabilization. You must investigate existing pavements that are being considered as candidates for recycling to determine their current structural capacity, composition, and variability. There are a number of ways to carry out this investigation work. Investigation may include one or more of the following techniques:

- measure pavement deflections and curvatures using falling weight deflectometer (FWD),
- assess bearing capacity using a dynamic cone penetrometer (DCP),
- excavate test pits to measure material properties and sample materials, and
- perform materials mix design in the laboratory.

Apart from the properties of the layer to be recycled, the structural properties of the subgrade and all the other layers are required as input into

the structural design process that again is carried out in accordance with Figure 3.1.

3.3.4 Design Traffic

Design traffic is the total traffic loading over the design period of the pavement. Keep in mind that cementitiously bound materials and asphalt-bound materials have different fatigue performance relationships. This means you will have to determine separately for each material the number of equivalent single axle loads (ESALs) that will cause the same level of accumulated damage as the actual traffic spectrum.

3.4 Construction Considerations

3.4.1 Stabilization Equipment

Stabilization may be carried out using one of two methods: (1) mix-in-place or in-situ stabilization, and (2) stationary or pugmill type stabilization. For in-situ stabilization, two types of equipment are generally used: an additive spreader and a mixer or reclaimer. The additive spreader is used to distribute the additive uniformly on the soil to be treated. These include spreaders that are capable of applying either dry or liquid additives. The mixer is used to thoroughly mix the additive with the soil. These machines come in various sizes and include

- motor patrols,
- rotary type mixers, and
- reclaimers/stabilizers.

For mixing in place with cement and lime, it is essential to have a water truck connected to the mixer to introduce water into the mixing unit. More information on the types of equipment that are used can be obtained from the manufacturers.

When stationary plants are employed, the materials and additives are blended through a pugmill, which then discharges the material into a truck. The blended materials are normally laid with a paver.

3.4.2 Compaction Equipment

Compaction is generally achieved with conventional compaction equipment. Static steel-wheeled compactors of at least 14 tons will be required when the pavement layers approach 200 mm (8 inches). Use care when using heavy vibratory rollers since they can damage underground structures or the stabilized layer.

3.4.3 Procedures and Operations

While the detailed procedures vary depending on the type of additive used, the following procedures are common in most circumstances:

- First, prepare the materials to be stabilized so that the final grade is as planned.
- Then spread the additive to the recommended quantity using an appropriate spreader.
- Next mix the additive into the host materials and add water as needed. Different types of additives have different requirements for degree of mixing.
- After mixing, begin compaction. In deep stabilization techniques, padfoot rollers are used until walk-out occurs. Smooth steel vibratory rollers are then used to complete the compaction process.
- While most pavements stabilized with a binder having a cementing component may be trafficked immediately after compaction, it is very important that the pavements be cured to ensure they have adequate strength to carry the traffic.

3.5 References

- Hicks, R.G., *Use of Layered Theory in the Design and Evaluation of Pavement Systems*, FHWA-AK-RD-83-8, ADOT&PF, July 1982.
- McHattie, Robert, Billy Connor, and David Esch, *Pavement Structure Evaluation of Alaska Highways*, FHWA-AK-KD-80-1, March 1980.
- "Designing Pavements for Alaska," course notes, Research & Technology Transfer, Alaska DOT&PF, Fairbanks, AK, December 2001.

4.0 ASPHALT STABILIZATION

4.1 General

Asphalt stabilization of pavement material is usually intended either to introduce some cohesion into nonplastic materials or to make a cohesive material less sensitive to loss of stability with increased moisture. Various bituminous materials can be used for this purpose. The process is more successful with granular material than with cohesive material. Asphalt stabilization is therefore primarily used on base and, to a lesser extent, subbase materials.

Using a mixture of bituminous and cementitious binders together has the advantage of improving strength as well as increasing cohesion and reducing moisture susceptibility. Using these types of stabilizing agents, even with poor quality pavement materials, improves performance.

4.2 Materials

4.2.1 Suitability of Materials

A relatively wide range of materials is suitable for asphalt stabilization, including materials that have been pretreated with lime. Figure 4.1 summarizes the broad selection process to determine an appropriate asphalt stabilizing agent, while Table 2.2 indicates material types suitable for stabilization with asphalt and asphalt/cement blends.

4.2.2 Asphalt Materials

Bituminous stabilization may be carried out with any of the following materials:

- hot asphalt cement,
- cutback asphalt,
- asphalt emulsion, either as cationic or anionic emulsion, and
- the above with cementitious binders used in conjunction.

Typical binder contents range from 4% to 8%.

4.2.3 Stabilization with Hot Asphalt

Stabilization with hot asphalt involves a temporary change of state of the stabilizing agent by

significantly increasing its surface area at the point of mixing. The two main methods are:

- foamed asphalt process, and
- high-impact process (HIP).

These processes require using specialized equipment to distribute the binder. They also eliminate the extra manufacturing process required in the use of cutback asphalt or asphalt emulsion. Stabilization can be carried out in place or in a central plant.

Several types of asphalt can be used in the hot process. Asphalt produced by the propane precipitation process includes an antifoaming agent. This ingredient must be neutralized before the asphalt can be used for foamed asphalt. Both foamed asphalt and the high impact process allow only a very short mixing time while the asphalt is in a finely dispersed condition. Mixing must be completed and the particles coated soon after the application of the binder.

Both methods require mixing to be carried out at or near optimum fluid (asphalt plus moisture) content. The fine aggregate particles are preferentially coated, leaving the coarse particles relatively uncoated with asphalt. You can place it immediately, but you must be careful with initial compaction to prevent instability. The foamed asphalt process results in materials with the desirable properties of asphalt mixtures: durability, flexibility (crack resistance, stabilization of fines against weakening by moisture), better cohesion, and decreased permeability.

The performance of hot asphalt stabilized materials varies with the quality of the material stabilized. A maximum plasticity index of 6–15 for the material to be stabilized is recommended for this type of treatment (Figure 4.1). Secondary additives such as fly ash, cement works flue dust, or lime may be added to alter the characteristics of the finished product or to make it more amenable to treatment with the asphalt binders.

Cementitious additives should not exceed 2% by mass, to avoid possible shrinkage cracking.

4.2.4 Stabilization with Cutbacks

Cutbacks are asphalt mixed with a light cutter oil, producing binders that are fluid at ambient temperatures. The cutback asphalt can be sprayed cold or with slight heating and mixed with pre-moistened soil. This method of stabilization results in a material that gains strength very slowly and as a result is not used very often. Environmental constraints often limit the use of cutbacks in urban areas.

4.2.5 Stabilization with Asphalt Emulsion

Asphalt emulsions may be readily mixed with damp soil to produce a good dispersion of asphalt throughout the soil. Asphalt emulsions are most widely used for soil stabilization.

4.2.5.1 Classes and Types of Emulsion. Asphalt emulsions are manufactured to comply with Alaska Standard Specifications 702, which allows for two classes, depending on the charge of the suspended particles:

- anionic asphalt emulsion, where the particles of asphalt are negatively charged, or
- cationic asphalt emulsion, where the particles of asphalt are positively charged.

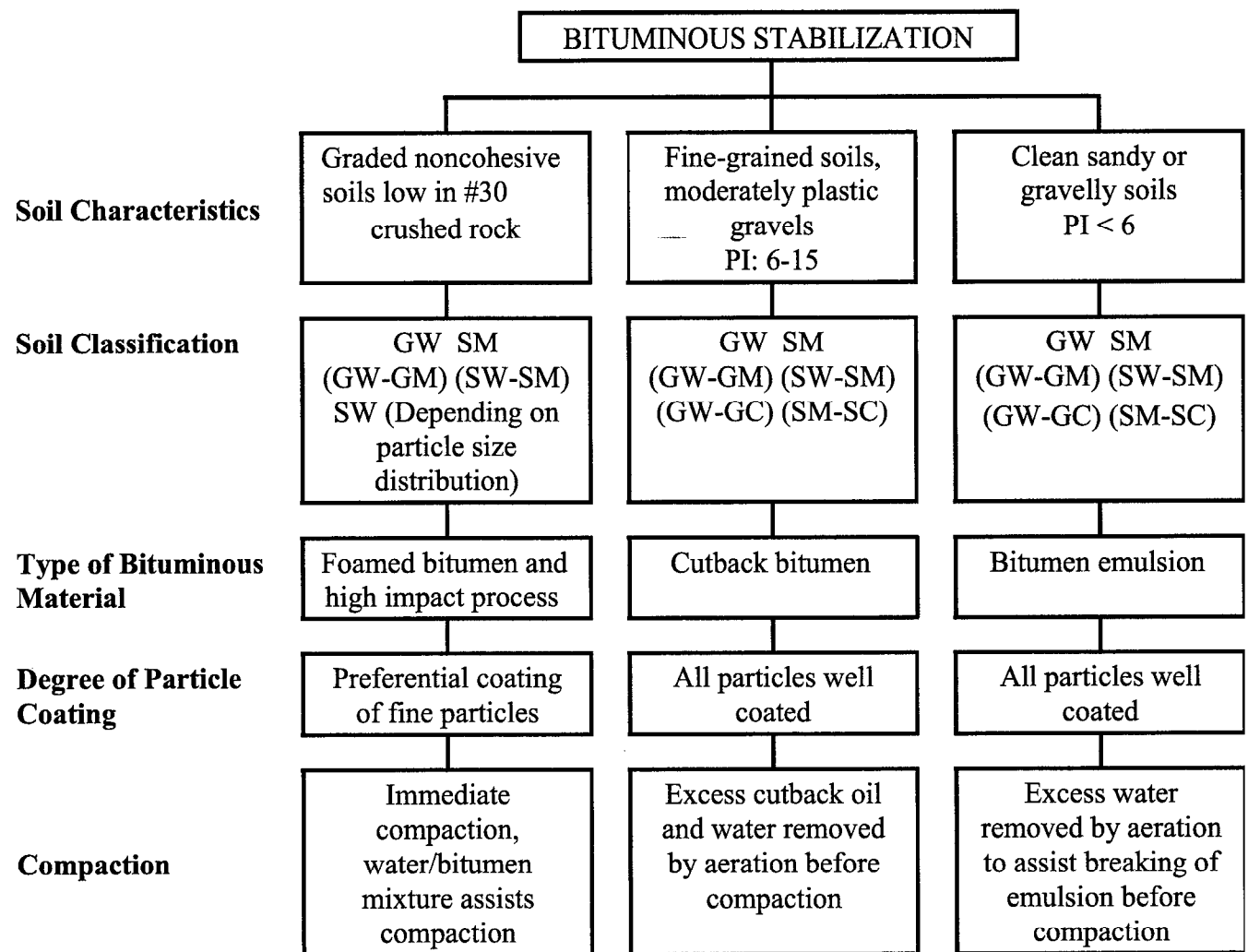


Figure 4.1. Guide to the Selection of Asphalt Materials for Stabilization

Both classes of asphalt emulsion are prepared in three grades, rapid setting (RS), medium setting (MS), and slow setting (SS). Only the medium- and slow-setting grades are suitable for use in stabilization.

Most manufacturers make all classes and grades. However, emulsions of the same class made by different manufacturers may react differently with the same soil. Therefore, it is important to test the emulsion first in the laboratory.

Asphalt emulsion is normally manufactured with 120–180 pen asphalt, which is usually satisfactory for soils with lower fines content (0–10% passing the 75- μ m sieve). For soils with higher fines content (15–25% passing the 75- μ m sieve), it may be necessary to use a softer asphalt if you have difficulty in distribution. Under extreme conditions, emulsion containing oil may have to be used (for example, CMS-2S).

Application rates of 2 to 3%* of residual binder are commonly used. Lower rates of about 0.5% to 1% may be satisfactory for well-graded materials in dry climates. Lower application rates, when added to granular base materials, can be useful as a construction expedient to reduce raveling and potholing under traffic. In any case, determine the application rate by laboratory testing.

If there are no other data available, a guide to the amount of emulsion to form a heavily bound material may be obtained from the following equation (Asphalt Institute, 1989):

$$\% \text{ (by mass)} = 0.75 (0.05 A + 0.10 B + 0.50 C) \quad (4.1)$$

where:

A = % retained on 2.36 mm (#8) sieve,

B = % passing the 2.36 mm (#8) sieve but retained on the 75 μ m sieve (#200), and

C = % passing the 75 μ m (#200) sieve.

4.2.5.2 Conditions of Mixing. The soil moisture content influences how efficiently the emulsion gets distributed throughout the soil. Dry soil

causes the emulsion to break prematurely, resulting in the asphalt forming blobs and not spreading evenly. As the moisture content of the soil increases, the tendency of the emulsion to break prematurely is reduced and the time of mixing can be extended to enable improved distribution. If the moisture content becomes too high, you may have to aerate the soil to remove excess moisture before compaction begins. Once the soil has been compacted, it is difficult to get any more water to penetrate.

4.2.6 Additives

You can improve the stabilization of gap-graded granular materials and/or materials with smooth rounded grains by adding mineral filler, rock dust, fly ash, etc. Hydrated lime or portland cement (1 to 2%) may also be added as a secondary additive to improve particle coating. You can also use lime as a preliminary modifying treatment to render particular soils more amenable to receiving an asphalt stabilizing agent (for example, asphalt emulsions).

You can improve the bond between soil particles with asphalt binders by using surface active agents or antistripping additives. These agents usually improve the wet strength and water absorption resistance, and you can mix them with the soil before adding the asphalt binder or combine them with the asphalt binder before use. The proportion of such additives is usually only about 0.3 to 1.0% by mass of the stabilizing agent.

You can successfully use a mixture of portland cement and asphalt to improve the properties of low-grade pavement materials by increasing stiffness and reducing permeability. Also, adding cement promotes the removal of excess water and helps the emulsion to break.

