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# **User's Manual for WSPRO**



## A Computer Model for Water Surface Profile Computations

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16. Abstract				
WSPRO, a water-surface profile computation model, can be used to analyze one-dimensional, gradually-varied, steady flow in open channels. WSPRO also can be used to analyze flow through bridges and culverts, embankment overflow, and multiple-opening stream crossings.				
This user's manual provides guidance for using the WSPRO model and updates a previous edition dated September, 1990. presents a general overview of input data requirements, uses conceptual examples to illustrate typical data sequences for various model applications, and provides detailed instructions for preparation of input data. Several example problems are included to illustrate both input data and model output. Analysis and interpretation of model output is discussed in detail.				
Two additional appendices have been added. Section 10 documents operation of the HYDRAIN WSPRO Input/Output Program. Section 11 discusses the operation of the SMS WSPRO interface. Both programs automate user interaction with the computer model.				
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#### **SECTION 1 - INTRODUCTION**

This manual presents data preparation instructions for WSPRO, a water-surface profile computation model. Version numbers are used to identify the various modifications and enhancements of the model. The version number consists of the letter V followed by the six-digit date (month, day, year). This manual corresponds with version V061698 of the program. Before studying these instructions, one should be aware of the basic computer resources and the user experience level required for WSPRO applications.

The WSPRO model consists of about 25,000 lines of Fortran source code. The code conforms to the American National Standards Institute (ANSI) Fortran 90 Standards. The program can be implemented on mainframe computers and microcomputers. Execution of the program requires 400 kilobytes of memory on microcomputers. Standard card reader logic is used for data input. A printer-compatible output file is created with successful model execution. Section 8 of this report presents additional information regarding implementation of the model on different computer systems.

Successful application of the model and proper interpretation of output from the model requires that the user have a background in surface-water hydraulics. Even with sufficient experience, however, it is not always easy to determine what data are necessary to adequately define the physical system for numerical analysis. Similarly, determining whether or not the output from a model adequately represents the real-world situation can be very difficult. The computational theory incorporated into this model is summarized in a previous report (Shearman and others, 1986)<sup>1</sup>. References cited in that report and in this manual provide additional detail on theory and methodology. Many users also may find it helpful to consult references that deal more directly with data requirements. Field-survey procedures are discussed by Benson and Dalrymple  $(1967)^2$ . Roughness coefficients are discussed in most standard hydraulics texts, such as those by Chow (1959)<sup>3</sup> and Henderson (1966)<sup>4</sup>. Barnes (1967)<sup>5</sup> presents color photos of sites for which roughness coefficients have been computed for measured discharges. Davidian  $(1984)^{6}$  discusses proper location and subdivision of cross-sections as well as additional topics related to water-surface profile computations. Matthai (1967)' discusses measurement of peak discharge at width contractions by indirect methods. Hydraulic analysis of flow through culverts is discussed in detail in several miscellaneous FHWA publications.<sup>8,9,10,11</sup> Shearman (1976)<sup>12</sup> discusses computer applications for step-backwater and floodway analysis and Schneider and others (1977)<sup>13</sup> discusses computation of backwater and discharge at width constrictions of heavily vegetated flood plains.

This manual attempts to simplify the instructions by prefacing detailed coding instructions with: (1) an overview of input data requirements without regard to actual data arrangement, and (2) a discussion of typical data sequences without detailed discussion of each pertinent parameter. These input data discussions are followed by an overview of model output and discussion of messages that may be generated during model execution. Detailed examples are then presented to illustrate specific input/output features of the model.

Section 2 presents an overview of all input data and discusses the individual data records in general terms without specifically defining the parameters within each data record. Section 3 presents typical data sequences for various model applications. These examples are conceptual inasmuch as they concentrate on the data sequences and not the individual parameters. These conceptual model applications, presented in order of progressively increased data requirements, are: (1) water-surface profile computations without considering bridges; (2) analysis of a basic, single-opening bridge situation; (3) analysis of a bridge with guidebanks; (4) single-opening analysis with consideration of embankment overflow; (5) analysis of several bridge design alternatives in a single model execution; and (6) analysis of a multiple-opening bridge situation. The objective of the overview and conceptual examples is to provide a background for the detailed coding instructions. Section 4 discusses the general rules and conventions for coding and then presents, in alphabetical order, detailed coding instructions for individual data records. Section 5 discusses the various forms of output that can be generated by the model. Section 6 summarizes all messages that may be generated during model execution. Several examples, complete with input data and model output, are presented in Section 7 to demonstrate model applications. Stand alone operation of the WSPRO executable is discussed in Section 8, Computer Considerations. A discussion on the operation of the HYDRAIN WSPRO Input/Output Program is presented in Appendix B. Appendix C contains a discussion of the Surface Water Modeling System's (SMS) graphical user interface for WSPRO.

## **SECTION 2 - INPUT DATA OVERVIEW**

It is convenient to categorize the input data into five general groups as follows: (1) title information; (2) job parameters; (3) profile control data; (4) cross-section definition; and (5) data display commands. Within each of these groups, the various parameters are further classified into subgroups, based on either their similarity to each other or the need to separate them from the others for ease of data modifications. These subgroups are allocated to different data records. The record types are defined by a one- or two-character identifier. These record identifiers were chosen with the intent that they be indicative of the data coded in that data record and (or) the purpose of that data record. A general discussion of the individual data records pertaining to each of the above groups follows. This information is summarized in Table 2-1.

#### 2.1 TITLE INFORMATION

Title information is used only for output identification. No other data (with the exception of the \* or \*F command) can precede the title information. Up to three title information records may be input (with identifiers of T1, T2, and T3), but it is permissible to use none. Except for column restrictions, there are no specific coding requirements. Therefore, the user has great flexibility to provide unique identification for individual jobs.

#### 2.2 JOB PARAMETERS

Two data records (with identifiers of J1 and UT) are available to define parameters that pertain to the entirety of the profile computations. The J1 record can be used to define such things as error tolerances, parameter test values and computational increments. Reasonable default values are provided for each of the parameters, thus negating the need for J1 data for many relatively standard applications. The UT record (which replaces the J3 record used in previous releases of the program) can be used to select parameters to be included in optional user-defined output tables.

#### 2.3 PROFILE CONTROL DATA

For each water-surface profile to be computed, up to 20 profiles per run, the model requires information regarding (1) discharge, (2) starting water-surface elevation, and (3) computation direction. Discharge data are coded in one or more Q records, one discharge value for each water-surface profile to be computed. A starting water-surface elevation for each of the discharges must be specified by the user or computed by the model. User-specified water-surface elevations are coded on one or more WS records.

TITLE INFORMATION	T1, T2	, T3 -	Alphanumeric data for identification of output.
JOB PARAMETERS	J1 UT	- -	Error tolerances, test values, and computational increments. User defined special tabling parameters.
PROFILE CONTROL DATA	Q	-	Discharge(s) for profile computation(s).
	WS	-	Starting water-surface elevation(s).
	SK	-	Energy gradient(s) for slope-conveyance computation of starting water-surface elevation(s).
	EX ER	-	Execution instruction, computation direction(s), and floodways. Indicates end of input (end-of-run).
CROSS-SECTION DEFINITION			• • •
Header Records	XS	-	Unconstricted valley section.
	BR	-	Bridge-opening section.
	GB	-	Guidebank section (SD record still recognized).
	XR	-	Road-grade section.
	AS	-	Approach section (no longer required but still recognized).
	CV	-	Culvert section.
	XT	-	Template section.
Cross-Sectional Geometry Data	GR	-	x,y-coordinates of ground points in a cross-section with exceptions at bridges, guidebanks, and culverts; and for data propagation.
Roughness Data	Ν	-	Roughness coefficients ('n'-values).
C .	SA	-	x-coordinates of subarea breakpoints in cross-section.
	ND	-	Hydraulic-depth breakpoints for vertical variation of roughness.
Flow Length Data	FL	-	Flow lengths and (or) friction slope averaging technique.
Component Mode Bridge Data	BL	-	Bridge length and location.
	BC	-	Bridge low-chord parameters (BD record is still recognized).
	AB	-	Abutment slopes.
	CD	-	Opening type and configuration.
	PD	-	Pier or pile data (PW record is still recognized).
	KQ	-	Conveyance breakpoints (KD record is still recognized).
Coordinate Mode Bridge Data	AB	-	Abutment toe elevations.
	CD	-	Opening type and configuration.
	PD	-	Pier or pile data (PW record is still recognized).
	KQ	-	Conveyance breakpoints (KD record is still recognized).
Approach/Road-grade Section Data	BP	-	Horizontal datum correction between bridge and approach and/or
			bridge and road-grade sections.
Culvert Section Data	CG	-	Culvert geometry.
	CC	-	Culvert coefficients.
OTHER RECORDS			
Scour Computation Data	DA	-	Abutment scour data.
	DC	-	Contraction scour data.
	DP	-	Local pier scour data.
Effective Flow Data	EF	-	Effective flow input.
Floodway Computation Data	FW	-	Mandatory record for floodway analysis to specify encroachment method, surcharge, and left and right encroachment limits.
	FS	-	Optional control record to vary surcharge for a range of flows.
Metric/English Units	SI	-	Metric/English input/output
Template Geometry Propagation	GT	-	Replaces GR data when propagating template section geometry.
Channel Hydraulic Properties HP	-	Produ distrib	ce tables of cross-sectional properties or velocity and conveyance bution.
Comments/Remarks	*	-	Insert comments and (or) blank lines in the input data sequence.
Free Format Input Dataset	*F	-	Allows free-format input (must be first record).

## Table 2-1.Tabulation of data records by group.

The user may choose to have the model compute the starting water-surface elevation. The model can compute either a "normal" water-surface elevation (by slope-conveyance method) or a critical water-surface elevation (based on minimum specific energy). A slope-conveyance computation can be obtained by specifying an energy gradient rather than a water-surface elevation. Energy gradients are specified in one or more SK records and take precedence over specified water-surface elevations. Specifying an elevation in the WS record that is less than the critical water-surface elevation for the initial cross-section (this is easily assured by coding a value below channel bottom) will cause the model to default to the critical-flow computation when no (or a negative) energy gradient is specified. The model is designed to compute profiles in both an upstream direction (for subcritical and (or) critical flow) and in a downstream direction (for supercritical and (or) critical flow). Computation direction for each profile to be computed is specified in an EX record whenever any downstream computations are involved (upstream is the default direction). The EX record also instructs the model to begin the profile computations. If the EX record is absent, the model will input and check the data but will not compute profiles. Obviously, a Q record must be coded for any job. Depending on the user's choice of options, one or both or neither of the WS and SK data records are required. Care must be taken to provide a consistent number of entries and maintain a one-to-one correspondence between the entries in the Q, WS, SK, and EX records. This will be illustrated in later examples.

#### 2.4 CROSS-SECTION DEFINITION

The majority of the input data is the cross-section data required to describe the physical system. Water-surface profile computations may require several different types of cross-sections. Regardless of the type of cross-section, the user must define the location, geometry, and roughness of each cross-section and perhaps other coefficients and parameters associated with the cross-section which influence the profile computations. The model is limited to a total of 100 cross-sections in a single job.

#### 2.4.1 Header Information

Header information is required for each cross-section. Unique record identifiers for the various header records serve to identify the different cross-section types. Certain coefficients and parameters associated with each cross-section are coded in the header record.

The header record identifiers and the cross-sections to which they apply are:

XS All unconstricted valley sections. (1)(2)BR Bridge-opening section. \_ GB Guidebank section. (3) (4) XR Road-grade section. \_ (5) CV Culvert section. -XT (6) Template section. \_

A cross-section identification code and a section reference distance are required in every header record. The section reference distance defines the location of the cross-section in the study reach relative to other cross-sections. It also provides one method of defining subreach flow lengths. Optional parameters common to the first five header records are cross-section skew, expansion and contraction coefficients, and valley slope. The BR, GB, XR, and CV records provide for additional parameters unique to bridges, guidebanks, road-grades, and culvert sections, respectively. The model provides reasonable default values for many of these optional and additional parameters.

#### 2.4.2 Cross-Sectional Geometry

Cross-sectional geometry is defined for most cross-sections using GR records. Pairs of x,y-coordinates representing the horizontal station and ground elevation of each ground point are coded on one or more GR records. Horizontal stationing is measured from any arbitrary datum on the left bank except at bridges where certain cross-sections must be referenced to a common horizontal datum. The model uses left to right as a positive direction and assumes that left and right are defined by looking downstream. Ground elevations at all cross-sections must be referenced to a common vertical datum. GR data are not coded for most bridge design applications (unless using coordinate mode to specify the shape of the bridge opening), some guidebank situations, culvert cross-sections, and when propagating cross-sectional geometry.

#### 2.4.3 Roughness Data

Manning's roughness coefficients may be specified in N records for all cross-sections except culverts. When a cross-section is subdivided for roughness or geometry, the x-coordinates of the subdivision points are coded in SA records. Hydraulic-depth breakpoints for vertical variation of roughness are coded in ND records. When propagating roughness data, only the data that has changed from that of the downstream cross-section is required. It is not necessary that x-coordinates for subdivision points correspond to x-coordinates for points defining the shape of the cross-section.

#### 2.4.4 Flow Lengths

Two options exist for defining flow length between successive cross-sections. The default option uses the difference between the section reference distances of successive header records. However, some users prefer a reference distance tied into a datum that does not necessarily reflect flow length (for example, centerline distance along a meandering stream). Another problem with using a cumulative distance is that a revision or error in one cross-section influences additional cross-sections. Sometimes it also is desirable to vary the flow length across

the valley between two cross-sections. Therefore, the user may choose to use FL records to define from one to three flow lengths between successive cross-sections.

#### 2.4.5 Bridge-Opening Cross-Sections

Two conventions are available for coding bridge-opening cross-sections. The first method (referred to as *component mode*) uses easy to define *components* to describe the shape of the bridge opening. The second method (referred to as *coordinate mode*) allows the user to describe the actual shape of the opening using *coordinate* points similar to how cross-section shapes are defined. The advantage of the *component mode* lies in the ability to easily alter the bridge type, length, and shape which allows convenient analysis of different design alternatives. The advantage of the *coordinate mode* is in the ability to model unusual bridge opening shapes or openings that don't conform to the coordinate mode guidelines.

#### 2.4.5.1 Component Mode

The component mode minimizes input data requirements for definition of (and subsequent modifications) bridge geometry by using information specified on records to define specific components of the bridge geometry. Component mode simply provides the user with the flexibility to easily analyze different design alternatives or bridge configurations. The model itself has no design capabilities. The record identifiers and the data contained in the additional records are:

(1)	BL	-	parameters defining bridge length and horizontal location of the
			opening.
(2)	BC	-	parameters defining bottom chord information for the bridge.
(3)	AB	-	parameters defining abutment slopes for the bridge.

In component mode, a bridge-opening cross-section is created by combining the data from the BL, BC, and AB records with the GR data of the full-valley cross-section (input immediately prior to the bridge-opening cross-section). An AB record is required in component mode only for spill-through type abutments (Type III bridge). The BL and BC records are always required in component mode for initial definition of the bridge opening. Subsequent bridge geometry modifications (i.e., for alternative designs in a single model execution) require recoding only those record type(s) pertaining to the parameter(s) being changed.

#### 2.4.5.2 Coordinate Mode

Some bridge geometries, such as arched openings, defy adequate definition by the component mode. Also, for existing bridges it is highly preferable to not have to break down the

geometry into components if nothing is going to be modified. The coordinate mode permits coding a bridge opening as a closed polygon using GR data. With left and right defined relative to looking downstream, the minimum (leftmost) horizontal station and its elevation must be the first x,y-coordinate coded. If the left edge of the bridge is vertical, the coordinate pair for the highest elevation must be coded first. The remaining x,y-coordinates for the ground, abutments, and low steel must be coded in counter-clockwise order. The first coordinate pair must be repeated as the last coordinate pair in the GR data.

Whichever of the above procedures is followed, other applicable data records are:

(1)	CD	-	parameters defining bridge and embankment configuration.
(2)	KQ	-	parameters for user override of stationing related to conveyance
			breakpoints in the approach section.
(3)	PD	-	parameters defining piers or piles.

The CD record is mandatory, the KQ and PD records are optional.

#### 2.4.6 Guidebanks

Two conventions are also available for defining guidebank geometry. One of the parameters in the GB header record is the horizontal offset of the guidebank relative to the bridge abutments (measured normal to the flow at the mouth of the guidebank). In component mode, if the user codes a value for this parameter, the model will construct a cross-section at the mouth of the guidebank. The top width of that section will be equal to the top width of the bridge opening plus twice the offset value coded. The side slopes will equal the bridge abutment slopes and the channel bottom geometry is propagated from the full-valley section at the bridge. The coordinate mode requires defining guidebank section geometry with GR data. Defining the shape of the guidebank cross-section using coordinate mode is often necessary when the guidebanks are not symmetrical. The guidebank cross-section should be constructed at the location of the mouth of the guidebanks.

#### 2.4.7 Road Grades

The geometry of a road-grade section is defined by x,y-coordinates in GR records in a manner similar to other cross-sections. The x,y-coordinates should define a transect along whatever part of the embankment will act as the weir crest (perhaps the centerline (crown) in a rural situation and sidewalk (or curb) in an urban situation). Roughness data are not input for road-grade sections (the model output reflects propagated roughness data for the road grade but the original purpose for that was never implemented in the model). Cautions that need to be exercised in coding road-grade sections are discussed in Section 4.3.9.

#### 2.4.8 Approach Sections

The only difference between an approach cross-section and any other unconstricted valley cross-section is that the model assumes that the first unconstricted valley cross-section (defined with an XS header record) upstream of the bridge opening is the approach section. Cautions to be exercised in coding approach sections are presented in Section 4.3.8.

#### 2.4.9 Culverts

Culvert hydraulics are analyzed using FHWA methodology (U.S. Federal Highway Administration, 1982; 1980; 1979). Shearman and others (1986) present a full summary of the culvert computations. Culverts may be analyzed both in multiple-opening situations, as standalone (single- or multi-barrel) installations, and as an intermediate structure in a continuous water-surface profile computation. A culvert can be defined with: (1) a CV header record; (2) a CG record to define culvert geometry; and (3) a CC record to define applicable coefficients. Default values are provided for all of the parameters in the CC record.

#### 2.4.10 Datum Corrections

All cross-sections must be referenced to a common vertical datum. An individual crosssection surveyed to an incorrect datum could be adjusted using the template section features of the model (see Section 2.4.12). The field-survey data could be coded as a template section and the YSHIFT parameter of the GT record applied to make the appropriate elevation adjustment. This procedure could also be used to change the datum for all cross-sections (e.g., to convert an arbitrary datum to mean sea level datum). This would be somewhat cumbersome since it would require coding each cross-section as a template section to apply the necessary adjustment.

One-dimensional models generally do not require a common horizontal datum from cross-section to cross-section. However, WSPRO's analysis of single-opening bridge hydraulics requires common horizontal datums for: (1) bridge-opening and approach sections (for effective flow length computations); and (2) bridge-opening and road-grade sections (for determining correct location of left and right road overflow segments). The user may choose to make such horizontal datum corrections by coding a BP record in conjunction with road-grade and (or) approach section data (instead of adjusting field-survey or design data prior to input). BP record input data for this purpose is the horizontal station on the road-grade and (or) approach section(s) that coincide with certain reference points on the bridge section. The reference points depend on whether component mode or coordinate mode is being used. Horizontal stationing of the bridge opening may vary for different alternatives in component mode. Therefore, the location constraining stations (XCONLT and XCONRT in the BL record) are the logical reference points if using component mode. In coordinate mode, the minimum and maximum horizontal stations of the bridge-opening section are used as reference points. The horizontal station on the road-

grade section that coincides with the intercept of a vertical projection from the appropriate left reference point of the bridge is sufficient to relate the horizontal data of the bridge-opening and road-grade sections. The horizontal station at the approach section that coincides with the intercept of a line projected parallel to the flow from the appropriate left reference point of the bridge is sufficient to relate the horizontal data of the bridge and approach sections, except for cases of curvilinear flow.

If curvilinear flow exists, a BP record should be included with the approach section data to assure an adequate estimate of effective flow length, even if there is no horizontal datum problem. For this purpose, lines parallel to the flow are projected from both the left and right reference points of the bridge section to the approach section. The horizontal stations at the approach section of both flow-line intercepts and the length of both flow lines are coded in the BP record.

Multiple-opening analysis involve projections of horizontal stationing from section to section to define valley strips and to fabricate intermediate exit and approach cross-sections. Therefore, all cross-sections, from the downstream match section through the upstream match section, inclusive, must be referenced to a common horizontal datum. The user must make all necessary adjustments to field-survey or design data prior to input to the model.

#### 2.4.11 Data Propagation

WSPRO is designed to minimize coding of repetitive data. Data applicable to successive cross-sections are automatically propagated by the model. That is, once values are established (by specification or default) for certain parameters, those values will be used at each succeeding cross-section until new values are specified. The changed value will then be propagated until another value is specified. Three parameters in the header records (expansion and contraction coefficients and valley slope) are automatically propagated. When fabricating a series of crosssections from one known cross-section it is quite likely that all of the data in the N, ND, and SA records are identical for all of the fabricated sections. Coding N, ND, and SA data for the first (most downstream) section of such a series of cross-sections is sufficient and appropriate. Even when not fabricating cross-sections, it is fairly common for the N and ND data to be identical for successive cross-sections but for the SA data (subdivision points) to vary. In such cases, coding the N and ND data for the downstream section and coding only the SA data for succeeding sections is sufficient. A change in the number of subdivisions nullifies the propagation of all roughness data. However, a change of magnitude of values in one record type does not require recoding all roughness data. For example, if the roughness coefficients change but the vertical breakpoints remain applicable, only the N data need recoding and the ND data can be propagated.

It also is possible to repetitively apply geometry data. Omitting GR data from the crosssection definition will cause the model to propagate GR data from the preceding (downstream) section with no changes. Coding valley slope in the header record and omitting GR data will result in geometry with x-coordinates identical to the preceding section and y-coordinates equal to those of the preceding section plus the product of valley slope and the difference in section reference distances of the two cross-sections.

#### 2.4.12 Template Sections

Additional flexibility for repetitive use of geometric data is possible using template sections. A template section is defined with an XT header record and GR records. Once defined, the template section is retained in its original form until replaced with a new template section. There are no limits to the number or location of cross-sections that may be fabricated using the template-section geometry. The horizontal geometry of the template section can be: (1) used unadjusted; (2) expanded or contracted by a scale factor; or (3) partially used by "chopping off" portions of the left and (or) right sides of the section. Vertical geometry of the template section may be: (1) used unadjusted; (2) shifted by a constant; or (3) shifted by the product of valley slope and section reference distance difference. Use of the template section to fabricate a cross-section is implemented using a GT record instead of GR records for a section. The GT record introduces the desired scaling and shifting parameters.

The roughness coefficients (N records) and hydraulic-depth breakpoints (ND records) must not be included with the template section data but with the data of the section(s) being fabricated. SA records (subarea breakpoint stations) may appear with either the template section or the section(s) being fabricated. However, when the scale factor option is to be applied, the subarea breakpoints are not scaled by the model. Therefore, the SA data must be included with each fabricated section and the breakpoints adjusted to be consistent with the scaled x,y-coordinate data.

#### 2.5 OTHER RECORDS

#### 2.5.1 Floodway

The data requirements for a floodway analysis are accomplished using FW and FS data records. The FW record allows the user to complete a floodway analysis for individual cross-sections or for part of or all of the cross-sections in a reach. Parameters associated with the FW record are an option code, a surcharge limit, left horizontal encroachment station, and right horizontal encroachment station. With the exception of the option code, the remaining parameters can be left blank to allow default computations to occur. Each FW record can have a separate surcharge value as a parameter. However, when a surcharge value is provided on the first FW record encountered in the input data set, WSPRO will automatically propagate this value to subsequent FW records. An optional, separate FS record allows introduction of global

surcharge values that are associated with each discharge on the Q record. The FS record must be grouped with the profile control information before any header records are encountered.

#### 2.5.2 Scour

The data requirements for scour computations are dependent on the type of analysis desired. Pier scour analysis requires coding of the necessary parameters and constants for use with a pier scour prediction equation. This information is input by using a DP record. Estimates of contraction scour are dependent on estimation equation parameters which are input on the DC record. Abutment scour is computed using parameters coded on a DA record. Detailed information on the theory and application of the pier, contraction, and abutment scour computation methodologies can be found in HEC-18, Evaluating Scour at Bridges, Richardson and Davis (1995)<sup>14</sup>. Multiple DP, DC, and DA scour records can be used in a separate program execution.

#### 2.5.3 HP Records

Tables of cross-sectional properties and (or) velocity and conveyance distribution are optional forms of model output. Tables of cross-sectional properties and (or) velocity and conveyance distribution can be obtained using HP records. The appropriate command record(s) must be input for each cross-section for which such output is desired.

#### **SECTION 3 - TYPICAL DATA SEQUENCES**

This section presents examples of data sequences for various model applications. The primary objective is to illustrate data sequences with little attention to individual parameters within each record. Generally, only three parameters in the cross-section header records are shown. These parameters are the record identifier (columns 1-2), the section identification code (columns 6-10), and the section reference distance (first parameter after column 10). Parameters are indicated only if they provide additional clarity to the example, and then they are indicated by name rather than actual numerical value. Figure 3-1 illustrates all record types required to describe an unconstricted, valley cross-section data group. The GR records must always be in a contiguous group and in sequence of ascending horizontal stationing. There are no other restrictions as to the ordering of the record types illustrated. The following sections describe various arrangements of these data, first without consideration of bridges, then with additional data to describe both single- and multiple-opening bridge situations and to obtain results for alternative designs.

	80 character record
1-2 4 6-10	11-80
ID# SECID	User's Format
XS secid GR GR R N ND SA FL	<pre>srd [,other variables as needed] x,y-coordinates continued Manning's roughness coefficients hydraulic-depth breakpoints for vertical roughness variation station breakpoints for horizontal subdivisions flow length/friction-loss computation data</pre>

Figure 3-1. Record types for defining unconstricted valley cross-sections.

#### 3.1 REACHES WITH NO BRIDGES

The first example is a curvilinear stream reach with variable land cover and a slightly meandering main channel (Figure 3-2). The data sequence in Figure 3-3 reflects assumptions as follows: (1) four cross-sections (three subreaches) are needed to define the total reach; (2) a rating curve (relation of water-surface elevation and discharge ) is known at the downstream cross-section (XSEC1); (3) cross-sectional geometry varies significantly (thus GR data are

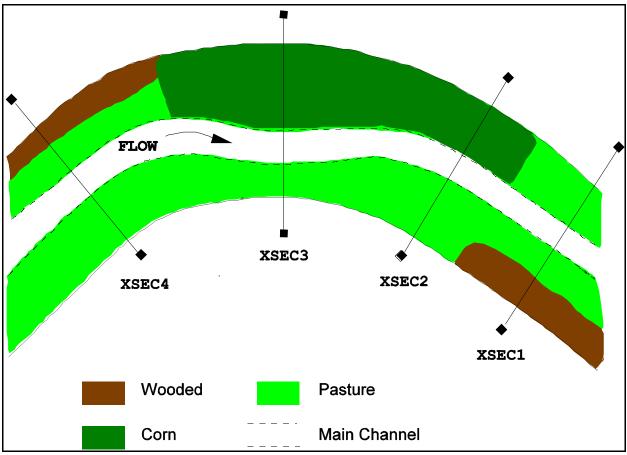


Figure 3-2. Definition sketch: curvilinear reach with variable land cover.

needed for each section); (4) roughness is to be varied both horizontally and vertically at each cross-section; and (5) overbank and main-channel flow lengths are significantly different in both the second and third subreaches. The user should note the cross-section locations in Figure 3-2. A cross-section should describe average geometry and roughness for a subreach. The computational procedure assumes that a subreach extends half the distance to adjacent cross-sections in both the upstream and downstream directions.

	80 character record
1-2 4 6-10	11-80
ID# SECID	User's Format
GR GR ND SA XS XSEC N GR GR SA ND XS XSEC FL SA GR GR	<pre>T1, T2, T3 records as desired J1 record [as needed] UT record [as needed] q1, q2, q3, ws1, ws2, ws3, 1 srdl, [as needed] x1,y1 x2,y2  last x,y-coordinate nbot1, ntop1, nbot4, ntop4 botd1, ntop1, botd4, top4 xsa1, xsa2, xsa3 2 srd2, [as needed] nbot1, ntop1, nbot3, ntop3 x1,y1 x2,y2  last x,y-coordinate xsa1, xsa2 botd1, topd1, botd3, topd3 3 srd3, [as needed] flen1, xlb, flen2, xrb, flen3 xsa1, xsa2 xl,y1 x2,y2  last x,y-coordinate 4 srd4, [as needed] x1,y1 x2,y2  last x,y-coordinate 4 srd4, [as needed] x1,y1 x2,y2  last x,y-coordinate xsa1, xsa2 botd1, topd1, botd4, top4 nbot1, ntop1, nbot4, ntop4 flen1, xlb, flen2, xrb, flen3</pre>

Figure 3-3. Input data sequence: curvilinear reach of Figure 3-2.

The title information (T1, T2, and T3 records) and job parameters (J1 and UT records) must precede all other data (with the exception of the \* and \*F command). All job-parameter data may be omitted if all default values are satisfactory and no special tables are desired. Because a rating curve exists for XSEC1, a WS record is used to specify a beginning water-surface elevation for each discharge specified in the Q record. In addition to the header record and GR data, a complete description of roughness (N, ND, and SA records) is required for each of the first two sections. Because the number of subareas and the roughness values differ from XSEC1 to XSEC2 (pasture, main channel, pasture and woods versus corn, main channel and pasture), roughness data cannot be propagated from XSEC1 to XSEC2. However, the N and ND data for XSEC2 can be propagated to XSEC3. SA data are required at XSEC3 because the

subdivision stations are different than those at XSEC2. XSEC4 also requires complete roughness data because of four versus three subareas and different roughness coefficients than XSEC3. FL data are never required at the most downstream cross-section because friction-loss computations for the downstream subreach rely on the upstream FL data. FL data are not required for XSEC2 in this example because the lengths along the overbanks and main channel are equal, thus the difference between the section reference distances of XSEC2 and XSEC1 is used for flow length. FL data are used at XSEC3 and XSEC4 to account for the different flow lengths in the overbanks and main channel. The EX record instructs the model to compute profiles for the preceding data; one profile is computed for each discharge specified in the Q record.

The second example reach (rather idealistic) has uniform geometry and the same roughness characteristics throughout. Cross-sectional geometry for XS1 was obtained by field survey. Channel slope varies as shown in Figure 3-4 and the stage-discharge relation is unknown.

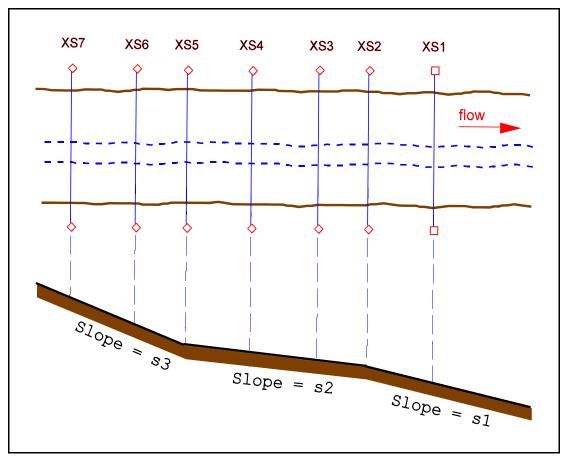


Figure 3-4. Definition sketch: idealistic reach; data propagation.

Figure 3-5 illustrates the data sequence. An SK record (rather than a WS record) is appropriate to define the starting conditions for profile computations if one assumes that the slope, s1, extends a significant distance down- stream and there are no downstream controls to affect the water-surface profile. In that case normal flow can be assumed at XS1. Thus, slopeconveyance is appropriate for computing the starting water-surface elevation at XS1. Complete description of the geometry and roughness is required for the first section; note that both horizontal and vertical variations of roughness are used. Subsequent cross-sections require only a header record because the uniform reach permits propagation of all geometry and roughness data. Valley slope specified in a header record is applicable to the subreach extending to the adjacent downstream cross-section, as well as to successive upstream subreaches until a different valley slope is specified. Therefore, coding the three slopes (s1, s2, s3) in the header record of the first section where each slope is applicable is sufficient to adjust the cross-section elevations. The [as needed] entry in the header records indicate additional variables that might be coded in a header record.

	80 character record			
1-2	4 6-10	11-80		
ID#	SECID	User's Format		
T_ J1 UT Q SK XS GR GR GR ND SA XS XS XS	XS1 XS2 XS3 XS4	<pre>T1, T2, T3 records as desired J1 record [as needed] UT record [as needed] q1, q2, q3, s1, s1, s1, srd1, [as needed] x1,y1 x2,y2  etc., etc.,  last x,y-coordinate nbot1, ntop1, nbot2, ntop2, nbot3, ntop3 botd1, topd1, botd2, topd2, botd3, topd3 xsa1, xsa2 srd2, [as needed], s1 srd3, [as needed], s2 srd4, [as needed]</pre>		
XS	XS3	srd3, [as needed], s2		



The third example illustrates use of a template cross-section. The three cross-sections (XSEC7, XSEC8, XSEC9) shown in Figure 3-6 were obtained by field survey After initial analyses it was judged that the shaded area is ineffective flow area and that two additional sections are needed to adequately model the flow expansion and contraction in the reach. Truncation of the middle section (XSEC8) to better model the effective flow area could be accomplished by discarding data left of the effective-flow cutoff point and coding a vertical wall at that point. Also, one could fabricate the intermediate sections (XS8-A and XS8-C) using x,y-coordinate data from XSEC8 (with elevations adjusted for valley slope, if necessary) and coding a vertical wall at the appropriate cutoff point.

WSPRO can perform this truncation and fabrication by means of the template crosssection feature. This is especially convenient when the need for modified and (or) additional cross-sections is discovered after the data have already been coded. In this case the surveyed XSEC8 can serve as a template cross-section. A template section can be placed anywhere in the data sequence prior to its first use. The template section follows XSEC7 in the data sequence in Figure 3-7. The section reference distance and valley slope coded in the XT header record are used to adjust elevations of any cross-section fabricated from the template section. XS8-A is

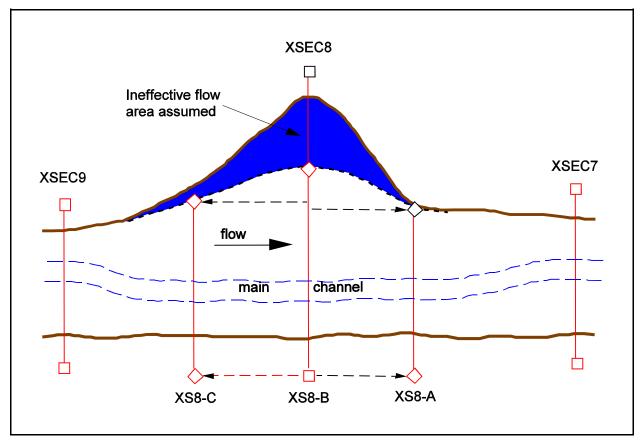


Figure 3-6. Definition sketch: example application of a template cross-section.

defined by an XS header record, a GT record, and an SA record. The horizontal station of the cutoff point (at the tail-end of the arrow pointing to XS8-A) is coded as the XLIML parameter, the second parameter in a GT record. The null value (\*) is coded because the first parameter is not being used but proper parameter positioning must be maintained. The coefficients and vertical variation of roughness at XSEC7 are assumed applicable and propagated to XS8-A. SA data are recoded to reflect XSEC8 subarea breakpoints. Assuming that subarea stations would be the same for all sections fabricated from the template section, XS8-B and XS8-C require only XS and GT records. As it was for XS8-A, the XLIML parameter on the GT record for XS8-B and XS8-C is used to specify the horizontal station of the appropriate cutoff point on the left overbank (at the diamond for XS8-B and at the tail-end of the arrow pointing to XS8-C).

	80 character record
1-2 4 6-10	11-80
ID# SECID	User's Format
XS XSEC	7 srd7, [as needed] [ GR, N, ND, and SA data, as appropriate ]
XT XSEC8 GR GR *	3 srd8, vslope x1,y1 x2,y2 last x,y-coordinate
XS XS8-A GT SA *	A srd8-A, [as needed] *, xliml xsal, xsa2
XS XS8-E GT *	3 srd8, [as needed] *, xliml
XS XS8-0 GT *	C srd8-C, [as needed] *, xliml
XS XSECS	9 srd9, [as needed] [ GR, N, ND, and SA data, as appropriate ].

Figure 3-7. Input data sequence: template section application for Figure 3-6.

#### 3.2 SINGLE-OPENING BRIDGE SITUATIONS

Computations for bridges having a single opening require definition of a minimum of four cross-sections. These sections consist of three unconstricted valley sections in addition to the bridge-opening section. Presence of guidebanks and (or) possible road overflow introduces the possibility for an additional one or two optional cross-sections.

#### 3.2.1 Basic Bridge Situation, Component Mode

Figure 3-8 is a sketch of a typical single-opening bridge situation.

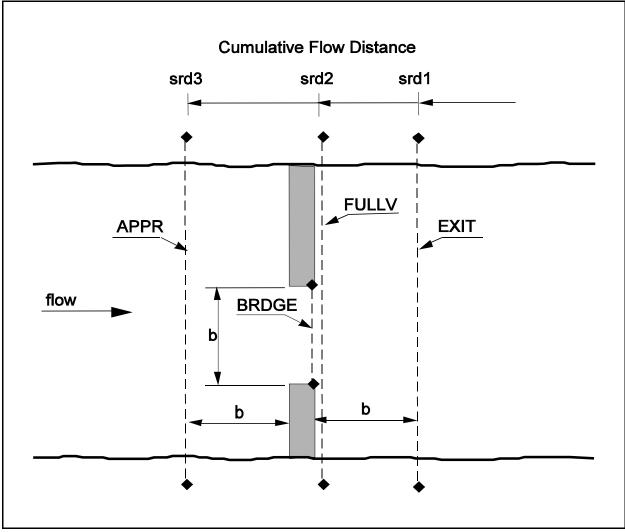


Figure 3-8. Definition sketch: cross-section locations, single-opening bridge.

Figure 3-9 illustrates a data sequence for a case where: (1) component mode is being used for the bridge opening (i.e., bridge design data are to be superimposed on the full-valley section geometry); (2) there are no guidebanks; and (3) there is no chance of overtopping the embankment (thereby eliminating the need to code a road-grade section).

