Jemuary 7, 1914.

In re investigation of accident on Southern Railway near Oyana, N. C., on March Sl. 1913.

On March 31, 1913, there was a derailment of a freight train on the Southern Mailway near Oyana, N. C., resulting in the death of 5 employees and the injury of a trespassor on the train. After investigation as to the nature and cause of this accident, and the circumstances connected therewith, the Chief Inspector of Safety A. Liances reports as follows:

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The Asheville Division of the Southern Railway, on which this accident occurred, is a single track line operated under the manual block system. At the point of derailment the track runs east and west; it is laid with 35-foot, 80-pound steel rails, single spiked to oak ties, 18 or 19 to the rail, with 8 inches of rock ballast, the track being maintained in good condition.

Westbound freight train No. 73, running from Salisbury, N. C., to Asheville, N. C., a distance of 141 miles, consisted of engine No. 646, 17 loaded and 17 empty cars and a caboose, and was in charge of Conductor Boyle and Engineman Nagle. This train left Salisbury at 2: 21 a.m., 21 minutes late, and left Newton, the last telegraph station east of the point of derailment, at 4:43 a.c., 28 minutes late. The derailment occurred 2.2 miles e at of Cyama, N. C., near mile-post 5-21.6, at about 4:50 a.m. The engine, tender and 16 cars are derailed on the touth side of the track. It cars sere destroyed, and the track was torn up for a distance of 330 feat. The conductor, engineer and fireman were killed.

The speed of the train at the time of the derailment was 25 or 30 miles per hour. There was a speed restriction of 30 miles per hour for freight trains in effect on this division. At the time of the accident the weather was foggy.

Approaching the scene of the accident from the east there is a 2 degree curve to the south 2300 feet long, followed by a tangent 2700 feet in length. Following this tangent is a 1 degree curve to the north 5400 feet in length. About 1800 feet of this tangent is an a grade of .95% descending from the east, this grade being immediately followed by an ascending grade of 0.9% about 1400 feet in length. The derailment occurred about 1122 feet from the east end of the tangent, 300 feet from the foot of the descending grade, on a fill 5 feet high on the south and 2½ feet high on the north. It was caused by a broken rail on the south of the truck. The rail was broken in 8 places, the shortest piece being about 7 inches long and the longest piece 18 feet 2½ inches long.

Engine 646 weighed 197,750 pounds, with 176,650 pounds on drivers. This engine was built in 1904; it was rebuilt in 1912, and at the time of the accident was in good condition. The last preceding train over this track was eastbound freight train third No. 72, which passed the scene of derailment at approximately 3:05 c.m. This train was hauled by engine 823, of the same class an engine 646, and consisted of 20 loaled cars. The engineer of this train stated that at this point where derailment occurred his train was running at the rate of 20 or 25 miles per hour and he did not notice engine unusual when his train passed that point.

The section on which the derailment securred included 6% miles of main track. The section foreman stated that ordinarily he had four men in his gang, but at the time of the accident he had but 3. He stated that this section of the track was carefully gone ever by his men 3 times each week and that he made an examination of the track at least twice a week for the express purpose of looking for broken rails; he had not young a broken rail on this division for at least 3 menths.

As this accident was caused by a broken rail arrangements were made with the Bureau of Standards, Department of Com-erce. for the purpose of having this rail examined and the cause of its failure accordance. This examination was consucted by Mr. James E. Howard, Engineer-Physicist, of the Bureau of Standards, and the report regarding his examination, with the accordanying illustrations, is attached to and made a part of this report.

The broken rail which coused this accident was naturactured by the Tennensee Coal & Iron Company; it was 33 feet long, reighed 60 younds to the yard; was rolled in November, 1904, and laid in the wrack in January, 1905. The first brook in this rail was found 18 fact of inches from the east end, while that , art of the rail west of this fracture was broken in 9 pirces, renging from 6-5/8 inches to 5 feet 5 inc es in length, the last break bing near the splice plate at the west end. From the appearance of the fractures and the battered ends of the pieces of rail it is believed that the first break occurred either between pieces Nos. 2 and 3 or between pieces Nos. 3 and 4. shown in illustration No. 1. A transverse fissure one inch in diameter was found at the facture between pieces Nos. 3 and 4, and that fact together with other evidence brought out by Mr. Howard's exumination in leates that this fracture was the first one to be formed. The factures to the east of this initial fracture differ from those to the west in that the fragments were buised on different ends. from which is is inferred that the several frictures were caused by the trains which moved in colonite directions. It also appears that the receiving ends of fragments Nos. 1 and 2 fero bruised and displaced in such a marner as to indicate that this had been done by an eastbound

train. The manner in which the rail broke is clearly shown in illustrations Nos. 1. 2 and 5. Illustration No. 4 shows the transverse fiscure one inch in dismeter on the fractured surface of fragment No. 4. It will be noted that this transverse fiscure was located on the gauge side of the head nearly over the geb. This fissure is similar to the fiscures found in the broken rails which caused the accidents on the Lehigh Valley Railroad at Manchester, N. Y., August 25, 1911, and on the Louisville & Mashville Railroad near May's Mill, Oct. 1, 1912, upon with reports have been made.

The 17 other rails which had to be replaced were bent and twisted out of shape but were not fractured.

In view of the fact that the formation of transverse fissures in rails is a grave menace to the safety of railroad travel, the facts disclosed by this investigation and the conclusions reached by Mr. Howard as a result of his study and tests of this rail are of particular interest and value.

Mr. Boward points out that transverse fissures are progressive in their character and development. They have been observed at different stages in their growth from ".30 diameter, to a maximum of 2½". The material and locality in which they exist have been derined. They have been found only in steel rails, where they commonly are developed on the gauge side of the head, or ever the web. In their development from a minimum to a maximum diameter the extension takes place while the rails are under service condition in the track.

Purthermore, Mr. Howard calls ettention to the fact that:

"The rate of development of a transverse fissure should be an accelerating one, since the resistance of the rail diminishes as the fissure extends.

It is a question of interest whether an effect caused by an excessive load is further accentuated by the application of loger loads; that is, whether the effect of an everload coming from the drivers of the engine would be further increased by the lesser wheel loads of the train. Laboratory tests on the effects of repeated stresses have shown, however, that many million repetitions of a lesser load may be applied to a steel bar without causing rupture, while substantially the normal number of repetitions of a maximum load will thereafter effect a fracture.

This is so vital a feature in the use of railway material that confirmatory late are desirable to ecquire from independent sources. Provided these indications are trustworthy, it follows that the limit of endurance of a steal rail is not necessarily measured

by total tomage, but rather by the number of repetitions of high wheel loads to which it is exposed.

Frequent rail failures are regarded as suffioient warning that railway practice is approaching the limit of endurance of rail steel.

The facts disclosed by the investigation of this derailment emphasize the statements made in the report regarding the derailment on the Louisville & Mashville Railroad, near Hay's Mill, Ala., where it was stated that the combined bending stresses and intense wheel contact stresses which attend the service conditions of steel rail appear to be the cause of and formation of these fissures. The insidious character of these fissures and their memace to safe travel by rail justify the conclusion reached that there is an absolute necessity of making a complete investigation of track and wheel conditions for the purpose of determining the effect thereon of the recent types of locomotives and cars with their greatly increased wheel loads.