4.2.7 Water Quality

The quality of compaction water used in asphalt stabilization is not critical. Salty water has been used with no apparent harm. You may have to be

* This is often limited by the natural moisture content of the aggregate.

careful to prevent damage by salt accumulation, which may lead to asphalt surfacing failures. You should avoid using salt water to dilute emulsions.

4.3 Mix Design

4.3.1 General

You can design asphalt-stabilized materials in a number of ways. Historically, such design has relied on strength and water absorption testing. However, a similar approach that is now being used for cementitious stabilized materials and asphalt is gaining in acceptance.

The mix design for asphalt stabilized materials must ensure the best composition of mix components to meet the mix design criteria and to realize the structural parameters required. Your assessment of the materials should follow three basic steps for all types of asphalt stabilization:

1. Test and classify the material to be stabilized (grading and Atterberg limits).
2. Select the type of additive, depending on material to be stabilized, climate, and construction equipment availability and determine the laboratory optimum fluid content (OFC) and compacted density.
3. Determine target residual asphalt contents, optimum moisture content, and required density.

The Asphalt Institute design manuals (1989, 2000) give detailed guidance of the procedures involved. The various factors that affect the behavior and design of asphalt stabilized materials are shown in Figure 4.2.

4.3.1.1 Classifying Material. Determine grading and Atterberg limits in accordance with AASHTO test methods. If you plan to use lime, cement, or other secondary additives in the stabilization process, you should incorporate them in the material to be tested.

4.3.1.2 Determining Laboratory Optimum Fluid Content and Compacted Density. Optimum fluid content (OFC) is defined as the fluid content at which maximum dry density is

achieved where the total fluid content consists of the asphalt stabilizing agent plus compaction moisture. You should do this testing on the likely blend to be used in the field.

4.3.1.3 Determining Target Residual Asphalt Content. The target residual asphalt content will depend on the performance required for the stabilized material in the field as well as the economics of the mix. The testing regime to determine the target residual asphalt content will depend on the performance criteria adopted. For heavily bound materials, you may need to perform stiffness and indirect tensile testing, as well as an assessment of fatigue performance, to optimize structural performance. For lightly bound materials, you should assess stiffness and deformation resistance or Marshall stability and flow.

You also need to assess water absorption, usually by capillary rise of water in compacted cylinders that have been oven dried at 60°C. Evaluate moisture sensitivity by either long-term soaking or vacuum soaking. In any case, try to make the testing regime reflect construction, curing, and performance conditions likely to occur in the field.

4.3.2 Testing and Design Criteria

For the purpose of pavement design, asphalt stabilized materials are either unbound, modified, or bound materials, depending on the type and quantity of binder. Poisson's ratio would vary between 0.20 and 0.45 (0.20 for bound, 0.45 for unbound).

As a guide to selecting suitable binder contents for preparing test specimens, soils and granular materials normally require adding 2 to 5% by mass of residual asphalt. You can use the diametral modulus test to test for resilient modulus (Figure 4.3), deformation, and fatigue characteristics of asphalt stabilized materials.

If the diametral test is not available, use the indirect tensile strength (ITS) test to obtain an indication of resilient modulus.

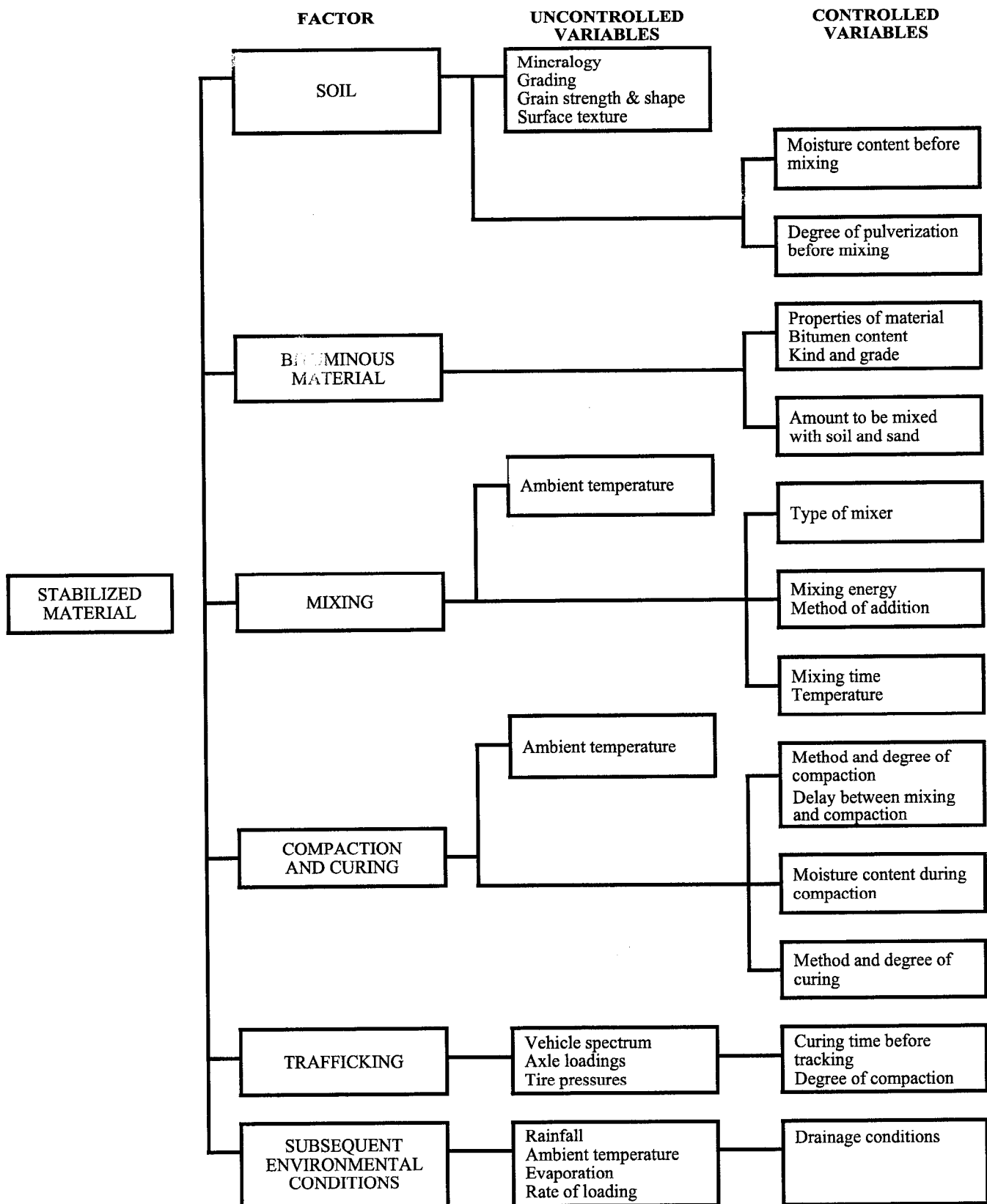


Figure 4.2. Factors Affecting the Design and Behavior of Asphalt Stabilized Materials

Alternatively, the resilient modulus (E) may then be estimated using the equation:

$$E \text{ (MPa)} = 2.2 \times \text{ITS (kPa)} + 168 \quad (4.2)$$

A minimum value of ITS of 100 kPa (14.5 psi) is recommended.

4.4 Construction Factors Affecting Design Considerations

You should stabilize with emulsion and cutback asphalts when conditions are dry and warm. In hot, dry areas, medium- to slow-setting cutback asphalts can be used, depending on the soil type, but in cooler areas, medium- to rapid-setting cutbacks would be required.

Foam asphalt stabilization is not subject to climatic restrictions for mixing and compaction. You must in all cases prewet the soil with water to achieve better dispersion of the asphalt binder. It is quite difficult to add water after the mixing has been carried out.

When preparing test specimens of mixes incorporating asphalt binders other than hot asphalt, it is essential that you aerate the mixture before compacting. The time of aeration should be enough for the excess water to escape from emulsions and volatiles from cutback asphalt. Don't stabilize with asphalt binders, particularly

cutbacks and emulsions, if rain is likely before the process is completed.

To achieve good results with asphalt-stabilized materials in the field, you must

- thoroughly mix the stabilizing agent throughout the soil,
- ensure that the soil is compacted at a uniform moisture condition, and
- ensure adequate aeration of emulsion and cutback-stabilized materials to allow the excess moisture and/or volatiles to escape.

Asphalt stabilized materials are much slower setting than cementitiously treated materials, and you must not allow traffic on these materials until they have gained adequate strength. A limited amount of controlled traffic after setting is advantageous before sealing.

4.5 Expected Performance and Costs

Asphalt stabilized materials (using emulsions) have been widely used in Alaska with varying degrees of success. Local materials have been stabilized or pavements recycled using asphalt emulsions. Typical costs of using emulsions in highway construction in Alaska are not available.

4.6 References

- Asphalt Institute, *Asphalt Cold Mix Manual*, Manual Series No. 14, 3rd Edition, 1989.
- Asphalt Institute, *A Basic Asphalt Emulsion Manual*, Manual Series No. 19, 3rd Ed., March 2000.
- Asphalt Recycling and Reclaiming Association, *Basic Asphalt Recycling Manual*, 2001.
- Chevron USA, *Bituminous Mix Manual*, Chevron Asphalt Division, San Francisco, 1977.
- Puzinauskas, V. P., and R. N. Jester, "Design of Emulsified Asphalt Mixtures," *NCHRP Report 259*, Transportation Research Board, 1984.
- Salomon, A., and D. E. Newcomb, *Cold In-Place Recycling Literature Review and Mix Design Procedure*, MN/RC-2000-21, Minnesota DOT, August 2000.

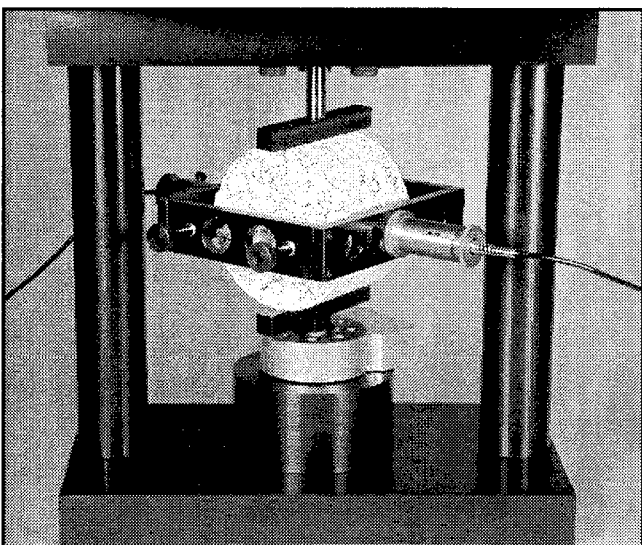


Figure 4.3. Diametral Modulus Testing
(Courtesy of OEM, Corvallis, OR)

5.0 CEMENT STABILIZATION

5.1 General

Cement stabilization refers to stabilizing soils with portland cement. The primary reaction is with the water in the soil that leads to the formation of a cementitious material. These reactions occur almost independently of the nature of the soil and for this reason portland cement can be used to stabilize a wide range of materials.

Although there are several types of cement-stabilized soils, there are two types associated with highway construction:

1. Soil-cement—it contains enough cement (usually > 3%) to pass standard durability tests and achieves significant strength increase.
2. Cement-modified soil—an unhardened or semihardened mixture of soil, water, and small quantities of cement.

In Alaska, soil-cement is the primary product used and it is the only one discussed in this chapter.

5.2 Materials

5.2.1 Types of Cement

Portland cement is a finely ground inorganic material that possesses strong hydraulic binding action when mixed with water to produce a stable, durable product. Several different cement types have been successfully used for cement stabilization of soils:

1. normal portland cement (Type I),
2. sulfate resistant (Type II), and
3. high early strength (Type III).

The most common cement used in Alaska is Type I. The portland cement used for stabilization should conform to Alaska Specifications, Section 701.

5.2.2 Soil-Cement Reactions

Regardless of the type used, the portland cement acts both as a cementing agent and a modifier. In primarily coarse-graded soils, the cement paste

bonds the soil particles together by surface adhesion forces between the cement gel and the particle surfaces. In fine-grained soils, the clay phase may also contribute to the stabilization process through reaction of the free lime from the cement. In this manner, the cement acts as a modifier by reducing the plasticity and expansion properties of the soil.

5.2.3 Soils Suitable for Cement Stabilization

A wide range of soil types may be stabilized using portland cement (see Table 2.2 and Figure 2.1). It is generally more effective and economical to use it with granular soils due to the ease of pulverization and mixing and the smaller quantities of cement required. Fine-grained soils of low to medium plasticity can also be stabilized, but not as effectively as coarse-grained soils. If the PI exceeds about 30, cement becomes difficult to mix with the soil. In these cases, lime can be added first to reduce the PI and improve workability before adding the cement.

5.3 Design Considerations

5.3.1 Mix Design

Table 5.1 identifies the usual cement requirements for soil-cement for various soil types. You can select an approximate cement content from this table. However, note that the cement content ranges are for soil-cement, a hardened material that passes rather severe durability tests.

For major projects, you should use a more detailed testing program, as shown in Figure 5.1. Detailed test procedures are given in the *Soil-Cement Laboratory Handbook* (PCA, 1971). Criteria for satisfactory performance of soil cement in the durability tests are listed in Table 5.2.