------80 character record ------1-2 4 6-10 11-80 ID# SECID User's Format XS EXIT srd1 [,as needed] [ GR, N, ND, and SA data, as appropriate ] \* XS FULLV srd2 [,as needed] [ GR, N, ND, and SA data, as appropriate ] \* BR BRDGE srd2 [,as needed] [ N, ND, and SA data, if appropriate ] BL brlen, xconlt, xconrt BC girdep, bdelev [,as needed] brtype, brwdth [,as needed] CD AB optional records PD \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ KQ XS APPR srd3 [,as needed] [ GR, N, ND, and SA data, as appropriate ]

Figure 3-9. Input data sequence: single-opening bridge of Figure 3-8, component mode.

This sequence begins with definition of the exit section which is one bridge length downstream from the bridge-opening cross-section. Data defining title information, job parameters, and profile control data are not illustrated. Also, additional cross-sections could be defined, if desired, both downstream from the exit cross-section and upstream from the approach cross-section. The exit and full-valley sections are each illustrated in Figure 3-9 with an XS header record followed by GR data to describe geometry and N, ND, and SA data to define roughness. The bridge section is introduced with a BR header record which is followed by roughness data (if different from the full-valley section roughness) and the bridge parameter data. The BL and BC records are mandatory in component mode to define the size and location of the opening. The bridge length (distance between the tops of the abutments) and the horizontal constraints for the location of the abutment toes must be specified in the BL record to define the horizontal dimension of the bridge and the extent of the full-valley section geometry to be used, respectively. The BC record supplies data required to place the "top" on the opening. Girder depth and deck elevation are required and, for sloping decks, the slope and the x-coordinate corresponding the specified deck elevation (y-coordinate) must be coded. The CD record also is

mandatory and provides data required for determining the coefficient of discharge and the flow length through the bridge. The type of bridge opening and the bridge width (dimension in direction of flow) are always required and other parameter requirements depend on the bridge type. The remaining records are required only under certain circumstances or are entirely optional. A brief summary of data requirements for the bridge opening using component mode follows.

- (1) In component mode the AB record is only required to specify abutment slopes for a type 3 opening (all other opening types have vertical abutments).
- (2) Only when there are piers or piles to be defined is it necessary to code PD data.
- (3) The KQ record is required only when it is necessary to override the default determination of the computed boundaries of the K<sub>q</sub> segment of the approach cross-section which influences the computation of the flow-contraction ratio. These requirements are further defined in the KQ record explanation in Section 4.

Because there are no guidebanks nor road-grade sections involved, definition of the approach section immediately follows the definition of the bridge-opening section. An unconstricted valley cross-section record (XS record) and the appropriate GR, N, ND, and SA data are required to define the approach section.

#### 3.2.2 Basic Bridge Situation, Coordinate Mode

Existing bridges, or bridges that do not conform to the component mode data requirements (e.g., arch bridges), can be defined using the coordinate mode. This convention requires using GR data to describe the opening as an irregular polygon. Very rigid rules apply to the specification of x,y-coordinates around this polygon. The minimum (leftmost) horizontal station and its elevation must be the first x,y-coordinate specified. If the left abutment is vertical, x,y-coordinates for the highest elevation must be coded first. Subsequent coordinates for points along the abutments, ground surface, and low chord must be specified in a counter-clockwise direction. The last x,y-coordinates must be an exact duplicate of the first x,y-coordinates. Subdivision station(s) (SA record) for unconstricted valley cross-sections do not have to coincide with any station(s) in the GR records. However, if a bridge-opening cross-section must be subdivided because of geometry and (or) variations in roughness, there must be a distinct relation between the SA and GR data. GR data must include x,y-coordinates at each subarea breakpoint along the bottom of the bridge-opening polygon.

Figure 3-10 illustrates the coordinate mode data sequence for the preceding example. The BL and BC data are not used because the GR data defines the bridge-opening geometry. As is the case for component mode, the CD record is mandatory and the PD and KQ records are optional for the coordinate mode. The AB record is again optional but is used to satisfy a different requirement. The AB record is required in coordinate mode only to specify elevations at the toes of the abutments for a type 2 opening. These elevations (determined by the model in the component mode) are needed to compute one of the adjustment factors for the coefficient of discharge.

	80 character record				
1-2	4 6-10	11-80			
ID#	SECID	User's Format			
XS * XS		<pre>srd1 [,as needed] [GR, N, ND, and SA data, as appropriate] srd2 [,as needed]</pre>			
* BR CD		[GR, N, ND, and SA data, as appropriate] srd2 [,as needed] [GR, N, ND, and SA data, as appropriate] brtype, brwdth [,as needed]			
AB PD KQ *		optional records			
XS	APPR	srd3 [,as needed] [GR, N, ND, and SA data, as appropriate]			

Figure 3-10. Input data sequence: single-opening bridge of Figure 3-8, coordinate mode.

#### 3.2.3 Addition of Guidebank Data

Figure 3-11 is a definition sketch of a bridge with guidebanks. As with bridge sections, there are two conventions applicable to definition of a guidebank section. Component mode permits superimposing guidebanks on the full-valley section. This is only applicable when component mode is also being used for the bridge-opening section. The GBOFF parameter (horizontal offset) on the GB header record initiates component mode for guidebanks (see Table 4-25). Side slopes of the guidebanks are equal to the slope(s) of the bridge abutments. These data, combined with the appropriate segment of the full-valley section, define guidebank geometry. Coordinate mode requires GR data to define guidebank section geometry. In either mode, roughness data are specified in N, ND, and SA records as appropriate for the situation and parameters for determining the guidebank adjustment factor applied to the coefficient of discharge are coded in the GB header record.

Figure 3-12 illustrates a data sequence for the sketch of Figure 3-11, including GR data to define guidebank geometry. Guidebank section data are always input immediately following the input data for the bridge opening to which the guidebanks are attached.

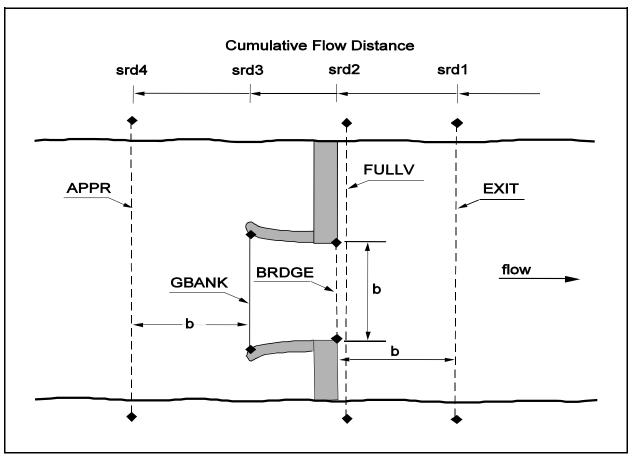


Figure 3-11. Definition sketch: cross-section locations, single-opening bridge with guidebanks.

		80 character record
1-2 4	6-10	11-80
ID#	SECID	User's Format
XS	EXIT	srd1 [,as needed] [GR, N, ND, and SA data, as appropriate]
XS	FULLV	srd2 [,as needed]
		[GR, N, ND, and SA data, as appropriate]
BR	BRDGE	srd2 [,as needed]
		[BL and BC data or GR data, and
CD		N, ND, and SA data, as appropriate] brtype, brwdth [,as needed]
GB	CDDNIK	[AB, PD, and KQ data as appropriate]
GB	GDBNK	srd3, gbtype, bsubd [,as needed] [GR, N, ND, and SA data, as appropriate]

XS APPR srd4 [,as needed]
\_\_\_ [GR, N, ND, and SA data, as appropriate]

#### 3.2.4 Addition of Road-Grade Data

Figure 3-13 illustrates the addition of a road-grade cross-section to a data sequence for a basic bridge situation. The road-grade cross-section data immediately precede the data describing the approach cross-section. A default value is provided within the model for each of the parameters in the road-grade header record except for section reference distance. Explanation of each of these default values is presented in the detailed coding instructions for the XR header record. When these default values are not sufficient, they can be overridden with appropriate user-specified values. Geometry of the top of the embankment defining the "potential" weir crest are described by specifying x,y-coordinates in GR records in the same manner that unconstricted valley sections are defined.

		80 character record
1-2	4 6-10	11-80
ID#	SECID	User's Format
XS *	EXIT	srdl [,as needed] [GR, N, ND, and SA data, as appropriate]
XS *	FULLV	srd2 [,as needed] [GR, N, ND, and SA data, as appropriate]
BR — CD *		srd2 [,as needed] [BL and BC data or GR data, and N, ND, and SA data, as appropriate] brtype, brwdth [,as needed] [AB, PD, and KQ data as appropriate]
XR *	ROAD	srdrd [,as needed] [GR data]
XS	APPR	srd4 [,as needed] [GR, N, ND, and SA data, as appropriate]

Figure 3-13. Input data sequence: single-opening bridge with road-grade (no definition sketch).

#### 3.2.5 Alternative Design Data

Figure 3-12. Input data sequence: single-opening bridge with guidebanks as shown in Figure 3-11.

Figure 3-14 illustrates a data sequence for obtaining computations for alternative designs. Profiles are required for every combination of three alternative bridge lengths (brlen1, brlen2, and brlen3) and two road-grade alternatives (a "high" embankment and a "low" embankment) for a range of five discharges (q1, q2, q3, q4, and q5). When varying both road-grade and bridge data, it will normally be prudent to hold the road-grade data constant for all bridge alternatives because usually it takes less coding for bridge modifications; to modify a road grade all data must be recoded. The first EX record (after the approach section data) will initiate profile computations for each of the five discharges for the brlen1 and "high" embankment combination. The next two four-record sequences (T3, BR, BL, and EX records) will result in two additional sets of five profiles for combinations of brlen2 and brlen3 with the "high" embankment. The next part of the sequence (T3, XR, GR, and EX records) will result in (1) the "high" embankment data being replaced by the "low" embankment data and (2) computation of five profiles for the brlen3 and "low" embankment combination. The next two four-record sequences (T3, BR, BL, and EX records) will result in the "high" embankment data being replaced by the "low" embankment data and (2) computation of five profiles for the brlen3 and "low" embankment combination. The next two four-record sequences (T3, BR, BL, and EX records) will result in the "low" embankment combination.

	80 character record				
1-2	4 6-10	11-80			
ID#	SECID	User's Format			
T1 T2 T3 *	HYPOT	ANALYSIS FOR HWY-77 BRIDGE HETICAL CREEK NEAR NOWHERE, USA H = brlenl / "high" EMBANKMENT			
WS Q *		ws2, ws3, ws4, ws5 q2, q3, q4, q5			
XS *	EXIT	srd1[,as needed] [exit section data]			
XS *	FULLV	srd2 [,as needed] [full-valley section data]			
BR BL 	BRDGE	srd2 [,as needed] brlen1, xconlt, xconrt [remaining bridge data]			
XR *	ROAD	srdrd [,as needed] [road-grade data, "high" embankment]			
XS EX *	APPR	<pre>srd3 [,as needed] [approach section data]</pre>			
T3 BR BL	LENGT BRDGE	H = brlen2/"high" EMBANKMENT brlen2			

```
ЕΧ
*
т3
    LENGTH = brlen3/"high" EMBANKMENT
BR
    BRDGE
           brlen3
BL
ΕX
Т3
    LENGTH = brlen3/"low" EMBANKMENT
XR
     ROAD
           [road-grade data, "low" embankment]
ΕX
*
Т3
    LENGTH = brlen2/"low" EMBANKMENT
BR
    BRDGE
ΒL
    brlen2
ΕX
Т3
    LENGTH = brlen1/"low" EMBANKMENT
BR
    BRDGE
ΒL
           brlen1
ΕX
ER
```

Figure 3-14. Input data sequence: alternative designs (no definition sketch).

To revise or replace data the SECID in subsequent header records must be identical to the original SECID for that cross-section, including the position of any blank characters. In this example the T3 record was revised for each alternative to provide unique output identification. Any or all of the title information records may be modified for this purpose. This example generates a total of thirty profiles, thus, users may wish to be prudent in the number of alternatives input and (or) the amount of output requested for each alternative to avoid an "overwhelming" volume of results.

### 3.3 MULTIPLE-OPENING BRIDGE SITUATION

Figure 3-15 is a definition sketch of a long, flood-plain crossing.

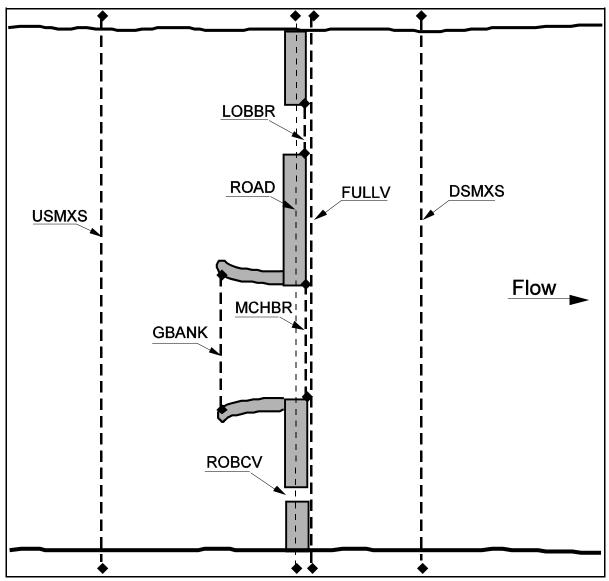


Figure 3-15. Definition sketch: multiple-opening stream crossing.

The crossing consists of three openings as follows: (1) a primary bridge spanning the main channel; (2) a relief bridge on the left overbank spanning a secondary channel; and (3) a culvert on the right overbank, primarily to provide low-flow passage for a wide, shallow drainage ditch, but large enough to pass a significant portion of the total flow at higher discharges. The primary bridge also has elliptical guidebanks. Figure 3-16 illustrates the data sequence required

to define this situation from the downstream match section through the upstream match section, inclusive. The sections are defined in the following order: (1) downstream match section (analogous to the exit section of single-opening bridges); (2) full-valley section; (3) bridge section for the relief bridge on left overbank; (4) bridge section for the primary bridge; (5) guidebank section; (6) culvert section for the culvert on right overbank; (7) road-grade section; and (8) upstream match section (analogous to the approach section of single-opening bridges and requires an XS header record).

	80 character record				
1-2 4 6-10 11-80					
ID#	SECID	User's Format			
XS 	DSMXS [	[downstream match section data]			
XS *	FULLV	[full-valley section data]			
BR 	LOBBR	[bridge section data]			
BR 	MCHBR	[bridge section data]			
GB 	GDBNK	[guidebank data]			
CV	ROBCV	[culvert data]			
XR *	ROAD	[road-grade data]			
XS	USMXS	[upstream match section data]			

Figure 3-16. Input data sequence: multiple-opening stream crossing of Figure 3-15.

## 3.4 CULVERT HYDRAULICS

Figure 3-17 shows a typical series of input data for a reach of a stream where culvert hydraulics are to be computed. A continuous water surface profile can be generated by placing a CV header record at the location shown in Figure 3-17. The culvert header record (CV record) is placed at the same section reference distance as the full valley cross-section. The xctr variable on the CV record locates the culvert laterally in the flood plain. The culvert geometry is entered on a CG record and if desired default culvert coefficients can be overridden through the use of a CC record.

80 character record				
1-2 4 6-10	11-80			
ID# SECID	User's Format			

XS	EXIT	<pre>srd1 [,as needed]</pre>
*		[ GR, N, ND, and SA data, as appropriate ]
XS	FULLV	srd2 [,as needed]
		[ GR, N, ND, and SA data, as appropriate ]
*		
CV	CULV	<pre>srd2, xctr, cvleng, dsinv, usinv, [, as needed]</pre>
CG		icode, rise, [ as needed ]
CC		[ optional as needed ]
*		
XS	APPR	<pre>srd3 [,as needed]</pre>
		[ GR, N, ND, and SA data, as appropriate ]

Figure 3-17. Input data sequence: continuous profile culvert computations .

### 3.5 SCOUR COMPUTATIONS

Figure 3-18 shows a conceptual data file that uses the DC, DP, and DA records to compute local scour at a bridge opening. The records are located after the last cross-section has been defined and before the EX record. The DC record is used to compute live-bed and clear-water contraction scour. The DP record is used to compute local scour at piers and the DA record is used to compute local scour at abutments. Multiple DC, DP, and DA records can be used to analyze different scour conditions or to examine scour sensitivity.

------80 character record ------1-2 4 6-10 11 - 80ID# SECID User's Format XS EXIT srd1 [,as needed] [ GR, N, ND, and SA data, as appropriate ] \* FULLV srd2 [,as needed] XS [ GR, N, ND, and SA data, as appropriate ] \* BR BRDGE srd2 [,as needed] [ N, ND, and SA data, if appropriate ] ΒL brlen, xconlt, xconrt BC girdep, bdelev [,as needed] brtype, brwdth [,as needed] CD AB \_ \_ \_ \_ \_ \_ \_ PD optional records ΚQ . . . . . . . . . .

XS APPR srd3 [,as needed] [ GR, N, ND, and SA data, as appropriate ] \* DC BRDGE [ as needed] [ additional DC records as desired ] \* DP BRDGE [ as needed] [ additional DP records as desired ] \* BRDGE [ as needed] DA [ additional DA records as desired ] \* ЕΧ

ER

Figure 3-18. Input data sequence: single-opening bridge of Figure 3-8, scour analysis.

#### 3.6 FLOODWAY RECORDS

Floodway computations can be accomplished through the use of FS and/or FW records. The example shown in Figure 3-19 uses FW records to specify the desired surcharge on a cross-section by cross-section basis. The encroachment method, the desired surcharge, and encroachment constraints can be specified on the FW record for each of the cross-sections where and encroachment is to be analyzed.

	80 character record				
1-2 4 6-10	11-80				
ID# SECID	User's Format				
 FW *	Title Information discharge information starting water surface elevation information srd1 [,as needed] [ GR, N, ND, and SA data, as appropriate ] [ surcharge ] srd2 [,as needed] [ GR, N, ND, and SA data, as appropriate ] [ surcharge ]				
* XS XS#3 FW EX ER	srd3 [,as needed] [ GR, N, ND, and SA data, as appropriate ] [ surcharge ]				

Figure 3-19. Input data sequence: floodway computations.

## **SECTION 4 - DETAILED INPUT DATA CODING INSTRUCTIONS**

## 4.1 GENERAL

WSPRO input data preparation has been greatly simplified by minimizing the use of fixed-field formats. The title information records (T1, T2, T3) are the only data records read with an 80-column fixed-field format. Only three data items in the remaining input data are constrained by fixed-field formats. They are a record identifier in columns 1 and 2, an option code in column 4, and a cross-section identification code in columns 6 through 10. Therefore, columns 1 through 10 of each record are reserved in fixed-field format to specify these three data items in accordance with the specifications shown in Table 4-1. When the option code and (or) the section identification code are not required in a data record, columns 3 through 5 and (or) 6 through 10 of that data record should be blank. Columns 11 through 80 are available to use as a free-format area for coding the remaining data.

Card Column(s)	Input Format	Data	
1-2	A2		Alphanumeric record identifier
3	1x		Blank space
4	I1		Integer option code
5	1x		Blank space
6-10	A5		Alphanumeric cross-section identification code, SECID

Table 4-1.Fixed-field input format for columns 1 through 10 of all data records.

The record identifier, consisting of one or two characters, was introduced in the previous two sections. Table 4-2 summarizes, in alphabetical order, all of the valid record identifiers and the purpose of each data record. The model relies on the record identifier to determine the type of incoming data. Obviously, incorrect coding of a record identifier will result in some degree of

job failure. The option code is a single-digit entry which is used in a few of the data records to select certain input/output options or optional computational techniques. The section identification, SECID, may consist of from one to five alphanumeric characters. The model uses SECID to identify cross-sections, both internally and in model output. Because of the alternative design features of the model, any data associated with a repeated SECID will replace data that were previously input with that SECID. Therefore, each cross-section must be identified with a unique SECID.

Primary objectives in adopting free-format input were to: (1) provide a "user-friendly" input scheme and; (2) eliminate many common coding errors that frequently occur when restricted by rigidly formatted input schemes. Free-format coding can be used to input a continuous string of numbers with appropriate data-field delimiters. Such input is usually objectionable to most users, however, because it is very difficult to visually interpret and (or) check such data strings. Most users will find it convenient to space the entries within the free-format area to enhance visual scanning of the data. Enough flexibility exists for users to tailor input patterns to suit the type and quantity of data and the users' individual tastes.

Free-format input eliminates fixed column requirements, yet there are certain coding conventions that must be followed. Also, unlike the flexibility in the ordering of data records, the individual data items in each data record must be coded in the specified order. The following conventions apply to coding data in the free-format area.

- (1) Individual data items must be separated by either: a comma; one or more blanks; or any combination of a comma and one or more blanks.
- (2) When a data item is not required (or it is to be assigned a default value) a null value may have to be specified, depending on the location of the data item within the parameter list. A null value must be specified for an omitted data item in the middle of a list. A null value in the middle of a list may be indicated by either two successive commas (with or without intervening blanks), or an asterisk (\*). A null value must be indicated (with either a single comma or an asterisk) when omitting the first item in a list. Missing items at the end of a list are automatically assigned default values and need not be indicated at all, although the appropriate number of either successive commas or asterisks is generally allowed. The exception to this rule is in the Q, WS, and SK records where the last entry **must not** be a null value. When commas are used to indicate null values, they may be preceded and (or) followed by one or more blanks. When asterisks are used, they may be separated from adjacent data items (default or specified) as per rule number 1, above.

(3) Data values may be coded to whatever precision is appropriate for the type of data and the situation. Any number with a decimal fraction must be coded with a decimal point. Any value coded without a decimal point is input as a whole number.

A few examples should clarify application of rules 1 and 2, above. Assume a data list contains four variables (var1, var2, var3, and var4). Equivalent coding sequences (not all inclusive) for several situations follow.

Example 1. All four variables are required by the model. (a) var1, var2, var3, var4

(b) var1 var2 var3 var4 (c) var1 ,var2 , var3 var4

Example 2. Default values are sufficient for the second and third variables.

- (a) var1,,,var4
- (b) var1 \* \* var4
- (c) var1,\*,\* var4

Example 3. Only var1 is required.

- (a) var1
- (b) var1 \* \* \*

Example 4. Only var2 and var3 are required.

(a) , var2 var3 (b) \* var2 var3 \* (c) \*,var2,var3

The input examples in later sections should provide additional understanding of the free-format coding conventions.

An alternative method of providing input has been provided. By placing a \*F as the first record in an input data string the entire data file format can be considered free format. However, the user needs to be aware that of the fact that if the \*F coding convention is followed that all input in columns 1-10 must be provided.

Record identifier Purpose of data record			
AB	Specifying (1) abutment slopes for type 3 openings <b>in component mode or</b> (2) abutment toe elevations for type 2 openings <b>in coordinate mode</b> .		
BC	Specifying bridge low-chord parameters (component mode only).		
BL	Specifying bridge length and abutment location constraints (component mode only).		
BP	Specifying relation of bridge-opening horizontal datum to the horizontal datum of road and (or) approach sections.		
BR	Header record for bridge-opening cross-section.		
CC	Specifying culvert coefficients.		
CD	Specifying parameters used for computing the flow length and the coefficient of discharge for a bridge opening.		
CG	Specifying culvert section geometry.		
CV	Header record for culvert cross-section.		
DA	Specifying parameters used for computing local scour at abutments.		
DC	Specifying parameters used for computing contraction scour.		
DP	Specifying parameters used for computing local pier scour.		
EF	Specifying effective flow input.		
ER	Specifying end of run.		
EX	Instructing model to begin execution of profile computations and specifying computation directions.		
FL	Specifying friction slope averaging technique and (or) variable flow length(s) between sections.		
FS	Specifying parameters for global floodway analysis.		
FW	Specifying parameters for floodway analysis.		
GB	Header record for guidebank cross-section.		
GR	Specifying x,y-coordinates to define cross-section geometry.		

## Table 4-2.Record identifiers and purpose of data record.

Table 4-2.	Record identifiers	and purpose c	of data record	(continued).
1 4010 1 21		and purpose o	n aada reeora	(commaca).

Record identi	fier Purpose of data record			
GT	Replaces GR data for sections being fabricated from template section.			
HP	Requesting tables of cross-sectional properties and (or) velocity and conveyance distribution.			
J1	Specifying parameters for computational control.			
KQ	Specifying user-selected breakpoints to override the default location of the Kq segment of the approach cross-section.			
Ν	Specifying roughness (Manning's "n") coefficients.			
ND	Specifying hydraulic-depth breakpoints for vertical variation of roughness.			
PD	Specifying pier or pile data.			
Q	Specifying discharge for each profile to be computed.			
SA	Specifying x-coordinates of breakpoints for subdivision of cross-sections for roughness and (or) geometry variation.			
SI	Specifying metric or English input/output.			
SK	Specifying energy gradient(s) for computing starting water-surface elevation(s) by slope conveyance.			
T1,T2,T3	Specifying title information for identification of model output.			
UT	Specifying user defined special tabling parameters.			
WS	Specifying starting water-surface elevation(s) for profile computations.			
XR	Header record for road-grade cross-section.			
XS	Header record for unconstricted valley and approach cross-sections.			
XT	Header record for template cross-section.			
*	Inserting comments and (or) blank lines into the input data sequence.			
*F	Specifying free-format input mode.			

## 4.2 INDIVIDUAL DATA RECORDS

The following tables present coding details for each of the individual data records, including (1) the variables to be coded, (2) definition of the variables, and (3) default values (where applicable). Variables listed outside of brackets are mandatory. Variables listed between brackets are optional but some or all of those optional variables may be required under certain circumstances. The tables are arranged in alphabetized order of the record identifiers.

Table 4-3.Description of format and contents of AB record.

## AB AB Record AB

Purpose:Mandatory record to specify abutment slope(s) for type 3 openings in component<br/>mode (not used for other opening types in component mode). Mandatory record to<br/>specify abutment toe elevations for type 2 openings in coordinate mode (not used for<br/>other opening types in coordinate mode mode).

Format:			
Columns:	Format:	Contents:	
1-2	A2	AB	
3	1X	Blank	
4	1X	Blank	
5	1X	Blank	
6-10	A5	Blank	
11-80	Free	ABSLPL, [ABSLPR] or	
		YABLT, YABRT	

#### **Definition of variables:**

ABSLPL, ABSLPR Slope (horizontal distance per meter change in elevation) of the left and right abutments, respectively. Required only for type 3 openings when using COMPONENT MODE for defining a bridge section. If both abutments have the same slope, only ABSLPL is required. See Figure 4-9 for illustration of these parameters.

YABLT, YABRT
 Ground elevation at the toe of the left and right abutments, respectively.
 Required only for type 2 openings when using COORDINATE mode (in component mode the model determines these elevations). The depths of water at the toes of the abutments are used to compute an adjustment factor for the coefficient of discharge. See Figure 4-8 for illustration of these parameters.

Note:

1) See **Table 4-11** for an explanation of bridge opening types.

Table 4-4.Description of format and contents of AS record.

AS AS Record	AS
--------------	----

**Purpose:** Optional header record for approach cross-section.

degrees.

Format:		
Columns:	Format:	Contents:
1-2	A2	AS
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, [SKEW,EK,CK,VSLOPE]

#### **Definition of variables:**

SECID Unique cross-section identification code (see Section 5.1 for additional discussion of SECID). SRD Section reference distance. A cumulative distance, in meters, along the stream measured from any arbitrary zero reference point (SRD may be negative). Unless an FL record is coded for the approach section, the difference in the fullvalley section **SRD** and the approach section **SRD** is used as the flow length to compute friction losses in the unconstricted profile computations. The effective flow length (Schneider, et al., 1977) is used for friction-loss computations for bridge backwater regardless of SRD or FL data. The SRD value for the approach section should be approximately equal to the SRD value of the fullvalley section plus the bridge width plus the bridge length (plus the length of guidebanks if they exist). SKEW The acute angle that the section must be rotated to orient the section normal to the flow direction. The model applies the cosine of **SKEW** to the horizontal dimension of the section to compute cross-sectional properties. Default is zero

EK, CK	Coefficients used to compute expansion and contraction losses, respectively, for the energy equation balance. These coefficients do not apply to bridge-backwater computations, but are used for computing the unconstricted profile in the full-valley to approach section subreach in upstream computations and are used in the approach to bridge section subreach in downstream computations. Default values are either $\mathbf{EK} = 0.5$ and $\mathbf{CK} = 0.0$ or the current values being propagated from downstream data.
VSLOPE	Valley slope, in meters per meter. Used to adjust elevations of propagated geometry data. Also used to adjust elevations of approach section data used to fabricate intermediate approach cross-sections in a multiple-opening situation. Default value is either zero or the current value being propagated from downstream data.

#### Note:

1) The AS Header Record has been replaced by a regular full valley cross-section (as defined by an XS Header Record) in the current version of WSPRO. For purposes of bridge hydraulic computations, the program assumes that the first cross-section upstream of the bridge is the approach cross-section. WSPRO still recognizes an AS Header Record but will prompt the user with a message in the input data summary stating that the AS Header Record has been replaced with a XS Header Record.

Table 4-5.Description of format and contents of BC record.

BC	BC Record	BC

Purpose: Specifying bridge deck parameters. Mandatory for component mode, not used for coordinate mode.

Format:		
Columns:	Format:	Contents:
1-2	A2	BC
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	BCELEV, [BCSLP, BCSTA]

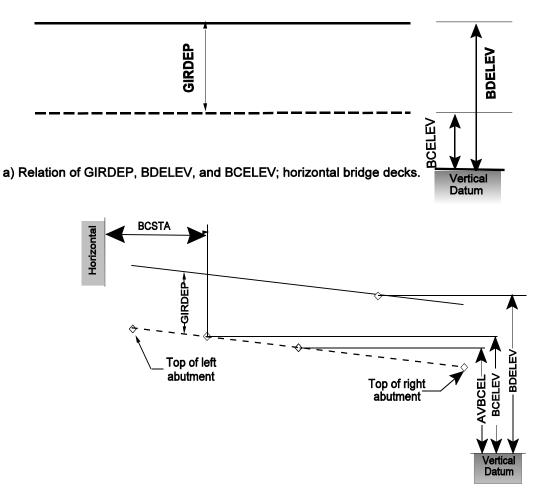
#### **Definition of variables:**

BCELEV	Low bridge-chord elevation. If bridge deck is not horizontal, <b>BCSLP</b> and <b>BCSTA</b> also are required.
BCSLP	Bridge deck slope, in meters per meter; left to right fall is negative.
BCSTA	The x-coordinate corresponding to the y-coordinate of <b>BCELEV</b> .

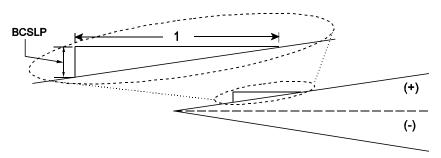
The above data provide the information necessary to connect the tops of the abutments to the low chord. Figure 4-1 illustrates the **BC** record parameters.

#### Note:

1) The BC Record replaces the BD Record used in earlier versions of WSPRO. The program still recognizes a BD Record but will prompt the user with a message in the input data summary stating that the BD Record has been replaced with the BC Record.



b) Relation of GIRDEP, BDELEV, BCSTA, AVBCEL, and BCELEV; sloping bridge decks.



c) Definition of and sign convention for BCSLP

Figure 4-1. Definition sketch: BC parameters.

Table 4-6.Description of format and contents of BL record.

BL	BL Record	BL

**Purpose:** Specifying bridge length and abutment location constraints (COMPONENT MODE ONLY).

Format:		
Columns:	Format:	Contents:
1-2	A2	BL
3	1X	Blank
4	1X	LOCOPT
5	1X	Blank
6-10	A5	Blank
11-80	Free	BRLEN, XCONLT, XCONRT

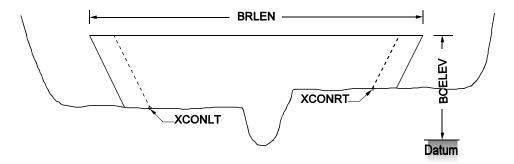
#### **Definition of variables:**

LOCOPT Bridge-location option to specify location of the specified bridge length (BRLEN) with respect to the specified horizontal stationing (XCONLT, XCONRT). Three choices are available as follows:

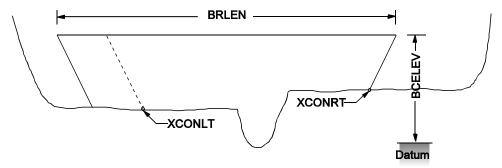
- BRLEN is centered at the midpoint of the minimum bridge length (i.e., the midpoint between the tops of the abutments with the abutment toes located at the constraining points, XCONLT and XCONRT).
   LOCOPT = 0 is the default option.
- 1 The toe of the right abutment is placed at the location specified by **XCONRT**.
- 2 The toe of the left abutment is placed at the location specified by **XCONLT**.

Figure 4-2 illustrates the effect of **LOCOPT** on the location of the bridge opening in COMPONENT MODE. Figure 4-3 illustrates the determination of the minimum bridge length in COMPONENT MODE's "bridge-building" process.

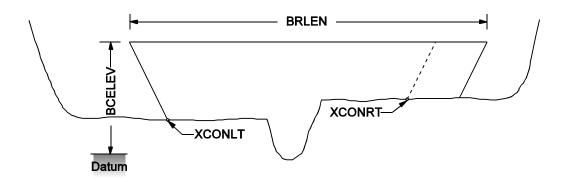
BRLEN	The length of the bridge (between the tops of the abutments) in meters. Must be specified for the first alternative of a series, but is optional on subsequent alternative designs.	
XCONLT, XCONRT	Horizontal stationing controlling the location of the bridge opening. These values serve as constraints on the abutment locations and bridge length. <b>The model will not locate the toe of either abutment between XCONLT and XCONRT</b> . Any specified <b>BRLEN</b> which would result in such a situation is not analyzed. It is mandatory to specify these variables for the first alternative of a series, but optional on subsequent alternative designs.	



a) LOCOPT = 0, bridge opening centered at midpoint of tops of abutments



b) LOCOPT= 1, right abutment toe fixed at XCONRT



c) LOCOPT = 2, left abutment toe fixed at XCONLT

Figure 4-2. Definition sketch: effect of LOCOPT on bridge-opening location.

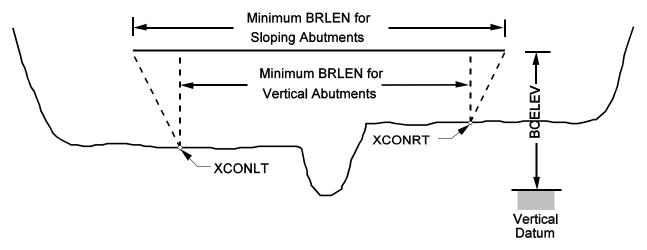


Figure 4-3. Definition sketch: BL record parameters.

Table 4-7.Description of format and contents of BP record.

BP	<b>BP</b> Record	BP

**Purpose:** Specifying relation of the horizontal datum of the bridge opening to the horizontal datum of road-grade and (or) approach section(s) (**only applicable to single-opening situations**).

Format:		
Columns:	Format:	Contents:
1-2	A2	BP
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	XREFLT, [XREFRT, FDSTLT, FDSTRT]

#### **Definition of variables:**

XREFLT

x-coordinate on the road-grade or the approach section that coincides with the projection from a reference point in the bridge section. The left reference point is either (1) **XCONLT** (in **BL** record) for COMPONENT MODE or (2) the minimum x-coordinate of the bridge section for COORDINATE MODE. The projection is made vertically to the road-grade section and along a line parallel to the flow to the approach section. **XREFLT** is the only parameter needed in the **BP** record to provide a common horizontal datum for bridge and road-grade sections (see Figure 4-4). **XREFLT** is also the only parameter needed in the **BP** record to provide a common horizontal datum for bridge and approach sections if their horizontal stationing is not aligned properly and (or) if either (or both) of those sections are skewed to the direction of flow (see Figure 4-5)

Curvilinear flow requires three additional parameters. A typical situation is illustrated in Figure 4-6. The bridge and approach sections must be aligned normal to the flow. The variable flow length due to nonparallel sections is described by the additional parameters.

XREFRT	x-coordinate of the approach section which coincides with the projection (parallel to the flow) of a right-hand reference point in the bridge section, which is either (1) <b>XCONRT</b> (in <b>BL</b> record) for COMPONENT MODE or (2) maximum x-coordinate of the bridge section for COORDINATE MODE.
FDSTLT, FDSTRT	The flow distances measured along the left and right projection lines, respectively.
A <b>BP</b> record for datum	n correction between the road grade and the bridge is <b>included with the road</b> -

**grade data**. A **BP** record for datum correction between the road grade and the bridge is **included with the road**grade data. A **BP** record for datum correction (or to account for curvilinear flow) between the bridge and the approach section is **included with the approach-section data**.

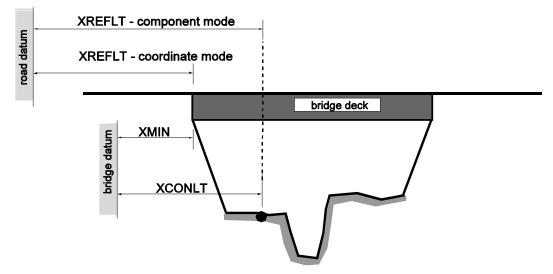


Figure 4-4. Definition sketch: application of BP record for datum correction between bridge-opening and road-grade sections.

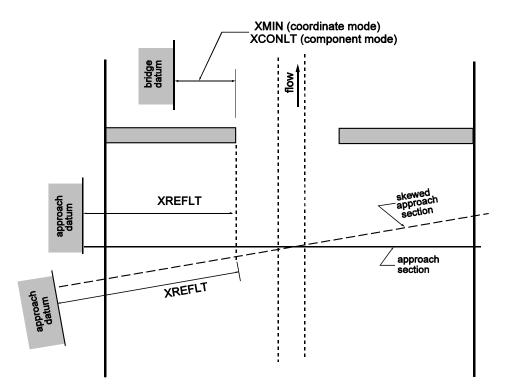


Figure 4-5. Definition sketch: application of BP record for datum correction between bridge-opening and approach sections.

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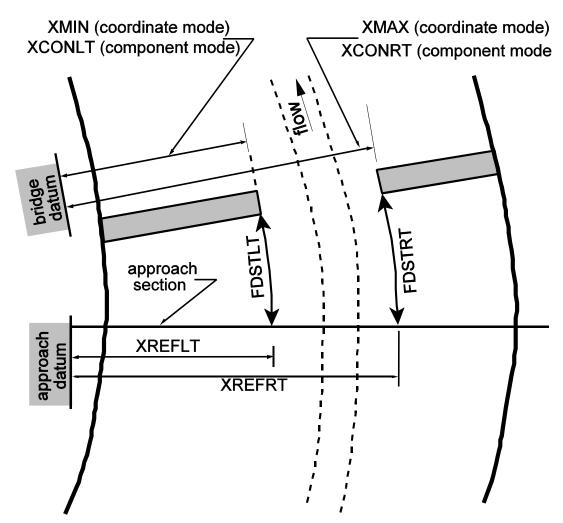


Figure 4-6. Definition sketch: application of BP record for curvilinear flow between approach and bridge.

Table 4-8.Description of format and contents of BR record.

BR	<b>BR Record</b>	BR
----	------------------	----

**Purpose:** Header record for bridge cross-section.

Format:		
Columns:	Format:	Contents:
1-2	A2	BR
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, [PFELEV,SKEW,EK,CK,USERCD]

#### **Definition of variables:**

SECID	Unique cross-section identification code.
SRD	Section reference distance; must be assigned the same value as the full-valley section.
PFELEV	Elevation of the low chord of the bridge opening. Should be coded for <b>COORDINATE</b> mode if there is any chance for pressure flow. <b>WSPRO</b> <b>cannot check for (nor compute) pressure flow without this elevation.</b> The model computes a low chord elevation in <b>COMPONENT MODE</b> . An average elevation is computed for sloping decks. Users may <b>override</b> the <b>COMPONENT MODE</b> computed value by coding <b>PFELEV</b> in the <b>BR</b> header record. See Section 4.3.7 and Figure 4-1 for additional information on <b>PFELEV</b> .
SKEW	The acute angle that the section must be rotated to orient the section normal to the flow direction. The model applies the cosine of <b>SKEW</b> to the horizontal dimension of the section to compute cross-sectional properties. Default is zero degrees. See Section 4.3.6 for additional information.

EK, CK	Coefficients used to compute expansion and contraction losses, respectively, for the energy equation balance. These coefficients are not used in bridge-backwater computations. They are used for supercritical flow computations from the bridge section to the exit section. Default values are either $\mathbf{EK} = 0.5$ and $\mathbf{CK} = 0.0$ or the current values being propagated from downstream data.
USERCD	User-specified coefficient of discharge for a bridge. WSPRO uses this override value instead of the computed value.

Table 4-9.Description of format and contents of CC record.

CC	CC Record	CC

**Purpose**: Specifying culvert coefficients. This record is not required if all of the default values are considered acceptable.

Format:		
Columns:	Format:	Contents:
1-2	A2	CC
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	IEQNO, [CKE, CVALPH, CN]

#### **Definition of variables:**

IEQNO	The inlet equation number.
CKE	The culvert entrance-loss coefficient, k <sub>e</sub> .
CVALPH	The velocity head correction coefficient, a, for the culvert.
CN	Manning's roughness coefficient, n, for the culvert.
	Default values for <b>CKE</b> , <b>CVALPH</b> , and <b>CN</b> are based on the value <b>ICODE</b> in the <b>CG</b> record. Default values for <b>CKE</b> are tabulated in the last column of Table 4-13. Default values for <b>CVALPH</b> and <b>CN</b> are tabulated in Table 4-10.

	n		а	
Shape		Box	Circle	Arch
Material	0.010	1.00		
Concrete	0.012	1.00	1.04	1.05
Corrugated metal	0.035		1.12	1.16

Table 4-10.Default values for roughness coefficient, n, and velocity head correction coefficient, a,<br/>for culverts.

Table 4-11.Description of format and contents of CD record.

# CD CD Record CD

**Purpose:** Mandatory record for ALL bridge openings to specify parameters used for computing the flow length and the coefficient of discharge for a bridge.

Format:		
Columns:	Format:	Contents:
1-2	A2	CD
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	BRTYPE, BRWDTH, EMBSS, EMBELV, WWANGL, WWWID, ENTRND

## **Definition of variables**:

BRTYPE	Indicates the type of bridge opening, as follows:		
	1 Vertical embankments AND vertical abutments, with or without wingwalls (Figure 4-7).		
	2 Sloping embankments AND vertical abutments without wingwalls (Figure 4-8).		
	3 Sloping embankments AND sloping spillthrough abutments (Figure 4-9).		
	4 Sloping embankments AND vertical abutments with wingwalls (Figure 4-10).		
BRWDTH	Total width (in direction of flow) of the bridge deck. For type 1 openings <b>BRWDTH</b> should include the length of the upstream wing-walls. For the other types <b>BRWDTH</b> should reflect only the deck dimension. The model computes the x-components using the values coded for <b>EMBSS</b> and <b>EMBELV</b> .		
EMBSS	Embankment side slope, expressed in the horizontal change in meters per meter change of elevation (e.g., 3 to 1 is expressed as 3.0 OR 2 <sup>1</sup> / <sub>2</sub> to 1 is expressed as 2.5). Default value is zero. <b>Must be specified for BRTYPE 2, 3, and 4</b> .		

EMBELV	Embankment elevation <b>must be provided for BRTYPE 2, 3, and 4</b> . A representative elevation in the vicinity of the bridge opening should be used when the top of the embankment is not horizontal. <b>EMBELV</b> and <b>EMBSS</b> are used to compute x-component(s) of flow length through bridge (see Figure 4-11).
WWANGL	Wingwall angle, in degrees. Required <b>only</b> for <b>type 1</b> openings with wingwalls (Figure 4-7) and <b>type 4</b> openings (Figure 4-10). Default is zero degrees.
WWWID	Wingwall width, in meters. Required <b>only</b> for <b>type 1</b> openings with wingwalls (see Figure 4-7). Default is zero.
ENTRND	Radius of entrance rounding, in meters. Required <b>only</b> for <b>type 1</b> openings with rounded entrance corners. Default is zero.

Table 4-11.Description of format and contents of CD record (continued).

## CD CD Record CD

Notes:

- 1) **BRTYPE** and **BRWDTH must be coded** for **all** opening types.
- 2) For **BRTYPE 1**, additional parameters may be applicable as follows:
  - a) **WWANGL** and **WWWID** both parameters must be specified when wingwalls are present.
  - b) **ENTRND** wingwalls are not present and entrance corners are rounded.
  - c) No additional parameters when neither wingwalls nor entrance rounding exists.
- 3) For **BRTYPE 1**, **EMBSS** and **EMBELV** should be allowed to default.
- 4) **EMBSS** and **EMBELV must be coded** for **BRTYPE 2, 3, and 4**.
- 5) There are **NO** additional parameters for **BRTYPE 2 and 3**.
- 6) **WWANGL** should be coded for **BRTYPE 4**.
- 7) A complete explanation of the variables presented in Figures 4-8, 4-9, 4-10, and 4-11 can be found in the Bridge Waterways Analysis Model, Research Report (Shearman and others, 1986).

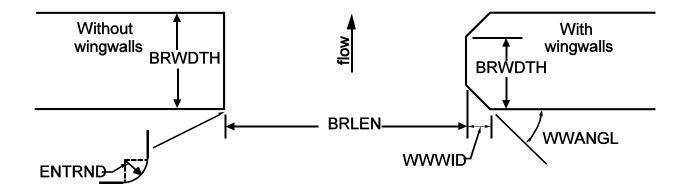
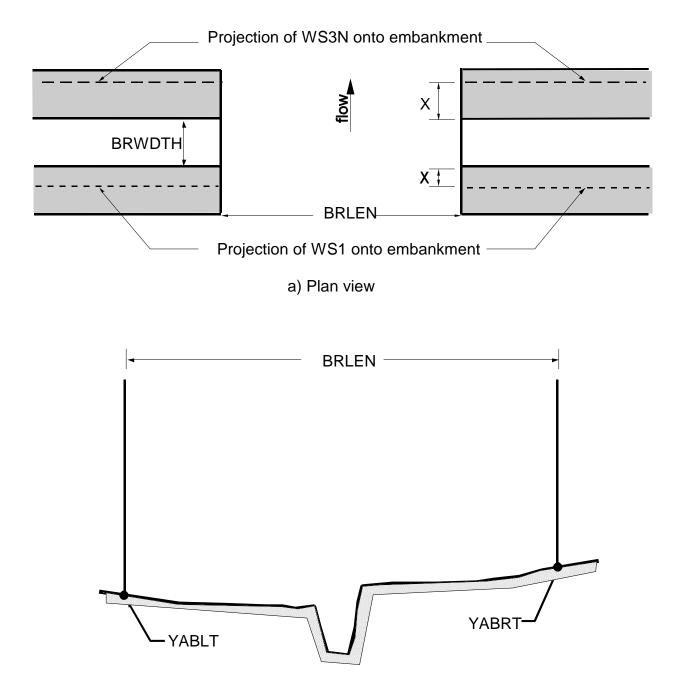
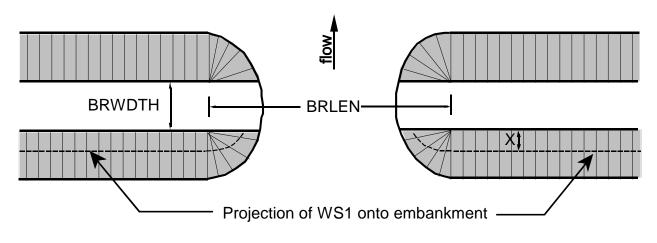


Figure 4-7. Definition sketch: type 1 bridge opening (BRTYPE = 1).

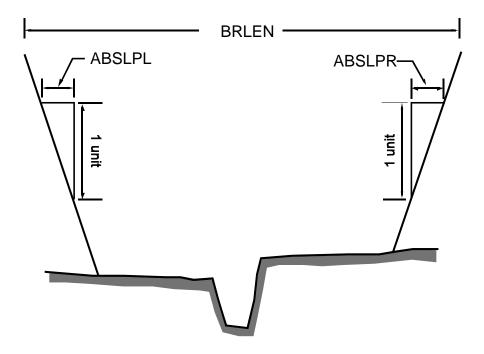


b) Elevation view, upstream side of bridge

Figure 4-8. Definition sketch: type 2 bridge opening (BRTYPE = 2).



a) Plan view



b) Elevation view, upstream side of bridge

Figure 4-9. Definition sketch: type 3 bridge opening (BRTYPE = 3).

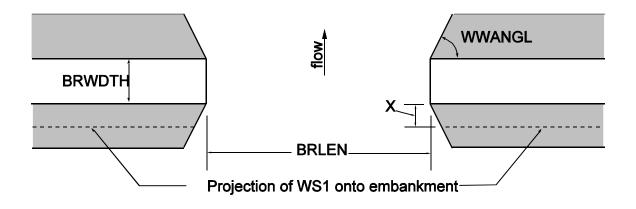


Figure 4-10. Definition sketch: type 4 bridge opening (BRTYPE = 4).

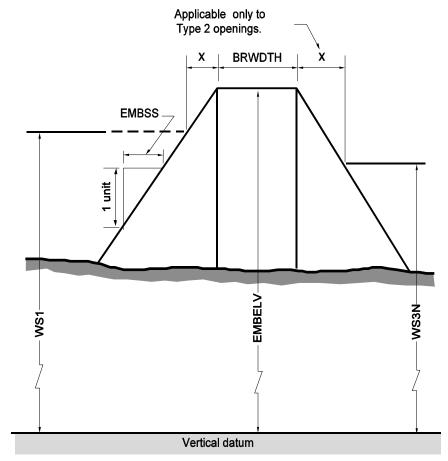


Figure 4-11. Definition sketch: embankment parameters (BRTYPE 2,3, and 4)

Table 4-12.Description of format and contents of CG record.

CG	CG Record	CG
----	-----------	----

**Purpose:** Specifying culvert section geometry.

Format:		
Columns:	Format:	Contents:
1-2	A2	CG
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	ICODE, RISE, [SPAN, BOTRAD, TOPRAD, CORRAD]

#### **Definition of variables**:

ICODE	Three digit culvert code (i.e., IJK) in which the individual digits indicate the following:		
	<ul> <li>I Shape code: 1 = BOX; 2 = CIRCULAR; and 3 = ARCH.</li> <li>J Material code: 1 = CONCRETE; 2 = CORRUGATED METAL PIPE (steel); and 3 = ALUMINUM.</li> <li>K Inlet code (type of inlet column in Table 4-13).</li> </ul>		
	Table 4-13 lists valid combinations of I, J, and K. <b>ICODE</b> also controls default values for <b>CKE</b> , <b>CVALPH</b> , and <b>CN</b> (see Tables 4-9, 4-10, and 4-13).		
RISE	The maximum vertical dimension, <b>in millimeters</b> , of the culvert barrel. The rise equals the diameter for circular culverts.		
SPAN	The maximum horizontal dimension of the culvert barrel, <b>in millimeters</b> . <b>SPAN must be</b> coded for box and pipe-arch culverts but <b>should not be</b> coded for circular culverts.		
BOTRAD	Bottom radii of pipe-arch culvert barrel, <b>in millimeters</b> . Appropriate values are tabulated in CDS-4 (FHWA, 1982). If not specified, approximate values of this,		

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and following two parameters will be computed on the basis of **ICODE**, **SPAN**, and **RISE** using the equations tabulated in Table 4-14.