5.3.2 Structural Design

High-strength stabilized materials have been used in many asphalt pavements throughout the world. The typical pavement section includes a minimum

Table 5.1. Cement Requirements for Various Soils

AASHTO Soil Classification	Unified Soil Classification*	Usual Range in Cement Requirement		Estimated Cement Content and That Used in Moisture-Density Test Percent by Weight	Cement Contents for Wet-Dry and Freeze-Thaw Tests Percent by Weight
		Percent by Volume	Percent by Weight		
A-1-a	GW, GP, GM, SW, SP, SM	5–7	3–5	5	3–5–7
A-1-b	GM, GP, SM, SP	7–9	5–8	6	4–6–8
A-2	GM, GC, SM, SC	7–10	5–9	7	5–7–9
A-3	SP	8–12	7–11	9	7–9–11
A-4	CL, ML	8–12	7–12	10	8–10–12
A-5	ML, MH, CH	8–12	8–13	10	8–10–12
A-6	CL, CH	10–14	9–15	12	10–12–14
A-7	OH, MH, CH	10–14	10–16	13	11–13–15

*Based on correlation presented by Air Force.

Table 5.2. Criteria for Soil-Cement as Indicated by Wet-Dry and Freeze-Thaw Durability Tests

AASHTO Soil Group	Unified Soil Group	Maximum Allowable Weight Loss, Percent
A-1-a	GW, GPP, GM, SW, SP, SM	14
A-1-b	GM, GP, SM, SP	14
A-2	GM, GC, SM, SC	14
A-3	SP	14*
A-4	CL, ML	10
A-5	ML, MH, CH	10
A-6	CL, CH	7
A-7	OH, MH, CH	7

*The maximum allowable weight loss for A-2-6 and A-2-7 is 10%.

thickness asphalt concrete surface course over the stabilized base. In many applications, only an asphalt surface treatment is used. You can use the AASHTO design procedure to establish thickness requirements; however, the mechanistic-empirical design method is preferred.

Cementitious stabilizers typically increase the strength properties and modulus of elasticity. Also, stabilization enhances freeze-thaw and moisture resistance. Flexural strength, modulus, and thickness of the stabilized layer, as well as the subgrade modulus and strength, influence the structural response and performance of cement stabilized layers. Some agencies limit the compressive strength to 700 psi maximum to minimize the potential for shrinkage cracks.

5.3.2.1 Strength Values. The strength of the stabilized material is a fundamental property of design and is often specified and used for con-

struction control. The types of tests normally used are the flexural beam test, the split tensile test, and the unconfined compression test. The latter is normally used because of its simplicity.

5.3.2.2 Stress-Strain Relationship. This behavior is normally expressed in terms of an elastic or resilient modulus. For cement-stabilized soils, selecting an appropriate modulus value is complicated because:

- different test methods give different values,
- the relationship can be nonlinear at higher stress levels, and
- the modulus is lower in tension than in compression.

Because of this, one can use a relationship between compressive strength and modulus of elasticity in lieu of testing.

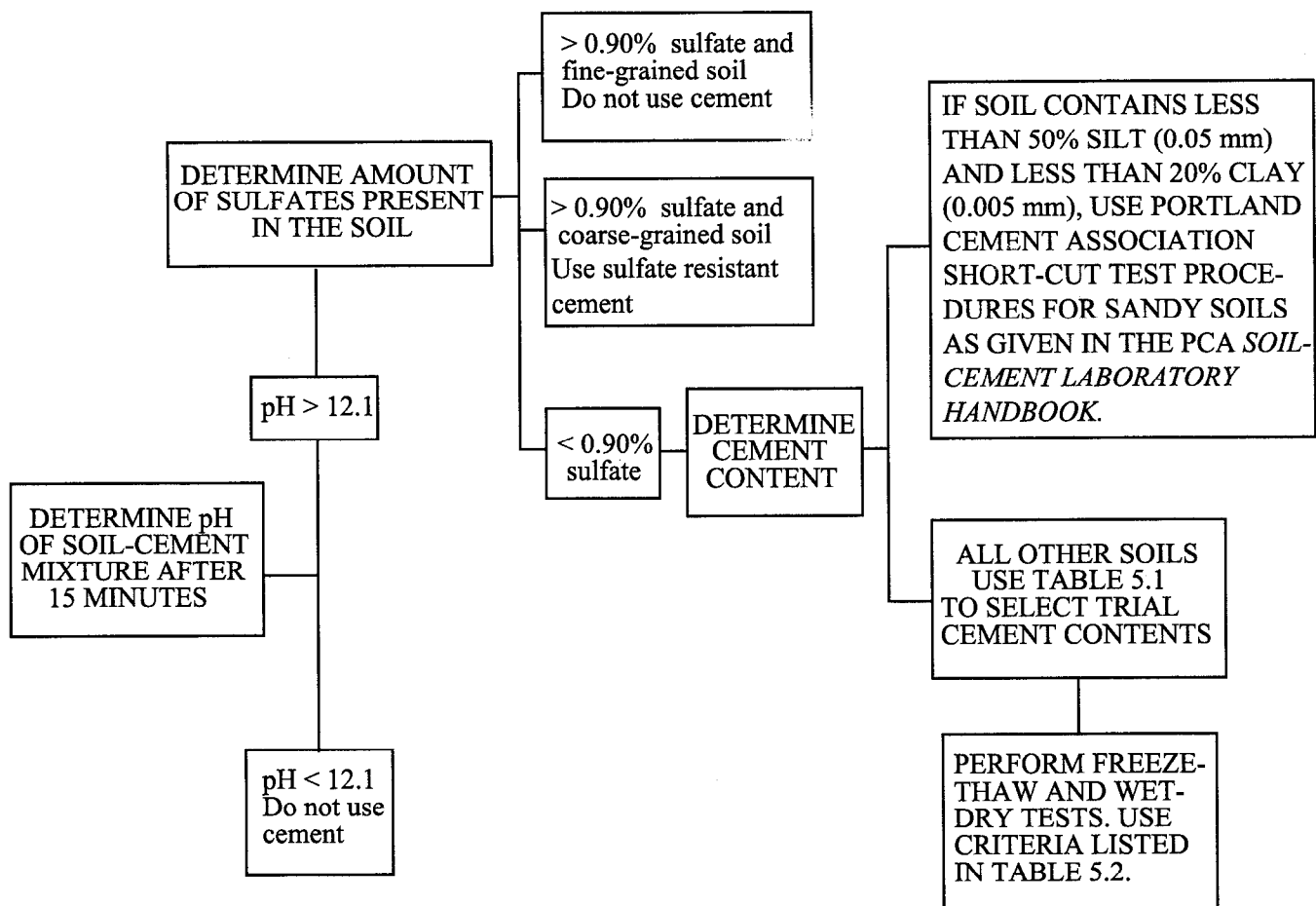


Figure 5.1. Subsystem for Base Course Stabilization with Cement

$$E = 1000 (f_c) \quad (5.1)$$

where

E = modulus of elasticity, psi

f_c = unconfined compressive strength, psi

5.3.2.3 Fatigue Characteristics. The fatigue characteristics of cement stabilized materials are normally reported in terms of stress ratio vs. number of repetitions as shown below:

$$\text{Log } N = (0.9722 - S)/0.0825 \quad (5.2)$$

where

N = allowable number of repetitions

S = flexural stress/flexural strength

This relationship, developed by the PCA, is also illustrated in Figure 5.2. You can also use this relationship to establish the thickness requirement when employing a mechanistic design approach.

5.4 Construction

When working with cement, always follow the manufacturer's instructions and seek advice about worker safety. The Portland Cement Association

is a good source of information for construction of soil-cement types of pavements (see website www.portcement.org).

5.5 Expected Performance and Costs

Cement stabilized materials have been widely used in parts of Alaska with varying degrees of success. Local materials have been stabilized for use in highways and airfields. Typical costs of using portland cement are not available.

5.6 References

- American Concrete Institute, State of the Art on Soil Cement, *ACI Materials Journal*, July-Aug. 1990.
- Portland Cement Association, *Soil-Cement Laboratory Handbook*, 1992.
- Portland Cement Association, *Soil-Cement Construction Handbook*, 1995.
- Portland Cement Association, *Thickness Design for Soil-Cement Pavements*, 1990.
- Raad, L., *Design Criteria for Soil-Cement Bases*, Ph.D. dissertation, University of California, Berkeley, 1976.

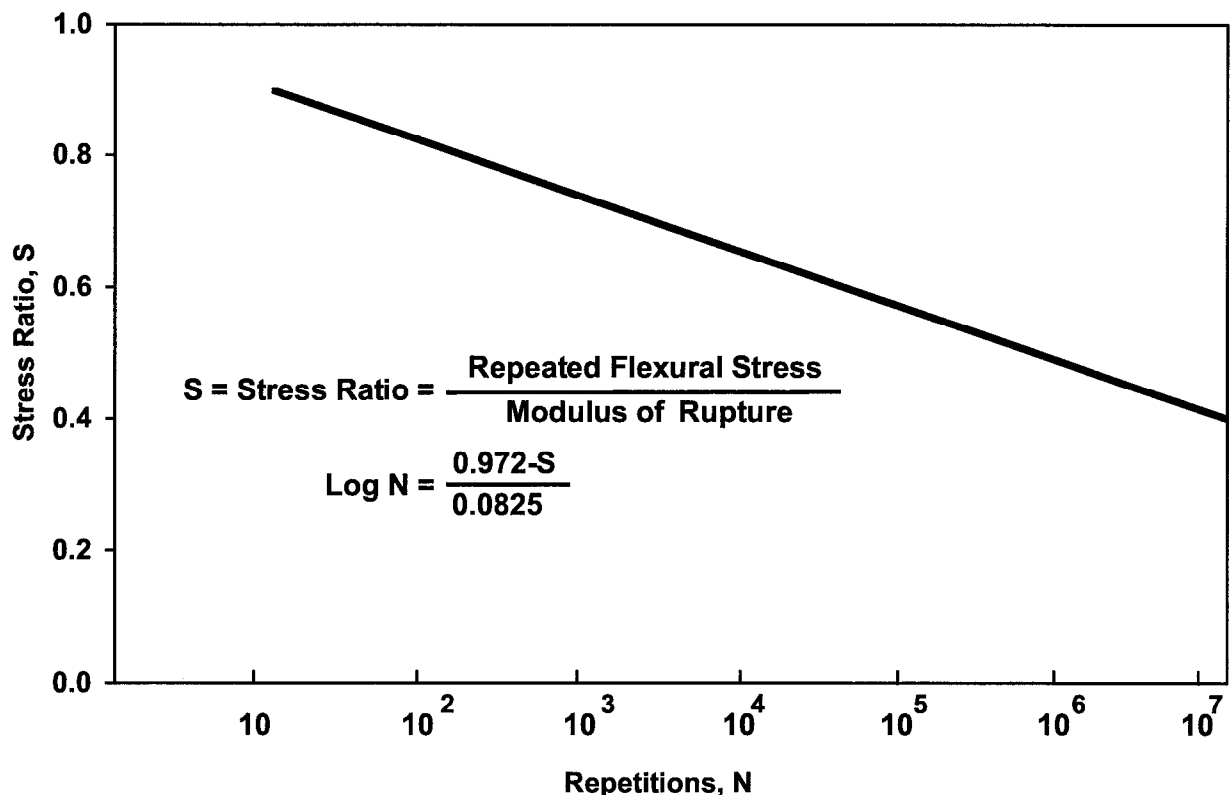


Figure 5.2. Recommended Stress Ratio-Fatigue Relation for Cement-Stabilized Materials

6.0 LIME AND LIME/FLY ASH STABILIZATION

6.1 General

Lime is an effective additive for plastic soils, improving both workability and strength. It is not effective in cohesionless or low cohesion materials without the addition of pozzolanic additives. There are many similarities between materials stabilized with cementitious stabilizing agents and lime. They have similar composition, resulting in comparable behavior, and require similar materials characterization, structural design procedures, and construction considerations.

However, there are significant differences in the nature and rate of the cementitious reactions, and these differences often provide a basis for choice between cementitious stabilizing agents and lime.

6.2 Materials

6.2.1 Types of Lime

Lime comes in a number of forms:

- hydrated (or slaked) lime (calcium hydroxide),
- quicklime (calcium oxide);
- dolomite lime (calcium/magnesium oxide), and
- agricultural lime (calcium carbonate).

Agricultural lime is not suitable for stabilization, and dolomitic lime is not usually as effective as hydrated lime or quicklime.

All commercial lime products have impurities such as carbonates, silica, alumina, etc., which dilute the active additive but are not harmful to the stabilization reaction.

Hydrated lime comes as a dry, very fine powder or as a slurry. The water contents of common lime slurries can range from 80 to 200%. Quicklime and dolomitic limes are commonly much more granular than the hydrated products and are available only as a dry product. These forms of lime react rapidly with available water, producing hydrated lime and releasing considerable amounts of heat. Table 6.1 summarizes the properties of lime. Lime contents are expressed as the equivalent of 100% pure hydrated lime.

6.2.2 Reactions

Because the oxide reacts immediately with any available water to form hydroxides, the main reactions between all common lime types and materials are alike. Adding lime results in the following:

- It has an immediate effect on clay, improving its grading and handling properties by promoting flocculation of the clay particles. The effect varies with the actual clay minerals present (that is, it depends on the degree of pozzolanic material in the soil). The effect is large with montmorillonite group clays and low with the kaolinite clay groups.

Table 6.1. Properties of Different Types of Lime

	Hydrated Lime	Quicklime	Slurry Lime
Composition	Ca(OH)_2	CaO^*	Ca(OH)_2
Form	fine powder	granular	slurry
Equivalent Ca(OH)_2 /Unit Mass (Available Lime)	1.00	1.32	0.56 to 0.33**
Bulk Density (tons/yd ³)***	0.32 to 0.39	0.73	0.875

* $\text{CaO} + \text{H}_2\text{O} \rightarrow \text{Ca(OH)}_2 + \text{Heat}$

** Moisture contents of slurries can vary from 80% to over 200%

*** tons/yd³ = 0.695 tonnes/m³

- It has long-term strength gains.
- It allows reduced pavement thickness since the stabilized material can be treated as a base/subbase layer.

Long-term strengthening (pozzolanic reactions) occurs in the highly alkaline environment ($\text{pH} > 12.3$) that promotes the dissolution of the clay, particularly at the edges of the clay plates, and permits the formation of calcium silicates and aluminates at these sites. These cementitious products are similar in composition to those of portland cement.