TOPRAD	Top radii of pipe-arch culvert barrel, in millimeters
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```
CORRAD Corner radii of pipe-arch culvert barrel, in millimeters.
```

IJK	Type of inlet		Ke
BOX	Culverts		
1 1 1		30E - 75E flare; square top edge	0.4
1 1 2	HEADWALL:	(a) normal or (b) 45E skew; square edge	0.5
1 1 3	WINGWALLS:	15E flare with square edges	0.5
1 1 4	WINGWALLS:	extended with 0E flare; square top edge	0.7
1 1 5	HEADWALL:	(a) normal or (b) 45E skew; 1:1 bevels	0.2
1 1 6	WINGWALLS:	(a) 18E - 33.7E flare with 1.5:1 top bevel; or (b) 45E flare with 1:1 top bevel	0.2
117	HEADWALL:	normal with 1.5:1 bevel on three sides	0.2
	JLAR Culverts		
2 1 1	PROJECTING:	socket end	0.2
2 1 2	HEADWALL:	socket end	0.2
2 1 3	PROJECTING:		0.5
2 1 4	HEADWALL:	square edge	0.5
2 1 5	END SECTION		0.5
2 2 5		: corrugated metal	0.5
216	BEVEL: concrete		0.2
$\begin{array}{cccc} 2 & 2 & 6 \\ 2 & 1 & 7 \end{array}$		ted metal with 1:1 bevels e with 1.5:1 bevels	0.2 0.2
2 2 7		ted metal with 1.5:1 bevels	0.2
2 2 1		corrugated metal	0.2
	MITERED:	corrugated metal	0.7
2 2 3	HEADWALL:	corrugated metal	0.5
ARCH	[ Culverts		
3 1 1	HEADWALL:	square edge	0.5
3 1 2	HEADWALL:	grooved end	0.2
3 1 3	PROJECTING:	grooved end	0.2
3 2 1	PROJECTING:	corner radius of pipe-arch culvert < 457 mm (18.0 in)	0.9
3 2 2	PROJECTING:	corner radius of pipe-arch culvert = $787 \text{ mm}$ (31.0 in)	0.9
3 2 3	PROJECTING:	corner radius of pipe-arch culvert = 1194 mm (47.0 in)	0.9
3 2 4	MITERED:	corner radius of pipe-arch culvert $< 457 \text{ mm}$ (18.0 in)	0.7
3 3 1		corner radius of pipe-arch culvert = $808 \text{ mm}$ (31.8 in)	0.9
3 2 4	MITERED:	corner radius of pipe-arch culvert < 457 mm (18.0 in)	0.7
3 2 5	MITERED:	corner radius of pipe-arch culvert = $787 \text{ mm}$ (31.0 in)	0.7
326	MITERED:	corner radius of pipe-arch culvert = $1194 \text{ mm} (47.0 \text{ in})$	0.7
3 3 2	MITERED:	corner radius of pipe-arch culvert = $808 \text{ mm}$ (31.8 in)	0.7
327	HEADWALL: HEADWALL:	corner radius of pipe-arch culvert $< 457 \text{ mm}$ (18.0 in)	0.5
328 329	HEADWALL: HEADWALL:	corner radius of pipe-arch culvert = $787 \text{ mm}$ (31.0 in) corner radius of pipe-arch culvert = $1194 \text{ mm}$ (47.0 in)	0.5
3 3 3	HEADWALL: HEADWALL:	corner radius of pipe-arch culvert = $1194 \text{ mm} (47.0 \text{ m})$ corner radius of pipe-arch culvert = $808 \text{ mm} (31.8 \text{ in})$	0.5 0.5

Table 4-13.Coefficients used in the analysis of culverts.

 Table 4-14.
 Approximate formulas for pipe-arch auxiliary dimensions.

NOTE: All Dimensions in millimeters.

R = rise B = span = top radius r<sub>t</sub> = bottom radius r<sub>b</sub> = corner radius r<sub>c</sub> **REINFORCED CONCRETE PIPE ARCH**  $r_c = 15.19 + 0.243 \mathbf{R}$  $r_t = 30.73 + 0.499B$  $r_{\rm b} = -1527.3 + 2.106 \mathbf{R} + 0.583(|\mathbf{B} - 2413|)$ CORRUGATED METAL PIPE ARCH, Corner Radius of pipe-arch culvert < 457 mm (18.0 in)  $r_c = 29.0 + 0.205 R$ if **R** < 1400 mm if **R** > 1400 mm  $r_{c} = 457.0$  $r_t = 15.09 + 0.498\mathbf{B}$  $r_{\rm b} = 177.8 - 2.036 \mathbf{R} + 2.741 \mathbf{B}$ CORRUGATED METAL PIPE ARCH, Corner Radius of pipe-arch culvert = 787 mm (31.0 in)  $r_t = -8.79 + 0.505 \mathbf{B}$  $r_{\rm b} = -24300 + 29.39 \textbf{R} - 13.49 \textbf{B}$ 

CORRUGATED METAL PIPE ARCH, Corner Radius of pipe-arch culvert = 1194 mm (47.0 in)  $r_t = -83.06 + 0.521$ B  $r_b = -23579 + 18.44$ R - 7.81B

ALUMINUM PIPE ARCH, Corner Radius of pipe-arch culvert = 808 mm (31.8 in)  $r_t = -17.68 + 0.522B$  $r_b = 9220 - 9.64R + 6.38B$  Table 4-15.Description of format and contents of CV record.

**Purpose:** Header record for culvert cross-section.

Format:		
Columns:	Format:	Contents:
1-2	A2	CV
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, XCTR, CVLENG, DSINV, USINV, [NBBL]

#### **Definition of variables:**

SECID	Unique cross-section identification code.	
SRD	Section reference distance. The SRD for the culvert should reflect the location of the downstream end of the barrel and should be the same as the <b>SRD</b> of the full-valley cross-section when none of sections are skewed to the flow.	
XCTR	The horizontal stationing of the center of the culvert measured relative to an arbitrary origin on the left bank. This stationing must be consistent with the stationing defining a multiple-opening situation.	
CVLENG	Length of the culvert barrel(s), in meters.	
DSINV	Elevation of downstream invert, in meters above the common elevation datum.	
USINV	Elevation of upstream invert, in meters above the common elevation datum.	
NBBL	Number of culvert barrels (default is one barrel).	

For a stand-alone culvert analysis, **SRD** and **XCTR** have no useful purpose but should be assigned some arbitrary value to prevent input problems.

Table 4-16.Description of format and contents of DA record.

DA DA Record	DA
--------------	----

**Purpose:** Specifying parameters for the computation of abutment scour.

Format:		
Columns:	Format:	Contents:
1-2	A2	DA
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	[K1, K2, YAL, YAR, Q, FS]

#### **Definition of variables:**

SECID	Unique cross-section identification code. SECID is optional (depending on placement in the data stream as with the Q and HP Records.
K1, K2	K1 and K2 are the two correction/adjustment factors for abutment shape and embankment skew (default value is 1.0 for both factors).
YAL, YAR	User-specified overrides for the left and right abutment depth, $y_a$ , in meters. Use these parameters for cases when an average overbank depth may not be realistic for scour computations (e.g., if the abutment location is within the main channel, some weighted average value of $y_a$ may yield more meaningful estimates of scour depths).
Q	Discharge through the bridge opening (cubic meters per second).
FS	Factor of safety (default = $1.0$ ).

The standard/default mode of WSPRO computations will consist of:

 Compute the length of embankment projected normal to flow, a', for the left abutment as LEW(bridge) - LEW(approach) and for the right abutment as REW(approach) - REW(bridge).

- 2) Compute left/right abutment obstructed flow areas, A<sub>e</sub>, for left/right a' segments at the approach section.
- 3) Compute left/right  $y_a$  values by dividing left/right flow areas,  $A_e$ , by the right/left a' values.
- 4) Compute left/right abutment obstructed flow, Q<sub>e</sub>, based on ratio of the left/right a' segment conveyances to the total approach section conveyance.
- 5) Compute left/right abutment index velocities,  $V_e$ , by dividing left/right values of  $Q_e$  by left/right values of  $A_e$ .
- 6) Compute Froude number as a function of  $V_e$  and  $y_a$ .

### Notes:

- 1) A message will be generated when  $a'/y_a > 25$  so the user can decide if an alternate methodology should be attempted.
- 2) Definition and explanation of the variables used in the abutment scour equation can be found in Hydraulic Engineering Circular 18, Evaluating Scour At Bridges (Richardson and Davis, 1995).
- 3.) The current version of WSPRO does not support abutment scour computations in coordinate mode. This capability is to be added at a future date.

Table 4-17.Description of format and contents of DC record.

# DC DC Record DC

**Purpose:** Specifying parameters to perform live-bed and clear-water contraction scour depth computations.

Format:		
Columns:	Format:	Contents:
1-2	A2	DC
3	1X	Blank
4	1X	<b>0</b> , (live-bed contraction scour)
5	1X	Blank
6-10	A5	SECID
11-80	Free	[BXL, BXR, AXL, AXR, K1, PW, YB, YA]

#### ---- or ----

Format:		
Columns:	Format:	Contents:
1-2	A2	DC
3	1X	Blank
4	1X	1, (clear-water contraction scour)
5	1X	Blank
6-10	A5	SECID
11-80	Free	[BXL, BXR, AXL, AXR, D <sub>50</sub> , PW, YB, YA]

SECID	Unique cross-section identification code. SECID is optional (depending on placement in the data stream as with the Q and HP Records.	
BXL, BXR, AXL, AXR	BXL, BXR, AXL, AXR are the left and right horizontal limits of the bridge (BX section) and approach (AX section) channel segments, respectively. Defaults for these limits are the left and right edges of water for the bridge and approach sections. AX and BX will be limited, of course, to the difference between left-and right-edges of water.	
K1	The $k_1$ exponent in the contraction scour estimation equation (default 0.64).	
PW	Cumulative pier width. Without a value for this parameter, the value of $W_2$ used in either the clear-water or live-bed contraction scour equations will be reduced by gross pier width based on PD record input data. If a PD record is not used this value must be coded on the DC record.	
YB	Depth of flow in bridge opening.	
YA	Depth of flow in channel or floodplain.	
D <sub>50</sub>	Median diameter of bed material in bridge opening or in the floodplain, mm. This value is required when using the DC record to compute clear-water contraction scour.	
Notes:		

- Multiple DC records can be used to evaluate different combinations of left and right horizontal limits and/or different values of k<sub>1</sub>.
- 2) It is also possible to add variables YB (water-surface elevation in contracted section), QB (flow through bridge opening), WS1 (water-surface elevation at approach section), QT (total flow at the approach section) to permit contraction scour computations 'divorced' from profile computations.
- 3) Samples of how BXL, BXR, AXL, and AXR are located are demonstrated in Figures 7-26 and 7-27.
- Definition and explanation of the variables used in the contraction scour equations can be found in Hydraulic Engineering Circular 18, Evaluating Scour At Bridges (Richardson and Davis, 1995).

Table 4-18.Description of format and contents of DP record.

DP	DP Record	DP
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**Purpose:** Specifying parameters to perform local pier scour computations.

Format:		
Columns:	Format:	Contents:
1-2	A2	DP
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	[BXL, BXR, PW, YB, QB, K1, K2, K3, K4, V1M, Y1M]

SECID	Unique cross-section identification code.	
BXL	Left horizontal limit of channel segment to be examined for pier scour computations.	
BXR	Right horizontal limit of channel segment to be examined for pier scour computations.	
PW	Individual pier width.	
YB	User-specified override for the water-surface elevation in the bridge opening.	
QB	Discharge through the bridge opening.	
K1,K2, K3,K4	Correction/adjustment factors applied to the scour estimation equation (default values are 1.1, 1.0, 1.0, and 1.0 for K1, K2, K3, and K4 respectively).	
V1M	Velocity multiplier.	
Y1M	Depth multiplier.	

Notes:

- 1) The **V1M** and **Y1M** parameters are multipliers to increase or decrease the maximum velocity and depth values. These parameters could be used as safety factors and/or to perform fundamental sensitivity analyses to show variation in scour depth for different combinations of velocity and depth. Multiple DP records can be used to specify different **V1M** and **Y1M** values for sensitivity analyses.
- 2) Pier scour computations can be divorced from the profile computations by coding the YB and QB parameters. This feature is useful when it is deemed necessary to adjust computed elevations and/or in complex flow situations where water-surface elevations are not determined by WSPRO computations. Velocity and depth values are determined as per the above discussion. These computations could be accomplished by inputting only the bridge section.
- Definition and explanation of the variables used in the pier scour equation can be found in Hydraulic Engineering Circular 18, Evaluating Scour At Bridges (Richardson and Davis, 1995).

Table 4-19.Description of format and contents of EF record.

EF	EF Record	EF
----	-----------	----

**Purpose:** Allows user to set effective flow limits and user-defined stagnation points.

Format:		
Columns:	Format:	Contents:
1-2	A2	EF
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	XLEF, YLEF, XREF, YREF

eft station for effective flow calculations, in meters.
Depth at left station where flow limits no longer take effect.
Right station for effective flow calculations, in meters.
Depth at right station where flow limits no longer take effect.

Table 4-20.Description of format and contents of ER record.

**Purpose:** Specifying end of input data. (Begin computations.)

Format:		
Columns:	Format:	Contents:
1-2	A2	ER
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	No Parameters

A harmless message about input end-of-file occurs if this record is missing.

Table 4-21.Description of format and contents of EX record.

EX	EX Record	EX

Purpose:Instructing model to execute profile computations and specifying computation direction.No profiles are computed if an EX record is not coded.

Format:		
Columns:	Format:	Contents:
1-2	A2	EX
3	1X	Blank
4	1X	IEX
5	1X	Blank
6-10	A5	Blank
11-80	Free	IDIR(1), IDIR(2) , IDIR (NPROF)

The parenthetical notation indicates the order number, i, assigned by the model to each profile to be computed; i = 1 and i = NPROF for the first and last profiles to be computed with  $NPROF \le 20$ .

IEX	If all profiles are to be computed in the downstream direction, coding $IEX = 1$ is equivalent to coding $IDIR = 1$ for each profile
<b>IDIR</b> (i)	Computation-direction code for i <sup>th</sup> profile. <b>IDIR</b> = 0 for upstream (subcritical and/or critical) computations. <b>IDIR</b> = 1 for downstream (supercritical and/or critical) computations. No entries are required if all profiles are to be computed in the upstream direction. Any combination of upstream and downstream computations requires that <b>IDIR</b> be specified for each discharge in <b>Q</b> record. See Sections 4.3.2 and 4.3.3 and Figure 4-12 for additional information.

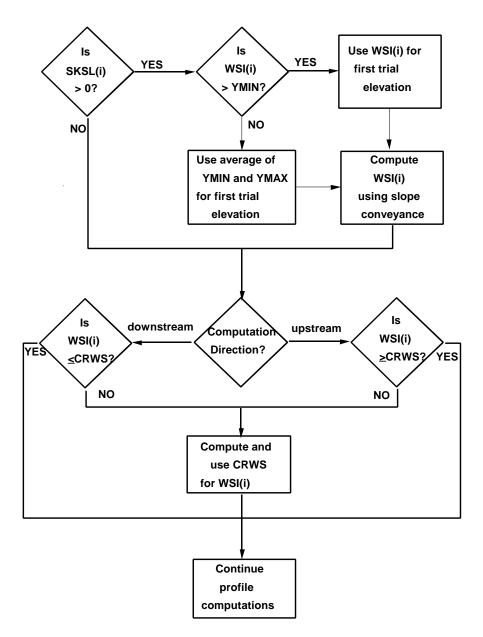


Figure 4-12. Flow chart: computational path for various combinations of WS, SK, and EX data.

Table 4-22.Description of format and contents of FL record.

FL	FL Record	FL

**Purpose:** Specifying friction slope averaging technique and (or) variable flow length(s) between sections.

Format:		
Columns:	Format:	Contents:
1-2	A2	FL
3	1X	Blank
4	1X	IHFNO
5	1X	Blank
6-10	A5	Blank
11-80	Free	[ FLEN(1) [, XFL (1), FLEN (2) ] [, XFL (2), FLEN (3)]

#### **Definition of variables:**

**IHFNO** Code to select the friction slope (or conveyance) averaging technique in the friction-loss computations. Valid entries are:

- 0 Uses geometric mean of conveyance.
- 1 Uses arithmetic average of conveyance.
- 2 Uses arithmetic average of friction slope.
- 3 Uses harmonic mean of friction slope.

**IHFNO** is propagated from section to section until a different value is introduced. **IHFNO** does not apply to bridge backwater computations (geometric mean of conveyance is always used). **IHFNO** is overridden if **IHFNOJ** is coded on the **J1** record.

FLENFlow length between the current cross-section and the adjacent downstream<br/>cross-section. Up to three values may be specified, and these lengths override<br/>SRD values except in bridge backwater computations. When more than one<br/>FLEN value is specified, each length applies to a segment of the cross-section<br/>(which does not have to bear any relation to the subdivisions specified on SA

records). The conveyance of the segments are used to compute a conveyance-weighted average flow length.

XFLx-coordinate of breakpoints between the segments of the cross-section for which<br/>multiple FLEN values are to be applied. Figure 4-13 illustrates the FLEN and<br/>XFL parameters.

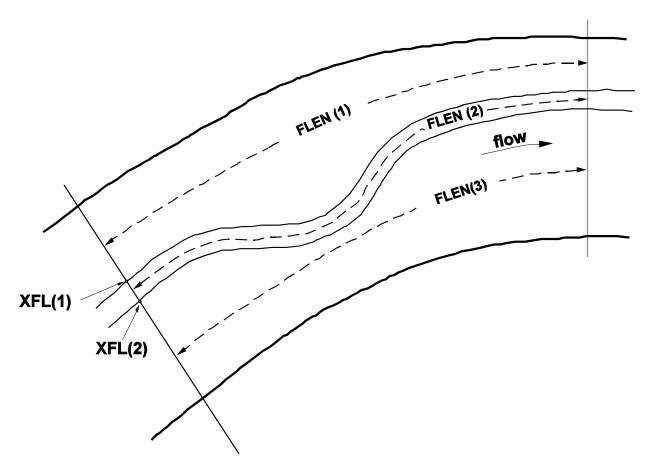


Figure 4-13. Definition sketch: FL record parameters.

Table 4-23.Description of format and contents of FS record.

FS	FS Record	FS

**Purpose:** Optional record for floodway analysis to specify global surcharges that vary based on some corresponding flow on the Q record.

Format:		
Columns:	Format:	Contents:
1-2	A2	FS
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	<b>FS</b> (1), <b>FS</b> (2),, <b>FS</b> ( <b>NPROF</b> )

The parenthetical notation indicates the order number, i, assigned by the model to each profile to be computed; i = 1 and i = NPROF for the first and last profiles to be computed with  $NPROF \le 20$ .

### **Definition of variables**:

FS(i)

Floodway surcharge, in meters, representing the allowable surcharge to be calculated at each section with a corresponding FW record. If a **FS** record is used, a surcharge **must be provided** for **FS** for each discharge specified in the **Q** record. Values on the FW record can override these surcharges for a specific cross-section.

Table 4-24.Description of format and contents of FW record.

<b>FW FW Record FW</b>
------------------------

**Purpose:** Mandatory record for floodway analysis to specify the encroachment method, desired surcharge, and left and right encroachment limits.

Format:		
Columns:	Format:	Contents:
1-2	A2	FW
3	1X	Blank
4	1X	MTHD
5	1X	Blank
6-10	A5	Blank
11-80	Free	TARGET, [XENCLT, XENCRT]

MTHD	Encroachment method option code. Valid values are:
	0 Equal encroachment.
	1 Fixed limits on left bank.
	2 Fixed limits on right bank.
	3 Fixed limits on both left and right banks (no TARGET surcharge).
TARGET	The desired surcharge, in meters. (This value overrides all surcharges that may be specified on the FS record.)
XENCLT	The x-station of the encroachment constraint on left bank. (Not used for option code 0.)
XENCRT	The x-station of the encroachment constraint on right bank. (Not used for option code 0.)

Table 4-25.Description of format and contents of GB record.

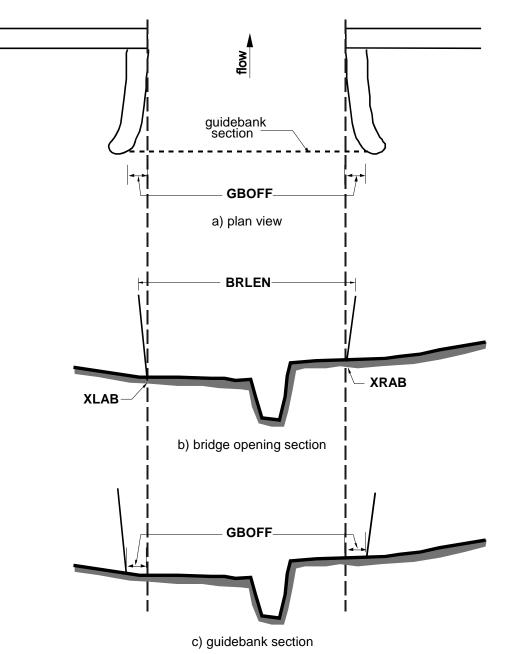
GB GB Record	GB
--------------	----

**Purpose:** Header record for guidebanks cross-section.

Format:		
Columns:	Format:	Contents:
1-2	A2	GB
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, GBTYPE, [BSUBD, GBOFF, SKEW, EK, CK, VSLOPE]

SECID	Unique cross-section identification code.	
SRD	Section reference distance.	
GBTYPE	<ul> <li>Code to indicate the type of guidebanks. Valid entries are:</li> <li>1 Elliptical guidebanks, no skew.</li> <li>2 Elliptical guidebanks, skewed.</li> <li>3 Straight guidebanks, no offset.</li> <li>4 Straight guidebanks, with offset.</li> </ul>	
BSUBD	Distance, in meters, that straight banks are offset from the bridge abutments (not to be confused with <b>GBOFF</b> below). Only relevant when <b>GBTYPE</b> = 4; for all other guide bank types, place asterisk (*) in field as a place holder.	
GBOFF	Measured normal to flow at the guidebank mouths, the horizontal offset, in meters, between guidebanks and bridge abutments. Use an average value if left and right offsets are not equal. WSPRO places base of the guidebanks at this distance from the abutment stations (see Figure 4-14). Guidebank side slopes are equal to bridge abutment slopes. Remaining ground points are obtained from the full-valley cross-section.	

SKEW	The acute angle that section must be rotated to orient the section normal to the flow direction. WSPRO applies the cosine of <b>SKEW</b> to horizontal dimension of section to compute cross-sectional properties. Default is zero degrees.
EK, CK	Expansion and contraction loss coefficients, respectively, for the energy equation balance. Coefficients would apply only to downstream computations from the guidebank to bridge opening. Default values are either $\mathbf{EX} = 0.5$ and $\mathbf{CK} = 0.0$ or the current values being propagated from downstream data.
VSLOPE	Valley slope. Used to adjust elevations of x,y-coordinates for the guidebank when geometric data are propagated. Default value is zero or the current value being propagated from downstream data.



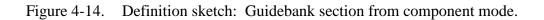


Table 4-26.Description of format and contents of GR record.

**Purpose:** Specifying x,y-coordinates to define cross-sectional geometry.

Format:		
Columns:	Format:	Contents:
1-2	A2	GR
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	X(1), Y(1), X(2), Y(2), X(NGP), Y(NGP)

The parenthetical notation indicates the order number, i, assigned by the model to each x,y-coordinate pair; i = 1 and i = NGP for the first and last coordinate pairs with  $NGP \le 100$ .

#### **Definition of variables:**

X(i)

x-coordinate, in meters from an arbitrary datum on the left bank, of the i<sup>th</sup> ground point (left and right defined by facing downstream.

Y(i) y-coordinate of the i<sup>th</sup> ground point, in meters above elevation datum.

Successive coordinates are coded from left to right (counterclockwise) and each x-coordinate must be equal to or greater than the previous x-coordinate (except for closing bridge openings). There is no limit on the number of **GR** records used. Figure 4-15 illustrates the x,y-coordinate system and its interrelation with roughness data.

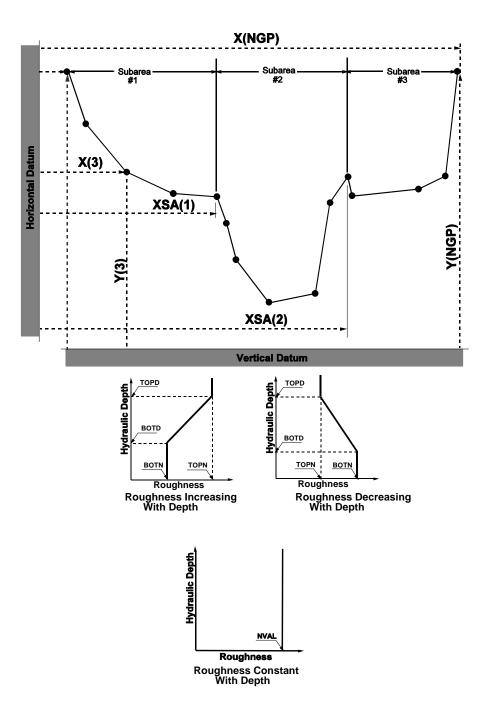


Figure 4-15. Definition sketch: GR, N, ND, and SA parameters.

Table 4-27.Description of format and contents of GT record.

GT GT Record	GT
--------------	----

**Purpose:** Replaces GR data for cross-sections being fabricated from template section.

Format:		
Columns:	Format:	Contents:
1-2	A2	GT
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	YSHIFT, XLIML, XLIMR, SCALE, XORIG

YSHIFT	Vertical distance, in meters, to apply to template section elevations to obtain desired elevations for the fabricated cross-section.
XLIML, XLIMR	x-coordinates of the left and right limits of the segment of the template cross- section to be retained for the fabricated cross-section. Neither value needs to coincide with x,y-coordinates specified in <b>GR</b> records. The model places vertical walls that extend to the maximum elevation of the cross-section at <b>XLIML</b> and (or) <b>XLIMR</b> .
SCALE	Scale factor use to expand (stretch) or contract (shrink) the template section horizontally; <b>SCALE</b> = 1.1 stretches a section by 10 percent and <b>SCALE</b> = $0.9$ shrinks a section by 10 percent).
XORIG	x-coordinate in the template section which is held constant when the <b>SCALE</b> factor is used. This maintains cross-section alignment if so desired; <b>SCALE</b> alters the x-coordinates on either side of <b>XORIG</b> .

Table 4-28.Description of format and contents of HP record.

HP	HP Record	HP

**Purpose:** Instructs the model to produce tables of cross-sectional properties and (or) velocity and conveyance distribution.

Format:		
Columns:	Format:	Contents:
1-2	A2	НР
3	1X	Blank
4	1X	IHP
5	1X	Blank
6-10	A5	[SECID]
11-80	Free	[ELMIN, YINC, ELMAX, Q]

IHP	Option code indicating: (1) table of cross-sectional properties for the entire cross-section ( <b>IHP</b> = 0); (2) table of cross-sectional properties for each subarea as well as the entire section ( <b>IHP</b> = 1); or (3) table(s) of velocity and conveyance distribution ( <b>IHP</b> = 2).
SECID	Section identification code, not required if <b>HP</b> record included with the section data.
ELMIN	The minimum elevation in the cross-section for which computations are desired. Default value is one-fourth of the difference between the maximum and minimum ground elevations above channel bottom, rounded to the nearest whole meter.
YINC	The elevation increment between successive elevations for which computations are desired. Defaults to <b>DELTAY</b> ( <b>J1</b> record).
ELMAX	The maximum elevation for which computations are desired. Default value is the maximum elevation in the cross-section.

**Q** Discharge (coded only for **IHP** = 2) is required to compute the velocity and conveyance distribution.

Multiple **HP** records may be coded for a section to obtain properties and (or) distribution computations (for one or more discharges) at one time. See Sections 4.3.3 and 7.6 for additional information.

Table 4-29.Description of format and contents of J1 record.

J1	J1 Record	J	L
JI	JI RECOLU	LP	L

**Purpose:** Specifying computational control parameters.

Format:		
Columns:	Format:	Contents:
1-2	A2	J1
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	[DELTAY, YTOL, QTOL, FNTEST, IHFNOJ]

DELTAY	Stepping increment, in meters, for successive trial water-surface elevations to balance the energy equation. Default value is 1.0.
YTOL	Allowable tolerance (error), in meters, between successive computed elevations for acceptable energy equation balance. Default value is 0.006 meters.
QTOL	Allowable tolerance (error), in hundredths, between the input discharge ( $\mathbf{Q}$ record) and the total computed discharge for bridge flow and road overflow. Default value is 2 percent (entered as 0.02).
FNTEST	Froude number test value. A computed water-surface elevation with a computed Froude number greater than <b>FNTEST</b> is (at least temporarily) considered an invalid solution. Default value is 0.8.

IHFNOJ Code for the friction slope (or conveyance) averaging technique used in frictionloss computations. Valid entries are: 0 for geometric mean conveyance (default value); 1 for arithmetic average conveyance; 2 for arithmetic average friction slope; and 3 for harmonic mean friction slope. IHFNOJ is applicable for all subreaches except for bridge computations which use geometric mean conveyance. To vary the averaging technique within a job requires use of FL records. IHFNOJ overrides all IHFNO values coded on FL records. Table 4-30.Description of format and contents of KQ record.

**Purpose:** Specifying user-selected breakpoints of the KQ segment of the approach section.

Format:		
Columns:	Format:	Contents:
1-2	A2	KQ
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	XLKQ, XRKQ

XLKQ	x-coordinate of the left limit of the conveyance (flow) distribution for the KQ-section.
XRKQ	x-coordinate of the right limit of conveyance (flow) distribution for the KQ-section.
Notes:	

- 1) Figure 4-16 illustrates application of the KQ record.
- 2) The model, unless overridden by some combination of **XLKQ** and **XRKQ**, will place the KQ-section based on the location of the computed centroid of conveyance.

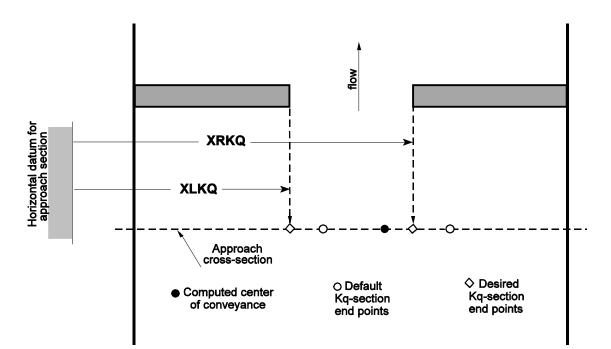


Figure 4-16. Definition sketch: KQ record parameters.

Table 4-31.Description of format and contents of N record.

Ν

**Purpose:** Specifying roughness (Manning's "n") coefficients.

Format:			
Columns:	Format:	Contents:	
1-2	A2	Ν	
3	1X	Blank	
4	1X	Blank	
5	1X	Blank	
6-10	A5	Blank	
11-80	Free	BOTN (1) [, BOTN (2),, BOTN (NSA)]	Option 1

#### -- or --

Format:		
Columns:	Format:	Contents:
1-2	A2	Ν
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	BOTN(1), TOPN(1) [,, BOTN(NSA), TOPN(NSA) ]Option 2

#### **Definition of variables:**

**BOTN**(i)

N-value for the i<sup>th</sup> subarea. In the absence of **ND** record data, this coefficient is applied over the entire range of depths. If **ND** command data are applicable, **BOTN**(i) is applied for the range of hydraulic depth, d of 0 < d **BOTD**(i).

**TOPN**(i)When ND command data are applicable, TOPN(i) is applied for the range of<br/>hydraulic depth, d of d TOPD(i). TOPD(i) values must not be coded when ND<br/>command data are not applicable.

Notes:

- 1) See Figure 4-14 and Tables 4-32 and 4-35 for additional information.
- 2) NSA is the number of subareas in the channel cross-section.

Table 4-32.Description of format and contents of ND record.

ND ND Record	ND
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**Purpose:** Specifying depth breakpoints for vertical variation of roughness.

Format:		
Columns:	Format:	Contents:
1-2	A2	ND
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	BOTD(1),TOPD(1),BOTD(2),TOPD(2), BOTD(NSA),TOPD(NSA)

The parenthetical notation indicates the subarea number, i, assigned by the model to each subarea in a cross-section; i = 1 and i = NSA for the leftmost and rightmost subareas with  $NSA \le 20$ .

#### **Definition of variables**:

- **BOTD**(i) Hydraulic depth (area divided by top width) in the i<sup>th</sup> subarea at or below which **BOTN** (specified for the i<sup>th</sup> subarea in the **N** record) is applicable.
- **TOPD**(i) Hydraulic depth in the i<sup>th</sup> subarea at or above which **TOPN** (specified for the i<sup>th</sup> subarea in the **N** record) is applicable.

Roughness coefficients for hydraulic depths between **BOTD** and **TOPD** are computed by straight-line interpolation. Values of **BOTD**, **TOPD**, **BOTN**, and **TOPN** must be supplied for each subarea when **ND** data are coded. Code equal top and bottom values for any subarea(s) with constant roughness. See Figure 4-15 and Tables 4-31 and 4-35 for additional information.

Table 4-33.Description of format and contents of PD record.

PD	PD Record	PD
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**Purpose:** Specifying pier or pile data.

Format:		
Columns:	Format:	Contents:
1-2	A2	PD
3	1X	Blank
4	1X	PPCD
5	1X	Blank
6-10	A5	Blank
11-80	Free	PELV(1), PDTH(1), PNUM(1),, PELV(NPD), PDTH(NPD), PNUM(NPD)

The parenthetical notation indicates the order number, i, assigned by the model to each **PELV**, **PDTH**, and **PNUM** data pair; i = 1 and i = NPD for the lowest and highest **PELV** values with NPD  $\leq 25$ .

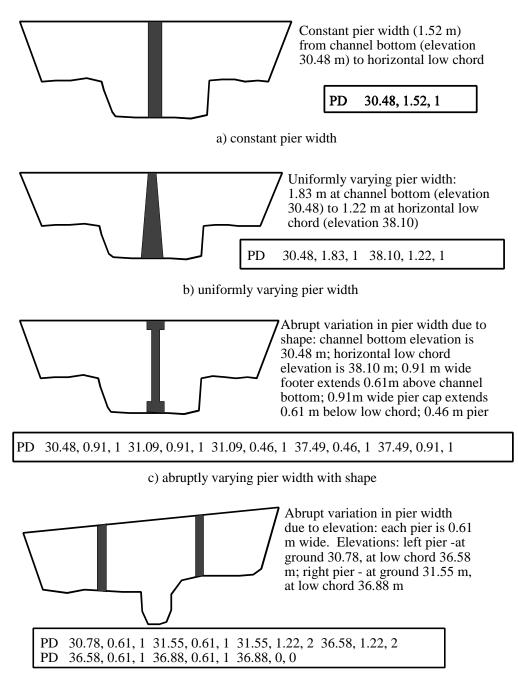
#### **Definition of variables:**

PPCD	Code to identify piers ( <b>PPCD</b> = 0) or piles ( <b>PPCD</b> = 1); affects the pier adjustment to the coefficient of discharge.
<b>PELV</b> (i)	The elevation, in meters above the common elevation datum, of the i <sup>th</sup> pair of elevation-width data.
<b>PDTH</b> (i)	The <b>gross</b> width, in meters, of <b>all</b> piers (and/or pile bents) for the $i^{th}$ pair of elevation-width data.
<b>PNUM</b> (i)	The number of piers/piles at the specific elevation <b>PELV</b> (i).

Figure 4-16 illustrates pier data requirements. An elevation-area relation is computed from the elevationwidth data. Areas between specified elevations are based on straight-line interpolation. Constant width is assumed between the highest **PELV** and the maximum bridge-opening elevation. Coding a **PELV** value lower than the minimum bridge-opening elevation is an abortive error. The minimum data requirement is an elevation-width pair at the minimum elevation at which piers (piles) begins. No additional data are required if the gross width is constant between the specified **PELV** and maximum bridge-opening elevation (see Figure 4-17a).

If the gross pier (pile) width varies uniformly between the minimum **PELV** and maximum bridgeopening elevation, a second elevation-width pair is needed for the maximum elevation (see Figure 4-17b).

Non-uniform variation of gross width requires two elevation-width pairs at each elevation that gross width changes abruptly. An abrupt change can be: (1) additional piers coming into effect with increasing elevation; (2) changes in pier dimensions; and (3) loss of piers with increasing elevation (sloping low chord). Figures 5-17c and 5-17d illustrate the latter cases.



d) abrupt changes in pier width with elevation

Figure 4-17. Definition sketch: PD record parameters.

Table 4-34.Description of format and contents of Q record.

Q	Q Record	Q
<u>V</u>	Q Record	(

**Purpose:** Specifying discharge for each profile to be computed.

Format:		
Columns:	Format:	Contents:
1-2	A2	Q
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	$Q(1), Q(2), \ldots, Q(NPROF)$

The parenthetical notation indicates the order number, i, assigned by the model to each profile to be computed; i = 1 and i = NPROF for the first and last profiles to be computed with  $NPROF \le 20$ .

#### **Definition of variables:**

**Q(i)** 

Discharge, in cubic meters per second, for each water-surface profile to be computed. The final value of **NPROF** is determined by the model based on the total number (specified or default) of corresponding, valid entries in the **Q**, **WS**, **SK**, and **EX** records. All entries in the **Q** record must be positive values (no zero or default values are permitted). See Sections 4.3.2, 4.3.3, and 7.6 for additional information.

Table 4-35.	Description of format and contents of SA record.
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SA	SA Record	SA

**Purpose:** Specifying horizontal breakpoints for subdivision of cross-section for roughness and (or) geometry variations.

Format:			
Columns:	Format:	Contents:	
1-2	A2	SA	
3	1X	Blank	
4	1X	Blank	
5	1X	Blank	
6-10	A5	Blank	
11-80	Free	XSA(1), XSA(2), XSA(NSA-1)	

The parenthetical notation indicates the subarea number, i, assigned by the model to each subarea in a cross-section; i = 1 and i = NSA for the leftmost and rightmost subareas with NSA  $\leq 19$ .

#### **Definition of variables:**

XSA(i)

x-coordinate of the rightmost limit of the i<sup>th</sup> subdivision. The last **XSA** value coded is the horizontal station of the subdivision point between the last two subareas. **XSA** values are not required to match with any x-coordinates in the **GR** data **except in bridge-opening cross-sections**.

Table 4-36.Description of format and contents of SI record.

**Purpose:** Allows user the option of specifying the unit system for both input and output.

Format:		
Columns:	Format:	Contents:
1-2	A2	SI
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	(code number, 0, 1, 2, or 3)

#### **Definition of variables**:

code number:	0	<u>Input</u> ENGLISH	<u>Output</u> ENGLISH
	1	METRIC	METRIC
	2	ENGLISH	METRIC
	3	METRIC	ENGLISH

Table 4-37.Description of format and contents of SK record.

SK	SK Record	SK

**Purpose:** Specifying friction slope(s) (energy gradient(s)) for computing starting water-surface elevation(s) by slope conveyance.

Format:		
Columns:	Format:	Contents:
1-2	A2	SK
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	SKSL(1), SKSL(2), SKSL(NPROF)

The parenthetical notation indicates the order number, i, assigned by the model to each profile to be computed; i = 1 and i = NPROF for the first and last profiles to be computed with  $NPROF \le 20$ .

#### **Definition of variables**:

SKSL(i)
Friction slope (energy gradient), in meters per meter, to compute the initial water-surface elevation for the i<sup>th</sup> profile by slope conveyance. Computations are not made for any SKSL with a zero, negative, or null value. The number of entries (specified or default) in the SK record must be identical to the number of entries in the Q record. The last entry in an SK record must not be a null value; instead code a negative slope. See Section 4.3.2 and Figure 4-12 for additional information.

Table 4-38.Description of format and contents of T1, T2, and T3 records.

# T1[,T2,T3] T1[,T2,T3] Record T1[,T2,T3]

**Purpose**: To present title information for identification of model input dataset.

Format:		
Columns:	Format:	Contents:
1-2	A2	T1[,T2,T3]
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	70A1	Any alphanumeric character string

Format: Notes:	<ul> <li>T1 [up to 70 alphanumeric characters to provide first title line]</li> <li>T2 [optional - up to 70 alphanumeric characters to provide second title line]</li> <li>T3 [optional - up to 70 alphanumeric characters to provide third title line]</li> </ul>
1) 2)	This record is used in conjunction with the T2 and T3 commands. The information in the free-format area of the T1, T2, and T3 command are printed on essentially every page of printed output, along with the date and time of job execution. When analyzing a series of alternative designs, it is possible to change some of the title information for each alternative without recoding all three commands. Depending on the amount of information to be changed, the user may choose to provide a new T2 and T3 command or just a new T3 command for each alternative. If a new T2 command is coded without a new T3 command, a blank line is printed for T3 command information.

Table 4-39.Description of format and contents of UT record.

UT	UT Record	UT

**Purpose:** Specifying parameters for user-defined output tables. (Up to three tables can be obtained).

Format:		
Columns:	Format:	Contents:
1-2	A2	UT
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	VARNOS (list 1), VARNOS (list 2), VARNOS (list 3)

#### **Definition of variables:**

VARNOS

Code numbers of stored output variables. The total number of variables in the three lists cannot exceed 50. Up to 3 tables containing no more than 50 total variables can be printed. An asterisk in the data field on the UT record separates the user-defined tables. The number of variables in each individual list is constrained only by printer line length (80 columns maximum). The model automatically uses 12 columns, thus leaving a maximum of 68 columns for the user. Table 5.1 provides a listing of variables from which user-defined tables can be developed.

Table 4-40.Description of format and contents of WS record.

WS WS Record	WS
--------------	----

**Purpose:** Specifying starting water-surface elevations for profile computations.

Format:		
Columns:	Format:	Contents:
1-2	A2	WS
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	WSI(1), WSI(2),, WSI(NPROF)

The parenthetical notation indicates the order number, i, assigned by the model to each profile to be computed; i = 1 and i = NPROF for the first and last profiles to be computed with  $NPROF \le 20$ .

#### **Definition of variables:**

WSI(i)
 Elevation, in meters, above the common elevation datum, representing the water-surface elevation to be used at the first cross-section for the i<sup>th</sup> profile computation. If a WS record is used, an elevation or a null value (\*) must be provided for WSI for each discharge specified in the Q record. A null value must not be specified for WSI for the last profile; instead code an elevation lower than the minimum elevation in the initial section. See Section 4.3.2 and Figure 4-11 for additional information.

Table 4-41.Description of format and contents of XR record.

XR	XR Record	XR
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**Purpose:** Header record for road-grade cross-section parameters.

Format:		
Columns:	Format:	Contents:
1-2	A2	XR
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, EMBWID, [IPAVE, USERCF, SKEW, EK, CK, VSLOPE]

#### **Definition of variables**:

SECID	Unique cross-section identification code.
SRD	Section reference distance. Should reflect the location of the centerline of the road near the center of the bridge.
EMBWID	Embankment top width, in meters. This distance should reflect the breadth (measured in the direction of flow) of the broad-crested weir that the embankment becomes when overtopped. Default value is <b>BRWDTH</b> ( <b>CD</b> record) which is frequently narrower than most of the embankment width.
IPAVE	Code to indicate the road surface material. Default is paved (IPAVE = 1); graveled (or otherwise rough) is indicated by IPAVE = 2.
USERCF	User-specified coefficient for unsubmerged weir flow. This value will override the coefficient computed by the model. The model will apply an adjustment factor for submerged weir flow for either a user-specified or computed coefficient.

SKEW	The acute angle that the section must be rotated to orient the section normal to the flow direction. The model applies the cosine of <b>SKEW</b> to the horizontal dimension of the section to compute the weir length perpendicular to the flow. Sometimes it is more correct to use the default of zero degrees for a skewed cross-section because upstream ponding causes the overflow to cross the road at a 90-degree angle, thus the skewed length is the more appropriate weir length to be used.
ЕК, СК	Coefficients used to compute expansion and contraction losses, respectively, for the energy equation balance. Default values are either $\mathbf{EK} = 0.5$ and $\mathbf{CK} = 0.0$ or the current values being propagated from downstream data.
VSLOPE	Valley slope, meters per meter. Used for adjusting elevations of propagated geometry data. (Default value is either 0.0 or the last valley slope that was input for a previous cross-section).

Table 4-42.Description of format and contents of XS record.

# XS XS Record XS

Format:		
Columns:	Format:	Contents:
1-2	A2	XS
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, [SKEW, EK, CK, VSLOPE]

**Purpose:** Header record for unconstricted valley cross-section.

#### **Definition of variables**:

SECID	Unique cross-section identification code.
SRD	Section reference distance, in meters. Cumulative distance along the stream measured from an arbitrary zero reference point ( <b>SRD</b> may be negative). Unless overridden by <b>FL</b> data, the difference between the <b>SRD</b> values of successive cross-sections is assumed to represent the flow distance between those sections and is used to compute friction losses for the energy equation balance.
SKEW	The acute angle that the section must be rotated to orient the section normal to the flow direction. The model applies the cosine of <b>SKEW</b> to the horizontal dimension of the section to compute cross-sectional properties. Default is zero degrees. See Section 4.3.6 for additional information.
EK, CK	Coefficients used to compute expansion and contraction losses, respectively, for the energy equation balance. Default values are either $\mathbf{EK} = 0.5$ and $\mathbf{CK} = 0.0$ or the current values being propagated from downstream data.
VSLOPE	Valley slope, in meters per meter, used for adjusting cross-section elevations when the geometry data for the section are being propagated from a template

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section or from a previously input section. Default value is either zero or the current value being propagated from downstream data.

Table 4-43.Description of format and contents of XT record.

# XT XT Record XT

Format:		
Columns:	Format:	Contents:
1-2	A2	XT
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	SECID
11-80	Free	SRD, [VSLOPE]

**Purpose:** Header record for template cross-section.

#### **Definition of variables**:

**SECID** Unique cross-section identification code.

**SRD** Section reference distance, in meters. This provides the reference point for elevation adjustments by valley slope.

**VSLOPE** Valley slope, in meters per meter. Alternatively, the valley slope may be specified in the header record of the cross-section being fabricated.

Only geometry data (**GR** records) are input for a template section. Roughness data (**SA**, **N**, and **ND** records) are included with the input data for the cross-section(s) fabricated from the template section.

Table 4-44.Description of format and contents of \* record.

*	* Record	*
---	----------	---

**Purpose:** Inserting comments (or blank lines) in the input data sequence.

Format:		
Columns:	Format:	Contents:
1-2	A2	
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	[up to 70 alphanumeric characters to insert comments]

**Note:** The free-format area can be used to code notes that may help the user keep track of the input data, or simply left blank to separate different input data (e.g., between cross-sections) to improve readability of printouts.

Table 4-45.Description of format and contents of \*F record.

F	*F Record	* <b>F</b>
F	*F Record	

Format:		
Columns:	Format:	Contents:
1-2	A2	• F
3	1X	Blank
4	1X	Blank
5	1X	Blank
6-10	A5	Blank
11-80	Free	(no parameters)

**Purpose:** To allow data to be entered in free format.

**Note:** This command should be the first command in the data file if the data following are in free format.

#### 4.3 SPECIAL CONSIDERATIONS

The purpose of this section is to somewhat consolidate several aspects of data input that appear to cause the majority of problems in WSPRO applications. The intent of the following subsections is to highlight potential problems and discuss preventative action(s). Much of the discussion consists of cross references to applicable sections, figures, and tables.

#### 4.3.1 Data Record Continuation

Continuation of some data records would be totally illogical (e.g., T1, T2, and T3 records). There would never be a logical need for continuation of many of the data record types because of the limited number of parameters (e.g., header records, data display commands, and others). Cross-section data (GR, N, ND, and SA records) are easily continued by simply providing a sufficient number of records with the same record identifier to contain all the data items. There are no rules as to the number of data items per record nor where in the sequence of data items the switch is made to the next record. However, a special rule exists for Q, WS, SK, and UT records which often require more than a single record. To use more than a single record for these data types, the last entry in all but the last record of the group must be a comma. The model will not recognize the intended continuation of the data if this rule is not followed.

#### 4.3.2 Profile Control Data

A discharge must be specified in the Q record(s) for each profile to be computed. In addition to discharge the model must be provided information, either explicitly or implicitly, regarding boundary condition(s) (i.e., water-surface elevation or energy gradient at the initial section and computational direction) for each profile. No default values are permitted in the discharge data; if multiple profiles are to be computed for the same discharge for different initial water-surface elevations, that discharge must be coded for each profile to be computed.

With one exception there must be either a WS or an SK record in the data sequence, in some cases both are necessary. An SK record is not required when the user does not want the initial water-surface elevation for any profile(s) to be determined by slope-conveyance computations. A WS record is required when the user wants to explicitly assign the initial water-surface elevation for any profile(s). If a combination of energy gradient(s) and explicitly assigned water-surface elevation(s) are to be used for initial conditions, both WS and SK records are required. Neither WS nor SK data are required if all profiles are to be computed using critical water-surface elevation at the initial cross-section. For any profile that the user does not provide a "valid" entry for either energy gradient or water-surface elevation, the model defaults to using critical water-surface elevation at the initial cross-section. A "valid" entry for slope is any number greater than zero (no check is made as to reasonable magnitude). A "valid" entry for water-surface elevation that is within the range of the minimum and

maximum ground elevations at the initial cross-section; and (2) in the correct flow regime (subcritical or critical for upstream computations; supercritical or critical for downstream computations). Figure 4-12 illustrates the computational path for various combinations of data in the WS, SK, and EX records. A "valid" entry in the SK record takes precedence over a specified water-surface elevation and will be used for slope-conveyance computation of the initial water-surface elevation (WSI). If a corresponding "valid" entry is coded in the WS record, it will be used as the "first guess" in the trial-and-error slope-conveyance solution. This feature was designed to speed up the trial-and-error solution. However, the solution is efficient enough that the user should not be too greatly concerned with making such "guesses." In the absence of a "valid" entry in the SK record, a "valid" entry in either WS or SK record(s), WSI is set equal to the critical-flow water-surface elevation, CRWS, which is based on minimum specific energy.

The model initially assumes the total number of profiles to be computed is equal to the number of discharges coded in the Q record(s). However, the model reduces that number if it does not find corresponding data (specified or default) in the WS and (or) SK and EX records. A complicating factor is that "trailing" default values (those appearing after the last specified numerical value) are not "counted." Therefore, do not allow a default value for the last profile in either SK or WS records. If SK data are being used, but slope conveyance is not applicable to the last profile, code a negative value for the energy gradient for the last profile. Likewise, if WS data are being used, but an explicitly assigned water-surface elevation is not applicable to the last profile, code a value that is less than the minimum ground elevation in the initial cross-section. The rules for providing appropriate corresponding entries in the EX record (by specification or default) are explained in Table 4-21.

## 4.3.3 Placement of Q and HP Records in Data Sequence

There are alternate means of specifying discharges and requesting plots and properties of cross-sections. Also, unlike most of the data types, these data records require specific ordering within the overall data sequence. Refer to Section 7.6 for an example that illustrates these rules.

## 4.3.4 Section Reference Distance

Section reference distance (SRD) must be coded in each header record (can never accept a default value in this field). Most of the cross-sections are input in downstream to upstream order (except if both guidebank and road-grade sections are coded at a bridge, and in some multiple-opening situations). Therefore, each SRD should be equal to (for full-valley and bridgeopening sections and for some multiple-opening situations) or greater than the preceding SRD (except in some multiple-opening situations). No internal check (error trap) exists for SRD coding mistakes. Therefore, the profile computations output should be scanned to detect such errors; the SRDL variable should reflect positive (negative) values of reasonable magnitude for upstream (downstream) computations.

The SRD generally reflects points located along the main channel. In multiple-opening situations data are input for each opening from left to right across the valley. SRD for each opening should reflect the location along the valley. A multiple-opening crossing that is not skewed would have the same SRD value for all bridge-opening (and culvert) sections. However, for a skewed crossing having its right end further downstream than its left end, the SRD of each opening should be unequal and in descending order.

## 4.3.5 Elevation Limits for Computing Profiles

WSPRO's chances of successfully balancing (1) the energy equation for both openchannel and bridge-backwater computations and (2) the discharge for combined flow through the bridge and over the road are constrained by both the input data and flow regime.

For upstream computations in open channels, WSPRO is limited to a range of elevation bounded by a minimum of the water-surface elevation for critical flow (CRWS) and a maximum of the highest ground elevation (YMAX) coded for the cross-section. If YMAX is the elevation of the leftmost (rightmost) point in the cross-section, WSPRO "builds" a wall at the rightmost (leftmost) point in the cross-section extending vertically to YMAX. If YMAX occurs at an interior point in a cross-section, both banks will be extended vertically to YMAX. It follows that the user should attempt to code any open-channel cross-section to include at least one ground point higher than any computed water-surface elevation that can reasonably be expected.

For upstream computations of bridge hydraulics, the lower elevation limit for any crosssection is the water-surface elevation for critical flow. The upper limit is governed by YMAX of the approach cross-section. Not coding the approach section to a sufficiently high elevation is a frequent cause for termination of computations.

## 4.3.6 Skewed Cross-Sections

The model requires cross-sectional properties (e.g., area, conveyance, wetted perimeter, etc.) that reflect a plane normal to the direction of flow. Normally this should be accomplished when the section is surveyed. In the simplest cases, a single straight line across the valley can be surveyed normal to the flow. If such a section is later determined to be actually skewed to the direction of flow (or field conditions dictate surveying along a line skewed to the flow), the skew angle can be coded in the header record and the model will use the cosine of the angle to convert the surveyed plane to a plane normal to the flow. However, this "rotation" of the cross-section is

strictly horizontal (i.e., there is no accounting for elevation differences between the points in the surveyed cross-section relative to "true" ground elevations along the plane normal to the flow). Thus, if valley slope is significant, the "corrected" cross-section does not truly represent the physical field conditions. Sometimes more than one straight-line segment (or perhaps a curvilinear line) is required to obtain a plane normal to the flow. In such cases the section must be surveyed (or a surveyed section modified) to directly reflect the appropriate plane in the coded x,y-coordinates. Cross-sections that are corrected for skew have their x-coordinates adjusted about the minimum y-coordinate (YMIN) in the cross-section.

Bridge openings can also be easily corrected for skew. Logically, one would usually survey the full valley cross-section with the same orientation to the floodplain as the bridge cross-section. However, this is not a requirement. The user must be aware that there is no assumed relationship between values of skew coded on the full valley cross-section and values of skew coded on the bridge cross-section. If both the full valley and bridge cross-sections are skewed to the floodplain, then the user would code a value of skew on each cross-sections header record.

The user should seriously question the applicability of WSPRO for severely skewed stream crossings. In addition to the "rotation" versus true elevations alluded to above, a skewed bridge situation (either single or multiple openings) may create complex flow patterns that in order to analyze may require a two-dimensional (2-D) analyses. Any bridge flow exhibits some degree of 2-D flow. WSPRO can use one-dimensional (1-D) computational techniques to successfully analyze the hydraulics of bridges that are not terribly complex. This is accomplished by use of certain coefficients, effective flow length (which reflects a quasi-2-D component), and cross-sections located close to, but not in, the definite 2-D flow zones. A skewed crossing, however, introduces increased opportunity for 2-D flow to the extent that the flow patterns assumed for the WSPRO computations may be totally invalidated. Two significant problems (relative to 1-D flow) occur at the upstream side of a skewed crossing: (1) the water tends to pond (stagnate) along the segment of the embankment that is furthest downstream; and (2) the segment of the embankment that is furthest upstream tends to intercept the flow and "funnel" the intercepted flow (especially if there is a cleared right-of-way) along the embankment to the bridge opening. The resulting flow pattern is far different from that which occurs at a perpendicular crossing. Similar problems can be caused by complex topography and (or) complex patterns of roughness distribution.

Another problem that may arise at skewed crossings is cross-section location (which is a moot point if flow pattern is a problem). For larger skew angles, the exit and (or) approach sections may intersect with the embankment when an attempt is made to locate them normal to the flow at the prescribed distance from the bridge opening. If the "rotational" problem is not severe, it is probably best to survey all bridge-related sections parallel to each other, code the

appropriate skew angle, and let the model adjust them accordingly. If that is not considered appropriate, sound engineering judgment must be exercised in locating the cross-sections.

Surveying all bridge-related cross-sections parallel to each other should also be considered for skewed multiple-opening stream crossings. In some instances when making a multiple-opening analysis, the model has trouble making the various projections for the needed intermediate exit and approach sections when there is a combination of skewed and unskewed cross-sections. The above discussion pertaining to 2-D versus 1-D analyses is also strongly applicable to multiple-opening situations.

## 4.3.7 Bridge-Backwater Computations

See Section 4.3.5 for elevation limits for the backwater computations.

See Figures 3-8 through 3-13 and related discussion in Section 3.2 for cross-section data requirements for bridge-backwater computations.

See Section 4.3.8 for information pertaining to approach sections.

The parameter that controls whether or not the model will check for the possibility of and compute pressure flow is the low-steel elevation (PFELEV), which may be coded in the BR header record. In component mode the model computes a value of PFELEV (average of the elevations of abutment tops) and the user is not required to assign a value on the BR record. However, in coordinate mode the model does not have sufficient information to compute PFELEV. Therefore, the user must code PFELEV in the BR record for coordinate mode if there is any possibility of pressure flow. Without a value for PFELEV, either computed (in component mode) or specified (in coordinate mode), the model cannot test for the possibility of pressure flow and thus will not compute pressure flow regardless of the degree of submergence of the opening.

The PFELEV parameter can also be used to control the type of solution obtained (i.e., pressure versus free surface). Because the bridge opening is completely described in the form of a polygon, the PFELEV is not tied into the computation of the bridge-opening properties. Thus the user may "mislead" the model by coding a PFELEV value that is higher or lower than the actual low-chord elevation to dictate the flow class that is computed. This is possible in both component and coordinate modes. For example, the user may wish to obtain a free-surface flow solution to compare with a borderline submerged orifice flow situation. The model is a slave to precision. If PFELEV (computed in component mode or specified by the user for coordinate mode) is 100.000 and the computed tailwater elevation is 100.010 or 100.001), the model will compute submerged pressure flow which is obviously a very borderline case. By specifying a value of PFELEV that is higher than the tailwater elevation, the user can force a free-surface

solution. Sometimes it is also possible to vary PFELEV to dictate free-surface versus unsubmerged pressure flow computations.

WSPRO is not universally applicable to dual bridges or other closely-spaced bridge situations. Typical dual bridges do not usually pose a major problem inasmuch as the flow is essentially continuous through the two structures. Even if the abutments of the two structures are not continuous, the distance between the structures is generally too short to permit expansion of the flow between the two structures to a degree that the flow must undergo significant contraction to pass through the downstream structure. Under such conditions the dual bridges can be treated as an extra-wide bridge (BRWDTH in the CD record). Water may actually be flowing into any gap between the embankments associated with each structure. Such flow is not, however, flowing in the direction that the flow is being modeled. If the quantity of such flow between the embankments is significant and (or) the multi-directional flow pattern is thought to have significant effect on the bridge backwater, a one-dimensional model such as WSPRO is not the appropriate model for analyzing that flow situation.

Some dual bridges are too far apart to satisfy the above assumption of essentially continuous flow through the two structures. In some cases, especially urban situations, bridges are so closely spaced that cross-section location requirements for WSPRO's computational techniques cannot be strictly satisfied. The most important question to be considered under such circumstances is whether or not the true physical system can be adequately described for 1-D analysis. If the situation is obviously a 2-D flow situation, WSPRO is not applicable to the problem. When attempting to describe closely spaced bridges for 1-D analysis, consideration must be given as to how much of the valley width will actually be flowing effectively. It may be prudent to arbitrarily (using engineering judgment) decrease the width of the approach section of the downstream bridge by constructing walls at the boundaries of the assumed effective flow width (likewise, the exit section of the upstream bridge). In some cases of very closely-spaced bridges, it is probably best to abandon "bridge hydraulics" and attempt to model the flow situation using straight step-backwater computations with appropriate expansion and contraction coefficients.

#### 4.3.8 Approach Sections

See Section 4.3.5 for discussion regarding the effect of the maximum ground elevation in the approach section on bridge-backwater computations.

If a BR Header Record is coded, the program recognizes the first cross-section upstream as the approach cross-section.

The approach cross-section should be located at a distance upstream from the bridge opening that is approximately equal to the sum of the BRWDTH and BRLEN (plus the distance from the upstream face of the bridge to the dike section if guidebanks are present). Some latitude is permitted on this requirement; misplacement by as much as 20 percent of the BRLEN will usually have insignificant effect on the results. A major problem can arise if the approach section is too close to the bridge. The effective flow length computations may encounter a "divide by zero" or a "square root of a negative number" computational problem, thus terminating the model computations. The user should check for possible datum problems between, or perhaps x,y-coordinate coding problems in, the bridge and approach sections. Two parameters in the profile output for the constricted results at the approach section provide a means for this check. If FLEN (the effective flow length) is drastically different than the SRD difference (SRDL), a problem exists. The BP record provides a solution for datum problems for single-opening situations.

#### 4.3.9 Road-Grade Sections

Care should be taken to provide consistent data for the approach, road-grade, and exit sections; do not describe a situation that is hydraulically impossible. If the road-grade is to act as a weir, the flow must have access from the approach section and egress to the exit section for the entire length of the road-grade that the model is using for a weir. Frequently a road-grade will exit the flooded width of the valley through a "cut" section in the side hill(s). The inundated length of roadway can thus be much greater than the effective flow width of the valley. However, the coded road-grade section should reflect only that part of the road-grade that can act as a weir; slack-water segments outside of the effective flow width of the approach and (or) exit sections must be excluded.

Road-grades may not always act as weirs, especially when the embankment height is very small and (or) depths of water over the road are quite high. Instead of acting as a weir, the road-grade is essentially a vertical contraction of a section of the valley. In such cases it sometimes becomes necessary to abandon the "bridge and weir" hydraulics in favor of the "composite-section" method described by Shearman and others (1986).

## 4.3.10 Multiple-Opening Situations

The upstream match section is analogous to the approach section of single-opening bridges and must be coded with an XS header record. The maximum ground elevation influences bridge backwater computations (see Section 4.3.5).

See Section 4.3.4 for discussion related to section reference distances.

The requirement of a common horizontal datum for all related cross-sections (downstream match section through upstream match section, inclusive) cannot be overstressed. There is no opportunity to correct datum problems with BP records in multiple-opening situations. All cross-sections must be aligned or computations related to the projection of data between and among sections will fail.

All openings must have flow through them or the model will experience computational problems and will either run forever or terminate. For situations where one or more openings are "dry" for lower discharges of interest, the analysis must be segmented such that all openings have flow for the discharges being analyzed.

## **SECTION 5 - MODEL OUTPUT OVERVIEW**

WSPRO automatically generates rather detailed output describing the processing of the input data and the results of all profile computations. This output, depending on the user's system and mode of operation, may be automatically printed and (or) stored in a "print" file for viewing on the screen of a terminal or microcomputer. The model offers no options to suppress any of this output, but users can edit out unwanted segments of the file before printing. Output generated for cross-sectional properties and user-defined tables also are directed to this "print" file. WSPRO uses a temporary direct-access file for storage and retrieval of cross-section data. Another temporary direct-access file stores computed results and key input parameters. Additional detail is presented in the following paragraphs and examples of model output are presented in Section 7. A glossary in the appendix contains definitions and (or) cross references for the many acronyms that appear in the output.

## 5.1 INPUT-DATA-PROCESSING INFORMATION

This output echoes each input data record and provides detailed information on how WSPRO has interpreted and processed each data record. This information includes messages related to possible input data problems; these messages are discussed in Section 6.1. A header record indicates the end of input data for the current section being processed (or the beginning of input data for the next section). Upon encountering a header record while processing cross-section data, WSPRO determines how much (if any) data are needed from previously input section data to provide complete section definition. After determining that a section has been completely defined, the current section is output to file 13 for subsequent retrieval for profile computations.

## 5.2 CROSS-SECTIONAL PROPERTIES

HP records can be used to generate tables of cross-sectional properties and (or) velocity and conveyance distribution for any section(s) except road grades and culverts. Properties can be obtained for the total cross-section, with or without a subarea breakdown. Cross-sectional geometry errors may be revealed by inconsistencies (and (or) absurd values) of properties, either within a cross-section or from section to section in a reach. Velocity and conveyance distribution can be obtained for one or more discharges at one or more elevations. Coding strategies for HP records are discussed in Section 4.3.3 with examples in Section 7.

#### 5.3 STANDARD TABLE OF COMPUTED PROFILE RESULTS

This output, also directed to the "print" file, contains a section-by-section tabular summary of a considerable number of parameters from the profile computations. It also may contain messages pertaining to the profile computations; these messages are discussed in Section 6.2. Thorough examination of this output to evaluate the adequacy of the analysis should be considered an integral part of any modeling effort. There is generally sufficient information in this output for the user to readily determine if there are any significant problems in the hydraulic computations. However, it may be necessary to generate supplemental output, such as properties computations, to determine corrective action(s) required (e.g., perhaps more (or better) section data). Although the headings for the various parameters were intended to be descriptive, the infrequent or less experienced user may not find all of them meaningful. The appendix provides a glossary containing brief definitions and (or) cross references for input/output acronyms.

#### 5.4 USER-DEFINED TABLES

Tables of selected parameters can be generated in the print file by coding a sequence of variable code numbers on UT record(s). Table 5-1 lists the variables that can be printed in table format along with the headings used and variable parameter code numbers. User-defined tables cannot be generated for multiple-opening analyses. As can be seen in the table, up to 80 variables can be printed in tables. Up to 3 tables with no more than 50 total variables can be printed. An asterisk in the data field on the UT record separates the user-defined tables.

		,
	Variable code number	Heading
Cross-section conveyance	1	К
Cross-section area	2	AREA
Velocity head correction factor, a	3	ALPH
Momentum correction factor, ß	4	BETA
Water-surface elevation	5	WSEL
Velocity head	6	VHD
Discharge	7	Q
Section reference distance	8	SRD
Maximum station in cross-section	9	XMAX
Maximum elevation in cross-section	10	YMAX
Minimum station in cross-section	11	XMIN
Minimum elevation in cross-section	12	YMIN
Boundary cross-section conveyance	13	Κ
Boundary cross-section area	14	AREA

Table 5-1.Variables available for user-defined tables (UT record).

#### (CONTINUED ON NEXT PAGE)

	Variable code number	Heading
Boundary velocity head correction factor, a	15	ALPH
Boundary momentum correction factor, ß	16	BETA
Boundary water-surface elevation	17	WSEL
Boundary velocity head	18	VHD
Boundary discharge	19	Q
Boundary section reference distance	20	SRD
Boundary maximum station in cross-section	21	XMAX
Boundary maximum elevation in cross-section	22	YMAX
Boundary minimum station in cross-section	23	XMIN
Boundary minimum elevation in cross-section	24	YMIN
Energy grade line	25	EGL
Velocity	26	VEL
Froude number	27	FR#
Critical water-surface elevation	28	CRWS
Minimum flow depth	29	DMIN
Friction loss	30	HF
Other losses (expansion/contraction)	31	НО
Expansion loss coefficient	32	EK
Contraction loss coefficient	33	СК
Friction slope	34	SF
Error in energy/discharge balance	35	ERR
Flow distance	36	FLEN
Straight-line (SRD) distance	37	SRDL
Stagnation point, left	38	SPLT
Stagnation point, right	39	SPRT
Skew of cross-section	40	SKEW
Cross-section wetted perimeter	41	XSWP
Cross-section top width	42	XSTW
Left edge of water	43	LEW
Right edge of water	44	REW
Low steel (submergence) elevation	45	PFELEV
Bridge opening length	46	BLEN
Bridge opening type	47	TYPE
Flow classification code	48	FLOW
Abutment station, left toe	49	XLAB
Abutment station, right toe	50	XRAB
Coefficient of discharge	51	С
Pier or pile code	52	PPCD
Pier area ratio	53	P/A
Road overtopping elevation	54	OTEL
Left Road Section		
Flow over road	55	Q

#### Table 5-1.Variables available for user-defined tables (UT record) (continued).

#### (CONTINUED ON NEXT PAGE)

	Variable code number	Heading
Weir length	56	WLEN
Left edge of water	57	LEW
Right edge of water	58	REW
Maximum depth of flow	59	DMAX
Average depth of flow	60	DAVG
Average total head	61	HAVG
Average weir coefficient	62	CAVG
Maximum velocity	63	VMAX
Average velocity	64	VAVG
Right Road Section		
Flow over road	65	Q
Weir length	66	WLEN
Left edge of water	67	LEW
Right edge of water	68	REW
Maximum depth of flow	69	DMAX
Average depth of flow	70	DAVG
Average total head	71	HAVG
Average weir coefficient	72	CAVG
Maximum velocity	73	VMAX
Average velocity	74	VAVG
Approach Section Information		
Flow contraction ratio (conveyance)	75	M(K)
Geometric contraction ratio (width)	76	M(G)
Conveyance of Kq-section	77	KQ
Left edge of Kq-section	78	XLKQ
Right edge of Kq-section	79	XRKQ

 Table 5-1.
 Variables available for user-defined tables (UT record) (continued).

Heading	Code number <sup>1</sup>
ALPH	3, 15
AREA	2, 14
BETA	4, 16
BLEN	46
С	51
CAVG	62, 72
СК	33
CRWS	28
DAVG	60, 70
DMAX	59, 69
DMIN	29
EGL	25
EK	32
ERR	35
FLEN	36
FLOW	48
FR#	27
HAVG	61, 71
HF	30
НО	31
K	1, 13
KQ	77
LEW	43, 57, 67
M(G)	76
M(K)	75
OTEL	54
P/A	53
PFELEV	45
PPCD	52
Q	7, 19, 55, 65
REW	44, 58, 68
SF	34
SKEW	40
SPLT	38
SPRT	39
SRD	8, 20

Table 5-2.Alphabetized list of output headings and associated parameter code<br/>numbers for user-defined tables.

Heading	Code number <sup>1</sup>
SRDL	37
TYPE	47
VAVG	64, 74
VEL	26
VHD	6, 18
VMAX	63, 73
WLEN	56, 66
WSEL	5, 17
XLAB	49
XLKQ	78
XMAX	9, 21
XMIN	11, 23
XRAB	50
XRKQ	79
XSTW	42
XSWP	41
YMAX	10, 22
YMIN	12, 24

Table 5-2.	Alphabetized list of output headings and associated parameter code
	numbers for user-defined tables (continued).

1 Parameter code numbers in Table 5-1.

#### 5.5 CROSS-SECTION DATA

A direct-access file (logical unit 13) is used as a "work" file for storage and retrieval of the cross-section data. The model determines the definition of each cross-section from the input data sequence and constructs this file for subsequent input of cross-section data for profile computations. Each record in this file defines a single cross-section and contains supplemental data pertaining to that cross-section.

#### 5.6 SUMMARY OF PARAMETERS

Certain key input parameters and results from profile computations are stored in a directaccess file (logical unit 14) and are accessed for producing user-defined tables. The parameters currently available are discussed with user-defined tables (Section 5.4).

## **SECTION 6 - MESSAGES**

WSPRO generates two types of messages during execution; one for those messages related to input data and the second for those concerning profile computations. Each set of messages has a separate numbering sequence.

#### 6.1 INPUT DATA MESSAGES

Messages related to input data are tabulated in Table 6-1. Each message begins with +++xxx where xxx is a three-digit sequence number for table look-up convenience. The three leading plus signs provide a convenient method for scanning the output for input data messages, especially when interactively previewing the output on a terminal or microcomputer. The lower-case acronyms to the right of or on the line below the message represent variables that are output to aid the user in understanding the message and (or) solving the problem for those cases that the message indicates that a problem exists. When a problem does exist, the message attempts to explain the problem and, when possible, indicate possible user action to remedy the situation.

+++xxx MESSAG	E: pertinent discussion
+++001	WARNING: Record Type "XX" Out Of Order, Data Not
	Used.
+++002	FATAL ERROR: Record "XX" Has Invalid Format (See
	Users Manual).
+++003	CAUTION: Profile Control Data Changes After EX
	Record Ignored.
+++004	CAUTION: Unexpected continuation - record
	skipped.
+++005	WARNING: Ignoring Unrecognized Record "XX".
+++006	ERROR: Invalid Characters "xxx" in Columns 3
	through 5.
+++010	WSi below YMIN at SECID "": Used WSi = CRWS.
	YMIN, WSi, CRWS = .0 .00 .00
+++011	WSi in wrong flow regime at secid "": Used WSi =
	CRWS.
	WSi,CRWS = .00 .00
+++014	WARNING: Excess data items ignored.

Table 6-1.Input Data Messages.

+++xxx MESSA	GE :
	pertinent discussion
+++015	WARNING: Too many data items on J3 record; Only 99 items used.
+++016	CAUTION: Ignoring extraneous option/secid.
+++017	ERROR: Data count ( 99 ) inconsistent with profile count ( 88 ).
+++018	ERROR: Unknown Secid "XXXXX" (Ignoring This Data Element).
+++019	ERROR: Invalid option code ignored ( X ).
+++020	ERROR: Computation direction established by EX Record.
+++021	ERROR: Data count ( 99 ) inconsistent with profile count ( 88 ).
+++022	ERROR: Profile count ( 99 ) reduced to data count ( 88 ).
+++023	ERROR: Discharge data #99 invalid or missing.
+++024	WARNING: Profile computations may be incomplete as a result of inconsistent profile control data.
+++025	FATAL ERROR: Profile computations bypassed as a result of inconsistent profile control data.
+++026	FATAL ERROR: Invalid continuation or redefinition for record "XX".
+++027	FATAL: Data count ( 88 ) for Record "XX" exceeds maximum ( 99 ).
+++028	FATAL ERROR: Mismatch between Header Record (XX) and Data Record (YY).
+++029	ERROR: Missing flow length or breakpoint station - 99
+++030	ERROR: Invalid friction loss option code "*".
+++031	FATAL ERROR: No X/Y-Coordinates for SECID "XXXXX".
+++032	FATAL ERROR: Major reversal at X-coordinate #99.
+++033	FATAL ERROR: Total number of X and Y values is an odd value ( 99 ).
+++034	WARNING: No input data for section reference distance (assuming a value of zero).
+++035	FATAL ERROR: Bridge opening cross-section must be closed.
	X(1): 88.000 Y(1): 12345.000 X(99): 67890.000 Y(99): 24680.000

## Table 6-1.Input Data Messages (Continued).

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+++xxx MESS	AGE:
	pertinent discussion
+++036	ERROR: Culvert code ( 99 ) invalid for shape code *.
+++037	ERROR: Problem with either culvert length ( ******* ) or upstream/downstream inverts ( ****** / ****** ).
+++038	ERROR: Invalid culvert pipe-arch auxiliary dimensions: 12345.0 67890.0 24680.0
+++039	ERROR: Culvert rise ( ******* ) and/or span ( ******* ) invalid for shape code *
+++040	FATAL ERROR: Full Valley Section Not On File. Program Ending.
+++041	ERROR: Top of abutment ( 12345.000 ) outside valley limits ( 67890.000 to 24680.000 ).
+++042	ERROR: Top of abutment ( 12345.000 ) below ground surface ( 67890.000 to 24680.000 ).
+++044	ERROR: Bridge length ( ******* ) too short for the selected abutment stations ( *******, ******* ).
+++045	ERROR: Template Expansion Error; Negative Scale Factor ( ******* ).
+++046	ERROR: 12345.000 (from the GT Record) is greater than the maximum template X-coordinate ( 67890.000 ). The program will delete all ground points.
+++047	ERROR: Inconsistency with number of values ( 99, 88 ) in the roughness data.
+++048 +++049	ERROR: Cannot Propagate Required Roughness data. WARNING: Section Reference Distance Changed ( 12345.0 -> 67890.0 ).
+++050 +++051	ERROR: Section Type Code Changed ( 99 -> 88 ). ERROR: Q Record Out of Order - Ignored by Program.
+++052	ERROR: Q Record Ignored (No Room to Store It).

Table 6-1.Input Data Messages (Continued).

+++xxx N	IESSAG	F. :
		pertinent discussion
+++(	)54	ERROR: Inconsistency between number of pier/piles
		entered
		( 99 )and the number actually encountered ( 88 ).
+++(	)55	ERROR: First pier/pile elevation (12345.000) is
		below minimum elevation (67890.000). Replacing
		the input value with the minimum.
+++(	)56	ERROR: Pier/pile #99 has a negative width ( ***** ).
+++(	)57	ERROR: Pier/pile group #99 has a negative value ( **** ).
+++(	158	ERROR: Elevation reversal (12345.000 to 67890.000
1170		) encountered ( starting at pier/pile #99 ).
+++(	060	FATAL ERROR: First X-coordinate (12345.000) is
		not equal to minimum X-coordinate ( 67890.000 ).
+++(	061	FATAL ERROR: Last X-coordinate (12345.000) is
		not equal to maximum X-coordinate ( 67890.000 ).
+++(	062	FATAL ERROR: Successive X-reversals or X-reversal
		followed by vertical segment beginning at
		X-coordinate #99 cannot be adjusted.
+++(	063	FATAL ERROR: Cross-section "XXXXX" must be
		preceded by at least one other open-channel (i.e.,
		XS Header Record) cross-section.
+++(	064	FATAL ERROR: Section reference distance reversal
		between sections "XXXXX" (at srd 12345.000) and
		"XXXXX" (at srd 67890.000).
+++(	165	CAUTION: Only limited SRD and cross-section
		sequence checking performed on "XXXXX" for
+++(	166	<pre>multiple-openings. FATAL ERROR: Full-valley section "XXXXX" (at srd</pre>
+++(	000	FATAL ERROR: Full-valley section "XXXXX" (at srd 12345.000) and bridge-opening section "YYYYY" (at
		srd 67890.000) should be at the same location.
+++(	167	FATAL ERROR: Guide Bank "XXXXX" must be preceded
	507	by a bridge-opening.
+++(	168	FATAL ERROR: Road-grade "XXXXX" must be preceded
		by either a bridge-opening, guide-bank, or
		culvert.
+++(	)69	FATAL ERROR: Road-grade "XXXXX" must be preceded
		by either a bridge-opening, guide-bank, or
		culvert.

## Table 6-1.Input Data Messages (Continued).

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+++xxx MESSA	GE:				
pertinent discussion					
+++070	FATAL ERROR: Full-valley section "XXXXX" (at srd				
	12345.000) and culvert section "CCCCC" (at srd				
	67890.000) should be at the same location.				
+++072	NOTICE: X-coordinate #99 increased to eliminate				
	vertical segment.				
+++073	NOTICE: X-coordinate #99 increased to eliminate				
	negligible reversal.				
+++074	NOTICE: X-coordinates #99 and #** replaced with				
	"averaged" input values to eliminate minor				
	X-reversal. Also adjusted associated				
	Y-coordinates using linear interpolation.				
+++075	NOTICE: PW Record Replaced With PD Record (See				
	Users Manual).				
+++076	NOTICE: KD Record Replaced With KQ Record (See				
	Users Manual).				
+++077	NOTICE: BD Record Replaced With BC Record (See				
	Users Manual).				
+++078	NOTICE: AS Record Replaced With XS Record (See				
	Users Manual).				
+++079	NOTICE: SD Record Replaced With GB Record (See				
	Users Manual).				
+++081	NOTICE: PX Record Ignored. Graphics and Plotting				
	Supported Using WSSHL Program (See Users Manual).				
+++082	NOTICE: J3 Record Replaced With UT Record (See				
	Users Manual).				
+++095	ERROR: Pier Scour Analysis Missing Pier Width (DF				
	Record).				

# Table 6-1.Input Data Messages (Continued).

WSPRO uses two integer variables and the record type to trace the input data flow. These two variables, which appear as itype and jtype in some of the above messages, may be helpful in diagnosing some input problems. Table 6-2 cross references itype and jtype to the record type.

Record Type	itype	jtype	Record Type	itype	jtype	Record Type	itype	jtype
T1	1	1	HP	4	2	GR	7	1
T2		2				Ν		2
T3		3				SA		3
			WS	5	1	ND		4
UT	2	1	SK	2		FL		5
J3		3	Q		3	GT		6
						BP		8
XS	3	1	EX	6	1	BL	1	1
BR		2	ER		0	PD		12
GB		3				AB		13
XR		4				BC		14
XS		5				CD		16
CV		6				KQ		17
XT		7				CG		18
						CC		28
						···*››		29

Table 6-2.Cross-reference table for record type versus itype and jtype.

# 6.2 **PROFILE COMPUTATION MESSAGES**

Table 6-3 is a tabulation of messages associated with profile computations. Each message begins with ===xxx where xxx is the three-digit reference number for Table 6-3. The upper-case acronyms on the line(s) immediately below the message represent intermediate results that are output to provide assistance in understanding the message.

The existence of one or more messages does not necessarily indicate invalid results. Some messages are simply intended to alert the user that a potential problem exists. It is up to the user to assess the severity of such problems and to judge whether additional and (or) alternate analyses and (or) input data are warranted.

Sometimes a series of messages is printed to indicate what assumptions were made within the model and the resultant computational path followed by the model. Again, it is the user's responsibility to assess the end result and judge whether or not the assumptions and computational procedures were appropriate. If not, the user may wish to alter the input so that the model will arrive at a more appropriate solution.

Table 6-3.Profile Computation Messages.

===xxx MESSAGE pertinent variables
Comments
===010 WSI BELOW YMIN AT SECID "aaaaa": USED WSI = CRWS. YMIN,WSI,CRWS = xxxxxx.x xxxxx.xx xxxxx.xx
Starting water-surface elevation (WSI) specified on WS record is lower than the minimum ground elevation (YMIN) at the initial cross-section. Critical water-surface elevation (CRWS) is used for WSI.

===015 WSI IN WRONG FLOW REGIME AT SECID "aaaaa": USED WSI = CRWS. WSI,CRWS = xxxxx.xx xxxxx.xx

WSI, the starting water-surface elevation specified in the WS record is invalid. For upstream (subcritical) profile computations, WSI cannot be lower than CRWS, the critical water-surface elevation. For downstream (supercritical) profile computations, WSI cannot be higher than CRWS. WSI is set equal to CRWS.

Table 6-3.Profile Computation Messages (Continued).

===xxx MESSAGE	pertinent variables
Comments	

===020 SLOPE-CONVEYANCE CONVERGENCE FAILURE: USED FINAL TRIAL WS. QCOMP,WS = xxxxxxx. xxx

The iterative solution for determining the starting water-surface elevation by slope-conveyance did not converge in 20 iterations with the conveyance-error tolerance equal to one-tenth of one percent. The last trial elevation, WS, is used for the starting elevation. This problem may arise due to the convergence criteria being too stringent. In that case the computed discharge, QCOMP, will be essentially equal to the specified discharge and the computed profile should be acceptable. Otherwise there are probably cross-section coding errors that require correction.

===025 SLOPE-CONVEYANCE DISCHARGE OUT OF RANGE. WSMIN,QMIN,WSMAX,QMAX = xxxxx.xx xxxxxxx. xxxxxxx.

The user-specified discharge in the Q record is outside the possible range of discharges at the initial cross-section. Discharge limits are computed using the user-specified slope in the SK record. The upper and lower discharge limits, QMIN and QMAX, are computed for water-surface elevations WSMIN, 0.076 m above channel bottom, and WSMAX, 0.006 m below the maximum cross-section elevation. Message ===010 will follow because the model arbitrarily defaults to an elevation lower than minimum ground elevation.

===105 WSMIN BELOW YMIN AT SECID "aaaaa": USED WSMIN = CRWS. YMIN,WSMIN,CRWS = xxxxxx.x xxxxx.xx xxxxx.xx

WSMIN is the lower limit of trial water-surface elevations to be used in the attempt to balance the energy equation. During upstream (subcritical) profile computations, the initial WSMIN is based on the water-surface elevation of the previous cross-section. When the initial WSMIN is below the minimum ground elevation of the cross-section the model defaults to using CRWS, the critical water-surface elevation, for WSI.

===110 WSEL NOT FOUND AT SECID "aaaaa": REDUCED DELTAY. WSLIM1,WSLIM2,DELTAY = xxxxx.xx xxxxx.xx xx.xx

No sign change found in the energy-balance error between elevation limits WSLIM1 and WSLIM2. A smaller value of DELTAY, the elevation increment, might yield a sign change. A new DELTAY equal to half of the original (specified or default) value is used. For downstream computations the same elevation range will be investigated; see message ===115 for upstream computations.

===115 WSEL NOT FOUND AT SECID "aaaaa": USED WSMIN = CRWS. WSLIM1,WSLIM2,CRWS = xxxxx.xx xxxxx.xx xxxxx.xx

For upstream computations, in addition to reducing DELTAY, the lower elevation limit, WSMIN, will be set equal to the critical water-surface elevation, CRWS.

Table 6-3.Profile Computation Messages (Continued).

===xxx	MESSAGE	pertinent variables	-
Comment	ts		
===120	YTOL NOT	SATISFIED AT SECID "aaaaa": TRIALS CONTINUED. YTOL,WSLIM1,WSLIM2 = xx.xx xxxxx.xx xxxxx.xx	

A sign change in the energy-balance error occurred between WSLIM1 and WSLIM2. However, the iterative solution (method of false position) failed to find an energy balance meeting the elevation tolerance, YTOL, for any elevation between WSLIM1 and WSLIM2. For upstream [downstream] computations, the old upper [lower] limit becomes the new lower [upper] limit for continuing trials for a solution at higher [lower] elevations. A larger YTOL value may be required for high-gradient, high-velocity, or not-so-gradually-varied flow situations.

===125 FR# EXCEEDS FNTEST AT SECID "aaaaa": TRIALS CONTINUED. FNTEST,FR#,WSEL,CRWS = xx.xx xx.xx xxxxx.xx xxxxx.xx

WSEL, a water-surface elevation which balances the energy equation, has a computed Froude number, FR#, greater than FNTEST, the Froude number test value (default value of 0.8 or user-specified on the J1 record). WSPRO rejects WSEL and attempts to find a valid solution at a higher elevation. The model uses a new lower elevation limit which is the higher of 1) WSEL plus five times the tolerance, YTOL; or 2) the critical water-surface elevation, CRWS.

===130 CRITICAL WATER-SURFACE ELEVATION <u>ASSUMED</u> !!!!! ENERGY EQUATION <u>NOT BALANCED</u> AT SECID "aaaaa" WSBEG, WSEND, CRWS = xxxxx.xx xxxxx.xx xxxxx.xx

No satisfactory solution for WSEL was found between elevation limits WSBEG and WSEND at the section with SECID aaaaa. Instead of aborting profile computations completely, WSPRO assumes critical water-surface elevation, CRWS, at that section and attempts to perform profile computations for subsequent sections. The user must decide what further action (if any) is needed to obtain meaningful profile results.

===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS. SECID "aaaaa" KRATIO = xx.xx

KRATIO is the conveyance of the section with SECID aaaaa divided by the conveyance of the preceding section in the profile computations. This section-to-section conveyance ratio should be between 0.7 and 1.4 to ensure reliable profile computations. Additional cross-sections should be considered if the ratio significantly violates the recommended criteria, especially if friction losses between the two sections are significant.

 Table 6-3.
 Profile Computation Messages (Continued).

===xxx MESSAGE pertinent variables						
Comments						
===140 AT SECID "aaaaa":	END OF Cross-section EXTENDED VERTICALLY. WSEL,YLT,YRT = xxxxx.xx xxxxxx.x xxxxxx.x					

Either YLT or YRT (ground elevations at the section end points) was lower than the computed water-surface elevation, WSEL. No action is required if vertical extension of the section adequately defines cross-sectional geometry.

===195 PROFILE COMPUTATIONS <u>TERMINATED</u> !!!!! IPR = nn CHECK INPUT ERROR MESSAGES FOR SECID "aaaaa"

An input data problem has caused an error flag to be set for the cross-section with SECID aaaaa. If profile computations were allowed to proceed, severe (possibly irrecoverable) computational problems might occur. Therefore, the profile computations are terminated at the previous section. Check the input data interpretation output for that section (and perhaps adjacent sections) for input data error messages (also consult Table 7-1). IPR indicates the number of the profile being computed.

===210 QUESTIONABLE CRITICAL-FLOW SOLUTION. SECID "aaaaa" Q,CRWS = xxxxx.xx xxxxx.xx

The elevation of minimum specific energy coincides (within tolerance YTOL) with the maximum elevation in the cross-section with SECID aaaaa. Thus, it is highly unlikely that CRWS is a valid critical water-surface elevation for the discharge, Q. Computations continue and subsequent assumptions and actions within the model may yield a valid solution. However, the user should carefully scrutinize the results (if any) to determine their validity.

===215 FLOW CLASS 1 SOLUTION INDICATES POSSIBLE ROAD OVERFLOW. WS1,WSSD,WS3,RGMIN = xxxxx.xx xxxxx.xx xxxxx.xx xxxxx.xx

WS1, WSSD, and WS3 are the computed water-surface elevations for flow class 1 at the approach, grade-bank (if any), and bridge-opening sections. WS1 is higher than minimum road elevation, RGMIN. A flow class 4 solution will be attempted (message ===265 will follow).

===220 FLOW CLASS 1 (4) SOLUTION INDICATES POSSIBLE PRESSURE FLOW. WS3,WSIU,WS1,LSEL = XXXXX.XX XXXXXXXX XXXXXXXX XXXXXXXXX

For free-surface flow (flow class 1 or 4), the water-surface elevation immediately upstream from the bridge, WSIU, is higher than the low-steel elevation, LSEL. WSPRO assumes that pressure flow (flow class 2 or 5) might occur. The flow class 1 or 4 elevations at the tailwater and approach sections, WS3 and WS1, are printed for evaluation. Message ===245 will follow unless LSEL is undefined, in which case the flow class 1 or 4 results will be used.

 Table 6-3.
 Profile Computation Messages (Continued).

===xxx MESSAGE	pertinent variables
Comments	
===225 NO ENERGY	BALANCE IN 15 ITERATIONS. FLOW,Q = n xxxxxxx. WS1,WSSD,WS3 = xxxxx.xx xxxxx.xx xxxxx.xx
The iterative simultance	us solution of the energy helence for free surface bridge flow foiled to converge. The variab

The iterative, simultaneous solution of the energy balance for free-surface bridge flow failed to converge. The variables printed indicate the flow class, FLOW, flow through the bridge, Q, and water-surface elevations at the approach, grade-bank (if any), and bridge-opening sections, WS1, WSSD, and WS3, computed on the last iteration. Further action(s) by the model are indicated by additional ===**xxx** messages.

===230 REJECTED FLOW CLASS 1 SOLUTION. WS1,WSSD,WS3 = xxxxx.xx xxxxx.xx xxxxx.xx CRWS = xxxxx.xx xxxxx.xx xxxxx.xx YMAX = xxxxx.xx xxxxx.xx xxxxx.xx

All efforts to compute free-surface flow without road overflow failed. Subsequent ===xxx messages will indicate actions taken by the model. Low-steel elevation and road-grade data influence the action(s) taken. The three lines of variables printed indicate the final trial water-surface elevations, critical water-surface elevations, and maximum ground elevations at (from left to right) the approach, grade-bank and bridge-opening sections. One common cause of this problem is that YMAX of the approach section (which is the upper elevation limit for bridge computations) is too low to permit a solution.

===235 CONTINUE FLOW CLASS 4 COMPUTATIONS. ITER,QRD = nn xxxxxxx. WS,WSMIN,WSMAX = xxxxx.xx xxxxx.xx xxxxx.xx

When an energy balance failure (see message ==225) for the free-surface bridge flow occurs during class 4 flow computations, the model assumes that too much flow is being put through the bridge and that more road overflow is needed. Therefore, iterations are continued using the last trial elevation WS, as the new lower limit. The other variables are: ITER, the current iteration number; QRD, the computed flow over the road; and WSMIN and WSMAX, the previous elevation limits.

===240 NO DISCHARGE BALANCE IN 15 ITERATIONS. WS,QBO,QRD = xxxxx.xx xxxxxxx. xxxxxxxx.

Variables printed are the water-surface elevation for the last iteration, WS, and computed flows through the bridge, QBO, and over the road, QRD. The problem could be that the discharge tolerance is too stringent (in which case the sum of QBO and QRD would compare favorably with the specified discharge). In that case, increase QTOL (in the J1 record) and recompute. Otherwise it is likely that data input or design problems must be overcome. Further action(s) by the model are indicated by additional ===**xxx** messages.

Table 6-3.Profile Computation Messages (Continued).

===xxx MESSAGE

---- pertinent variables ----

Comments

===245 ATTEMPTING FLOW CLASS 2 (5) SOLUTION.

Indicates that WSPRO will attempt to find a solution for flow class 2 or 5. If a satisfactory solution is not found, message ===270 will follow.

===250 INSUFFICIENT HEAD FOR PRESSURE FLOW. YU/Z,WSIU,WS1 = x.xx xxxxx.xx xxxxx.xx

YU/Z, the ratio of mean depths in the approach and bridge-opening sections, is less than 1.10 (arbitrary lower limit of assumed pressure flow). WSPRO assumes that flow class 1 or 4 results (if any) are applicable. Flow class 2 or 5 elevations immediately upstream of the bridge, WSIU, and at the approach section, WS1, are printed for user evaluation (see message ===270).

===255 ATTEMPTING FLOW CLASS 3 (6) SOLUTION. WS3N,LSEL = xxxxx.xx xxxxx.xx

The tailwater elevation, WS3N, is higher than the low-steel elevation, LSEL. WSPRO assumes that flow class 3 or 6 will occur (submerged pressure flow). A flow class 1 or 4 solution may be forced by coding LSEL higher than actual low-steel elevation. It may be advisable to obtain both pressure and free-surface solutions for border line cases. A comparison of results can be used to select the most appropriate solution.

===260 ATTEMPTING FLOW CLASS 4 SOLUTION.

This message follows ===225 if road-grade data have been input, the assumption being that free-surface flow may occur in conjunction with road overflow. It is also printed after class 1 flow solutions if those results indicate possible road overflow.

===265 ROAD OVERFLOW APPEARS EXCESSIVE. QRD,QRDMAX,QRATIO = xxxxxxx. xxxxxx. xx.xx

QRATIO is QRD, the computed road overflow, divided by QRDMAX, an estimated maximum road overflow. QRDMAX is the total discharge multiplied by the conveyance of the horizontal slice of water in the approach section that is higher than minimum road-grade elevation and divided by the total approach-section conveyance. Consideration should be given to treating the road grade and bridge as a composite section since weir flow may not actually occur.

===270 REJECTED FLOW CLASS 2 (5) SOLUTION.

Flow class 2 or 5 results are either unacceptable, as per message ===250, or a solution is not obtained. In the former case, if flow class 1 or 4 results have been obtained, those results are assumed most applicable and are reflected in profile output and subsequent profile computations. In the latter case, or if flow class 1 or 4 results were not obtained, message ===295 will follow.

Table 6-3.Profile Computation Messages (Continued).

===xxx MESSAGE

---- pertinent variables ----

Comments

===275 REJECTED FLOW CLASS 3 (6) SOLUTION.

Similar to message ==280 except that class 3 or 6 computations have failed and the preceding successful computations may be either class 1 or 4 flow.

===280 REJECTED FLOW CLASS 4 SOLUTION.

Class 4 flow computations were unsuccessful (see message ===240). Preceding computations for class 1 flow were successful but indicated probable road overflow (see message ===225). The model will continue profile computations using the class 1 flow results instead of terminating computations. The user should scrutinize such results very closely because it is very possible that data input or design problems are causing the computational problems.

===285 CRITICAL WATER-SURFACE ELEVATION <u>ASSUMED</u> !!!!! SECID "aaaaa" Q,CRWS = xxxxxxx. xxxxx.xx

At the section with SECID of aaaaa, for the discharge, Q, a solution could not be found in the subcritical flow regime. Critical water-surface elevation, CRWS, is assumed so that computations may continue. If CRWS is reflected in the final solution, the results should be given very close scrutiny to determine the validity of the analyses. Downstream profile computations may be advisable to determine if supercritical flow occurs at such sections.

===295 PROFILE COMPUTATIONS <u>TERMINATED</u> !!!!! IPR = nn BRIDGE FLOW COMPUTATIONS <u>FAILED</u> !!!!!

All attempts to obtain acceptable bridge results have failed and the profile computations are terminated. IPR is the number of the profile being computed.

# **SECTION 7 - EXAMPLES OF MODEL INPUT AND OUTPUT**

Examples of various WSPRO applications are presented in this section. The examples do not provide much background detail about the input data (e.g., maps, cross-section plots, etc.) nor do they present in-depth discussions of all aspects of the related hydraulic analyses. The primary intent of the examples is to illustrate WSPRO input-data sequences and output.

The illustrations in this section show sample input and output data. To reduce the bulk of this manual, many listings have been shortened by deleting parts of the output. Generally, a note has been inserted to indicate where a deletion has been made.

## 7.1 EXAMPLE #1: SIMPLE, OPEN-CHANNEL REACH

Figure 7-1 illustrates input data for five sections which describe a stream reach in which there are no bridges or culverts. An XS header record must be coded for each section. The geometry of each section is defined by GR data. Each section is subdivided into three subareas for horizontal variation of roughness. The three roughness coefficients specified in the N record are propagated to each of the other sections because no other N and (or) ND data are coded. The x-coordinates of the subdivision breakpoints do vary from section to section as reflected in the SA records. A single profile is computed for the discharge specified in the Q record. The initial water-surface elevation is computed by slope conveyance using the energy gradient coded in the SK record.