This process is relatively slow because the available lime has to diffuse through both the matrix of the material and the initial cementitious products. The stabilization reactions cannot proceed unless there are clays or some other pozzolanic material within the pavement that will react with the lime. Lower temperatures (below 15°C) slow the lime-pavement material reactions, and high organic contents similarly impede those reactions.

6.2.3 Properties of Clays Stabilized with Lime

For clays, the effect of lime on volume and moisture stability, strength, and elastic behavior is similar to the effects of cement. The following sections deal only with those aspects of materials stabilized with lime that are significantly different from materials stabilized with portland cement.

Rate of Strength Gain

For materials stabilized with lime, the rate of strength gain (tensile strength or UCS) is considerably less than with cementitiously bound materials. Materials stabilized with lime and supplementary cementitious materials will continue to gain strength with time, provided curing is sustained. The rate of strength gain is temperature sensitive and also depends on the lime content. Therefore, exercise caution in accepting results of high-temperature accelerated testing without validation at field temperatures. High temperatures can cause other types of bonds to form that would not normally occur in the field.

Accelerated curing temperatures should not exceed 40°C .

Lime-stabilized materials are usually evaluated at both 7 and 28 days. High lime contents will not necessarily produce high early strengths. Figure 6.1 illustrates the variations in strength with time and lime content for lime-stabilized materials.

Moisture-Density Relationships

Increasing lime content increases the optimum moisture content of the material being stabilized, due to the fine-grained nature of hydrated lime. This effect is further increased by delaying compaction once the lime is added (see Figure 6.2).

6.3 Design Considerations

6.3.1 Appropriate Conditions for Stabilization with Lime

As with all forms of stabilization, there are two areas of consideration:

- material factors, dealing with the composition of the material to be stabilized and its response to lime, and
- production factors.

Material Factors

For lime to be effective, the material being treated must contain clay particles or pozzolanic materials that are reactive to lime. In general, the more plastic the clay fines and the higher the clay content, the larger the lime content required to produce a specific strength gain or other effect. However, the amount of bonding achievable with lime is limited by the amount of reactive material. Assess the initial lime demand for the soil to be stabilized. Then increase the lime slightly. This ensures that you achieve the stabilized long-term properties after the initial reaction of the lime with the soil.

The advantage of using lime instead of cementitious stabilizing agents increases with increasing plasticity and fines content. Generally, soils with a $\text{PI} < 10$ will respond better to cementitious binders. For plasticity index (PI) reduction and workability improvement using

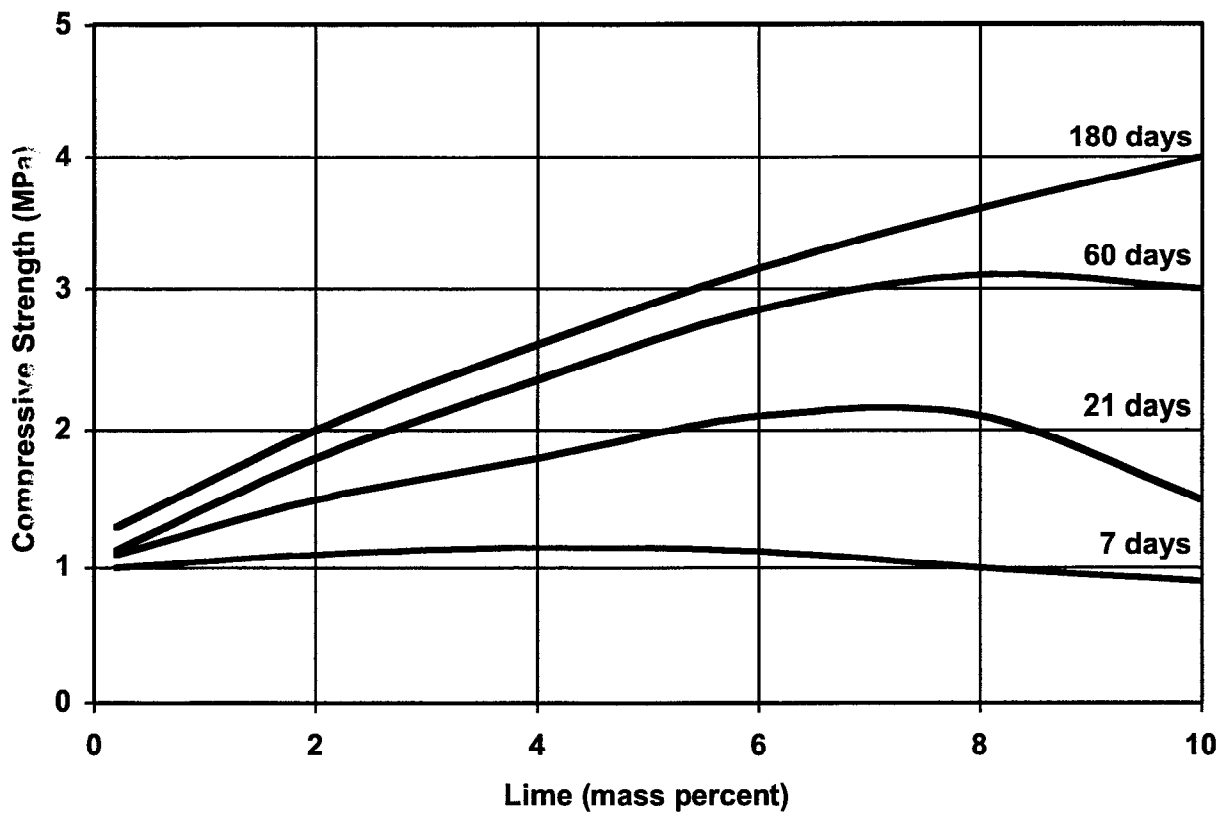


Figure 6.1. Variation in Compressive Strength as a Function of Lime Content and Time

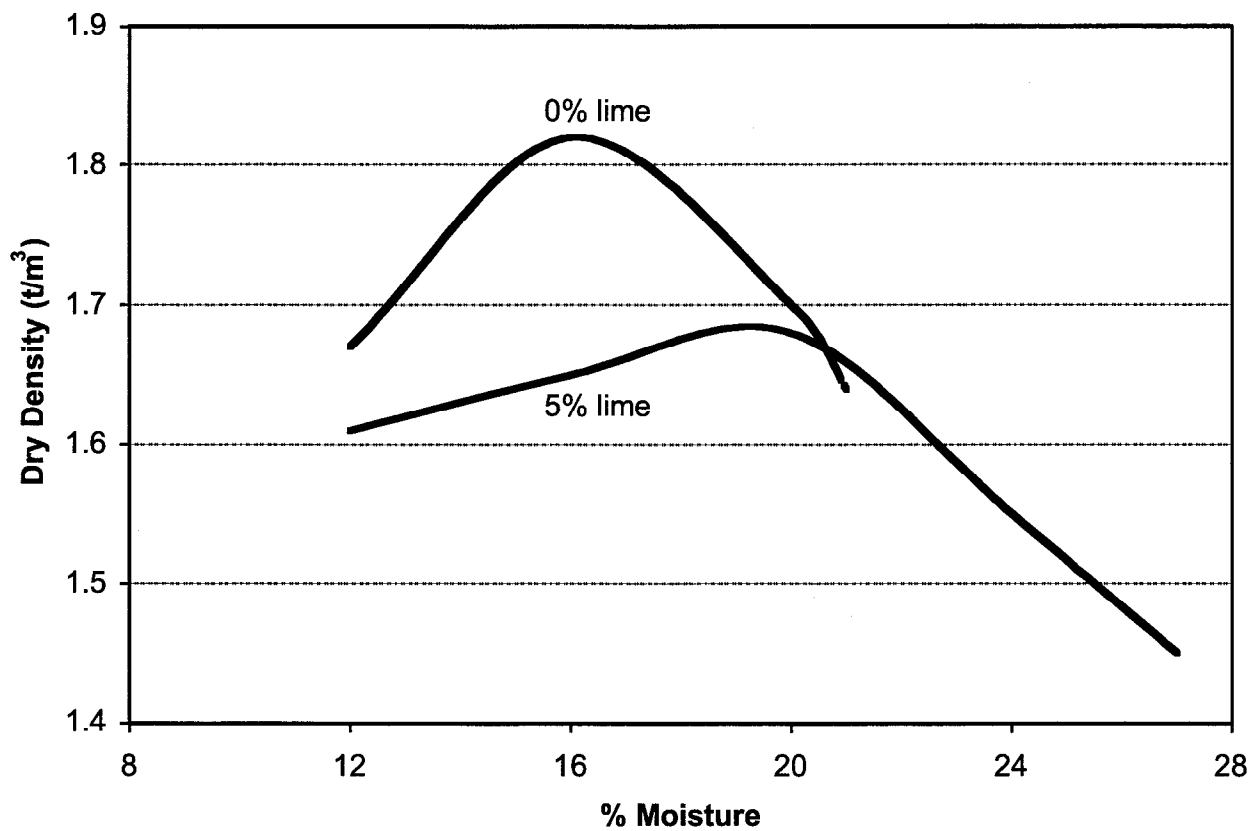


Figure 6.2. Effect of Lime on Optimum Moisture Content and Density

lime modification, add enough lime so that additional quantities do not result in further changes in PI.

For lime stabilization, use pH testing to determine whether a soil is reactive to lime and to estimate an approximate lime content, augmented by 28-day unconfined compressive strength (UCS) testing to establish the optimum lime content. The optimum lime content occurs when the plot of UCS vs. lime content peaks. An additional 1% is usually used to allow for losses and mixing variations. Sugars and reactive organic materials can retard the development of cementitious bonds with both cementitious binders and lime.

Production Factors

The following factors significantly affect the quality of lime-stabilized materials:

- quality of water,
- quality of lime,
- uniformity of mixing and curing,
- compaction, and
- clay content.

Adding lime normally promotes granulation of the material being stabilized. In materials that are difficult to break down, the lime-material mix is sometimes moist cured, from a few hours to a day, after light rolling to reduce contact with air, and then remixed. The initial lime addition may be a portion or the whole of the design lime content. This process is sometimes called mellowing.

Lime will diffuse slowly throughout clays and stabilize the lumps. Unless high early strength is particularly important, it is unnecessary to seek fine granulation. About 80 to 90% of the soil's clods should pass the 26.5-mm sieve. If temperatures are low at the time ($< 15^{\circ}\text{C}$), then more attention should be given to breakdown.

Using quicklime to establish a working platform on a wet clay is a useful construction expedient and uses the exothermic reaction of the lime as it hydrates to reduce the moisture content of the soil.

Compaction Process

The initial rate of reaction with lime allows time to achieve adequate compaction and riding qualities of lime-stabilized materials. If you are seeking high strengths, you need to perform early compaction to achieve as high a density as possible. Delayed compaction lowers the density but this is not as severe as cementitious binders.

6.3.2 Evaluation and Use of Materials Stabilized with Lime

The evaluation techniques and methods for materials stabilized with lime are similar to those used for cementitious binders. Lime is often used to modify materials, particularly those with high plasticity. If modification without achieving high strengths is the aim, the stabilized material can be reworked one or two days after initial compaction. If high strengths are required, you need to exercise careful control over field procedures, particularly moisture control, early rolling, and effective curing. Hydration cracking of lime-stabilized materials is not usually a major problem.

The similar range of materials for subgrade, subbase, and base can be treated with lime or cement. Certain conditions will favor the use of lime. Quicklime and, to a lesser extent, hydrated lime are particularly suitable for treating wet plastic clay subgrades. They provide effective working platforms from otherwise untraffickable situations. Lime slurry is not suitable for this application.

Lime is effective in modifying excessive plastic properties of subbase and base materials. Such modification of base materials is a widely accepted and successful practice. At lime contents of less than about 3%, the risk of undesirable shrinkage cracking is low, and it would rarely be necessary to take special measures to combat reflective cracking.

The use of lime slurry may have advantages in urban areas since it reduces environmental issues

such as skin irritation to workers and passersby during hot and/or windy conditions.

For lime pozzolan stabilization and other supplementary cementitious materials, the lime and the pozzolan (or other component additives) are dependent variables. This requires a comprehensive testing program to determine the optimum lime-to-pozzolan ratio (or ratio of other components). In this regard, take into account the following:

- the costs involved for each of the additive components, and
- the need for filler to correct a particle size distribution deficiency.

6.3.3 Choice of Lime Type

While the type of lime does not appear to be significant for determining the long-term structural properties of the stabilized materials, it has considerable influence on the construction processes. In selecting the type of lime for any particular job, take into account the following:

- Nuisance—hydrated lime can cause a dust problem even with very light winds. Minimize its use in urban or windy areas. Dust is not a serious problem with quicklime. Lime slurry is dust free.
- Soil moisture—quicklime and hydrated lime are effective in drying out wet soils, but slurry limes cannot be used for this purpose. Slurry limes are very suited to dry soil conditions where water may be required to achieve effective compaction.
- Lime content—if the content of the additive is to be kept low, quicklime is particularly effective. The total amount of lime slurry will usually be limited by the soil moisture content and, in general, lime slurry is limited to low additive contents (< 3%) and dry construction conditions.
- Available equipment and expertise—these factors are always important. Automated spreaders, adequate mixing, and compaction equipment are essential to achieve good results.

While quicklime can be immediately mixed into wet subgrades without problems, you should allow the lime to hydrate before it is mixed into subbase and base materials because unhydrated particles of quicklime may cause expansion with possible blowouts in the compacted materials.

6.4 Construction Considerations

The National Lime Association recommends that lime stabilization be conducted only when the temperature is above 40°F (and rising). Lime modification can be used in colder temperatures. Hydrated lime should not be applied on frozen ground. Lime-stabilized bases should be completed one month before the first hard freeze.

Basic instructions on first-aid procedures and appropriate facilities—such as protective creams, burn creams and ointment, fresh water, and eyewash glasses—should be available for the safe handling of these materials, particularly in hot weather.

6.5 Expected Performance and Cost

Lime has not been widely used in Alaska because of the lack of appropriate soils. Lime could be used with silty sands or sandy gravels if fly ash is added to facilitate the pozzolanic reaction. There is no cost data on lime-stabilized soils for the state of Alaska.

6.6 References

- Barenberg, E. J., "Behavior and Performance of Asphalt Pavements with Lime-Fly Ash-Aggregate Bases," *Proceedings*, Intl. Conference on Asphalt Pavements, Ann Arbor, MI, 1967.
- Little, D. N., *Handbook for Stabilization of Pavement Subgrades and Base Courses with Lime*, Kendall Hunt, 1995.
- National Lime Association publications: <http://www.lime.org/publications.html>
- Robnett, Q. L., and M. R. Thompson, "Effect of Lime Treatment on the Resilient Behavior of Fine Grained Soils," *Transportation Research Record 560*, Transportation Research Board, 1978.

Terrel, R. L., Epps, J. A., Barenberg, E. J., Mitchell, J. K., and Thompson, M. R., *Soil Stabilization in Pavement Structures—A Users Manual*, Vol. 1 & 2, U.S. Department of Transportation, Federal Highway Administration, October 1979.

Thompson, M. R., “Engineering Properties of Lime-Soil Mixtures,” *Journal of Materials*,

Vol. 4, No. 4, American Society for Testing and Materials, December 1969.

Transportation Research Board, “Lime-Fly Ash Stabilized Bases and Subbases,” *NCHRP Synthesis of Highway Practice 37*, Washington, DC, 1976.

Transportation Research Board, *Lime Stabilization: State-of-the-Art*, Report No. 5, 1987.

7.0 MECHANICAL STABILIZATION

7.1 General

Improving one material by blending it with one or more other granular materials is referred to as mechanical stabilization. This type of stabilization provides a direct means of altering the particle size distribution. Plasticity changes may also result.

Mechanical stabilization may involve the following:

- mixing materials from various parts of a deposit at the source of supply,
- mixing selected imported material with in-situ materials, and
- mixing two or more selected, imported natural gravels, soils, and/or quarry products on site or in a mixing plant.

Materials produced by mechanical stabilization have properties similar to conventional unbound materials and can be evaluated by conventional methods for unbound granular materials. Alaska DOT&PF has added silt and/or clay to gravel containing little or no fines.

7.2 Materials

Materials requiring mechanical stabilization have properties that make them deficient to be used as base or subbase materials. Typically, such materials are:

- poor-graded products;
- dune- or river-deposited sands;
- silty sands, sandy clays, silty clays;
- crusher run products;
- waste quarry products;
- industrial byproducts; and
- high-plasticity pavement materials.

7.3 Design Criteria for Granular Stabilization

The principal properties affecting the stability of base and subbase materials are internal friction and cohesion.

Internal friction is generated primarily as a result of the characteristics of the coarser soil particles

and the particle size distribution (PSD), or grading. Cohesion (and shrinkage, swelling, and compressibility) results primarily from the quantity and nature of the clay fraction as indicated by the plastic properties, sand equivalent, and maximum dry compressive strength (MDCS).

7.3.1 Particle Size Distribution

While maximum frictional strength does not necessarily coincide with maximum density, achieving high density will generally provide high frictional strength. Maximum density grading is obtained with the closest packing and minimum voids when:

$$p = \left[\frac{d}{D} \right]^n \quad (6.1)$$

where

p = % passing sieve size,

d = particle size

D = maximum particle size, and

n = 0.45 to 0.50 for most materials.

For materials with a maximum size of 19 mm, the amount of fines passing the 75- μ m (#200) sieve will be 6 to 8% for n values of 0.50 and 0.45, respectively. When relatively low permeability is required, materials should be of uniform particle size distribution within the limits of 0.50 and 0.33 for n .

Where n is less than 0.33, the fines content may be excessive. A high fines content will result in reduced permeability and may lead to the development of positive pore pressures and instability during compaction and in service. However, these materials will perform well if moisture conditions are controlled. When n is greater than 0.50, the material tends to be harsh and may be prone to segregation and raveling.

7.3.2 Plastic Properties

The limits for liquid limit and plasticity index (PI) given in Table 7.1 are generally accepted as satisfactory design criteria for granular stabilized

bases. Linear shrinkage (on material passing the 425- μm [#40] sieve) and the PI of a material are usually related. Linear shrinkage limits may be determined by test or estimated from PI values. Typical limiting values for linear shrinkage are 2% for sealed pavements and 3% for unsealed pavements.

Table 7.1. Desirable Limits for Plastic Properties of Granular Stabilized Base Materials

Pavement Type	Liquid Limit (%)	Plasticity Index (%)
Sealed		
Chip Seal	≤ 25	2 to 6
HMA	≤ 25	≤ 6
Unsealed	≤ 35	4 to 9

7.3.3 Strength Tests

For strength tests, such as the California R-value and the California bearing ratio (CBR), use the criteria normally specified for base and subbase materials. You may also use repeated load triaxial testing to characterize the elastic and plastic deformation characteristics of granular stabilized materials.

Selecting design criteria, particularly for lightly trafficked roads, should take into account local experience. Many materials that do not meet normal specifications perform well in lightly trafficked, well-drained situations.

7.4 Construction

In addition to adequate investigation and design, good construction practice and testing are essential to achieve a properly performing material. You must carefully proportion and thoroughly mix the constituent materials to produce a homogeneous material that can be compacted and finished as specified.

One of the major uses for granular stabilization is in constructing unsealed pavements to minimize dust. It involves mixing materials to ensure that

the correct grading and plasticity requirements for a low-dust wearing surface are met. This is usually a well-graded mixture with a specifically designed amount of cohesive fines. This is not easy to find without using special additions such as commercial clay (for example, montmorillonite or stabilite).

A suitable grading for wearing surfaces for unsealed pavements is given in Table 7.2.

Table 7.2. Suggested Particle Size Distribution for Unsealed Pavements

Sieve Size (mm)	% Passing Sieve	
	Wearing Course (Base)	Subbase
26.5	100	100
19.0	85–100	70–100
9.5	65–100	50–80
4.75	55–85	32–65
2.36 ¹	20–60	25–50
0.425	25–45	15–30
0.075 ²	10–25	5–15

1. The maximum particle size for subbase is often increased to 40 mm.
2. The 0.075 mm fraction is the fraction containing the dust particles.

7.5 Expected Life and Performance

Alaska DOT&PF has not used this technique extensively; hence, there is little performance or cost data available.

7.6 References

- Oglesby, Clarkson H., and R. Gary Hicks, *Highway Engineering*, 4th Ed., Wiley, 1982.
- Skorseth, Ken, and Ali A. Selim, *Gravel Roads – Maintenance and Design Manual*, South Dakota LTAP Program, Federal Highway Administration, November 2000.

8.0 SALT STABILIZATION

8.1 General

Salt stabilization has focused primarily on using calcium chloride as an additive to improve performance of granular materials. The stabilizing action of this additive is in its ability to attract and hold moisture and to reduce the void space of the compaction material. More information on this topic is available from the Salt Institute (www.saltinstitute.org).

8.2 Materials

The aggregate gradations suitable for salt stabilization are those typically recommended for aggregates used as an unsealed surface. These gradations have an increased amount of fines, or the percent passing the No. 200 sieve. A minimum value would be 5% with typical values in the range of 6% for base course and 10% for a wearing surface.

8.3 Design Considerations

Quantities vary slightly with application purpose and gradation. Typical application rates are from 1.5 to 2.0 lbs. of calcium chloride/square yard/year/inch of material for full stabilization efforts. The ability to hold moisture can improve the workability and maintainability of the aggregate surface, and the material may also be stronger. However, if the surface becomes wet, it may be less stable since the salt holds in the moisture for longer periods of time.

8.4 Construction Considerations

You should add aggregate material in layers to establish the specified grade and crown after compaction. Add the salt shortly afterward to take advantage of all subsequent mixing operations. Blend all materials thoroughly using a travel plant, a motor grader, or a pugmill. After the material is mixed, spread it out to provide a lift of at least 3 inches for compaction. Continue to compact until sufficient fines are brought to the surface to bind and seal the aggregate. You can leave the surface unsealed or seal it with a chip seal or a thin hot mix asphalt overlay.

8.5 Expected Performance and Costs

Most of the salt applications used in Alaska have been for dust control and not for full depth stabilization. As a result, there is little performance or cost data for these types of applications. Also, the salt can migrate over time, which causes a loss in the stabilizing effects.

8.6 References

- Dow Chemical, *Roadbase Stabilization with Calcium Chloride*, 1988.
- General Chemical, *Calcium Chloride—Roads That Last*, 1988.
- Salt Institute, The, *Salt for Road Stabilization*, Washington, DC, 1986.

9.0 CHEMICAL STABILIZATION

9.1 General

The vast majority of pavement stabilization carried out in Alaska is conducted using cement or asphalt stabilizers or blends of these stabilizing agents.

However, other forms of stabilization have been used in trial sections and these are briefly described in this section. Many of these chemical binders have been used as dust suppressants but may also have the ability to alter other properties such as strength and permeability. These binders should be assessed for their ability to improve the structural performance of pavements in a similar way to the cementitious, lime, and asphalt binders.

One area where other methods of stabilization are used is on unsealed pavements or pavements under construction to reduce dust nuisance and improve safety. This is an area that is continuously developing, so individual stabilizing agents should be assessed on the merits of their performance for a particular application. Broad guidance for the application of other stabilization agents follows.

9.2 Dust Suppression

Short of sealing a road, there are no known ways to eliminate dust emissions on a long-term basis by using a single process or just one application of a chemical dust suppressant. Dust suppression techniques fall into three main categories:

- good construction and maintenance practices,
- mechanical stabilization, and
- chemical dust suppressants.

9.2.1 Good Construction and Maintenance Techniques

Good construction and maintenance techniques are fundamental for a longer life and high level of service for unsealed roads. Providing a crown and adequate drainage are critical in retaining a hard road surface that minimizes dust.

9.2.2 Mechanical Stabilization

Section 7 describes mechanical stabilization. It involves mixing aggregate materials to ensure that the correct grading, plasticity, and strength requirements are met.

9.2.3 Chemical Binders

If you continue to have a lot of dust and can't achieve dust suppression through mechanical stabilization, you should consider using chemical dust suppressants as an adjunct to the other methods mentioned in sections 4–6. Chemical dust suppressions generally have a limited life and require regular applications to maintain satisfactory control of dust on a long-term basis. Selecting a particular type of dust suppressant depends on material composition and climatic factors.

Chemical dust suppressants can be broadly classified as

- organic nonasphalt products,
- water attracting chemicals,
- waste oil,
- petroleum-based products,
- electrochemical products,
- microbiological binders, and
- polymers.

Organic Nonasphalt Products

Organic nonasphalt products consist primarily of lignin sulphonates, a byproduct of the paper pulping industry. Their action in the soil is to adhere to and “glue” together soil particles. They also act as a clay dispersant, making clay more plastic and increasing its density after compaction. Lignosulphonates are effective dispersing agents for clays. During rain, the dispersed clays in the soil swell, filling the pore spaces, which tends to reduce water infiltration. During drying out, the lignosulphonate distributed throughout the soil reduces the rate of evaporation. These products are water soluble and have a limited lifespan if used in wet environments.

Water-Attracting Chemicals

Water-attracting chemicals consist of hygroscopic (water loving) materials, primarily chlorides and salts (see Section 8). Salts suppress dust by attracting and trapping moisture from the air, keeping the pavement wearing surface moist.

Petroleum-Based Products

Petroleum-based products are recycled waste oils, asphalt emulsions, and tars. Their effect is to produce heavy agglomerations of fine dust particles. Oils are often the longest lasting dust suppressants, but may carry the highest environmental consequences, particularly if waste oil is used.

Electrochemical Stabilizers

Electrochemical stabilizers consist of enzymes and sulphonated petroleum and are highly ionic. Electrochemical dust suppressants work by expelling adsorbed water from the soil, decreasing air voids and increasing compaction. Most of these products need some clay particles to work in the material. If traffic occurs during wet weather, you should apply a wearing surface to the electrolyte-treated material. The maximum strength of the material may not be attained for up to 20 days following application. This type of stabilization produces a more cohesive road surface, effectively increasing the energy required to dislodge particles.

9.3 Construction Considerations

Work safely with stabilizing agents. Always follow the manufacturer's instructions and seek

advice about working safely with the products from the manufacturer or contact the Research Office of Alaska DOT&PF.

9.4 Expected Performance and Costs

Chemicals have been used in Alaska with mixed success (Table 9.1). They have also been used only in small test sections. Hence, good performance and cost data are not available.

Table 9.1. Chemical Products Evaluated by Alaska DOT&PF^{1,2}

Class	Product
Clay Additives	Stabilite
Enzymes/Electrolytes	EMC ² Perma-Zyme
Tree Resins	Road Oyl

1. Stabilite, EMC², and Road Oyl are products from Soil Stabilization Products Co., Inc. (www.sspco.com)

2. Perma-Zyme is from the Charbon Group Inc. (www.natural-industrial.com)

9.5 References

- Bolander, Peter, *Stabilization with Standard and Nonstandard Stabilizers*, USDA-Forest Service, Portland, OR, November 1995.
- Bolander, Peter, and Alan Yamada, *Dust Palliative Selection and Application Guide*, USDA Forest Service, November 1999.
- Federal Highway Administration, *Non-Standard Stabilizers*, Report # FHWA-FLP-92-011, July 1992.