```
Т1
             Example 1 - Some Creek Near Anywhere, USA
т2
             Simple Open-Channel Profile Example
т3
             Constant Discharge Subcritical Flow
*
SI 1
                283.168
Q
SK
                0.0023
      SEC-A 30.480
XS
                32.614,189.829 40.234,186.263 51.816,183.429 57.912,181.051
75.590,179.984 82.601,179.984 89.306,179.862 94.488,179.984
96.926,180.777 103.022,181.630 106.680,183.032 112.166,184.038
GR
GR
GR
                115.824,184.404 121.920,184.465
GR
Ν
                0.065 0.027
                                        0.065
                     51.816 112.166
SA
XS
      SEC-B 64.008
              43.891,189.616
              43.891,189.61655.474,184.55662.484,181.50866.751,180.92967.666,180.96083.820,179.80290.526,179.77196.317,179.832
GR
GR
              105.156,180.015 108.814,180.960 117.043,184.130 117.348,184.465
GR
GR
              121.920 184.587
SA
               55.474 117.043
XS
      SEC-C 114.300
              60.046,188.793 68.275,185.440 75.286,181.661 83.515,180.289
84.125,179.680 89.611,179.619 92.964,179.375 96.622,179.253
GR
GR
```

GR GR SA *		99.670,179.375 107.594,179.741 108.204,180.685 113.995,181.722 119.786,183.916 121.006,184.282 121.920,184.313 68.275 121.006
XS GR GR GR SA *	SEC-D	152.400 67.056,188.702 73.152,184.008 77.114,180.899 83.210,180.807 84.125,180.076 89.916,180.015 95.098,180.015 96.012,179.527 99.974,179.558 103.632,179.588 103.937,180.076 113.386,181.295 118.567,183.916 121.920,184.008 73.152 117.043
XS GR GR SA * EX ER	SEC-E	195.072 62.179,188.671 73.152,180.777 78.638,180.807 84.125,180.533 89.306,180.472 94.793,180.472 100.279,180.381 105.766,180.746 111.252,180.685 117.043,183.185 121.920,183.490 122.225,185.318 75.895 111.343

Figure 7-1. Input data for the simple, open-channel reach of example #1.

Figure 7-2 illustrates the output for this example. The output summarizes the input data with an echo of each input data records. After finding a section header record, WSPRO continues input of subsequent records until a record type other than GR, N, ND, SA, FL, or \* is found. WSPRO suspends input at that point to process the section input data. Any missing data are obtained from the previous section. Data for the current section are then summarized in a neat, tabular format. WSPRO's subsequent actions depend on the record type that initiated processing of the section data. The possibilities are: (1) another header record will cause it to repeat the above procedure; (2) a display command (HP record) will result in generation of the output requested by that command; (3) a Q record would indicate that discharge is to be changed at this section and the values would be stored accordingly; (4) an EX record will result in profile computation and output; and (5) an ER record would cause the model to come to a normal stop. Any other record type would be invalid at this point and would cause generation of an error message. Part of the desired output may be obtained despite the error, the amount depending on where the problem occurs.

	Model for Wat Run Date & Time: Input File: EX1.D	** W S P R O ****** nistration - U. S. er-Surface Profile Co 9/9/97 9:07 am V AT Output File	Geological Survey mputations. ersion V070197 : EX1.LST
	EXAMPLE 1 - SOME SIMPLE OPEN-CHAN CONSTANT DISCHAR L	CREEK NEAR ANYWHERE, NEL PROFILE EXAMPLE	USA
	Quantity	SI Unit	Precision
	Velocity Discharge Slope Angles	<pre>meters meters meters/second cubic meters/second meter/meter</pre>	0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001 0.001
Q	283.168		

\*\*\* Processing Flow Data; Placing Information into Sequence 1 \*\*\*

SK 0.0023

Federa	**************************************	nistration er-Surface B Metric / C	- U. S. Geol Profile Comput Output Units:	ogical Surv ations. Metric	ey
	EXAMPLE 1 - S SIMPLE OPE	OME CREEK NE N-CHANNEL PF	CAR ANYWHERE, ROFILE EXAMPLE SCRITICAL FLOW	USA	
*	Starting T	o Process He	eader Record S	EC-A *	
GR 11	30.480 32.614,189.829 .590,179.984 .926,180.777 1 5.824,184.404 0.065 0.027 51.816 1	121.920,184 0.065	5.263 51.816, 984 89.306,17 530 106.680,18 1.465	183.429 57 9.862 94.4 3.032 112.1	.912,181.051 88,179.984 66,184.038
	eted Reading D g X-Section Da				
*** SRD Locatio Valley Slop Energy Loss	Data Sum n: 30. e: .00000 Coefficients	Cross-Secti Averaging (	ader Record SE on Skew: . Conveyance By .on: .50 C	0 Error C Geometric M	ode 0 ean.
Х	Y		Y	х	Y
32.614 57.912 89.306 103.022 115.824	189.829 181.051 179.862 181.630 184.404	40.234 75.590 94.488 106.680 121.920	186,263	51.816 82.601 96.926 112.166	183.429 179.984 180.777 184.038
Minimum X- Maximum X- Minimum Y- Maximum Y-	Minimum a Station: Station: 1 Elevation: 1 Elevation: 1	32.614 ( as 21.920 ( as 79.862 ( as	X,Y-coordinate ssociated Y-El ssociated Y-El ssociated X-St ssociated X-St	evation: 1 evation: 1 ation:	89.829 ) 84.465 ) 89.306 ) 32.614 )
	SubArea	Roughness Coefficient	3 SubAreas ) Horizontal Breakpoint		
	1	.065	  51.816	-	
	2	.027	112.166		
	3	.065		_	
* * *	Finished P	rocessing He	eader Record S	* EC-A *	

	Model for Wa Input Units:	inistration ter-Surface I Metric / (	Dutput Units:	tations. Metric	
	EXAMPLE 1 - SIMPLE OF CONSTANT	SOME CREEK NE EN-CHANNEL PE DISCHARGE SUE	EAR ANYWHERE, ROFILE EXAMPL BCRITICAL FLO	USA E W	
	Starting	To Process He	eader Record	SEC-B	*
GR 67.6 GR 105 GR 121 SA 5!	3.891,189.616 566,180.960 156,180.015 920 184.587 5.474 117.04		302 90.526,1 960 117.043,1	79.771 96. 84.130 117.	317,179.832 348,184.465
*** Storing	g X-Section D	Data Associat Data In Tempor	ary File As	Record Numb	er 2 ***
Valley Slope	e: .00000	mmary For Hea Cross-Secti Averaging C -> Expansi	Conveyance By	Geometric I	Mean.
x	Х, Y- Ү	coordinates ( X	(13 pairs) Y	Х	Y
43.891 66.751 90.526 108.814 121.920	189.616 180.929 179.771 180.960 184.587	55.474 67.666 96.317 117.043	184.556 180.960 179.832 184.130	62.484 83.820 105.156 117.348	181.508 179.802 180.015 184.465
Minimum Y-H	Station: Station: Elevation: Elevation: Rough SubArea	Coefficient	ssociated Y-E ssociated Y-E ssociated X-S ssociated X-S 3 SubAreas ) Horizonta Breakpoin	<pre>levation: levation: tation: tation: l t</pre>	90.526 )
	1	.065	  FF 474		
	2	.027	55.474  117.043		
	3	.065			

Federa	l Highway Adm Model for Wa Input Units:	inistration ter-Surface I Metric / (	0 ********* - U. S. Geo Profile Comput Dutput Units:	logical Sur tations. Metric	vey			
	** EXAMPLE 1 - SOME CREEK NEAR ANYWHERE, USA SIMPLE OPEN-CHANNEL PROFILE EXAMPLE CONSTANT DISCHARGE SUBCRITICAL FLOW							
*	Starting	To Process He	eader Record S	SEC-C	*			
GR     84.       GR     99.       GR     119       SA     6	0.046,188.793 125,179.680 670,179.375 1 .786,183.916 8.275 121.00	89.611,179.63 07.594,179.74 121.006,184.3	.440 75.286,3 19 92.964,179 41 108.204,180 282 121.920,18	9.375 96.6 ).685 113.9 34.313	522,179.253 95,181.722			
			ted With Heade rary File As H					
Valley Slop	n: 114. e: .00000	Cross-Sect: Averaging (	ader Record SI ion Skew: Conveyance By ion: .50 (	.0 Error Geometric	Mean.			
X	Х, Ү- Ү	coordinates X	Ŷ	Х	Y			
119.786	188.793 180.289 179.375 179.741 183.916		185.440 179.680 179.253 180.685 184.282		181.661 179.619 179.375 181.722 184.313			
Minimum X- Maximum X- Minimum Y- Maximum Y-	Minimum and Maximum X,Y-coordinates Minimum X-Station: 60.046 (associated Y-Elevation: 188.793) Maximum X-Station: 121.920 (associated Y-Elevation: 184.313) Minimum Y-Elevation: 179.253 (associated X-Station: 96.622) Maximum Y-Elevation: 188.793 (associated X-Station: 60.046)							
Roughness Data ( 3 SubAreas ) Roughness Horizontal SubArea Coefficient Breakpoint								
1 .065 68.275								
2 .027 121.006								
	3	.065						
* * *	Finished		eader Record &		*			

*	Model for Wat Input Units:	Metric / O	Output Units:	Metric	*
	EXAMPLE 1 - S SIMPLE OPE	SOME CREEK NE EN-CHANNEL PE		USA E	
*	Starting T	o Process He	eader Record	SEC-D	*
R 84. R 99. R 118 A 7	52.400 7.056,188.702 125,180.076 8 974,179.558 10 .567,183.916 1 3.152 117.043 eted Reading I	89.916,180.01 03.632,179.58 21.920,184.0	L5 95.098,18 38 103.937,18 )08	0.015 96.0 0.076 113.3	12,179.527 86,181.295
*** SRD Locatio Valley Slop	g X-Section Da Data Sum n: 152. e: .00000 Coefficients	mary For Hea Cross-Secti Averaging (	ader Record S ion Skew: Conveyance By	EC-D .0 Error Geometric	*** Code O Mean.
х	Y	coordinates ( X	Ŷ	Х	Y
67.056 83.210 95.098 103.632	Y 188.702 180.807 180.015 179.588 183.916	X 73.152 84.125 96.012 103.937 121.920	Ŷ		
67.056 83.210 95.098 103.632 118.567 Minimum X- Maximum X-	Y 188.702 180.807 180.015 179.588 183.916 Minimum a Station: Station: Elevation: 1 Elevation: 1 Elevation: 1 Roughr SubArea	X 73.152 84.125 96.012 103.937 121.920 and Maximum 2 67.056 ( as 21.920 ( as .79.527 ( as .88.702 ( as hess Data ( Roughness Coefficient	Y 184.008 180.076 179.527 180.076 184.008 (,Y-coordinat ssociated Y-E ssociated Y-E ssociated X-S ssociated X-S 3 SubAreas ) Horizonta Breakpoin	77.114 89.916 99.974 113.386  es levation: levation: tation: tation: 1 t	180.899 180.015 179.558 181.295 188.702 ) 184.008 )
67.056 83.210 95.098 103.632 118.567 	Y 188.702 180.807 180.015 179.588 183.916 Minimum a Station: Station: Elevation: 1 Elevation: 1 Elevation: 1 Roughr SubArea	X 73.152 84.125 96.012 103.937 121.920 and Maximum 2 67.056 ( as 21.920 ( as .79.527 ( as .88.702 ( as hess Data ( Roughness Coefficient	Y 184.008 180.076 179.527 180.076 184.008 X,Y-coordinat ssociated Y-E ssociated Y-E ssociated X-S 3 SubAreas ) Horizonta Breakpoin	77.114 89.916 99.974 113.386 	180.899 180.015 179.558 181.295 188.702 ) 184.008 )
67.056 83.210 95.098 103.632 118.567 	Y 188.702 180.807 180.015 179.588 183.916 Minimum a Station: Station: Elevation: 1 Elevation: 1 Elevation: 1 Roughr SubArea	X 73.152 84.125 96.012 103.937 121.920 and Maximum X 67.056 ( as 21.920 ( as .21.920 ( as .88.702 ( as .88.702 ( as .88.702 ( as .88.702 ( as .065	Y 184.008 180.076 179.527 180.076 184.008 (,Y-coordinat ssociated Y-E ssociated Y-E ssociated X-S ssociated X-S 3 SubAreas ) Horizonta Breakpoin	77.114 89.916 99.974 113.386 	180.899 180.015 179.558 181.295 188.702 ) 184.008 )

Federal	l Highway Adm Model for Wa Input Units:	ninistration ater-Surface I Metric / 0	O ********* - U. S. Geo Profile Compu Output Units:	logical Sun tations. Metric	rvey
	EXAMPLE 1 - SIMPLE OF	SOME CREEK NI PEN-CHANNEL PI DISCHARGE SUI	EAR ANYWHERE, ROFILE EXAMPL BCRITICAL FLO	USA E W	
* * *		To Process H	eader Record	SEC-E	*
GR 89.3 GR 111	2.179,188.671 306,180.472	94.793,180.4 117.043,183.	.777 78.638, 72 100.279,18 185 121.920,1	0.381 105.	766,180.746
			ted With Head rary File As		
*** SRD Location Valley Slope Energy Loss	195. 195.	Cross-Sect: Averaging (	ader Record S ion Skew: Conveyance By ion: .50	.0 Error Geometric	*** Code 0 Mean. n: .00
х	Y	coordinates X	Ŷ	х	Y
			180.777 180.472 180.746 183.490		
Minimum X-S Maximum X-S Minimum Y-H Maximum Y-H	Station: Station: Elevation: Elevation: Rough	62.179 ( a: 122.225 ( a: 180.381 ( a: 188.671 ( a: nness Data (	X,Y-coordinat ssociated Y-E ssociated Y-E ssociated X-S ssociated X-S 3 SubAreas ) Horizonta t Breakpoin	levation: levation: tation: tation:	188.671 ) 185.318 ) 100.279 ) 62.179 )
	1 2		75.895		
	3	.065	111.343 		
* * *			eader Record		- * * - *

		Model	********* W S P ay Administration for Water-Surface Units: Metric /	n - U.S.G e Profile Com	putations.					
EX	* EXAMPLE 1 - SOME CREEK NEAR ANYWHERE, USA SIMPLE OPEN-CHANNEL PROFILE EXAMPLE CONSTANT DISCHARGE SUBCRITICAL FLOW									
			ry of Boundary Co							
	#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime					
	 	283.17	 ********	.0023	Sub-Critical					
			ginning 1 Profi	le Calculation	n(s) *					

**************************************								
EXAMPLE 1 - SOME CREEK NEAR ANYWHERE, USA SIMPLE OPEN-CHANNEL PROFILE EXAMPLE CONSTANT DISCHARGE SUBCRITICAL FLOW << Beginning Computations for Profile 1 >>								
	EGEL	VHD HF HO		AREA K SF	SRDL FLEN ALPHA	REW		
Section: SEC-A Header Type: XS SRD: 30.480	182.532 182.928 181.963	* * * * * *	283.168 2.782 .631	5902.40	******** ********* 1.000	54.112 105.378 *****		
Section: SEC-B Header Type: XS SRD: 64.008		.060	283.168 2.356 .503	7551.27	33.528	113.343		
Section: SEC-C Header Type: XS SRD: 114.300		.077	283.168 2.695 .553	6864.35				
Section: SEC-D Header Type: XS SRD: 152.399	182.776 183.217 182.092	.073	283.168 2.938 .617	96.356 6080.87 .0019	38.099	116.315		
Section: SEC-E Header Type: XS SRD: 195.072		.106	283.168 3.042 .747	5278.85		116.198		

<< Completed Computations of Profile 1 >>

ER

#### Output data. Figure 7-2.

The user should look for input data messages identified by "+++" and consult Table 6-1 for explanation(s) and possible solution(s) for any problem(s) that might exist. When previewing output on a terminal or microcomputer screen, it is convenient to use a "search" command to detect any +++ string(s).

Output for profile computation headings are defined in Section 5 and (or) Appendix A. As previously discussed, any modeling effort should include a thorough review of this output. Inconsistencies from section to section, unreasonable and (or) unexpected parameter values, results not in agreement with pre-conceived notions and (or) sound engineering judgment, etc., may indicate that additional and (or) modified input data are needed for the analysis. The final lines reflect the end of run with normal termination.

# 7.2 EXAMPLE #2: SINGLE-OPENING BRIDGE (COORDINATE MODE)

Figure 7-3 illustrates input data for a simple, single-opening bridge analysis. The minimum number of sections (see Figure 3-8) are shown in this example. Optional grade-bank and road-grade sections are not required if grade banks are not present and there is no possibility of road overflow.

```
т1
          Example 2: Dry Creek Near Barren Hills
т2
          Simple Bridge Example Fixed-geometry Mode
SI 1
          147.248
Q
          336.621
WS
*
 Exit Cross-Section - GR Data From X,Y Coordinates
XS
     EXIT 14554.200
GR
            0.000,337.597
                            21.336,337.078
                                              44.196,336.652
                                                               49.073,335.128
                            67.970,334.701
                                                               74.066,334.396
           56.998,334.183
                                              71.018,334.518
GR
                                                               93.878,333.512
GR
           78.638,333.634
                            83.210,333.329
                                              87.782,333.329
GR
           99.974,333.573
                           104.851,334.335
                                             106.680,334.609
                                                              111.862,334.457
GR
          118.872,334.274
                           124.054,333.939
                                             131.978,333.939
                                                              136.246,333.543
          141.732,333.390
                                             149.047,334.975 156.972,335.128
                           148.133,334.335
GR
GR
          167.335,335.890
                           172.822,337.718
Ν
          0.040
*
*
 Full-Valley Cross-Section - All Data Propagated From Exit Cross-Section
XS
     FULLV 14599.920
*
  Bridge Section - Component Mode
*
     BRIDG 14599.920 337.810
                                30.000
BR
           0.000,337.810
                           0.000,336.956
                                            4.877,335.036 15.240,333.177
GR
GR
          20.726,333.238
                          28.956,333.299
                                           35.662,334.396 41.758,336.591
          41.758,337.383
                           0.000,337.810
GR
          333.451,0.701,1
                           336.895,0.701,1 336.895,1.829,2 337.810,1.829,2
PD
CD
          2 18.410 1.000 338.785
AB
             *
                336.956 336.591
*
XS
     APPRO 14660.880
            0.000,337.322
                            22.860,337.017
                                              42.672,336.743
                                                               50.292,335.311
GR
           56.388,334.244
                            68.580,334.762
                                              71.628,334.518
                                                               74.676,334.488
GR
           79.248,333.665
                            83.820,333.360
                                              88.392,333.360
                                                               94.488,333.573
GR
          100.584,333.573
                           105.156,334.396
                                             106.680,334.670
                                                              112.776,334.518
GR
                                                              137.160,333.604
          118.872,334.335
                           123.444,334.000
                                             132.588,334.000
GR
GR
          141.732,333.451
                           147.828,334.396
                                             149.352,335.036
                                                              156.972,335.280
GR
          167.640,335.890
                           175.260,338.023
* BP Record Used To Adjust Horizontal Datum Between Bridge and Approach
          68.580
BP
*
ΕX
ER
```

Figure 7-3. Input data.

A single profile is computed for the specified discharge (shown in the Q record) using the water-surface elevation in the WS record. Exit-section geometry and roughness are propagated

to the full-valley section. Because valley slope is not coded in either XS header record, elevations are projected horizontally. Bridge-opening data begins with the required BR header record. WSPRO recognizes coordinate mode because GR data are used instead of BL and BC records and data to define bridge geometry. Missing roughness data are propagated from the fullvalley section. Because it is a type 2 opening (BRTYPE = 2 in the CD record), an AB record is required to specify abutment toe elevations (see Table 4-3). In the PD data note that: (1) the PPCD value of 0 indicates pier data; and (2) the last data pair is redundant (see Table 4-33 and Figure 4-16). Although the PW record is obsolete (replaced by the PD record), a PW record has been coded in this example to illustrate WSPRO's ability to detect and handle obsolete records. Approach-section data begins with the required XS header record and geometry is defined by GR data. Missing roughness data are propagated from the full-valley section (bridge data are never propagated). The first x-coordinate in both the bridge and the approach section is zero, which is only possible if the bridge opening is fully eccentric to the left. Assuming that is not the case here, a horizontal-datum problem exists. A BP record located with the approach section data provides the correction. A line projected parallel to the flow, from the minimum x-coordinate in the bridge (zero in this case), intersects the approach section at x-coordinate 68.580 (see Table 4-7 and Figure 4-5). Figure 7-4 shows a portion of the input-data-processing output for the bridge opening. Profile output is shown in Figure 7-5.

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_\_\_\_\* EXAMPLE 2: DRY CREEK NEAR BARREN HILLS SIMPLE BRIDGE EXAMPLE FIXED-GEOMETRY MODE \*\_\_\_\_\_\* Starting To Process Header Record BRIDG \*\_\_\_\_\_\* BR BRIDG 14599.920 337.810 30.000 0.000,337.810 0.000,336.956 4.877,335.036 15.240,333.177 GR 0.726,333.238 28.956,333.299 35.662,334.396 41.758,336.591 1.758,337.383 0.000,337.810 GR 2 GR 4 333.451,0.701,1 336.895,0.701,1 336.895,1.829,2 337.810,1.829,2 2 18.410 1.000 338.785 PD CD \* 336.956 336.591 AB \* \* \* Completed Reading Data Associated With Header Record BRIDG \* \* \* \*\*\* No Roughness Data Input, Propagating From Previous Section \* \* \* +++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment. +++072 NOTICE: X-coordinate # 9 increased to eliminate vertical segment. \* \* \* Storing Bridge Data In Temporary File As Record Number 3 \*\*\* Data Summary For Bridge Record BRIDG SRD Location: 14600. Cross-Section Skew: 30.0 Error Code Valley Slope: \*\*\*\*\*\* Averaging Conveyance Br Commission \* \* \* 0 Averaging Conveyance By Geometric Mean. Energy Loss Coefficients -> Expansion: .50 Contraction: .00 X,Y-coordinates (10 pairs) X Y X Y X Y .000 337.810 .100 336.956 4.877 335.036 15.240 333.177 20.726 333.238 28.956 333.299 35.662 334.396 41.758 336.591 41.858 337.383 \_\_\_\_\_

.000 337.810	
Minimum and Maximum X,Y-coordinatesMinimum X-Station:.000 ( associated Y-Elevation: 337.810Maximum X-Station:41.858 ( associated Y-Elevation: 337.383Minimum Y-Elevation:333.177 ( associated X-Station: 15.240Maximum Y-Elevation:337.810 ( associated X-Station: .000	) ) ) )
X-coordinates & Horizontal Breakpoints Translated by Skew Angl X Input X Skewed X Input X Skewed X Input X Ske	wed
.000         2.042         .100         2.128         4.877         6           15.240         15.240         20.726         19.991         28.956         27           35.662         32.926         41.758         38.205         41.858         38           .000         2.042	.265 .118 .292
Roughness Data ( 1 SubAreas ) Roughness Horizontal SubArea Coefficient Breakpoint	
1 .040	
Discharge coefficient parameters BRType BRWdth EMBSS EMBElv UserCD 2 18.410 1.00 338.785 ********	
Pressure flow elevations AVBCEL PFElev ******** 337.810	
Abutment Parameters ABSLPL ABSLPR XTOELT YTOELT XTOERT YTOERT ****** ******* ******** 336.956 ******** 336.591	
Pier/Pile Data ( 4 Group(s) ) Code Indicates Bridge Uses Piers Group Elevation Gross Width Number	
1333.451.70112336.895.70113336.8951.82924337.8101.8292	
** * Finished Processing Header Record BRIDG * **	

Figure 7-4. Summary of bridge section data for the single-opening bridge (coordinate mode) of example #2.

**************************************						
Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations.						
			/ Output (			
**						*
EXAM	PLE 2: DF	RY CREEK	NEAR BAF	RREN HILLS	5	
SIMPL	E BRIDGE	EXAMPLE	FIXED-GEC	OMETRY MOI	DE	
	<< Beginni	lng Compu	tations for	r Profile	1 >>	
		VHD	Q	AREA		LEW
	-	HF		K		REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: EXIT	336 621	.013	147 247	287.057	*******	44.294
Header Type: XS					* * * * * * * * *	169.529
SRD: 14554.200		* * * * * *			1.000	*****
Section: FULLV	336.627	.013	147.247	287.882	45.720	44.273
Header Type: FV	336.640	.006	.511	12472.93	45.720	169.549
SRD: 14599.920	334.608	.000	.108	.0001	1.000	.000
<<< The Preceding Data Reflect The "Unconstricted" Profile >>>						

Section: APPRO	336.636	.014	147.247	280.319	60.960	43.238
Header Type: AS	336.650	.008	.525	11828.16	60.960	170.305
SRD: 14660.880	334.667	.000	.113	.0001	1.000	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRIDG Header Type: BR SRD: 14599.920		.021	1.736		45.720 45.720 1.844	38.097
Specific Bridge I: Bridge Type 2 F						
Pier/Pile Code 0	том туре т	.7365	.026 337.	809 ******	* ******	* *******
Unconstricted Downstream Br Bridge DrawDo	idge Sectio wn Distanco	on Water e:		evation:	3	36.546 .081
	WSEL EGEL	VHD HF	Q V	AREA K	SRDL FLEN	LEW REW
	CRWS		FR #	SF	ALPHA	ERR
Section: APPRO Header Type: AS SRD: 14660.880		.023	.471			171.171

\*\* Change in Approach Section Water Surface Elevation: .242 \*\*

	I	/bi	pro	ach	Se	ctio	n APPRO	Flow	Contra	ction	Info	rmation
Μ	(	G	)	М (	Κ	)	KQ	2	KLKQ	XRF	Q	OTEL

Figure 7-5. Computed profile output for the single-opening bridge (coordinate mode) of example #2, first analysis.

# 7.3 EXAMPLE #3: BRIDGE-BACKWATER COMPUTATIONS USING TEMPLATE SECTION

A common practice is to design a bridge using only one valley cross-section, usually surveyed along the centerline of the proposed stream crossing. Slope-conveyance is generally used to determine the unconstricted (pre-bridge) water-surface elevation which subsequently serves as the base to determine the amount of backwater that may be created by the proposed bridge. It is not within the scope of this report to present an in-depth discussion of the potential problems associated with this "one-section" analytical procedure. However, a brief discussion is in order to establish a foundation for the following example. The most obvious constraint in this "one-section" method is the selection of an applicable energy gradient for the slope-conveyance computations. Assuming subcritical flow (the most frequent case), the applicable energy gradient depends on downstream conditions. The first question that must be addressed is: "Is the surveyed section truly representative of the valley geometry for a significant reach length downstream from the bridge site?" If that question cannot be answered in the affirmative, then obviously the "one-section" method is probably inadequate. The only reasonable alternative in that case is to survey a sufficient number of sections to adequately define the variable geometry and use step-backwater computation techniques to define the unconstricted water-surface profile. An affirmative answer still raises additional questions. The next question may be "Is there significant variation in roughness and (or) stream slope downstream from the bridge site?" Another logical question is "Are there any human induced influences on the water-surface profile downstream from the bridge site?" If responses to the last two questions are negative, using slope-conveyance and the "one-section" method can be quite adequate. If either or both of the latter two questions indicate a problem, but uniform (or uniformly varying) geometry is reasonably approximated, the template-section feature of WSPRO might be used to great advantage. This feature permits the user (within limits) to fabricate additional sections so that step-backwater computations can be used instead of slope-conveyance computations. Also, even though many agencies desire to apply WSPRO's bridge-backwater computational techniques, they are restricted by policy and (or) economics to surveying a single valley section. Assuming that slope-conveyance determination of the unconstricted water-surface elevation is applicable, such agencies could utilize the template-section feature to fabricate the additional sections required for the WSPRO analysis and use slope conveyance to determine the water-surface elevation at the exit section.

The following two sections illustrate use of the template-section feature subject to the following assumptions: (1) the surveyed section adequately represents the geometry of the entire length of reach being considered; (2) there are significant changes in valley slope in the stream reach downstream from the bridge site; (3) roughness varies both vertically and horizontally within each section but does not vary from section to section along the reach; and (4) a gaging station provides a known stage-discharge relation at a point downstream from the bridge site. The third and fourth assumptions shorten the illustrations and discussion. The analysis could easily include variation of roughness along the reach. Without a known stage-discharge relation, a similar data setup could be used to take advantage of the "self-correcting" and "converging profile" characteristics of step-backwater analyses discussed by Davidian (1984).

The example is separated into two logical steps: (1) transfer the known stage-discharge relation from the downstream point to the bridge-related sections; and (2) compute the bridge backwater (in this case for only one design alternative). A third step, not illustrated in this example, is computing step-backwater upstream from the bridge to determine the extent of the backwater effects (logically performed after reducing the number of feasible alternative designs).

# 7.3.1 Transfer Known Stage-Discharge Relation to Bridge Site

A plot of the surveyed section is presented in Figure 7-6 and the stream-bed profile plot is shown in Figure 7-7.

The section was surveyed along the centerline of the proposed stream crossing (SRD = 762). The known stage-discharge relation is available from a streamflow-gaging station at SRD = 304.8. The input data for transferring this known relation to the bridge site are illustrated in Figure 7-8. Profiles are computed for five discharges (Q record) using watersurface elevations (WS record) from the known stage-discharge relation. Because the surveyed section is located upstream from where section data is first needed, that section must be initially introduced as a template section. Template-section data consist of an XT header record and GR data. The required XS header record for the section needed at SRD = 304.800. The GT record in instructs the model to get the geometry for that section from the template section. The -0.671 coded in the GT record is the elevation difference from SRD = 762 to SRD = 304.8. That value is used to lower the template-section elevations to fabricate the section at SRD = 304.8. If a uniform valley slope exists between two points, the slope can be coded in either header record (XT or XS) to accomplish the necessary elevation adjustment. The roughness data for this most downstream section follows the header record. Each of the upstream sections require only an XS header record due to the assumptions of uniform geometry and constant roughness from section to section. Elevation adjustments are accomplished by valley slope propagation. Roughness data are also propagated. The SRD values of the last three sections reflect appropriate locations for the subsequent bridge-backwater analysis.

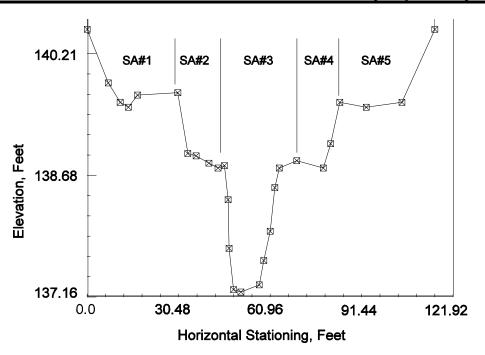


Figure 7-6. Plot of surveyed cross-section for example #3.

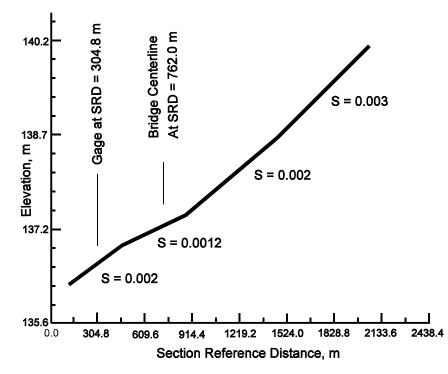


Figure 7-7. Plot of streambed profile for example #3.

T1 T2 T3 * SI 1 *	1	Example 3: Examples of Input & Output for WSPRO Computer Model FHWA/USGS Model for Water-surface Profile Computations <<<< Transfer Known Rating to Upstream Sections >>>>
Q WS *		84.951 99.109 127.426 155.743 212.376 139.056 139.181 139.394 139.574 139.879
XT GR GR GR GR GR GR GR *	SURVY	762.0000.000,141.7320.000,140.5137.010,139.84210.973,139.59813.716,139.53716.764,139.69030.175,139.72033.528,138.95836.271,138.92840.538,138.83643.586,138.77545.720,138.80646.939,138.37947.244,137.77048.768,137.25151.206,137.22157.302,137.31258.826,137.61760.960,137.98362.484,138.53264.008,138.77569.799,138.86778.638,138.77581.077,139.08084.125,139.59892.964,139.537104.851,139.598115.824,140.513
XS GT N SA ND ND	GAGE	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
XS XS XS XS XS * EX ER	XS3 EXIT FULLV	457.200 * * * 0.0020 609.600 * * * 0.0012 720.852 757.428 803.148

Figure 7-8. Input data for transferring stage-discharge relation to the bridge site, example #3.

The WSPRO generated output for the input data file shown in Figure 7-8 is presented in Figure 7-9.

SECTION 7. Examples of Model Input and Output

	Model for Wat Run Date & Time: Input File: EX3.D	nistration - U.S. er-Surface Profile C 9/10/97 7:31 am AT Output Fil	Geological Survey Computations. Version V070197	
T1 T2 T3 SI	EXAMPLE 3: EXAM FHWA/USGS MODEL <<<<< TRANSFER	IPLES OF INPUT & OU J FOR WATER-SURFACE	TTPUT FOR WSPRO COMPUTER MODEL PROFILE COMPUTATIONS UPSTREAM SECTIONS >>>>	
01		(SI) Units Used in	WSPRO	
	Quantity	SI Unit	Precision	
	Length Depth Elevation Widths	meters meters	0.001 0.001 0.001 0.001 0.001	
	Discharge Slope	meters/second cubic meters/second meter/meter degrees	1 0.001 0.001	
Q	84.951 99.10	9 127.426 155.743	212.376	
* *	* Processing Flow Dat	a; Placing Informati	on into Sequence 1 ***	
WS	139.056 139.18	1 139.394 139.574	139.879	

*			nistration er-Surface P Metric / O	-	ogical Surv ations. Metric	
1	FHWA/USG	EXAMPLES OF S MODEL FOR ANSFER KNOWN	INPUT & OUTP WATER-SURFA	UT FOR WSPR	O COMPUTER D COMPUTATION	S
	* * *		o Process He	ader Record S	URVY *	
XT GR GR GR GR GR GR GR	13 36 46 57 64 84		0.000,140. 16.764,139. 40.538,138. 47.244,137. 58.826,137. 69.799,138. 92.964,139.	83643.586,77048.768,61760.960,86778.638,	139.720 138.775 137.251 137.983 138.775	10.973,139.598 33.528,138.958 45.720,138.806 51.206,137.221 62.484,138.532 81.077,139.080 15.824,140.513
	NOTICE:	eted Reading D X-coordinate X-coordinate Storing Templ	<pre># 2 increas #29 increas</pre>	ed to elimina	te vertical te vertical	segment.
*** SRD	Locatio	Data Sum n: 762.	mary For Hea Valley Slo	der Record SU pe: ******	RVY Error C	*** ode 0
	Х	Y	oordinates ( X	29 pairs) Y	х	Y
	.000		.100	140.513	7.010	139.842

$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	.100 13.716 33.528 43.586 47.244 57.302 62.484 78.638 92.964 115.924	138.775 137.770 137.312 138.532 138.775	36.271 45.720 48.768 58.826 64.008	139.690 138.928 138.806 137.251 137.617 138.775 139.080
Minimum X-Station: Maximum X-Station: Minimum Y-Elevation: Maximum Y-Elevation: *	.000 ( 115.924 ( 137.221 ( 141.732 (	associated 2 associated 2 associated 2	Y-Elevation: Y-Elevation: X-Station: X-Station:	.000)

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \*\_\_\_\_\_ -----\* EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>> \_\_\_\_\_ \* Starting To Process Header Record GAGE \_\_\_\_\_ XS GAGE 304.800 GТ -0.671 0.055,0.050 0.065,0.060 0.040,0.040 N 0.065,0.060 0.055,0.050 30.175 45.720 64.008 84.125 0.305,0.914 0.610,1.524 0.000,0.305 Ν SA ND 0.305, 1.219 0.305, 0.914 ND \* \* \* \* \* \* Completed Reading Data Associated With Header Record GAGE \*\*\* Storing X-Section Data In Temporary File As Record Number 1 \*\*\* \*\*\*Data Summary For Header Record GAGE\*\*SRD Location:305.Cross-Section Skew:.0Error Code0Valley Slope:.00000Averaging Conveyance By Geometric Mean. \* \* \* Energy Loss Coefficients -> Expansion: .50 Contraction: .00 X,Y-coordinates (29 pairs) X Y v 
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 46.939
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 51.206
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 136.641
 58.826
 136.946

 60.960
 137.312
 62.484
 137.861
 64.008
 138.104

 69.799
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 Y х \_\_\_\_\_ Minimum and Maximum X, Y-coordinates Minimum X-Station:.000 ( associated Y-Elevation: 141.061 )Maximum X-Station:115.924 ( associated Y-Elevation: 141.061 )Minimum Y-Elevation:136.550 ( associated X-Station: 51.206 )Maximum Y-Elevation:141.061 ( associated X-Station: .000 ) Roughness Data ( 5 SubAreas ) Transition Roughness Hydraulic Horizontal Coefficient SubArea Point Depth Breakpoint 
 Bottom
 .055
 .305
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 Top
 .050
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 - -- 30.175

 Bottom
 .065
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 Top
 .060
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 - -- 45.720
 Bottom
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 Top
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 - -- -- 64.008
 \_\_\_\_ ----- ------ ------ ------1 2 3 64.008 4 \_ \_ \_ \_ \_ \_ 84.125 5 \_ \_ \_ \_ \_ \_ \_ \_\_\_\_\_ Finished Processing Header Record GAGE

M I	odel for Wat nput Units:	nistration - cer-Surface Pro Metric / Out	ofile Comput cput Units:	ations. Metric	-
XAMPLE 3: EX FHWA/USGS	AMPLES OF MODEL FOR	INPUT & OUTPUT WATER-SURFACI RATING TO U	Γ FOR WSPF E PROFILE	RO COMPUTER COMPUTATION	MODEL IS
*					
* *		Co Process Head			
S XS2 457	.200 * *	* 0.0020			
*** No Roug	hness Data 1	Data Associated Input, Propagat ata In Tempora	ing From Pr	revious Sect	ion ***
* * *	Data Sun	nmary For Heade	er Record	XS2	***
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		coordinates (29			
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.000	141.366	.100	140.147	7.010	
10.973	139.232 139.354	13.716 33.528	139.171 138.592	16.764 36.271	139.32
		43.586	138.409	45.720	138.44
46.939	138.470 138.013	43.586 47.244	137.404	48.768	
51.206	136.855	57.302	136.946 138.166	58.826	137.25 138.40
69.799	138.501	78.638	138.409	81.077	138.71
84.125 115.824	136.855 137.617 138.501 139.232 140.147	78.638 92.964 115.924	139.171 141.366	104.851	139.23
Minimum X-St Maximum X-St Minimum Y-El Maximum Y-El	ation: ation: 1	and Maximum X, .000 ( asso 15.924 ( asso .36.855 ( asso .41.366 ( asso	ociated Y-El ociated Y-El	evation: 1	.41.366 ) .41.366 ) 51.206 ) .000 )
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1	Bottom Top	.055 .050	.305 .914		
2	 Bottom Top 	.065 .060	.610 1.524	30.175  45.720	
3	Bottom Top	.040	.000	  64.008	
4	Bottom Top 	.065	.305 1.219	  84.125	
5	Bottom Top	.055 .050	.305 .914		

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XAMPLE 3: EX FHWA/USGS	AMPLES OF MODEL FOR	INPUT & OUTPU WATER-SURFACI RATING TO (	Γ FOR WSPF E PROFILE	RO COMPUTER COMPUTATIO	MODEL NS	
*					*	
		o Process Head				
S XS3 609	.600 * *	* 0.0012				
*** No Roug	hness Data I	ata Associated nput, Propagat ta In Tempora	ing From Pr	evious Sec	tion *	* * * * * *
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SRD Location: Valley Slope: Energy Loss Co	.00120 oefficients	Averaging Con -> Expansion	n Skew: . nveyance By n: .50 (	Geometric I Contraction	Code 0 Mean. : .00	
		oordinates (29				
X	-			X	¥	
	141.549	.100	140.330	7.010	139.	659
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	138.653	43.586 47.244	138.592 137.587	45.720	138. 137.	62
46.939	138.196	47.244	137.587	48.768	137.	06
51.206 60.960	137.038 137.800	57.302 62.484	137.129 138.349	58.826 64.008	137. 138.	43° 592
69.799 84.125	138.684	78.638 92.964	138.592	81.077	138.	89
84.125 115.824	140.330	92.964 115.924	139.354 141.549	104.851	139.	41
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SubArea	Transition Point	ess Data ( 5 Roughness Coefficient	Hydraulic Depth	Breakpoin	t	
1	Bottom Top	.055 .050	.305 .914		_	
2	 Bottom Top 	 .065 .060	 .610 1.524 	30.175		
3	Bottom Top	.040 .040	.000 .305	45.720		
4	 Bottom Top 	.065	.305 1.219	64.008   84.125		
5	Bottom Top	.055	.305 .914			

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EXAMPLE 3: EXA FHWA/USGS	AMPLES OF MODEL FOR	INPUT & OUTPU WATER-SURFACI RATING TO (	F FOR WSPR E PROFILE	O COMPUTER COMPUTATION	MODEL IS
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XS EXIT 720					
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Minimum X-Sta Maximum X-Sta Minimum Y-Ele Maximum Y-Ele	ation: 1	nd Maximum X,X .000 ( asso 15.924 ( asso 37.171 ( asso 41.682 ( asso	ociated Y-El ociated Y-El	evation: 1 evation: 1	41.682 ) 41.682 ) 51.206 ) .000 )
SubArea	Transition Point	ess Data ( 5 Roughness Coefficient	Hydraulic Depth	Breakpoint	
1	Bottom Top	.055 .050	.305 .914		
2	 Bottom Top 	 .065 .060	.610 1.524	30.175  45.720	
3	Bottom Top	.040	.000 .305		
4	 Bottom Top	.065 .060	.305 1.219	64.008	
5	 Bottom Top	.055 .050	.305 .914	84.125  	
* * *	Finished P	rocessing Head	ler Record	* EXIT * *	

Federal M	Highway Adm odel for Wat	*** WSPRO inistration - cer-SurfacePro Metric / Out	U. S. Geol ofile Comput	ogical Surve ations.	
* EXAMPLE 3: EX FHWA/USGS	AMPLES OF MODEL FOR		FOR WSPR PROFILE	O COMPUTER M COMPUTATIONS	ODEL
*	Starting 7	To Process Head	der Record F	ULLV *	
XS FULLV 757	.428				
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*** SRD Location: Valley Slope: Energy Loss C	757. .00120	nmary For Head Cross-Section Averaging Con -> Expansion	n Skew: . nveyance By	0 Error Co Geometric Me	*** de 0 an. .00
X	Y	coordinates (29 X	9 pairs) Y	x	Y
$\begin{array}{c} . 000 \\ 10.973 \\ 30.175 \\ 40.538 \\ 46.939 \\ 51.206 \\ 60.960 \\ 69.799 \\ 84.125 \end{array}$	141.726 139.592 139.714 138.830 138.373 137.215 137.977 138.861 139.592	.100 13.716 33.528 43.586 47.244 57.302 62.484 78.638 92.964	139.531 138.952 138.769 137.764 137.306 138.526	7.010 16.764 36.271 45.720 48.768 58.826 64.008 81.077 104.851	139.074
115.824  Minimum X-St Minimum Y-El Maximum Y-El	Minimum a ation:	115.924 and Maximum X, .000 ( asso 115.924 ( asso 137.215 ( asso 141.726 ( asso	ociated Y-El	evation: 14 evation: 14	1.726 ) 1.726 ) 1.206 ) .000 )
SubArea	Transition Point	ness Data ( 5 n Roughness Coefficient	Hydraulic Depth	Breakpoint	
1	Bottom Top 	.055 .050	.305 .914	  30.175	
2	Bottom Top 	.065 .060 	.610 1.524	45.720	
3	Bottom Top 	.040 .040	.000 .305	  64.008	
4	Bottom Top 	.065 .060 	.305 1.219 	  84.125	
ن *	Bottom Top	.055 .050	.305 .914	 	
* *		Processing Head			

Federal M I	************* Highway Admir odel for Wate nput Units: N	nistration - er-Surface Pr Metric / Ou	- U. S. Geol cofile Comput tput Units:	logical Surve tations. Metric	зХ
EXAMPLE 3: EX FHWA/USGS	AMPLES OF 1 MODEL FOR SFER KNOWN	NPUT & OUTPU WATER-SURFAC	JT FOR WSPI CE PROFILE	RO COMPUTER M COMPUTATIONS	IODEL S
*	Starting To	Process Hea	ader Record	APPR *	
XS APPR 803	.148				
*** No Roug	ed Reading Da hness Data Ir X-Section Dat	nput, Propaga	ating From Pi	revious Secti	.on ***
*** SRD Location: Valley Slope: Energy Loss C	803. .00120	hary For Head Cross-Sectio Averaging Co -> Expansio	on Skew: onveyance By	.0 Error Co Geometric Me	*** ode 0 ean. .00
x	Y	oordinates (2 X		x	Y
.000	141 781			7.010	139.891
10.973	139.647	13.716	139.586	16.764	139.739
30.175 40.538	139.769 138.885				
46.939	138 138	43.586 47.244	138.824 137.819	45.720 48.768	137.300
51.206 60.960	137.270	57.302	137.361	58.826	137.666
60.960 69 799	138.032 138.916	62.484 78.638	138.581	64.008 81 077	138.824
60.960 69.799 84.125 115.824	140.562	92.964 115.924	139.586 141.781	58.826 64.008 81.077 104.851	139.647
Minimum X-St Maximum X-St Minimum Y-El Maximum Y-El		nd Maximum X, .000 ( ass 5.924 ( ass	sociated Y-E	levation: 14 levation: 14	1.781 ) 1.781 ) 1.206 ) .000 )
	Roughne	ess Data ( 5 Roughness	SubAreas )	Horizontal	
SubArea	Point	Coefficient	Depth	Breakpoint	
1	Bottom	.055	.305		
	Тор 	.050	.914	30.175	
2	Bottom	.065	.610		
	Тор 	.060	1.524	 45.720	
3	Bottom	.040	.000	45.720	
	Top	.040	.305		
4	 Bottom	.065	.305	64.008	
-	Тор	.060	1.219		
5	 Bottom Top	 .055 .050	 .305 .914	84.125  	
* * *	 Finished Pr	cocessing Hea	ader Record		

	Model	******** W S P ay Administration for Water-Surface Units: Metric /	n – U.S.Ge e Profile Comp	putations.
FHW	,	L FOR WATER-SU		SPRO COMPUTER MODEL E COMPUTATIONS SECTIONS >>>>
	* Summa: *========	ry of Boundary Co	ondition Info ====================================	
#	Reach Discharge	Water Surface Elevation	Slope	Flow Regime
 1 2 3 4 5 	84.95 99.11 127.43 155.74 212.38	139.056 139.181 139.394 139.574 139.879		Sub-Critical Sub-Critical Sub-Critical Sub-Critical Sub-Critical
	*=====================================	ginning 5 Profi		n(s) *

EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: GAGE	139.056	.129	84.951		*********	8.875
Header Type: XS	139.185	*****	1.202		*********	106.398
SRD: 304.799	138.430	*****	.598		1.757	*****
Section: XS2	139.361	.129	84.951	70.733		8.861
Header Type: XS	139.490	.305	1.201	1899.47		106.408
SRD: 457.200	138.735	.000	.597	.0020		.001
Section: XS3	139.655	.101	84.951	81.727	152.399	7.059
Header Type: XS	139.757	.266	1.039	2175.65	152.399	107.738
SRD: 609.599	138.918	.000	.500	.0017	1.845	.000
Section: EXIT	139.825	.093	84.951	85.403	111.252	6.666
Header Type: XS	139.919	.162	.994	2273.79	111.252	108.174
SRD: 720.852	139.049	.000	.472	.0015	1.861	001
Section: FULLV	139.879	.091	84.951	86.424		6.563
Header Type: XS	139.971	.050	.982	2301.54		108.295
SRD: 757.428	139.093	.000	.465	.0014		.005
Section: APPR	139.944	.089	84.951	87.465	45.719	6.458
Header Type: XS	140.034	.061	.971	2330.06	45.719	108.417
SRD: 803.147	139.150	.000	.458	.0013	1.868	.005

<< Beginning Computations for Profile 1 >>

<< Completed Computations of Profile 1 >>

EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: GAGE	139.181		99.109	83.065	*********	6.904
Header Type: XS	139.315		1.193	2211.05	*********	107.897
SRD: 304.799	138.551		.572	*****	1.851	*****
Section: XS2	139.487	.134	99.109	83.220		6.888
Header Type: XS	139.621	.305	1.190	2215.18		107.916
SRD: 457.200	138.856	.000	.570	.0020		.001
Section: XS3	139.782	.105	99.109	94.690	152.399	5.734
Header Type: XS	139.887	.266	1.046	2534.38		109.261
SRD: 609.599	139.039	.000	.479	.0017		002
Section: EXIT Header Type: XS SRD: 720.852	139.953 140.050 139.172	.097 .162 .000	99.109 1.005 .454	98.549 2647.68 .0015		
Section: FULLV	140.007	.095	99.109	99.635		5.244
Header Type: XS	140.102	.050	.994	2680.13		109.830
SRD: 757.428	139.216	.000	.447	.0014		.005
Section: APPR	140.072	.092	99.109	100.734		5.136
Header Type: XS	140.165	.061	.983	2713.19		109.956
SRD: 803.147	139.271	.000	.440	.0014		.005

<< Beginning Computations for Profile 2 >>

<< Completed Computations of Profile 2 >>

EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: GAGE Header Type: XS SRD: 304.799	139.393 139.534 138.750		127.425 1.212 .532	105.081 2846.26 *****		4.711 110.451 *****
Section: XS2	139.700	.140	127.425	105.229		4.697
Header Type: XS	139.840	.304	1.210	2850.87		110.468
SRD: 457.200	139.055	.000	.531	.0020		.003
Section: XS3	139.996	.111	127.425	117.420	152.399	3.524
Header Type: XS	140.108	.267	1.085	3240.40	152.399	111.834
SRD: 609.599	139.238	.000	.454	.0018	1.857	.000
Section: EXIT	140.170	.103	127.425	121.775	111.252	3.112
Header Type: XS	140.273	.164	1.046	3384.60	111.252	112.314
SRD: 720.852	139.372	.000	.430	.0015	1.847	.002
Section: FULLV	140.225	.101	127.425	122.966	36.576	3.000
Header Type: XS	140.326	.051	1.036	3424.56	36.576	112.445
SRD: 757.428	139.417	.000	.424	.0014	1.845	.005
Section: APPR	140.290	.099	127.425	124.143	45.719	2.889
Header Type: XS	140.389	.062	1.026	3464.28	45.719	112.574
SRD: 803.147	139.470	.000	.418	.0014	1.842	.003

<< Beginning Computations for Profile 3 >>

<< Completed Computations of Profile 3 >>

EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: GAGE Header Type: XS SRD: 304.799	139.574 139.721 138.989		155.742 1.251 .509	124.477 3475.58 *****		2.858 112.610 *****
Section: XS2	139.880	.146	155.742	124.632		2.843
Header Type: XS	140.026	.305	1.249	3480.84		112.627
SRD: 457.200	139.294	.000	.508	.0020		.001
Section: XS3	140.178	.118	155.742	137.488		1.652
Header Type: XS	140.297	.270	1.132	3929.59		114.015
SRD: 609.599	139.477	.000	.439	.0018		.000
Section: EXIT	140.353	.109	155.742	142.169	111.252	1.225
Header Type: XS	140.463	.167	1.095	4099.20	111.252	114.513
SRD: 720.852	139.610	.000	.418	.0015	1.790	003
Section: FULLV	140.407	.107	155.742	143.328		1.119
Header Type: XS	140.515	.052	1.086	4141.67		114.635
SRD: 757.428	139.654	.000	.413	.0014		.000
Section: APPR	140.474	.105	155.742	144.664	45.719	.998
Header Type: XS	140.579	.063	1.076	4190.90	45.719	114.776
SRD: 803.147	139.709	.000	.407	.0014	1.783	.002

### << Beginning Computations for Profile 4 >>

<< Completed Computations of Profile 4 >>

EXAMPLE 3: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< TRANSFER KNOWN RATING TO UPSTREAM SECTIONS >>>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: GAGE	139.879		212.376	158.973	********	.096
Header Type: XS	140.036		1.335	4739.04	*********	115.826
SRD: 304.799	139.307		.479	*****	1.733	*****
Section: XS2	140.186	.157	212.376	159.281		.096
Header Type: XS	140.343	.305	1.333	4751.54		115.826
SRD: 457.200	139.612	.000	.478	.0020		.005
Section: XS3	140.486	.128	212.376	172.846	152.399	.087
Header Type: XS	140.615	.271	1.228	5320.61	152.399	115.836
SRD: 609.599	139.795	.000	.415	.0018	1.671	.000
Section: EXIT	140.665	.119	212.376	178.137	111.252	.083
Header Type: XS	140.785	.169	1.192	5548.59	111.252	115.840
SRD: 720.852	139.929	.000	.394	.0015	1.651	.001
Section: FULLV	140.720	.117	212.376	179.426	36.576	.082
Header Type: XS	140.838	.053	1.183	5604.11	36.576	115.841
SRD: 757.428	139.973	.000	.390	.0015	1.647	.000
Section: APPR	140.788	.115	212.376	180.863		.081
Header Type: XS	140.903	.064	1.174	5666.34		115.842
SRD: 803.147	140.027	.000	.384	.0014		.000

<< Beginning Computations for Profile 5 >>

ER

<< Completed Computations of Profile 5 >>

Figure 7-9. Output for transferring stage-discharge relation to the bridge site, example #3.

## 7.3.2 Bridge-Backwater Analysis (Component Mode) with Road Grade

Figure 7-10 illustrates the input data for analysis of a bridge to be located at SRD = 762. The results of the analysis in the preceding section provide known water-surface elevations at the exit section. Again the surveyed section is introduced as a template section (XT record). The exit section (XS record) is fabricated from the template-section geometry (GT record). Elevation adjustments are made using the product of the valley slope (which could alternatively been coded in the XT record) and the difference in section reference distances (i.e., 762.000-720.852). Roughness data for the exit section are coded in the following N, SA, and ND records.

geometry and roughness data for the full-valley section are propagated from the exit section with elevation adjustments again made using valley slope. Bridge data, in component form (no GR data) for component mode, is shown in the BR record section. Had roughness data not been coded for the bridge opening, it would have been obtained from the full-valley section. The road-grade section is coded in the XR and GR records which follow. Bridge data are never propagated, and even in component mode the bridge section does not interfere with upstream propagation of data from the full-valley section. Therefore, missing geometry and roughness data for the approach section (XS header record) are propagated from the full-valley section, with elevations adjusted for the specified valley slope.

т1 Example 3A: Examples of Input & Output for WSPRO Computer Model т2 FHWA/USGS Model for Water-surface Profile Computations <<<< Additional Bridge Backwater Computations >>>> ΤЗ SI 1 84.951 99.109 127.426 155.743 212.376 139.824 139.952 140.168 140.354 140.665 0 WS \* ΧТ SURVY 762.000 0.000,141.732 0.000,140.513 7.010,139.842 10.973,139.598 GR 13.716,139.537 16.764,139.690 30.175,139.720 33.528,138.958 GR 36.271,138.928 40.538,138.836 43.586,138.775 45.720,138.806 GR 47.244,137.770 48.768,137.251 58.826,137.617 60.960,137.983 51.206,137.221 62.484,138.532 46.939,138.379 GR GR 57.302,137.312 69.799,138.867 78.638,138.775 81.077,139.080 64.008,138.775 GR 84.125,139.598 92.964,139.537 104.851,139.598 115.824,140.513 GR 115.824,141.732 GR EXIT 720.852 \* \* \* 0.0012 XS GΤ -0.671Ν 0.055,0.050 0.065,0.060 0.040,0.040 0.065,0.060 0.055,0.050 Ν 30.175 45.720 64.008 84.125 0.305,0.914 0.610,1.524 0.000,0.305 SA ND 0.305,1.219 0.305,0.914 ND \* FULLV 757.428 XS BRDGE 757.428 BR BC 140.818 ΒL 36.576 41.148 68.580 1 9.144 CD 0.040 0.050 Ν 0.914 1.524 ND \* XR ROAD 762.000 9.144 0.000,140.818 36.576,141.122 73.152,141.122 115.824,140.818 GR \* APPR 803.148 XS ΕX ER

Figure 7-10. Input data for single-opening bridge (component mode) and road grade, example #3.

Figure 7-11 presents parts of the output for the bridge-backwater computations. The output for the fifth profile includes a series of ===xxx messages. Such messages are documented in Table 6-3. Following is a brief discussion of this particular series of messages.

\*\*\*\*\*\* W S P R O Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Run Date & Time: 9/10/97 10:58 am Version V070197 Input File: EX3A.DAT Output File: EX3A.LST EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>> т1 т2 т3 SI 1 Metric (SI) Units Used in WSPRO Quantity SI Unit Precision \_\_\_\_\_ Length meters Depth meters Elevation meters Widths meters 0.001 0.001 0.001 0.001 Velocitymeters/second0.001Dischargecubic meters/second0.001Slopemeter/meter0.001 Angles degrees 0.01 Q 84.951 99.109 127.426 155.743 212.376 \* \* \* Processing Flow Data; Placing Information into Sequence 1 \* \* \* WS 139.824 139.952 140.168 140.354 140.665

	Input Units	ater-Surface : : Metric / (	Profile Comp Output Units	: Metric	*
FHWA/U	: EXAMPLES O SGS MODEL FO ADDITIONAL	R WATER-SURF		COMPUTATIO	ONS
*	Starting	To Process H	eader Record	SURVY	*
GR GR GR GR GR GR	762.000 0.000,141.73 13.716,139.537 36.271,138.928 46.939,138.379 57.302,137.312 64.008,138.775 84.125,139.598 15.824,141.732	16.764,139 40.538,138 47.244,137 58.826,137 69.799,138 92.964,139	.690 30.17 .836 43.58	10,139.842 5,139.720 6,138.775 8,137.251 0,137.983 8,138.775 1,139.598	10.973,139.598 33.528,138.958 45.720,138.806 51.206,137.221 62.484,138.532 81.077,139.080 115.824,140.513
*** Com +++072 NOTIC +++072 NOTIC *** SRD Locat	E: X-coordina Storing Tem Data S	Data Associa te # 2 increas te #29 increas plate Header 1 ummary For Hea Valley Slo	sed to elimi: sed to elimi: Record Data ader Record	nate vertica nate vertica In Memory SURVY	al segment. al segment. ***
	Х,Ү	-coordinates	(29 pairs)		
Х			Ŷ	Х	Y
X .000 10.973 30.175 40.538 46.939 51.206 60.960 69.799 84.125 115.824	$141.732 \\ 139.598 \\ 139.720 \\ 138.836 \\ 138.379 \\ 137.221 \\ 137.983 \\ 138.867 \\ 139.598 \\ 140.513$	X .100 13.716 33.528 43.586 47.244 57.302 62.484 78.638 92.964 115.924		7.010 16.764 36.271 45.720 48.768	139.842 139.690 138.928 138.806 137.251 137.617 138.775 139.080
.000 10.973 30.175 40.538 46.939 51.206 60.960 69.799 84.125 115.824 	141.732 139.598 139.720 138.836 138.379 137.221 137.983 138.867 139.598 140.513	X .100 13.716 33.528 43.586 47.244 57.302 62.484 78.638 92.964 115.924 .000 (a:	Y 140.513 139.537 138.958 138.775 137.770 137.312 138.532 138.775 139.537 141.732 	7.010 16.764 36.271 45.720 48.768 58.826 64.008 81.077 104.851 	139.842 139.690 138.928 138.806 137.251 137.617 138.775 139.080 139.598

Mode	hway Administ: l for Water-Sit Units: Metri	ration – U urface Profi ic / Outpu	. S. Geolog le Computat t Units: Me	ical Survey ions. tric	
EXAMPLE 3A: EXAMI FHWA/USGS MOI <<<<< ADDIT	PLES OF INP DEL FOR WAT IONAL BRIDGE	JT & OUTPUT ER-SURFACE BACKWATER	FOR WSPRO PROFILE CO COMPUTATIO	COMPUTER MO MPUTATIONS NS >>>>>	
* St	carting To Pro	ocess Header	Record EX	IT *	
GT -0.671 N 0.055 N 0 SA 30.1 ND 0.305 ND 0	2 * * * 0 ,0.050 0.065 .065,0.060 0 175 45.720 ,0.914 0.610 .305,1.219 0 Reading Data 2	,0.060 0.04 .055,0.050 54.008 84.1 ,1.524 0.00 .305,0.914	25 0,0.305	Record EXT	Г ***
	ection Data I	n Temporary	File As Rec	ord Number	
SRD Location: Valley Slope: Energy Loss Coef:	Data Summary 721. Cros .00120 Ave: ficients ->	ss-Section S raging Conve	kew: .0 yance By Ge	Error Code ometric Mean	e 0 n.
х ү		inates (29 p. X Y 		X	Y
10.973       13         30.175       13         40.538       13         46.939       13         51.206       13         60.960       13         69.799       13         84.125       13	3.927       1         9.049       3         3.165       4         7.708       4         5.550       5         7.312       6	3.586       13         7.244       13         7.302       13         2.484       13         8.638       13         2.964       13	8.866 8.287 8.104 7.099 6.641 7.861	36.271 45.720 48.768 58.826 64.008 81.077	139.171 139.019 138.257 138.135 136.580 136.946 138.104 138.409 138.927
I Minimum X-Statio Maximum X-Statio Minimum Y-Elevat Maximum Y-Elevat	zion: 136.55 zion: 141.0	00 ( associ 24 ( associ 50 ( associ	ated Y-Eleva ated Y-Eleva ated X-Stat ated X-Stat	ion: 51	.206 )
T: SubArea	ransition Re		ydraulic H	orizontal reakpoint	
1	Bottom Top	.055 .050	.305 .914	  30.175	
2	Bottom Top	.065 .060	.610 1.524		
3	Bottom Top	 .040 .040	.000 .305	45.720	
4	Bottom Top	.065 .060	.305 1.219	64.008  	

5	Bottom Top	.055 .050	 .305 .914	84.125  
* * *	Finished	Processing Header		

Federal H Mc Ir	Highway Admin odel for Wate oput Units: N	nistration – er-Surface Pro Metric / Out	U. S. Geo ofile Compu- cput Units:		7
EXAMPLE 3A: EX FHWA/USGS <<<< ADI	CAMPLES OF MODEL FOR DITIONAL BRI	INPUT & OUTPU WATER-SURFACE IDGE BACKWATE	JT FOR WS E PROFILE ER COMPUTA	PRO COMPUTER N COMPUTATIONS TIONS >>>>>	
* * *		) Process Head	ler Record	FULLV *	
XS FULLV 757.	. 428				
*** No Rough	nness Data Ir	nput, Propagat	ing From P	er Record FULI revious Sectio Record Number	on ***
*** SRD Location: Valley Slope: Energy Loss Co	757. .00120	Averaging Cor	n Skew: nveyance By	.0 Error Coo Geometric Mea	an.
Х	Y	oordinates (29 X	) pairs) Y	Х	Y
.000 10.973 30.175 40.538 46.939 51.206 60.960 69.799 84.125	141.105 138.971 139.093 138.209 137.752 136.594 137.356 138.240 138.971		139.886 138.910 138.331 138.148 137.143 136.685 137.905 138.148	7.010 16.764 36.271 45.720 48.768 58.826 64.008 81.077 104.851	138.301 138.179 136.624 136.990 138.148 138.453
Minimum X-Sta Maximum X-Sta Minimum Y-Ele Maximum Y-Ele SubArea	ation: 12 ation: 12 evation: 13 evation: 14 Roughne Transition Point	1.105 ( asso ess Data ( 5 Roughness Coefficient	ociated Y-E ociated Y-E ociated X-S ociated X-S SubAreas ) Hydraulic Depth	levation: 14 levation: 14 tation: 5 tation: Horizontal Breakpoint	1.105 ) 1.105 ) 1.206 ) .000 )
1	Bottom Top	.055 .050	.305 .914		
2	 Bottom Top 	.065 .060	.610 1.524	30.175  45.720	
3	Bottom Top	.040	.000 .305		
4	 Bottom Top 	.065 .060	.305 1.219	64.008   84.125	
5	Bottom Top	.055	.305 .914		

\* Finished Processing Header Record FULLV \*

******************************** W S P R O *********************************
EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>> **
* Starting To Process Header Record BRDGE * **
BR       BRDGE       757.428         BC       140.818         BL       36.576       41.148       68.580         CD       1       9.144         N       0.040       0.050         ND       0.914       1.524
<pre>*** Completed Reading Data Associated With Header Record BRDGE *** +++072 NOTICE: X-coordinate # 2 increased to eliminate vertical segment. +++072 NOTICE: X-coordinate #17 increased to eliminate vertical segment. *** Storing Bridge Data In Temporary File As Record Number 3 ***</pre>
*** Data Summary For Bridge Record BRDGE *** SRD Location: 757. Cross-Section Skew: .0 Error Code 0 Valley Slope: ****** Averaging Conveyance By Geometric Mean. Energy Loss Coefficients -> Expansion: .50 Contraction: .00
X,Y-coordinates (18 pairs) X Y X Y X Y