10.0 OTHER STABILIZING TECHNIQUES

10.1 General

Other techniques, including drainage and geotechnical fabrics, have also been used in Alaska to stabilize soils. Although not used extensively, they provide other options for stabilizing soft or wet soils.

10.2 Drainage

Removing excess water using drainage techniques can help stabilize soft wet soils. This technique has been used successfully in combination with fabrics in southeast Alaska.

10.3 Geotechnical Fabrics

Fabrics have been used to reinforce soft soils as well as to separate soils from aggregate layers. This technique has also been used in southeastern Alaska. Northern Region has also used Presto products, a geoweb fabric grid that can be filled with sand (www.prestogeo.com).

10.4 Expected Life and Costs

The limited use of these techniques has not resulted in good life or cost data. Better information on life and costs is required.

10.5 References

- Cedergren, A. R., "Development of Guidelines for the Design of Subsurface Drainage Systems for Highway Pavement Structural Sections," FHWA-RD-73-14, Federal Highway Administration, 1973.
- Christopher, B. R., and R. D. Holtz, "Geotextile Engineering Manual," FHWA-TS-86/203, Federal Highway Administration, 1985.
- ERES Consultants, *Soil and Base Stabilization and Associated Drainage Considerations, Vol. I and II*, Report FHWA-SA-93-005, Federal Highway Administration, December 1992.
- Moulton, L. K., "Highway Subdrainage Design," FHWA-TS-80-224, 1980.

APPENDIX A: Glossary of Terms

Absorption—The penetration of binder into an aggregate or base.

Acidic Soil—A soil having a pH value less than 7.0 (see also **Alkaline Soil**).

Additive—A substance added in small amounts to help in the manufacture or handling of a product or to modify the end properties.

Alkaline Soil—A soil having a pH value greater than 7.0 (see also **Acidic Soil**).

Anionic Asphalt Emulsion—A type of asphalt emulsion in which the suspended particles are negatively charged.

Binder—A material used for the purpose of binding particles together as a coherent mass.

Bound Material—Granular material to which cement, asphalt, or similar binders are added to produce structural stiffness.

California Bearing Ratio (CBR)—A measure of the bearing capacity of a soil or granular material obtained from a standard soil test.

Cationic Asphalt Emulsion—A type of asphalt emulsion in which the suspended particles are positively charged.

Cemented Material—Materials produced by the addition of cement, lime, or other hydraulically binding agent to granular materials in sufficient quantities to produce a bound layer with significant tensile strength.

Chip Seal—A thin layer of asphalt material sprayed onto a pavement surface and having a layer of aggregate rolled in.

Cohesive Soil—A soil whose relevant behavioral characteristics are derived largely or entirely from the cohesive bonds associated with the fine fraction.

Compaction Test (Field)—To compare field compaction with maximum dry density of the soil or pavement material.

Compaction Test (Laboratory)—A laboratory test to determine the maximum dry density of a soil or pavement material under specified test conditions.

Cutback Asphalt—A material made from asphalt by the addition of cutter oil for a temporary reduction in viscosity.

Deep Lift—A pavement construction technique whereby stabilization is carried out to depths in excess of 200 mm.

Design Life—Time period during which the quality of a pavement, for example, riding quality, is expected to remain acceptable.

Design Traffic—Cumulative traffic, expressed in terms of equivalent standard axle loads, predicted to use road over time.

Dynamic Cone Penetration (DCP) Test—A test in which the effort to push or drive a standard steel cone into soil at a controlled rate is used as a measure of certain soil properties, such as the field CBR.

Equilibrium Moisture Content—The moisture content that is reached in a soil in a particular environment after moisture movements have ceased.

Equivalent Standard Axle Loads (ESAL)—The number of standard axle loads that are equivalent in damaging effect on a pavement to a given vehicle or axle loading.

Field Density—The density of earthworks or pavement material measured in place.

Fly Ash (FA)—A fine powder of pozzolanic material extracted from the flue gases of a boiler fired with pulverized coal.

Foamed Asphalt—Hot asphalt greatly expanded in volume by the introduction of steam or water.

Gap-Graded Material—Material having a substantially continuous distribution of sizes from coarse to fine, the largest size being several times larger than the smallest size.

Indirect Tensile Strength—The tensile strength of a pavement material measured across the diameter of the sample.

Isotropic—Having properties that are equal in all directions.

Layer—A sequence of one thickness of pavement material placed during one construction operation.

Leaching—The removal of soluble material and colloids by percolating water.

Load Equivalency Factors—The ratio of the number of repetitions of the standard axle load that the pavement can sustain to the number of repetitions of another axle load that the same pavement can sustain for given damage criteria.

Maximum Dry Density—The greatest dry density of a soil obtained when a soil or pavement material is compacted in a specified manner over a range of moisture content. The moisture content at which this density is reached is called the optimum moisture content. Two amounts of compactive effort are commonly specified, referred to as standard and modified.

Modification—Improving the properties of a material by adding small quantities of an additive.

Moisture Content—The quantity of water that can be removed from a material by heating at 105°C until no further significant change in mass occurs, usually expressed as a percentage of the dry mass.

Noncohesive Soil—A soil that is lacking the fine fraction, resulting in a loss of the cohesive bonds associated with this fraction. Could also have fines, which are nonplastic.

Nuclear Density Meter (NDM)—An instrument for the nondestructive determination of the density and moisture content of material, using a radioactive source for its operation.

Optimum Moisture Content (OMC)—That moisture content of a material at which it will produce the maximum dry density under a standard test.

Plasticity Index (PI)—The numerical difference between the value of the liquid limit and the value of the plastic limit of a soil.

Poissons Ratio—Ratio of radial to longitudinal strain.

Pozzolan—A siliceous or aluminosiliceous material which in itself possesses little or no cementitious value but which in finely divided form may be mixed with lime or portland cement to form a cementitious material.

Recycling—The reuse of pavement material by in situ or plant mixing, with or without the addition of new material components.

Reflection Cracking—A visible crack in the pavement surfacing resulting from the movement associated with cracks in the underlying pavement layer.

Rehabilitation—The restoration (that is, stabilization) of a distressed pavement so that it may be expected to function at a satisfactory level of serviceability for longer.

Relative Compaction—The percentage ratio of the field dry density to maximum dry density.

Resilient Modulus—The ratio of stress to recoverable strain under repeated loading conditions. Also referred to as elastic stiffness.

Rutting—The longitudinal vertical deformation of a pavement surface in a wheel path, measured relative to a straightedge placed at right angles to the traffic flow and across the wheel path.

Stabilized Soil—A material that has been modified to improve or maintain its load carrying capacity. Modification may be by the addition of other natural materials such as sand, loam, or clay, or of manufactured materials such as cement, lime, and asphalt.

Standard Axle Load (SAL)—A load of 80 kN (18,000 lbs) applied over a single axle with a dual wheel at each end.

Stiffness—A measure of the elastic behavior of a pavement material (σ/ϵ) and can be determined either in compression or tension.

Subbase—The material layer on the subgrade below the base, for the purpose of making up additional pavement thickness required over the subgrade, or to prevent intrusion of the subgrade into the base, or to provide a working surface on which the remainder of the pavement can be constructed.

Subgrade—The trimmed or prepared portion of the formation on which the pavement is constructed.

APPENDIX B: Reference List on Stabilization—Alaska DOT&PF

- Alaska DOT&PF, *Project History—Dennis Road and Cushman Extension*, 1975.
- Alaska DOT&PF, *Project History—College Road Recycle*, 1982.
- Alaska DOT&PF, *Project History—Alaska Highway—MP 1253 to 1235*, July 1984.
- Alaska DOT&PF, *Project History—Shishmaref Airport Improvements*, 1992.
- Alaska DOT&PF, *Project History—Sheep Creek Road Rehabilitation*, 1993.
- Alaska DOT&PF, *Project History—Goldstream Road Rehabilitation*, 1994.
- Alaska DOT&PF, *Project History—Steese Highway Overlay, MP11-22*, 1995.
- Brownfield, Boyd J., “Perma-Zyme—A New Approach to Road Stabilization,” *Alaska DOT&PF M&O Newsletter*, fall 1994.
- Coetzee, N. F., “Product Evaluation: Presto Roadbase Sand Confinement Grid,” ADOT&PF Final Report, June 1983.
- Danyluk, Lawrence S., *Stabilization of Fine-Gravel Soil for Road and Airfield Construction*, AI-RED-86-30, Final Report to Alaska DOT&PF, April 1986, 44 pp.
- Esch, David, Internal memos on frost susceptibility, 1974.
- Esch, David, Internal memos on frost susceptibility, 1978.
- Gartin, R. Scott, and David C. Esch, *Treated Base Course Performance in Alaska*, FHWA-AK-RD-91-13, Alaska DOT&PF, June 1991.
- Gentry, Charles W., and David C. Esch, *Soil Stabilization for Remote Area Roads*, AK-RD-86-08, Final Report to Alaska DOT&PF, August 1985, 142 pp.
- Hopper, Thad, Tracy Moore, and Dave Sterley, *Council Grounds Trail Experiment Soil Stabilization*, ADOT&PF Southeast Region, October 1997.
- Kinney, Thomas C., and M. Reckard, *Effects of Salts on Road Embankment Stability Under Freezing and Thawing Conditions*, AK-RD-87-09, Final Report to Alaska DOT&PF, December 1986, 34 pp.
- Koehmstedt, Paul L., *Soil Stabilization for Remote Airfields*, Final Report to ADOT&PF, January 1981.
- Kozisek, P. E., and J. W. Rooney, *Soil Stabilization Test Strips—Bethel, Alaska*, Final Report to Alaska DOT&PF, January 1986.
- McHattie, Robert L., Final Evaluation of Experimental Features Projects, AK8701A and AK8701B, internal Alaska DOT&PF memo, January 31, 1994.
- McHattie, Robert L., *Subbase Treatment Using EMC² Soil Stabilizer*, Interim Report, Alaska DOT&PF, October 1997.
- McHattie, Robert, Billy Conner, and David Esch, *Pavement Structure Evaluation of Alaska Highways*, FHWA-AK-RD-80-1, Alaska DOT&PF, March 1980.
- Naske, Claus M., *Alaska Road Commission Historical Narrative*, prepared for Alaska DOT&PF, June 1983.
- Oglesby, C. H., and R. G. Hicks, *Highway Engineering*, John Wiley and Sons, 1982.
- Peyton, H. R., C. F. Kennedy, and J. W. Lund, *Stabilization of Silty Soils in Alaska—Phase II*, report to Alaska Department of Highways, June 1966.
- Peyton, H. R., and J. W. Lund, *Stabilization of Silty Roads in Alaska*, University of Alaska, Fairbanks, November 1964.
- Phukan, Arvind, *A Literature Search for Substitute Materials in Frost Protecting Layers*, AK-RD-82-7, Final Report to Alaska DOT&PF, May 1981, 53 pp.
- USDA, *Exploratory Soil Survey of Alaska*, February 1979.
- Vita, Charles L., Ted S. Vinson, and James W. Rooney, *Bethel Airport CTB-AC Pavement Performance Evaluation*, Final Report to Alaska DOT&PF, March 1981.

APPENDIX C: Soils of Alaska

C.1 General

The information presented in this appendix comes from the *Exploratory Soil Survey of Alaska*, which has been prepared for many different users, including farmers, foresters, and agronomists. Great differences in soil properties occur even within short distances. Soils may be organic, seasonably wet, shallow over bedrock or permafrost, gravelly, and/or sandy. The type of soil in the region will have a profound effect on the type of additive that can be used to stabilize it.

C.2 Major Land Resource Areas

Alaska has 15 major land resource areas. Each is characterized by a unique pattern of topography, climate, vegetation, and soils.* They include the following:

- **Southeast Alaska.** This includes the mountains of the mainland east of the St. Elias Mountains and the islands of the Alexander Archipelago. It is dominated by rugged hills and mountains that rise from the sea. Strips of hilly moraines border most of the bays and valleys. The area has a cool climate characterized by high precipitation throughout the year. Frost-free seasons are long, but are offset by low summer temperatures and persistent cloud cover. The predominant soil types encountered include in road construction are sandy or silty soils and peat or muskeg.
- **Southcentral Alaska Mountains.** This area includes the St. Elias Mountains, Chugach, and the Kenai Mountains bordering the Gulf of Alaska and the Wrangell and Talkeetna Mountains farther inland. Moraines, outwash plains, and other glacier features are found throughout this area. The resource area has a variety of climates, ranging from high precipitation and moderated temperatures

along the coastal regions to areas with low precipitation and marked seasonal temperature differences in the Interior. At higher elevations, the precipitation is mainly snow and summer temperatures are so low that ice persists throughout the year. The predominant soil types encountered in road construction include silt, sand, and gravel.

- **Cook Inlet-Sustina Lowland.** This area is a long narrow basin between the Kenai, Chugach, and Talkeetna Mountains to the east and the Aleutian and Alaska Ranges to the west. Most of the northern half of the lowland is drained by the Susitna River and its tributaries. The southern half borders Cook Inlet. The Matanuska Valley is an eastern extension of the lowland from the head of the Cook Inlet. The entire basin is underlain by sediments of the Tertiary age, but the surface consists mainly of glacial deposits, including low moraines interspersed with many lakes, bogs, and broad outwash plains. The climate of the lowland has both maritime and continental characteristics. The Alaska Range protects the area from the extreme temperatures of interior Alaska. Precipitation is moderate in the southern part and fairly low in the central part. The predominant soil types encountered in road construction include silts, silty clays, and muskeg.
- **Alaska Peninsula and Southwestern Islands.** This area includes the Aleutian Range, the Alaska Peninsula, the Kodiak Island group, other small islands south and east of the peninsula, and the Aleutian Islands. The mountains of the Aleutian Range are mostly volcanic, whereas the Kodiak Mountains are a continuation of the non-volcanic mountains that border the Gulf of Alaska. The climate is generally maritime, but influenced by the mountainous terrain. Except in the high mountains, mean annual temperatures are above freezing. Summers are cool and winters relatively mild. Precipitation

* For expected soil types in each area, contact head ADOT&PF geologist in each respective region.

is heavy except in the northwest part of the area. The predominant soil types found in road construction are sand, silty sands, and muskeg.