36.576140.81836.676138.29440.538138.20943.586138.14845.720138.17946.939137.75247.244137.14348.768136.62451.206136.59457.302136.68558.826136.99060.960137.35662.484137.90564.008138.14869.799138.24073.152138.20573.252140.81836.576140.818
Minimum and Maximum X,Y-coordinates Minimum X-Station: 36.576 (associated Y-Elevation: 140.818) Maximum X-Station: 73.252 (associated Y-Elevation: 140.818) Minimum Y-Elevation: 136.594 (associated X-Station: 51.206) Maximum Y-Elevation: 140.818 (associated X-Station: 36.576)
Roughness Data ( 1 SubAreas ) Transition Roughness SubArea Point Coefficient Depth
1 Bottom .040 .914 Top .050 1.524
Discharge coefficient parameters BRType BRWdth WWAngl WWWdth EntRnd UserCD 1 9.144 ****** ****************************
Pressure flow elevations AVBCEL PFElev 140.818 140.818
Abutment Parameters ABSLPL ABSLPR XTOELT YTOELT XTOERT YTOERT

Federa	l Highway Adı Model for W Input Units	ministration ater-Surface : Metric /	CO ********** - U. S. Geol Profile Comput Output Units:	ogical Surve ations. Metric	
EXAMPLE 3A: FHWA/USG <<<<<	EXAMPLES O S MODEL FO ADDITIONAL	F INPUT & OL R WATER-SURF BRIDGE BACKW	TPUT FOR WSE ACE PROFILE ATER COMPUTAT	PRO COMPUTER COMPUTATIONS CIONS >>>>	
*	Starting	To Process H	leader Record	ROAD *	
	62.000 9.14 .000,140.818		122 73.152,14	1.122 115.8	324,140.818
			ted With Heade prary File As F		
Roadway Wid	n: 762. th: 9.144	Cross-Sect User-Speci	adway Record ion Skew: . fied Weir Coef Consists of a	0 Error Co ficient: **	* * * * *
Х	Х,Ү Ү	-coordinates X	( 4 pairs) Y	X	Y
.000			141.122	73.152	141.122
Minimum Y-	Station: Station: Elevation: Elevation:	.000 ( a 115.824 ( a 140.818 ( a 141.122 ( a	X,Y-coordinate associated Y-El associated Y-El associated X-St associated X-St	evation: 14 evation: 14 ation: 11	L5.824 )
*				****	

*	Finished	Processing	Header	Record	ROAD	*
*						*

Federal H Mc In	lighway Admi: odel for Wat put Units: 1	nistration – er-Surface Pr Metric / Ou	U. S. Geo ofile Compu tput Units:		У
EXAMPLE 3A: EX FHWA/USGS <<<< ADD	AMPLES OF MODEL FOR DITIONAL BR	INPUT & OUTP WATER-SURFAC IDGE BACKWAT	UT FOR WS E PROFILE ER COMPUTA	PRO COMPUTER 1 COMPUTATIONS TIONS >>>>	
* * *		o Process Hea	der Record		
XS APPR 803.	148				
*** No Rough	iness Data II	nput, Propaga	ting From P	er Record AP revious Section Record Number	on ***
*** SRD Location: Valley Slope: Energy Loss Co	803. .00120	Averaging Co	n Skew: nveyance By	.0 Error Co Geometric Me	an.
X	Y	oordinates (2 X	9 pairs) Y	X	Y
30.175 40.538 46.939 51.206 60.960 69.799 84.125			139.941 138.965 138.386 138.203 137.198 136.740 137.960 138.203 138.203 138.965 141.160	7.010 16.764 36.271 45.720 48.768 58.826 64.008 81.077 104.851	138.356 138.234 136.679 137.045 138.203 138.508
Minimum X-Sta Maximum X-Sta Minimum Y-Ele Maximum Y-Ele SubArea	tion: 1 evation: 1 evation: 1 Roughn Transition	nd Maximum X, .000 ( ass 15.924 ( ass 36.649 ( ass 41.160 ( ass ess Data ( 5 Roughness Coefficient	ociated Y-E ociated Y-E ociated X-S ociated X-S SubAreas ) Hydraulic	levation: 14 levation: 14 tation: 5 tation: Horizontal	1.160 ) 1.160 ) 1.206 ) .000 )
1	Bottom Top	.055 .050	 .305 .914		
2	Bottom Top	.065	.610 1.524	30.175  45.720	
3	Bottom Top	.040	.000 .305		
4	 Bottom Top 	.065 .060	.305 1.219	64.008   84.125	
5	Bottom Top	.055 .050	.305 .914	04.125  	

	Model f	WSF	n – U.S.Ge Profile Comp	
FHW	. ,	ES OF INPUT & ( G FOR WATER-SU NAL BRIDGE BAC	RFACE PROFILE	
	* Summar *========	ry of Boundary Co Water Surface	ondition Infor	rmation *
#	Discharge	Elevation	Slope	Flow Regime
 1 2 3 4 5 	84.95 99.11 127.43 155.74 212.38	139.824 139.952 140.168 140.354 140.665		Sub-Critical Sub-Critical Sub-Critical Sub-Critical Sub-Critical
	* Beg	ginning 5 Profi	le Calculation	n(s) *

**************************************							
** EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>> << Beginning Computations for Profile 1 >>							
	WSEL EGEL CRWS	HF	Q V FR #	K	FLEN		
Section: EXIT Header Type: XS SRD: 720.852	139.824 139.851 138.430	* * * * * *	.556	4488.66	******** ********* 1.758	.284 115.608 *****	
Section: FULLV Header Type: FV SRD: 757.428	139.866	.013	.570	4353.38	36.576	115.234	
<<< The Preceding Data Reflect The "Unconstricted" Profile >>>							
Section: APPR Header Type: AS SRD: 803.147	139.885	.018	.586	4193.28		114.783	

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #		AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDGE Header Type: BR SRD: 757.428	139.809 139.901 138.439		1.	951 061 229	80.045 2529.21 *****	36.576 36.576 1.600	36.615 73.213 .000
Specific Bridge I Bridge Type 1 F Pier/Pile Code **	low Type 1				V BLEN  17 36.57	XLAB  6 36.57	XRAB  6 73.152
Unconstricted Full Valley Section Water Surface Elevation: 139.836 Downstream Bridge Section Water Surface Elevation: 139.809 Bridge DrawDown Distance: .027							

\*\*\* Roadway Section Located at SRD 762.000 \*\*\*

## Section: ROAD Header Type: XR <<< Embankment Is Not Overtopped >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	139.908	.028	84.951	150.974	36.575	.430
Header Type: AS	139.937	.024	.562	4426.88	38.544	115.438
SRD: 803.147	138.529	.010	.208	.0004	1.763	.001

* *	Change in Appr	oach Section Wa	ater Surface	e Elevatio	on: .055	* *
		ction APPR Flo			mation OTEL	
	.678 .	274 3211.7	37.298	73.895	139.895	
	<<< End of	Bridge Hydrau	lics Computa	tions >>:	>	

<< Completed Computations of Profile 1 >>

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_ \_ \_ \_ \_ \_ EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>>> << Beginning Computations for Profile 2 >> WSEL. VHD AREA Q SRDL TIEW EGEL HF V Κ FLEN REW FR # SF CRWS НO ALPHA ERR \_\_\_\_\_ \_\_\_\_\_ \_ \_ \_ \_ \_ \_ \_ \_ \_ 99.109 Section: EXIT 167.420 \*\*\*\*\*\*\*\* .090 139.951 .030 139.982 \*\*\*\*\* 5089.29 \*\*\*\*\*\*\* Header Type: XS .591 115.832 138.551 \*\*\*\*\* .205 \* \* \* \* \* \* \*\*\*\*\* SRD: 720.852 1.695 Section: FULLV 139.965 .031 99.109 163.895 36.576 .093 115.830 Header Type: FV .604 139.997 .014 4941.29 36.576 757.428 1.711 SRD: 138.597 .000 .212 .0004 .000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section:	APPR	139.983	.034	99.109	159.652	45.719	.096
Header T	ype: AS	140.017	.019	.620	4766.64	45.719	115.827
SRD:	803.147	138.650	.001	.222	.0004	1.730	.001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #		AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDGE Header Type: BR SRD: 757.428	139.929 140.045 138.527	.116 .025 .037	1.	109 173 247	84.432 2753.53 *****	36.576 36.576 1.651	36.611 73.218 .000
Specific Bridge I Bridge Type 1 F Pier/Pile Code **	low Type 1			PFELEV  140.81	BLEN 7 36.57	XLAB  6 36.57	XRAB 6 73.152
Unconstricted Full Valley Section Water Surface Elevation: 139.965 Downstream Bridge Section Water Surface Elevation: 139.929 Bridge DrawDown Distance:							

\*\*\* Roadway Section Located at SRD 762.000 \*\*\*

# Section: ROAD Header Type: XR <<< Embankment Is Not Overtopped >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	140.054	.030	99.109	167.876	36.575	.090
Header Type: AS	140.084	.027	.590	5108.66	38.831	115.833
SRD: 803.147	138.650	.011	.204	.0004	1.693	.001

* *	Change in Appr	oach Section W	later Surface	e Elevatio	on: .071	* *
		ection APPR Fl :) KQ		on Infort XRKQ	mation OTEL	
	.684 .	307 3539.2	37.338	73.944	140.040	
	<<< End of	Bridge Hydrau	lics Computa	ations >>:	>	
	<< Compl	eted Computati	ons of Profi	le 2 >>		

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_ \_ \_ \_ \_ \_ EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>>> << Beginning Computations for Profile 3 >> WSEL. VHD AREA Q SRDL TIEW EGEL HF V Κ FLEN REW FR # SF CRWS НO ALPHA ERR \_\_\_\_\_ \_ \_\_\_\_\_ \_ \_ \_ \_ \_ \_ \_ \_ \_ 127.425 Section: EXIT 192.424 \*\*\*\*\*\*\*\* .073 140.168 .035 140.203 \*\*\*\*\* 6178.91 \*\*\*\*\*\*\*\* Header Type: XS .662 .208 115.850 138.750 \*\*\*\*\* \*\*\*\*\* \*\*\*\*\* SRD: 720.852 1.605 Section: FULLV 140.183 .037 127.425 189.099 36.576 .075 .673 115.848 Header Type: FV .015 6029.31 140.220 36.576 757.428 SRD: 138.796 .000 .214 .0004 1.616 .000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section	: APPR	140.203	.039	127.425	185.076	45.719	.078
Header	Type: AS	140.242	.021	.688	5850.63	45.719	115.845
SRD:	803.147	138.849	.000	.222	.0005	1.629	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #		AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDGE Header Type: BR SRD: 757.428	140.129 140.296 138.693	.167 .030 .061	- •	425 389 280	91.732 3141.05 *****	36.576 36.576 1.697	36.603 73.225 .000
Specific Bridge I Bridge Type 1 F Pier/Pile Code **	low Type 1			PFELE  140.8	V BLEN  17 36.57	XLAB  6 36.57	XRAB  6 73.152
Unconstricted Full Valley Section Water Surface Elevation: 140.183 Downstream Bridge Section Water Surface Elevation: 140.129 Bridge DrawDown Distance: .054							

\*\*\* Roadway Section Located at SRD 762.000 \*\*\*

# Section: ROAD Header Type: XR <<< Embankment Is Not Overtopped >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	140.309	.033	127.425	197.346	36.575	.069
Header Type: AS	140.343	.031	.645	6403.53	39.392	115.854
SRD: 803.147	138.849	.015	.199	.0005	1.590	.001

* *	Change in Ap	proach S	ection Wat	er Surface	e Elevati	on: .106	* *
	Approach M(G) M(		APPR Flow KQ		ion Infor XRKQ	mation OTEL	
	.684	.353	4142.3	37.407	74.029	140.294	
<pre>&lt;&lt;&lt; End of Bridge Hydraulics Computations &gt;&gt;&gt;</pre>							
	<< Com	pleted C	omputation	s of Profi	ile 3 >>		

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_ \_ \_ \_ \_ \_ EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>>> << Beginning Computations for Profile 4 >> WSEL. VHD AREA Q SRDL TIEW EGEL HF V Κ FLEN REW FR # SF CRWS НO ALPHA ERR \_\_\_\_\_ \_ \_\_\_\_\_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_\_\_\_\_ 155.742 Section: EXIT 213.962 \*\*\*\*\*\*\*\* .057 140.354 .041 140.395 \*\*\*\*\* 7189.39 \*\*\*\*\*\*\*\* Header Type: XS .727 115.865 138.989 \*\*\*\*\* .212 \* \* \* \* \* \* \*\*\*\*\* SRD: 720.852 1.544 Section: FULLV 140.371 .043 155.742 210.854 36.576 .060 .738 Header Type: FV 7039.24 .017 115.863 140.414 36.576 757.428 SRD: 139.031 .000 .218 .0005 1.552 .001

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section	: APPR	140.393	.045	155.742	207.058	45.719	.062
Header	Type: AS	140.438	.022	.752	6857.77	45.719	115.860
SRD:	803.147	139.088	.000	.225	.0005	1.563	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	ŧ	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDGE Header Type: BR SRD: 757.428	140.297 140.520 138.849	.222 .035 .088	- •	742 591 311	97.879 3480.61 *****	36.576 36.576 1.726	36.596 73.232 .000
Specific Bridge I Bridge Type 1 F Pier/Pile Code **	low Type 1			PFELE  140.8	V BLEN  17 36.57	XLAB  6 36.57	XRAB  6 73.152
Unconstricted Full Valley Section Water Surface Elevation: 140.371 Downstream Bridge Section Water Surface Elevation: 140.297 Bridge DrawDown Distance: .074							

\*\*\* Roadway Section Located at SRD 762.000 \*\*\*

# Section: ROAD Header Type: XR <<< Embankment Is Not Overtopped >>>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	140.536	.037	155.742	223.698	36.575	.051
Header Type: AS	140.574	.036	.696	7669.17	39.761	115.872
SRD: 803.147	139.088	.018	.197	.0005	1.519	.002

* *	Change in Approach S	Section Wat	er Surfac	e Elevati	on: .143	* *		
	Approach Section M(G) M(K)				mation OTEL			
	.684 .385	4714.0	37.466	74.102	140.521			
	<pre></pre>							
	<< Completed C	Computation	ns of Prof	ile 4 >>				

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_\_\_\_\_ EXAMPLE 3A: EXAMPLES OF INPUT & OUTPUT FOR WSPRO COMPUTER MODEL FHWA/USGS MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS <<<<< ADDITIONAL BRIDGE BACKWATER COMPUTATIONS >>>>> << Beginning Computations for Profile 5 >> WSEL. VHD AREA Q SRDL TIEW EGEL HF V Κ FLEN REW FR # SF CRWS HO ALPHA ERR \_\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_ 212.376 .032 249.987 \*\*\*\*\*\*\*\* Section: EXIT 140.665 .053 9033.11 \*\*\*\*\*\*\*\* 140.718 \*\*\*\*\* .849 115.891 Header Type: XS 139.307 \*\*\*\*\* \*\*\*\*\* \*\*\*\*\* SRD: 720.852 .223 1.461 Section: FULLV 140.685 .055 212.376 247.240 36.576 .034 Header Type: FV .858 .020 8885.99 115.889 140.740 36.576 SRD: 757.428 139.351 .000 .227 .0006 1.466 .001 <<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section	: APPR	140.711	.057	212.376	243.874	45.719	.036
Header	Type: AS	140.768	.026	.870	8707.19	45.719	115.887
SRD:	803.147	139.406	.000	.233	.0006	1.473	.000

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

===215 FLOW CLASS 1 SOLUTION INDICATES POSSIBLE ROAD OVERFLOW. WS1, WS2, WS3, RGMIN: 140.94 .00 140.57 140.82

===260 ATTEMPTING FLOW CLASS 4 SOLUTION.

- ===220 FLOW CLASS 1 ( 4 ) SOLUTION INDICATES POSSIBLE PRESSURE FLOW. WS3, WSIU, WS1, PFELV: 140.57 140.89 140.94 140.82
- ===245 ATTEMPTING FLOW CLASS 2 ( 5 ) SOLUTION.
- ===250 INSUFFICIENT HEAD FOR PRESSURE FLOW. YU/Z, WSIU, WS: 1.06 141.02 141.07
- ===270 REJECTED FLOW CLASS 2 ( 5 ) SOLUTION.

	WSEL EGEL CRWS	VHD HF HO	Q V FR #	:	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR
Section: BRDGE Header Type: BR SRD: 757.428	140.570 140.913 139.124	.343 .044 .150	211. 1.		107.906 4059.12 *****	36.576 36.576 1.759	36.585 73.242 .000
Specific Bridge I Bridge Type 1 F Pier/Pile Code **	low Type 4					XLAB 	XRAB  6 73.152
Unconstricted Full Valley Section Water Surface Elevation: 140.685 Downstream Bridge Section Water Surface Elevation: 140.570 Bridge DrawDown Distance: .115							

	WSEL EGEL	VHD HF	Q V	AREA ERR	FLEN SRD	LEW REW	
Section: ROAD Header Type: XR			1.369 .727			.000 115.823	
Hydraulic	Characteri	stics of	Left and F	Right Roadwa	ay Sections	3	
Left WeirRight WeirWeir Flow $(Q)$ .63.74Weir Length(WLEN)14.45716.867Weir LEW(LEW).00098.956Weir REW(REW)14.457115.823Maximum Depth(DMAX).120.120Average Depth(DAVG).060.060Maximum Velocity(VMAX).737.737Average Velocity(VAVG).727.727Average Head(HAVG).088.088Weir Coefficient(CAVG)2.9872.987							
	WSEL EGEL CRWS		Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR	
Section: APPR Header Type: AS SRD: 803.147	140.937 140.982	.044 .043	212.376 .786 .196	270.121 10142.84 .0006	36.575 40.308 1.423	.018 115.905 .000	
** Change	in Approa	ch Secti	on Water Su	urface Eleva	ation: .22	26 **	
			PR Flow Cont XLKQ				
.6	.42	8 579	7.1 37.5	543 74.19	99 ******	- *	
<pre>&lt;&lt;&lt; End of Bridge Hydraulics Computations &gt;&gt;&gt;</pre>							
<< Completed Computations of Profile 5 >>							
ER							
*************	*** Norma Elapsed	l end of Time:	WSPRO exec 0 Minutes	cution. ** 1 Seconds	* * * * * * * * * * * * * * * * * * * *	* * * * *	

Figure 7-11. Output for single-opening bridge (component mode) and road grade, example #3.

With one exception, WSPRO first attempts to compute free-surface flow with the entire flow passing through the bridge (class 1). The exception occurs when the tailwater is higher than the low-chord elevation, in which case WSPRO assumes submerged pressure flow, with or without road overflow (class 6 or 3). In this example, message ===215 appears because the class 1 solution has a water-surface elevation at the approach section higher than the minimum road-grade elevation (RGMIN = 140.82). Message ===260 indicates that a class 4 solution (free-surface bridge flow with road overflow) is attempted. The class 4 solution generates message ===220 because the water-surface elevation immediately upstream from the bridge (WSIU = 140.90) is higher than the low-chord elevation (PFELV = 14-.82). Message ===245

indicates that an unsubmerged pressure flow solution, with or without road overflow (class 2 or 5) is attempted. When no more such messages are generated, the appropriate flow type has been selected and computation continues. This is a good illustration that the messages do not always indicate problems. Frequently they simply indicate the assumptions and actions taken by the model.

## 7.4 EXAMPLE #4: UPSTREAM/DOWNSTREAM PROFILE COMPUTATIONS

WSPRO is designed to accommodate profile computations in both upstream and downstream directions during a single model execution. It is not always obvious that downstream computations are required; the need may not be recognized until upstream computations have been attempted. However, Figure 7-12 illustrates input data for a case where profiles are required for two discharges in a reach where it is quite certain there is a combination of subcritical and supercritical subreaches. Upstream profiles are computed with a starting water-surface elevation determined by slope conveyance; downstream profiles are computed with a critical-flow water-surface elevation at the initial section. This is reflected by the corresponding entries in the Q, SK, and EX records. The entries for the first profile reflect a discharge of 362.456 m<sup>3</sup>/s (cubic meters per second), an energy gradient of 0.0037, and upstream computations (0 in the EX record). Likewise, the second set of entries reflect: (1) the same discharge; (2) critical flow to be computed at the initial section (negative slope, no water-surface elevation); and (3) downstream computations (1 in the EX record).

т1 Example 4: Steep Creek near Nowhere, USA \* YMIN FR# CRWS SRD SRDL Q WSEL EGL UT 8 37 7 27 28 5 25 11 \* ST 1 \* Q 362.456 362.456 396.436 396.436 SK 0.0037 -1 0.0037 -1 \* XS XSEC1 30.480 32.614,189.829 51.816,183.429 57.912,181.051 GR 40.234,186.263 82.601,179.984 89.306,179.862 GR 75.590,179.984 94.488,179.984 96.926,180.777 103.022,181.630 106.680,183.032 GR Ν 0.065 0.027 0.065 SA 51.816 112.166 \* XS XSEC2 60.960 43.891,189.616 55.474,184.556 62.484,181.508 66.751,180.929 GR 83.820,179.802 90.526,179.771 96.317,179.832 67.666,180.960 GR GR 105.156,180.015 108.814,180.960 117.043,184.130 55.474 117.043 SA \* XS XSEC3 91.440 GR 60.046,188.793 68.275,185.745 75.286,181.935 83.515,180.594 89.611,179.923 92.964,179.680 96.622,179.558 GR 84.125,179.984 99.670,179.680 107.594,180.045 108.204,180.990 113.995,182.027 GR 119.786,184.221 GR 68.275 121.006 SA \* XS XSEC4 121.920 83.210,181.417 67.056,188.702 77.114,181.508 73.152,184.617 GR 73.152,184.617 77.114,181.508 89.916,180.624 95.098,180.624 84.125,180.685 96.012,180.137 GR 99.974,180.167 103.632,180.198 103.937,180.685 113.386,181.905 GR GR 118.567,184.526 117.043 73.152 SA \* XS XSEC5 165.811 62.179,188.671 75.895,181.844 78.638,181.874 84.125,181.600 GR GR 89.611,181.539 95.098,181.539 100.279,181.447 105.766,181.813 111.252,181.752 117.348,184.709 GR

SA *		75.895 111.343
XS GR GR SA	XSEC6	215.494         66.142,189.555       71.018,186.995       75.286,185.136       81.382,182.514         84.430,182.088       87.173,181.844       93.878,181.691       97.231,181.539         101.498,181.539       105.766,181.844       114.910,182.088       115.824,185.440         75.286       114.910
* GR GR GR SA *	XSEC7	245.974 69.494,189.860 78.638,184.861 85.039,182.606 85.039,182.148 88.392,182.057 90.830,181.722 97.231,181.600 100.279,181.478 104.242,181.569 107.290,181.905 117.958,182.088 118.872,185.654 78.638 117.958
EX ER		0 1 0 1

Figure 7-12. Input data for upstream/downstream profile computations.

Figure 7-13 shows the output for the subcritical and supercritical profile computations for the 362.456 m<sup>3</sup>/s discharge. The energy equation was successfully balanced at a subcritical water-surface elevation at XSEC2 and at XSEC3. When WSPRO cannot obtain a subcritical result at a section, it assumes that perhaps that section is a critical-flow section with supercritical flow between the critical-flow section and the next section downstream where subcritical flow was successfully computed. Thus WSPRO assumes a critical water-surface elevation (CRWS) at the section in question and attempts to balance the energy equation for a subcritical water-surface elevation at the next upstream section. The preceding was the case for XSEC4 through XSEC7 with the only success (subcritical result) occurring at XSEC7. The output for downstream computations (supercritical result) is presented on pages 213 through 215. When computing in the downstream direction, at any section where the energy equation cannot be balanced for supercritical flow, WSPRO again assumes the possibility of a critical-flow section. That is, it assumes CRWS at the section in question and attempts to balance the energy equation at the next downstream section. In this example, for CRWS at XSEC7, a supercritical result could not be obtained at XSEC6. However, for CRWS at XSEC6 a supercritical result could be obtained at XSEC5. WSPRO was also able to successfully balance the energy equation for supercritical flow from XSEC5 to XSEC4, XSEC4 to XSEC3, and XSEC3 to XSEC2. A supercritical watersurface elevation could not be computed at XSEC1.

* * *	Federal Highway Admi Model for Wat Run Date & Time: Input File: EX4.D	*** WSPRO ****** Inistration - U.S. er-Surface Profile Co 9/10/97 12:54 pm DAT Output File	mputations. Version V070197 : EX4.LST
T1 UT SI 1	EXAMPLE 4: STEE 8 37 7 1	EP CREEK NEAR NOWHERE, 11 27 28 5	USA 25
	Metric	c (SI) Units Used in W	ISPRO
	Quantity	SI Unit	Precision
	Length Depth Elevation Widths	meters	0.001 0.001 0.001 0.001 0.001
	Discharge	meters/second cubic meters/second meter/meter	0.001
	Angles	5	0.01
Q	362.456 362.45	56 396.436 396.436	
* * *	Processing Flow Dat	a; Placing Informatio	on into Sequence 1 ***
SK	0.0037 -	-1 0.0037 -1	

Federa	l Highway Adm Model for Wa	inistration ter-Surface 1	0 ********** - U. S. Geol Profile Comput Dutput Units:	ogical Surve ations.	ey
			NEAR NOWHERE,		
*	Starting	To Process H	eader Record X	SEC1 *	
S XSEC1 3 R 3 R 75.	2.614,189.829	40.234,18 82.601,179.	5.263 51.816 984 89.306,1	,183.429 79.862 94	57.912,181.0 .488,179.984
	926,180.777 .065 0.027 51.816 11	0.065	530 106.680,1	83.032	
			ted With Heade rary File As R		
*** SRD Locatio Valley Slop Energy Loss	n: 30. e: .00000	Cross-Sect Averaging (	ader Record XS ion Skew: . Conveyance By ion: .50 C	0 Error Co Geometric Me	ean.
x	Х, Y- Y	coordinates X	Ŷ	х	Y
32.614 57.912 89.306 103.022		40.234 75.590 94.488 106.680	186.263 179.984 179.984 183.032	51.816 82.601 96.926	183.429 179.984 180.777
Minimum X- Maximum X- Minimum Y- Maximum Y-	Station: Station: Elevation: Elevation: Rough	32.614 ( a: 106.680 ( a: 179.862 ( a: 189.829 ( a: ness Data (	X,Y-coordinate ssociated Y-El ssociated Y-El ssociated X-St ssociated X-St 3 SubAreas )	evation: 18 evation: 18 ation: 8 ation: 3	39.829) 33.032) 39.306) 32.614)
	SubArea	Roughness Coefficien	Horizontal t Breakpoint		
	1	.065	  51.816	-	
	2	.027	51.816  112.166		
	3	.065		-	
*		.065	eader Record X		

*	l Highway Adm Model for Wa Input Units:	ter-Surface P Metric / C	rofile Comput Output Units:	ations. Metric	
	EXAMPLE 4:				
* * *		To Process He	ader Record X	SEC2 *	
S XSEC2 6 R R 67	0.960 43.891,189.61 .666,180.960	6 55.474,18 83.820,179.	4.556 62.48 802 90.526,	4,181.508 179.771 9	66.751,180.9 6.317,179.832
	.156,180.015 5.474 117.0		960 117.043,	184.130	
*** Comple *** Storing	eted Reading 1 g X-Section Da	Data Associat ata In Tempor	ed With Heade ary File As R	er Record XS ecord Numbe	EC2 *** r 2 ***
*** SRD Location Valley Slop Energy Loss		Cross-Secti Averaging C	der Record XS on Skew: . onveyance By on: .50 C	0 Error C Geometric M	ean.
х	Y	coordinates ( X	Ŷ	X	Y
43.891 66.751 90.526 108.814	189.616 180.929 179.771 180.960	55.474 67.666 96.317 117.043	184.556 180.960 179.832 184.130	62.484 83.820 105.156	181.508 179.802 180.015
Minimum X- Maximum X- Minimum Y- Maximum Y-	Minimum a Station: Station: Elevation: Elevation:	and Maximum X	.Y-coordinate	evation: 1 evation: 1 ation: ation:	89.616 ) 84.130 ) 90.526 ) 43.891 )
	SubArea	Coefficient	Horizontal Breakpoint		
	1	.065	 55.474	-	
		.027			
	2	.027	117.043		
	2 3		117.043	-	

*	Model for Wat Input Units:	Metric / O	rofile Computa utput Units: N	letric	*
	EXAMPLE 4: S	TEEP CREEK N	EAR NOWHERE, U	JSA	
*	Starting I	o Process He	ader Record XS	SEC3 *	
S XSEC3 93 R G R 84	50.046,188.793	68.275,18 89.611,179.	5.745 75.286 923 92.964,1	5,181.935 79.680 90	83.515,180.5 6.622,179.558
r 99	.670,179.680	107.594,180.	045 108.204,1	80.990 113	3.995,182.027
	.786,184.221 3.275 121.00	6			
*** Comple *** Storing	eted Reading D g X-Section Da	ata Associat ta In Tempor	ed With Header ary File As Re	Record XSI cord Number	EC3 *** r 3 ***
Valley Slope	n: 91. e: .00000	Cross-Secti Averaging C	der Record XSF on Skew: .( onveyance By C on: .50 Cc	) Error Co Geometric Me	ean.
х	Y		13 pairs) Y	Х	Y
60.046 83.515 92.964 107.594 119.786			185.745 179.984 179.558 180.990	75.286 89.611 99.670 113.995	181.935 179.923 179.680 182.027
Minimum X-3 Maximum X-3 Minimum Y-1 Maximum Y-1	Station: 1 Station: 1 Slevation: 1 Slevation: 1 Roughn	60.046 ( as 19.786 ( as 79.558 ( as 88.793 ( as ess Data (	,Y-coordinates sociated Y-Ele sociated Y-Ele sociated X-Sta sociated X-Sta 3 SubAreas ) Horizontal	evation: 18 evation: 18 ation: 9 ation: 6	88.793 ) 84.221 ) 96.622 ) 60.046 )
		Coefficient	Horizontal Breakpoint	-	
	1 2	.065  .027	 68.275 		
	3	.065	121.006	-	

Federa	***************** l Highway Admi Model for Wat Input Units:	nistration er-Surface P Metric / C	- U. S. Geo Profile Compu Sutput Units:	logical Sur tations. Metric	rvey
	EXAMPLE 4: S	TEEP CREEK N	IEAR NOWHERE,	USA	
*	Starting 7	o Process He	ader Record :	XSEC4	*
XS XSEC4 12 GR GR 84	67.056.188.702	2 73.152,18 89.916,180.	4.617 77.1 624 95.098	14,181.508 ,180.624	83.210,181.417 96.012,180.137
GR 99	.974,180.167	103.632,180.	198 103.937	,180.685 1	13.386,181.905
	.567,184.526 3.152 117.04	13			
*** Comple *** Storing	eted Reading I g X-Section Da	oata Associat Ita In Tempor	ed With Head ary File As i	er Record X Record Numb	SEC4 *** Der 4 ***
*** SRD Location Valley Slope Energy Loss	Data Sum n: 122. e: .00000 Coefficients	Cross-Secti Averaging C	der Record X on Skew: onveyance By on: .50	.0 Error Geometric	*** Code 0 Mean. n: .00
x	Y	coordinates ( X	Ŷ	Х	Y
67.056 83.210 95.098 103.632 118.567	188.702 181.417 180.624 180.198	73,152	184.617	77.114 89.916 99.974 113.386	181.508 180.624 180.167 181.905
Minimum X-5 Maximum X-5 Minimum Y-1 Maximum Y-1	Minimum a Station: Station: 1 Elevation: 1 Elevation: 1	and Maximum X 67.056 ( as 18.567 ( as 80.137 ( as .88.702 ( as	C,Y-coordinat sociated Y-E sociated Y-E sociated X-S sociated X-S	es levation: levation: tation: tation:	188.702 ) 184.526 ) 96.012 ) 67.056 )
	SubArea	ness Data ( Roughness Coefficient	3 SubAreas ) Horizonta Breakpoin		
	1	.065	 73.152		
	2 3	.027  .065	117.043 		
* * *	 Finished F	Processing He	ader Record	  XSEC4 	-* * -*

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_\_\_\_\_ EXAMPLE 4: STEEP CREEK NEAR NOWHERE, USA \*\_\_\_\_\_\* \* Starting To Process Header Record XSEC5 \_\_\_\_\_ XS XSEC5 165.811 62.179,188.671 75.895,181.844 78.638,181.874 84.125,181.600 GR 89.611,181.539 95.098,181.539 100.279,181.447 105.766,181.813 GR 111.252,181.752 117.348,184.709 GR 75.895 111.343 SA \* \* \* \* \* \* Completed Reading Data Associated With Header Record XSEC5 Storing X-Section Data In Temporary File As Record Number 5 \*\*\* \* \* \* \*\*\* \* \* \* Data Summary For Header Record XSEC5 SRD Location:166.Cross-Section Skew:.0Error CodeValley Slope:.00000Averaging Conveyance By Geometric Mean. 0 Energy Loss Coefficients -> Expansion: .50 Contraction: .00 X,Y-coordinates (10 pairs) х ү Х Ү Х Y 
 A
 I
 A
 I
 X
 Y

 62.179
 188.671
 75.895
 181.844
 78.638
 181.874

 84.125
 181.600
 89.611
 181.539
 95.098
 181.539

 100.279
 181.447
 105.766
 181.813
 111.252
 181.752

 117.348
 184.709
 181.813
 111.252
 181.752
 \_\_\_\_\_ \_ \_\_\_\_\_ Minimum and Maximum X,Y-coordinates Minimum X-Station:62.179 (associated Y-Elevation:188.671 )Maximum X-Station:117.348 (associated Y-Elevation:184.709 )Minimum Y-Elevation:181.447 (associated X-Station:100.279 )Maximum Y-Elevation:188.671 (associated X-Station:62.179 ) Roughness Data ( 3 SubAreas ) Roughness Horizontal SubArea Coefficient Breakpoint ----------\_\_\_\_\_ .065 \_\_\_ 1 75.895 2 .027 \_\_\_ \_\_\_ 111.343 .065 3 \_\_\_ \_\_\_\_\_ \_\_\_\_ -----Finished Processing Header Record XSEC5 \* \*\_\_\_\_\_ ------

*	Model for Wa Input Units:	ter-Surface P Metric / O	utput Units:	Metric	*
	EXAMPLE 4:	STEEP CREEK N	EAR NOWHERE,	USA	
*	Starting	To Process He	ader Record X	SEC6	*
S XSEC6 21					
					81.382,182.5 97.231,181.539
a 101.	498,181.539	105.766,181.	844 114.910,	182.088 1	15.824,185.440
A 75	5.286 114.9	10			
		Data Associat ata In Tempor			
* * *	Data Su	mmary For Hea	der Record XS	EC6	* * *
RD Locatior Alley Slope Cnergy Loss	e: .00000	Cross-Secti Averaging C -> Expansi	onveyance By	Geometric	Mean.
x	Х, Y- Y	coordinates (	12 pairs) Y	x	Y
66.142 81.382	189.555 182.514	71.018 84.430	186.995	75.286 87.173	185.136 181.844
93.878 105.766	181.691 181.844	71.018 84.430 97.231 114.910	181.539 182.088	101.498 115.824	185.136 181.844 181.539 185.440
	Minimum Station: Station:	and Maximum X	.Y-coordinate	es evation: evation: ation:	189.555 ) 185.440 ) 101.498 )
Minimum X-S Maximum X-S Minimum Y-B Maximum Y-B	levation: levation:	189.555 ( as	sociated X-St	ation:	66.142 )
Minimum X-S Maximum X-S Minimum Y-F Maximum Y-F		ness Data (	3 SubAreas )		66.142 )
Minimum X-S Maximum X-S Minimum Y-F Maximum Y-F	Rough SubArea	ness Data ( Roughness Coefficient	3 SubAreas ) Horizontal Breakpoint		66.142 )
Minimum X-S Maximum X-S Minimum Y-F Maximum Y-F	Rough SubArea	ness Data ( Roughness	3 SubAreas ) Horizontal Breakpoint		66.142 )
Minimum X-S Maximum X-S Minimum Y-F Maximum Y-F	Rough SubArea	ness Data ( Roughness Coefficient .065  .027	3 SubAreas ) Horizontal Breakpoint  75.286 		66.142 )
Minimum X-S Maximum X-S Minimum Y-F Maximum Y-F	Rough SubArea 1	ness Data ( Roughness Coefficient .065 	3 SubAreas ) Horizontal Breakpoint  75.286  114.910 		66.142 )

	Federa	l Highway Adm Model for Wa Input Units:	*** WSPR( inistration - ter-Surface P Metric / Ou	- U. S. Geol rofile Comput utput Units:	ogical Sun ations. Metric		
		EXAMPLE 4:	STEEP CREEK NE	EAR NOWHERE,	USA		
	*	Starting	To Process Hea	ader Record X	SEC7	*	
XS GR GR	XSEC7 2 88	69.494,189.86	0 78.638,184 90.830,181.5	4.861 85.03 722 97.231,	9,182.606 181.600 1	85.039,182 L00.279,181.4	.148 78
GR	104	.242,181.569	107.290,181.9	905 117.958,	182.088 1	L18.872,185.6	54
SA	7	8.638 117.9	58				
** +++07 **	2 NOTICE:	X-coordinat	Data Associate e # 4 increase ata In Tempora	ed to elimina	te vertica	al segment.	
SR: Va	* D Locatio lley Slop ergy Loss	n: 246. e: .00000	mmary For Head Cross-Sectio Averaging Co -> Expansio	on Skew: . onveyance By	0 Error Geometric	Code O Mean.	
	Х	Х,Ү- Ү	coordinates (1 X	Y	X	Y	
	69.494 85.139 97.231 107.290	189.860 182.148 181.600 181.905		184.861 182.057 181.478 182.088	85.039 90.830 104.242	185.654	
M M M	inimum X- aximum X- inimum Y- aximum Y-	Minimum Station: Station: Elevation: Elevation:	and Maximum X 69.494 ( ass 118.872 ( ass 181.478 ( ass 189.860 ( ass	,Y-coordinate sociated Y-El sociated Y-El sociated X-St sociated X-St	s evation: evation: ation: ation:	189.860 ) 185.654 ) 100.279 ) 69.494 )	
		SubArea	ness Data ( 3 Roughness Coefficient	Horizontal Breakpoint			
		1	.065	78.638	_		
		2	.027				
		3	.065	117.958			
	+				-	*	
	^ * *		Processing Hea			*	
	^					- ^	

	EXAMPL 0 1 0	E 4: STEEP CREEP 1	K NEAR NOWHER	E, USA
	* Summa	ry of Boundary Co	ondition Info	rmation *
#	Reach Discharge	Water Surface Elevation	Friction Slope	Flow Regime
 1 2	362.46 362.46	 ******* ******	.0037 *****	Sub-Critical Super-Critical
3 4	396.44 396.44	* * * * * * * * * * * * * * *	.0037 *****	Sub-Critical Super-Critical

Model	***** W S P ay Administratio for Water-Surfac Units: Metric /	n – U.S. e Profile Co	Geological mputations	Survey	* *
*	E 4: STEEP CREE				_ *
	Beginning Comput			>>	
	WSEL VHD	0	AREA	SRDL	LEW REW ERR
Section: XSEC1 1 Header Type: XS 1 SRD: 30.480 1	82.544 .639 83.184 ***** 82.249 *****	362.456 3.540 .801	102.372 ** 5956.34 ** *****	****** ****** 1.000	54.082 105.409 *****
===135 CONVEYANCE RAT KRATIO: 1.40	IO OUTSIDE OF RE	COMMENDED LI	MITS AT SE	CID "XSEC	2".
Section: XSEC2 1 Header Type: XS 1 SRD: 60.960 1	82.858 .406 83.264 .080 82.086 .000	362.456 2.821 .586	128.469 8356.57 .0026	30.480 30.480 1.000	59.377 113.742 001
===125 FR# EXCEEDS FN FNTEST, FR#, W	TEST AT SECID "X SEL, CRWS: .80	SEC3": TRIA .84	LS CONTINU 182.76	ED. 182.49	
===110 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC3 , DELTAY: 182.				
===115 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC3 , CRWS: 182.49				
===135 CONVEYANCE RAT KRATIO: .68	IO OUTSIDE OF RE	COMMENDED LI	MITS AT SE	CID "XSEC	3".
Section: XSEC3 1 Header Type: XS 1 SRD: 91.440 1	82.757 .774 83.532 .083 82.488 .184	362.456 3.896 .838	93.019 5719.86 .0027	30.479 30.479 1.000	73.771 115.924 .000
===110 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC4 , DELTAY: 183.				
===115 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC4 , CRWS: 183.02	": USED WSM 188.70	IN = CRWS. 183.02		
===130 CRITICAL WATER ENERGY EQUATIO WSBEG, WSEND,	-SURFACE ELEVATI N N_O_T B_A_L_A CRWS: 183.02	NCED AT	' SECID "XS	EC4".	!!!!
Section: XSEC4 1 Header Type: XS 1 SRD: 121.920 1	83.022 1.010 84.033 ***** 83.022 *****	362.456 4.450 1.001	81.438 4695.78 .0005	30.480 30.480 1.000	75.183 115.594 *****
===110 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC5 , DELTAY: 183.		DELTAY. 7 .05		
===115 WSEL NOT FOUND WSLIM1, WSLIM2	AT SECID "XSEC5 , CRWS: 183.82				
	-SURFACE ELEVATI N N_O_T B_A_L_A CRWS: 183.82	_N_C_E_D AT	' SECID "XS		!!!!
Section: XSEC5 1	83.822 1.047	362.456	85.384	43.891	71.918

Header Type: XS SRD: 165.811		4.2444920.271.034.0008		
		5": REDUCED DELTAY. 11 189.55 .05	5	
		5": USED WSMIN = CRWS 189.55 184.5		
ENERGY EQUAT		CON A _ S _ S _ U _ M A_N_C_E_D AT SECID "2 189.55 184.11		
Section: XSEC6 Header Type: XS SRD: 215.493		362.45679.9284.5344838.671.002.0006	49.682	77.668 115.461 *****
Section: XSEC7 Header Type: XS SRD: 245.974	185.298 .126	362.456 97.768 3.707 6563.61 .755 .0041	30.480	79.413 118.598 .000

<< Completed Computations of Profile 1 >>

EXAMPLE 4: STEEP CREEK NEAR NOWHERE, USA

=== User Defined Table 1 of 1 ===

	SRD	SRDL	Q	XMIN	FR #	CRWS	WSEL
1 XSEC	1 30.480	* * * * * * * * *	362.456	32.613	.801	182.249	182.544
2 XSEC	2 60.960	30.480	362.456	43.890	.586	182.086	182.858
3 XSEC	3 91.440	30.479	362.456	60.045	.838	182.488	182.757
4 XSEC	4 121.920	30.480	362.456	67.056	1.001	183.022	183.022
5 XSEC	5 165.811	43.891	362.456	62.178	1.034	183.822	183.822
6 XSEC	6 215.493	49.682	362.456	66.141	1.002	184.111	184.111
7 XSEC	7 245.974	30.480	362.456	69.493	.755	184.119	184.587

		EGL
1	XSEC1	183.184
2	XSEC2	183.264
3	XSEC3	183.532
4	XSEC4	184.033
5	XSEC5	184.870
6	XSEC6	185.172
7	XSEC7	185.298

Federal Hig Mode Inpu	******** W hway Administra l for Water-Sur t Units: Metric	ation - U. face Profile c / Output	S. Geologic Computatio Units: Metr	al Survey ns. ic	
	IPLE 4: STEEP (				*
===100 WSI BELOW YM	< Beginning Com IIN AT SECID "XS RWS: 181.5	SEC7": SETTI	NG WSI = CR	2 >> WS.	
	WSEL VHD EGEL HF CRWS HO	FR #		ALPHA	
Section: XSEC7 Header Type: XS SRD: 245.974	184.119 1.065 185.185 ***** 184.119 *****	362.456 4.544 1.004	79.762 4802.12 *****	********* ********* 1.011	80.742 118.478 *****
===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, CRWS: 184	SEC6": USED 4.11 181.	WSMIN = CRW 60 184.	'S. 11	
===110 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, DELTAY: 1			5	
	ER-SURFACE ELEV ION N_O_T B_A_ 0, CRWS: 184.1	L_A_N_C_E_D	AT SECID "	XSEC6".	1111
Section: XSEC6 Header Type: XS SRD: 215.493	184.111 1.061 185.172 ***** 184.111 *****	362.456 4.534 1.002	79.928 4838.67 *****	-30.480 30.480 1.012	77.668 115.461 *****
===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, CRWS: 183	SEC5": USED 3.82 181.	WSMIN = CRW 51 183.	IS. 82	
Section: XSEC5 Header Type: XS SRD: 165.811	183.730 1.146 184.876 .295 183.822 .000	362.456           4.455           1.104	81.351 4569.79 .0059	-49.682 49.682 1.132	72.105 115.330 .003
===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, CRWS: 183				
===135 CONVEYANCE R KRATIO: .6		F RECOMMENDED	LIMITS AT	SECID "XSE	24".
Section: XSEC4 Header Type: XS SRD: 121.920	184.420 .455	6.248	2768.52	43.891	114.420
===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, CRWS: 182				
Section: XSEC3 Header Type: XS SRD: 91.440		6.306	57.475 2784.20 .0170	30.480	113.129
===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XS M2, CRWS: 182				
Section: XSEC2 Header Type: XS SRD: 60.960	181.653 1.515 183.168 .475 182.086 .256	5 5.450	3026.55	30.479	110.613

===115 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XSEC1 M2, CRWS: 182.25		
===110 WSEL NOT FOU WSLIM1, WSLI	ND AT SECID "XSEC1 M2, DELTAY: 182.		
	ER-SURFACE ELEVATI ION N_O_T B_A_L_A , CRWS: 182.25	_N_C_E_D AT SECI	"XSEC1".
Section: XSEC1 Header Type: XS SRD: 30.480	182.249 .877 183.126 ***** 182.249 *****		39 30.480 104.637

<< Completed Computations of Profile 2 >>

**************************************										
		EXAM	IPLE 4: STI	EEP CREEK N	EAR NOWHER	E, USA				
			=== U:	ser Defined	Table 1 o	f 1 ===				
		SRD	SRDL	Q	XMIN	FR #	CRWS	WSEL		
2 3 4 5 6	XSEC6 XSEC5 XSEC4 XSEC3 XSEC2	245.974 215.493 165.811 121.920 91.440 60.960 30.480	-30.480 -49.682 -43.891 -30.480 -30.479	362.456 362.456 362.456 362.456 362.456	66.141 62.178 67.056 60.045 43.890	1.002 1.104 1.625 1.626 1.486	184.111 183.822 183.022 182.488 182.086	184.111 183.730 182.428 181.871 181.653		
		EGL								
2 3 4 5 6	XSEC6 XSEC5 XSEC4 XSEC3	185.185 185.172 184.876 184.420 183.901 183.168 183.126	-							

#### Figure 7-13. Output for upstream/downstream profile computations.

The appropriate segments of each profile must be pieced together to form a hydraulically valid profile for the reach. It can be assumed that the subcritical initial water-surface elevation at XSEC1 is valid inasmuch as it yielded successful subcritical computations upstream and a supercritical result could not be obtained in the downstream computations. An assumption that XSEC6 is a critical-flow section can be supported based on the fact that for CRWS it yielded successful computations in both upstream and downstream directions (i.e., supercritical result at XSEC5 and subcritical result at XSEC7). Perhaps further proof of this should be obtained by attempting lower (supercritical) starting conditions at XSEC6 is a critical-flow section, the subcritical result at XSEC7 appears valid. It also seems quite certain, based on the "failure" of upstream computations and the "success" of downstream computations, that a supercritical profile is valid for XSEC5 and XSEC4.

A choice must then be made between two "valid" answers at both XSEC2 and XSEC3. Based on the highest energy-grade-line elevation, it can be assumed that subcritical flow occurs at XSEC2 and supercritical flow occurs at XSEC3. Based on the relatively low Froude numbers at these sections, it could further be assumed that the transition from supercritical flow at XSEC3 and subcritical flow at XSEC2 is in the form of a relatively weak hydraulic jump.

## 7.5 EXAMPLE #5: USER-DEFINED TABLES

Figure 7-14 shows the user-defined tables generated with the computed profiles of the previous section. User-defined tables are requested using the UT record. These tables are output immediately following their associated profile output. They were separated from that output and placed in Figure 7-14 to facilitate discussion. User-defined tables are created by using one (or more) UT record(s) as explained in Table 4-39 and Section 5. The series of numbers in the free-format area of the UT record represents a single "list" of parameter numbers (PARNOS) to generate one user-defined table. A second user-defined table could have been generated for each profile by coding an asterisk and another "list" of PARNOS following the existing "list" of PARNOS (and a third user-defined table by coding another asterisk and a third "list" of PARNOS following the second "list" of PARNOS).

-----Section of output deleted-----

=== User Defined Table 1 of 1 ===

	SRD	SRDL	Q	XMIN	FR #	CRWS	WSEL
1 XSEC1 2 XSEC2 3 XSEC3 4 XSEC4 5 XSEC5 6 XSEC6 7 XSEC7	30.480 60.960 91.440 121.920 165.811 215.493 245.974	******** 30.480 30.479 30.480 43.891 49.682 30.480	$\begin{array}{c} 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\end{array}$	32.613 43.890 60.045 67.056 62.178 66.141 69.493	.801 .586 .838 1.001 1.034 1.002 .755	182.249 182.086 182.488 183.022 183.822 184.111 184.119	182.544 182.858 182.757 183.022 183.822 184.111 184.587
	EGL						
1 XSEC1 2 XSEC2 3 XSEC3 4 XSEC4 5 XSEC5 6 XSEC6 7 XSEC7	183.184 183.264 183.532 184.033 184.870 185.172 185.298						

-----Section of output deleted-----

		SRD	SRDL	Q	XMIN	FR #	CRWS	WSEL
2 3 4 5 6	XSEC7 XSEC6 XSEC5 XSEC4 XSEC3 XSEC2 XSEC1	245.974 215.493 165.811 121.920 91.440 60.960 30.480	********* -30.480 -49.682 -43.891 -30.480 -30.479 -30.480	$\begin{array}{c} 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\\ 362.456\end{array}$	69.493 66.141 62.178 67.056 60.045 43.890 32.613	1.002 1.104 1.625 1.626	184.119 184.111 183.822 183.022 182.488 182.086 182.249	184.119 184.111 183.730 182.428 181.871 181.653 182.249
		EGL						
2 3 4 5 6	XSEC7 XSEC6 XSEC5 XSEC4 XSEC3 XSEC2 XSEC1	185.185 185.172 184.876 184.420 183.901 183.168 183.126						
			Remainder	of output	deleted			

=== User Defined Table 1 of 1 ===

Figure 7-14. User-defined tables.

User-defined tables summarize results in downstream to upstream progression regardless of computational direction. The second parameter selected, SRDL, is the section reference distance (SRD) difference between adjacent sections. Thus, positive values indicate upstream computations and negative values indicate downstream computations. The other parameters selected are pertinent to desirable plotting variables or otherwise associated with determining the "valid" segments of the upstream/downstream profile computations.

## 7.6 EXAMPLE #6: CODING OF DISCHARGE DATA AND DISPLAY COMMANDS

Placement of additional Q data to account for changing discharge(s) along a reach and HP record to obtain properties and (or) plots of sections is subject to specific rules of "order dependence" (unlike most of the record types for this model). However, despite the "order dependence," alternative coding schemes exist. The following illustrations are considerably shortened by eliminating most of the detail of associated record types.

The following examples are based on a reach of stream having five sections with SECID's of XSECA through XSECE. A tributary stream enters the stream reach being modeled between XSECB and XSECC. Hydrologic analyses indicate that for discharges of 141.6 and 99.12 m<sup>3</sup>/s at XSECA, the tributary stream contributes 28.32 and 19.82 m<sup>3</sup>/s, thus dictating the need to specify discharges of 113.28 and 79.30 m<sup>3</sup>/s at XSECC.

Figure 7-15a illustrates one alternative for coding the necessary discharge data. Q data that precede all cross-section data are assigned to the most downstream section. New discharge data may be coded with the cross-section data for the section at which the discharge changes. When coded in this fashion, no SECID is required in the Q record(s) that reflect the new discharge data. However, this Q data must follow all cross-section data (GR, N, ND, SA, and FL records) section at which the new discharge data are introduced.

Figure 7-15b presents a second alternative for coding the appropriate Q data. Q data for the initial section is coded as in the preceding discussion. The Q record(s) for the new discharge data required at XSECC (with the SECID) may be introduced any time after XSECC has been input (but not interspersed with any cross-section data). Most logically this would be immediately prior to the EX record as shown.

Figure 7-15c indicates that all Q data, appropriately identified with the applicable SECID, can be introduced after the involved cross-section data have been input. Again, the most logical placement would be immediately prior to the EX record. This latter alternative permits "grouping" of the Q data so that if any modification(s) and (or) additions/deletions are required, all Q data are readily displayed for revision(s). This can be highly advantageous because, of course, each "set" of Q data must have a consistent number of corresponding entries.