- **Copper River Plateau.** This area is a broad basin of rolling to hilly moraines and glacial sediment interspersed with many lakes. It is surrounded by mountains: the Chugach to the south; the Alaska Range to the north; and the Talkeetna mountains to the west. The plateau is drained by three major rivers—the Copper, the Matanuska, and the Susitna. The climate is strongly continental—winters are long and cold. Summers are short and warm. Mean annual temperatures are below freezing and precipitation is low to moderate. The predominant soil types found in road construction are silt, sand, and gravels.
- **Alaska Range.** The long narrow mountain chain arcs around southcentral Alaska and separates it from the interior. It is very rugged and has many peaks above 10,000 feet. Many of the rivers in southcentral Alaska originate in the Alaska Range region. Mean annual temperatures are well below freezing even in the low passes. Precipitation is fairly heavy on the southern and southeastern slopes, but lighter on the north and western slopes. The predominant soil types are sands and gravels.
- **Interior Alaska Lowlands.** This area includes broad valleys and plains between the Alaska Range on the south and east, the Brooks Range on the north, and the Norton Sound Highlands on the west. The area is divided into parts—the Yukon Flats and the Kanuti Flats, which are large basins surrounded by hills; and the Koyukuk-Innoko and the Tanana-Kuskokwim Lowlands, which border major rivers in the region. Also included are natural levees, glacial outwash plains, piedmont slopes, and some rolling hills. The climate is continental, characterized by long cold winters and short warm summers. The predominant soil types for road construction include silt, sand, gravel, and permafrost.
- **Kuskokwim Highlands.** This area includes hills and low mountains between the central

Yukon River and Bristol Bay. The northern part consists mostly of a series of rounded ridges 200 to 1,500 feet in elevation, separated by narrow valleys. The climate is strongly continental in most of the area but is modified by maritime influences near the Bering Sea. The mean annual temperature everywhere except the coast of Bristol Bay is below freezing. Precipitation is light in the north but increases southward toward the coast. The predominant soil types for road construction are coarse to fine sands.

- **Interior Alaska Highlands.** This area includes hills between the Tanana and Yukon Rivers and the Brooks Range and east of the Koyukuk and Selawik lowlands. The highlands consist mostly of rounded hills and ridges but include some mountains higher than 6,000 feet. Parts of the area adjacent to the major river valleys are as low as 300 feet. The higher parts have been affected by glaciers, but most of the area has never been ice covered. The climate is continental with long cold winters and short warm summers. The predominant soil types found in road construction are silt, sands, and gravels, and some low-PI clays.
- **Norton Sound Highlands.** This area consists of hills and low mountains on the Seward Peninsula and in the area east and south of Norton Sound. Elevations are generally less than 3,000 feet, though a few peaks are higher. Some of the mountainous areas were glaciated, but most of the area has always been free of ice. There is a significant maritime influence on the climate in the area. Mean annual temperatures are below freezing, but winters are milder and summers cooler than in the inland areas. Precipitation is moderated in the regions bordering the sound, but low in the northern Seward Peninsula. The predominant soil types found in road construction are sand and silty gravels.
- **Western Alaska Coastal Plains and Deltas.** This area is made up of the Selawik-Kobuk Delta and the Yukon-Kuskokwim Delta and the Bristol Bay coastal plain. All are low and

have very little relief. Permafrost underlies nearly all areas except the southern part of the Bristol Bay coastal plain. A cold maritime climate prevails. Mean air temperatures are generally below freezing except for Bethel, Kotzebue, and King Salmon. The predominant soil types found in road construction are silts, and sands.

- **Bering Sea Islands.** This area includes six islands of the Bering Sea—the Pribilofs, Nelson, St. Matthew, and St. Lawrence. The islands are volcanic rock and permafrost is virtually everywhere, except the Pribilof Islands. All of the islands have cool moist climates with mean annual temperatures increasing from the north to the south. The predominant soil types found in road construction are sands and gravels.
- **Brooks Range.** This area, the northern extension of the Rocky Mountains, extends across northern Alaska from Canada to about the Bering Strait and the Chukchi Sea. The southern slopes of the Brooks Range mark the northern limit of extensive forests in Alaska. In all but the southern slopes, the climate is arctic, with freezing temperatures occurring every month. Total precipitation is low. The

predominant soil types found in road construction are sand and silty gravel.

- **Arctic Foothills.** This is an area of low ridges and intervening swales north and west of the Brooks Range. The elevation is generally less than 2,000 feet. The area has an arctic climate, modified slightly in the western part by a maritime influence. The predominant soil types found in road construction are silt, sands, and gravels.
- **Arctic Coastal Plain.** This is a gently rolling treeless area with many shallow elongated lakes and naturally drained lake basins. Rivers flowing from the mountains to the south meander across the plains to the Arctic Ocean. The climate is arctic with low mean annual temperatures and very low precipitation rates. The predominant soil types found in road construction are sand and silty sands.

C.3 SUMMARY

This appendix briefly describes the different land resource areas in Alaska and the major soil types encountered in road construction. For more detail on soil types, the reader is referred to the USDA's soil conservation publication titled *Exploratory Soil Survey of Alaska*, dated February 1979.

APPENDIX D: Slide Presentation

Soil Stabilization Workshop
for
ADOT&PF
by
Gary Hicks
Professor Emeritus (OSU)
and
Project Manager

November 27-29, 2001
 **CIVIL, CONSTRUCTION AND ENVIRONMENTAL ENGINEERING**
OREGON STATE UNIVERSITY

1

Background

- ADOT&PF wishes to increase the use of locally available aggregates.
- Increased use of stabilizing additives can make this happen.
- ADOT&PF needs information to determine the right additive for a given soil type and given use, particularly for use as surface courses.

2

Purpose of Workshop

- Provide an overview of various soil stabilization techniques (asphalt portland cement, lime/lime-fly ash, salt, and chemicals)
- Discuss mix design, mix properties, construction, and performance
- Present do's and don'ts for the various stabilization procedures

3

Reasons for Using Stabilized Materials

- Temporary wearing surface
- Working platform for construction activities
- Improve poor subgrade conditions
- Upgrade marginal base materials
- Dust control
- Salvage old roads containing marginal materials

4

Scope of Workshop

- Introduction
- Overview of stabilization techniques
- Design Philosophy
- Asphalt Stabilization
- Cement Stabilization
- Lime and Lime/Fly ash Stabilization
- Mechanical stabilization
- Stabilization with other additives (salts and chemicals, drainage and geo-textiles)
- Wrap-up

5

Overview of Stabilization Techniques

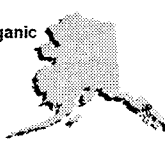
- Types of soils
- Types of stabilizers
- Selection of stabilizer type



6

Typical Soils in Alaska

- Poor to well-graded gravels
- Coarse to fine sands
- Inorganic to slightly organic silts
- Clayey gravels (rare)
- Clays (rare)
- Peat/Muskeg



7

Types of Stabilizers

- Cementing (strength gain)
 - lime
 - lime/fly ash
 - portland cement
 - asphalt
- Modifiers
 - lime
 - chemicals

8

Types of Stabilizers (cont.)

- Waterproofers
 - bitumens
 - chemicals
- Water-retainers
 - salts
 - others?

9

Most Widely Used Admixtures in Alaska

- Asphalt emulsions
- Portland Cement
- Salts

Question? Why have other admixtures not been widely used?

10

Description of Stabilizer Effects

- Cementing agents
 - Bond together individual or aggregates of soil particles (lime, cement, asphalt)
 - Portland cement and lime react with clay minerals (a *pozzolanic* reaction), lime/fly ash is effective when the clay mineral contents are low or non-existent.
 - Portland cement - hydrates and produces a strong cementing agent. increases *strength*. In roads, this can reduce the thickness of pavement needed.

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Description of Stabilizer Effect (cont.)

- Modifiers
 - Change properties of water (absorbed layer)
 - Reduce plasticity (raise optimum water content)
 - Improve workability
 - Modify clay minerals

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Description of Stabilizer Effect (cont.)

- Waterproofers
 - Retard or completely stop water absorption
 - Prevent water from reaching water sensitive soils
- Water retainers (e.g. calcium and sodium chloride)
 - Lower vapor pressure - soil stays moist
 - Lower freezing point - mitigates frost damage

13

Factors to Consider in Selection of Admixture

- Soil type (Gradation and Plasticity Index)
- Climate and drainage
- Availability of admixture and/or construction equipment
- Economics (equipment and materials and expected performance)

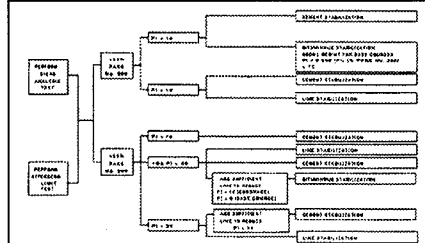
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Selection of Stabilizer Type

Soil Types	Most Effective Stabilizers
1. Coarse granular soils	Asphalt, portland cement, lime-fly ash
2. Fine granular soils	Portland cement, lime-fly ash, asphalt, chlorides
3. Clays of low plasticity	Portland cement, chemical waterproofers, lime, lime-fly ash
4. Clays of high plasticity, silts	Lime, chemicals

15

Guide to Choice of Admixtures



16

Climatic Limitations/ Construction Safety Procedures: Asphalt Products	
Climatic Limitations	Construction Safety Precautions
- Air temperature should be above 50°F (10°C) when using emulsions - Air temperature should be 40°F (5°C) and rising when placing thin lifts (1-in.) of hot mixed asphalt concrete - Hot, dry weather is preferred for all types of asphalt stabilization	- Some cutbacks have flash and fire points below 100°F (40°C) - Hot mixed asphalt concrete temperatures may be as high as 350°F (175°C)

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Climatic Limitations/ Construction Safety Procedures: Cement and Cement-Fly Ash	
Climatic Limitations	Construction Safety Precautions
- Do not use with frozen soils - Air temperature should be 40°F (5°C) and rising - Complete stabilized layer one week before first hard freeze	- Cement should not come in contact with moist skin for prolonged periods of time - Safety glasses and proper protective clothing should be worn at all times

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Climatic Limitations/ Construction Safety Procedures: Lime and Lime-Fly Ash	
Climatic Limitations	Construction Safety Precautions
- Do not use with frozen soils - Air temperature should be 40°F (5°C) and rising - Two weeks of warm to hot weather are desirable prior to fall and winter temperatures	- Quicklime should not come in contact with moist skin - Hydrated lime [Ca(OH)₂] should not come in contact with moist skin for prolonged periods of time - Safety glasses and proper protective clothing should be worn at all times

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Climatic Limitations/ Construction Safety Procedures: Salts	
Climatic Limitations	Construction Safety Precautions
- Do not use with frozen soils - Do not apply in the rain	- Require MSDS - May impact vegetation and vehicles

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Climatic Limitations/ Construction Safety Procedures: Chemicals	
Climatic Limitations	Construction Safety Precautions
- Do not use with frozen soils - Chemicals are more active at high temperatures - Others?	- Require MSDS for each product - Concentrate may pose some environmental concern

21

Design Philosophy	
- Mix Design - Structural Design - Construction consideration "These aspects are interrelated. The performance of the stabilized material depends on all 3 factors"	

22

Asphalt Stabilization	

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Stabilization with Asphalt	
- Suitable types of soils - Soil - asphalt reactions/asphalt breaking and curing - Properties of treated soils - Mix design - Construction - Do's and don'ts	

24

Reasons for Use

- Waterproofing fine-grained soils
- Upgrading marginal materials
- Provide temporary/permanent wearing surfaces
- Reduce dusting
- Recycling

25

Definitions

- **Asphalt Cement:** A flux or un-fluxed asphalt specially prepared as to quality and consistency for direct use in such construction industries as highways and structures.
- **Cutback Asphalt:** Asphalt cement that has been made liquid with the addition of petroleum diluents such as naphtha and kerosene.

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Definitions (cont.)

- **Emulsified Asphalt:** Asphalt cement that has been mechanically liquefied with the addition of emulsifying agents and water.
- **Foamed Asphalt:** Asphalt cement that is mixed with hot water and an emulsifying agent in the field to produce a product which increases in volume and capable of coating the untreated soil

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Types of Asphalts

- Emulsions (Anionic and cationic)
 - Slow setting (SS)
 - Medium setting (MS)
 - High floats (HF)
 - Lime or cement may be added
- Cutbacks
 - Slow cure (SC)
 - Medium cure (MC)
 - Rapid cure (RC)
- Asphalt cements

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Suitable Soils



- Non-plastic sands (PI < 10)
- Non-plastic gravels (PI < 6)
- Question- Where have asphalt products been most successful in Alaska?

29

Soil - Asphalt Mechanisms/ Asphalt Breaking and Curing

- Mechanisms
 - Waterproofing
 - Adhesion
- Breaking and curing
 - Breaking (chemical)
 - Curing (evaporation)

30

Properties of Treated Soils

- Strength and deformation characteristics
 - Compressive
 - Tensile
 - Elastic stiffness
- Durability
 - Freeze-thaw resistance
 - Stripping
- Fatigue performance
- Surface wear resistance

31

Mix Design

- Procedures
 - Selection of asphalt type and grade
 - Approximate quantities
 - Detailed testing procedures-see TAI
- Typical asphalt contents
 - 5 to 8%
 - Depends on aggregate gradation/absorption

32

Mix Properties
• Marshall Stability and flow • Stiffness-diametral modulus test • Fatigue performance • Rutting resistance

33

Construction
• Mixing (in-place or central plant) • Placement • Compaction • Curing • Need for surface (HMA or surface treatment)

34

Performance and Cost
• Performance - Has varied from good to poor • Cost - Good cost data not readily available

35

Do's and Don'ts
• Do's - Select the right asphalt for the job - Construct in dry and warm conditions - Aerate the emulsion or cutback before placing the surface layer • Don'ts - Use asphalt with high plasticity clays - Construct in cold weather

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Good web sites
• TAI (www.asphaltinstitute.org) • AEMA (www.aema.org) • ARRA (www.arra.org) • TRB (www.trb.org)

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Cement Stabilization

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Stabilization with Portland Cement
• Suitable types of soils • Soil-cement/cement hydration reactions • Mix Design • Properties of treated soils • Construction • Performance and Costs • Do's and don'ts

39

Portland Cement
• Produced by pulverizing clinker consisting of hydraulic calcium silicates, and usually containing one or more forms of calcium sulfate (ASTM C-1) • Reactions include both short- and long-term ones

40

Other Definitions

- **Cement Stabilized Soil:** Mixture of soil and measured amounts of portland cement and water which is thoroughly mixed, compacted to a high density and protected against moisture loss during a specific curing period.
- **Soil-Cement:** Hardened material formed by curing a mechanically compacted intimate mixture of pulverized soil, portland cement, and water. Soil-cement contains sufficient cement to pass specified durability tests.

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Other Definitions (cont.)

- **Cement-Modified Soil:** Unhardened or semi-hardened intimate mixture of pulverized soil, portland cement, and water. Significantly smaller cement contents are used in cement-modified soil than in soil-cement.

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Types of Portland cement

- I - normal
- II - modified
- III - high early strength
- IV - low heat
- V - sulfate resistant

43

Suitable Soil Types

- Good for most sands/gravels
 - Also suitable for fine-grained soils with low to medium plasticity (PI <30)
- Question- where has portland cement been used in Alaska?

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Soil-Cement Reactions

- Ion exchange
- Flocculation
- Pozzolanic reaction
- Carbonation
- Portland cement hydration- most important

45

Mix Design

- Procedures
 - Approximate
 - Detailed testing
- Typical cement contents
 - 5 to 10 %
 - Too much PC can lead to shrinkage cracking

46

Typical Cement Contents

Classifications		Recommended %PC* (wt)	Allowable Loss** (%)
AASHTO	USCS		
A-1-a	GW, GP, GM, SW, SP, SM	3-5	14
A-1-b	GM, GP, SM, SP	5-8	14
A-2	GM, GC, SM, SC	5-9	14
A-3	SP	7-11	14
A-4	CL, ML	7-12	10
A-5	ML, MH, CH	8-13	10
A-6	CL, CH	9-15	7
A-7	OH, MH, CH	10-16	7

* Range to achieve satisfactory strength and durability

** Loss in weight after freeze-thaw or wet-dry test (ASTM 598 or 560)

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Properties of Cement-Treated Soils

- Compaction characteristics
- Strength and deformation characteristics
 - Tensile or compressive
 - Modulus
 - Fatigue performance
- Durability
 - Shrinkage
 - Freeze-thaw resistance
- Wear resistance (only w/o HMA surface)

48

Construction

- Mixing (in-place or central plant)
- Placement
- Compaction
- Curing
- Need for surface layer (HMA or surface Treatment)

49

Performance and Costs

- Performance
 - Has varied from good to poor
 - Lack of good performance information
- Costs
 - Lack of good data in Alaska
 - Varies with size of job

50

Do's and Don'ts

- Do's
 - Use good quality cement
 - Provide for thorough mixing
 - Must provide for moist curing after compaction
- Don'ts
 - Use with plastic soils
 - Do place in cold weather conditions
 - Others?

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Good web sites

- PCA (www.portlandcement.org)
- ACI (aci-int.org)
- Porter Yett Construction (www.porteryettcompany.com)

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Lime/Lime Fly-ash Stabilization

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Stabilization with Lime/ Lime-Fly Ash

- Suitable types of soils
- Soil-lime/lime-fly ash reactions
- Mix design
- Properties of treated soils
- Construction
- Performance and Costs
- Do's and Don'ts

54

Definitions: Lime/Fly Ash

- Lime
 - All classes of *hydrated lime* or *quicklime*, both calcitic and dolomitic (ASTM C693). If there are little or no pozzolans in a soil, these can be provided by *fly ash*.
- Fly ash (pozzolan) - ASTM definition
 - "Siliceous or siliceous and aluminous materials which in themselves possess little or no cementitious value but will, in finely divided form and in the presence of moisture, chemically react with calcium hydroxide at ordinary temperatures to form compounds possessing cementitious properties."

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Types of Lime

- CaO - calcitic quicklime
- CaO+MgO - dolomitic quicklime
- Ca(OH)₂ - high calcium hydrated lime
- Ca(OH)₂+MgO - monohydrated dolomitic lime
- Ca(OH)₂+Mg(OH)₂ - dy-hydrated dolomitic lime

56

Types of Lime/Fly Ash Mixtures

- LFA - mixture of lime and fly ash with aggregate
- LCFA - mixture of lime, cement, and fly ash with aggregate
- LFS - mixture of lime and fly ash with soil

57

Suitable Soils

- Without fly ash
 - Clays
 - Clayey soils
 - With fly ash
 - Silty soils
 - Sands
 - Gravels
- Question-" Why hasn't lime been used in Alaska?"

58

Problem Soils

- High % clays
- Soils with sulfates
- Organics



59

Soil-Lime/Lime-Fly Ash Reactions

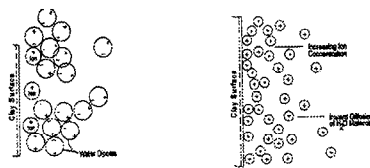
- Short term
 - Ion exchange
 - Flocculation
- Long term
 - Pozzolanic reaction-need an alkaline environment
 - Carbonation-not good

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Cation Exchange and Flocculation-Agglomeration

- Reduction in adsorbed water layer
- Increased shear strength
- Improved workability

Formation of Diffused Water Layer Around Clay Particle



Pozzolanic Reactions

- $\text{Ca}^{++} + \text{OH}^- + \text{SiO}_2 \rightarrow \text{CSH}$
- $\text{Ca}^{++} + \text{OH}^- + \text{A}_2\text{O}_3 \rightarrow \text{CAH}$



61

Carbonation



- $\text{CaCO}_3 + \text{heat} \rightarrow \text{CaO} + \text{CO}_2$
- $\text{CaO} + \text{H}_2\text{O} \rightarrow \text{Ca(OH)}_2$
- $\text{Ca(OH)}_2 + \text{CO}_2 \rightarrow \text{CaCO}_3$

62

Mix Design

- Procedures
 - Approximate
 - Based on strength
- Range in contents used
 - Lime - 2 to 8%
 - Fly ash - 8 to 36%

63

Selection of Lime Content

- PI vs. lime content
- pH 12.4 test
- Strength requirements



64

Lime-Fly Ash Typical Contents



- Lime - 2.5 to 4%
- Fly ash - 10 to 15%

65

Properties of Treated Soils

- Uncured mixtures
 - Reduced plasticity and improved workability
 - Moisture - density relations
 - Swell potential
- Cured mixtures
 - Strength and deformation properties
 - Shrinkage
 - Durability
 - Wear resistance

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Construction

- Mixing
- Placing
- Compaction
- Curing
- Need for surface layer (HMA or surface treatment)

67

Performance and Costs

- Performance
 - Not used in Alaska because of lack of clays
 - Can be used with fly ash and granular materials
- Costs
 - None available in Alaska

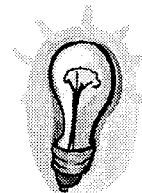
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Do's and Don'ts

- Do's
 - Use quality lime
 - Compact after flocculation has occurred
 - Watch out when using CaO
- Don'ts
 - Allow carbonation to take place
 - Use clay with NP soils or soils with organics

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Good web sites



- NLA (www.lime.org)
- NAA (www.acaa-usa.org)

70

Mechanical Stabilization

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Mechanical Stabilization
• General • Materials • Design • Construction • Performance and Costs • Do's and Don'ts

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Types of Mechanical Stabilization
• Mixing materials from various parts of the deposit • Mixing select or import material with in-situ materials • Mixing 2 or more materials on site or in a mixing plant

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Materials Requiring Mechanical Stabilization
• Poor-graded aggregates • Dune sands • Silty sands or clays • Crusher run products • Waste products

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Design Considerations
• Improving gradation • Changing plastic properties • Changing strength • Typically 1-3% of filler is added "Primary use is the minimize dust on aggregate surfaced roads"

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Construction
• Proportioning of materials • Mixing the materials- fines should be added dry to slightly moist • Compaction • Need for a surface layer

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Performance and Cost
• Performance - Used in Alaska on occasions - Generally mix clay or other fines - Performance not well documented • Costs - Not readily available for Alaska - More than aggregate base by \$1-3/ton in the lower 48

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Dos and Don'ts
• Dos - Used a suitable grading - Mix to correct proportions • Don'ts - Cover the material with a surface - Traps moisture

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Other Stabilization Techniques

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Stabilization with Other Additives

- Salts
- Lignins
- Dust oils
- Chemicals
- Fabrics
- Drainage

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Salts - General

- Can use either CaCl_2 or MgCl_2
- Important benefits
 - Improves compaction
 - Retains moisture
- Recommended with >10% P200
- Comes in liquid/pellet form
- Can be used in base or wearing surface

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Salts - Design

- Typically use between 6 to 10 lbs of solid per ton of aggregate
- Equates to 1.6 to 2.6 gal/ton of aggregate for 38% solids CaCl_2
- Most suppliers recommend 1 gal/sq/yd when treating a 4 inch layer

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Salts - Construction Hints

- May have to apply in a series of passes to prevent runoff
- Best to apply when aggregate is damp
- Compact close to optimum moisture content

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Salts - Maintenance Hints

- Can be bladed dry, but best if bladed with some moisture present
- Salts will leach or migrate over time

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Performance and Costs

- Performance
 - Salts can migrate
 - Expected life of treatment varies
 - Generally re-treated every year
- Costs
 - Information for Alaska not readily available

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Lignins - General

- By-product of wood pulping process
- Physically binds materials together
- Can be used to treat base/surface aggregates
- Generally darkens the material

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Lignins - Design

- Typically mix 1 part lignin to 1 part H₂O
- Total diluted application rate is close to 2 gals/sy for 4 to 6 in. depth
- Applied in multiple spray applications (~3)

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Lignins - Construction Hints

- Mixing
 - Blade
 - Rototiller
- Apply in 3 applications
 - 0.6 gal/sy
 - 1.0 gal/sy
 - 0.4 gal/sy

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Lignins - Maintenance Hints

- Hard crust is formed, so blade when road surface is moist
- Lignin is water soluble
- Will migrate under heavy rains

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Performance and Cost

- Performance
 - Not widely used in Available
 - Will wash out with rain
- Costs
 - Not available for Alaska conditions
 - In lower 48, \$0.05 to 1.50/sy

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Dust Oils

- Binds and/or agglomerates surface particles
- Serves to waterproof the road surface
- Others?

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General

- Includes asphalt emulsions, cutbacks, modified emulsions, other oil and waste products
- Can be used to treat either the base or wearing surfaces

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Design Considerations

- Amount of solids remaining determines the strength of the bond
- Typically apply the diluted emulsion at a rate of 0.25 to 1 gal/sy, diluted 1:1
- Each emulsion may have a different recommended dilution ratio

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Construction Hints

- Apply to a wet surface to get penetration.
- Emulsions do not penetrate as well as cutbacks. May have to mix
- For CSS-1 compact after the mixing is complete

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Maintenance Hints

- Once bladed, the bond is permanent.
- Re-blading will destroy the bond
- Other?

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Performance and Costs

- Performance
 - Varies with traffic and need for re-blading
- Costs
 - Not available for Alaska

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Chemicals

- Electrolytes, enzymes, and tree resins
 - Road Oyl, EMC2, Permazine and others used in Alaska
 - Use limited primarily for trial sections
- Other uses
 - Cosmetics, shampoos
 - detergents, cleaning agents

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Design Considerations

- Most chemicals need clay particles to stabilize
- Most do not change the color of the material
- Diluted 1 part to 100 or more parts of water

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Construction Hints

- Surface generally treated every year
- Compacted near optimum water content
- High densities essential to the process
- If traffic occurs during wet weather, may have to apply a wearing surface

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Maintenance Hints

- The stabilized material can be reworked
- It will harden again under traffic
- Blading in dry conditions will cause dusting and erosion

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Performance and Costs

- Performance
 - In Alaska, it is not well documents
 - Seem to be highly variable
- Costs
 - No cost data available for routine projects

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Drainage and Geotexties

- Removing water can help stabilize soft soils
- Fabric drains can be used for this
- Fabrics can also be used to reinforce soft soils to to separate soils from aggregates
- Presto Grids have been used by the northern region to stabilize sands

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Wrap-Up	
• General conclusions • Implementation of guide • Questions/answers	

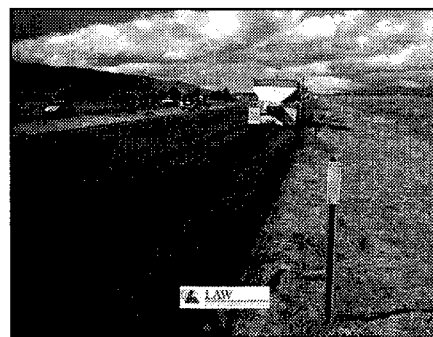
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General Conclusions
• Asphalt and cement have been the most widely used stabilizers • The performance of soil stabilized pavements has been mixed in Alaska • The guide provide suggestions for using the various stabilizers

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Implementation of Guide
• Purpose of guide is to expand use of stabilized materials • What is the best way to implement the guide • Training or finding cost effective solutions

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