```
0
         141.60 99.12
XS
   XSECA srd_A, [,as needed]
               [ GR, N, ND, and SA data as appropriate ]
XS
   XSECB
          srd_B, [,as needed]
               GR, N, ND, SA, and FL data as appropriate ]
             ſ
XS
          srd_C, [,as needed]
   XSECC
              [ GR, N, ND, SA, and FL data as appropriate ]
Q
         113.28 79.30
XS
   XSECD
          srd_D, [,as needed]
              [ GR, N, ND, SA, and FL data as appropriate ]
XS
   XSECE
          srd_E, [,as needed]
             [ GR, N, ND, SA, and FL data as appropriate ]
ΕХ
      a) new discharge data coded with cross-section data
         141.60 99.12
0
XS
   XSECA srd_A, [,as needed]
               [ GR, N, ND, and SA data as appropriate ]
XS
   XSECB
          srd_B, [,as needed]
                XS
   XSECC
          srd_C, [as needed]
             [ GR, N, ND, SA, and FL data as appropriate ]
XS
   XSECD
          srd_D, [,as needed]
             [ GR, N, ND, SA, and FL data as appropriate ]
XS
   XSECE
         srd_E, [,as needed]
              [ GR, N, ND, SA, and FL data as appropriate ]
Q
   XSECC 113.28
                79.30
ΕX
      b) new discharge data coded with cross-section data
XS
   XSECA
          srd_A, [,as needed]
               [ GR, N, ND, and SA data as appropriate ]
XS
   XSECB
          srd_B, [,as needed]
                GR, N, ND, SA, and FL data as appropriate ]
XS
   XSECC
          srd_C, [as needed]
              [ GR, N, ND, SA, and FL data as appropriate ]
XS
   XSECD
          srd_D, [,as needed]
             [ GR, N, ND, SA,
                              and FL data as appropriate ]
XS
   XSECE
          srd_E, [,as needed]
             [ GR, N, ND, SA, and FL data as appropriate ]
Q
   XSECA
          141.60 99.12
   XSECC
         113.28 79.30
Q
ΕХ
      c) all discharge data coded after all cross-section data
```

Figure 7-15. Alternatives for coding discharge data.

HP records placement follows the same rules as Q record placement, except there is no analogy for an "unidentified" HP record preceding all cross-section data as there is with Q data. HP records may be included with the data of the section(s) of interest, with no SECID required but following all GR, N, ND, SA, and FL data associated with the individual section(s). Alternatively, they may be "grouped," with SECID's, any time after the data for the pertinent section(s) have been input (again, most logically placed immediately prior to the EX record as per the Q records in Figure 7-15c). This latter convention permits ready display of all such records for: (1) any required modification(s); and (or) (2) deletion of some or all of such records if and when the user no longer needs that particular output.

## 7.7 EXAMPLE #7: CROSS-SECTIONAL PROPERTIES

Three different forms of cross-sectional properties are available as optional WSPRO output. Two forms of hydraulic-properties (e.g., area, conveyance, wetted perimeter, etc.) tables are available for a range of elevations with a specified (or default) elevation increment. One of these tables summarizes hydraulic properties for the total section; the second table provides a subarea breakdown of the same hydraulic properties. A third variation of cross-sectional properties can be obtained in the form of velocity and conveyance distribution in any section (except road-grade sections). All of these optional outputs can be obtained along with profile computations or generated separately with abbreviated input data. The following examples use the latter option.

### 7.7.1 Hydraulic Properties for Total Section

Data from the surveyed section in example #3 are used to illustrate the hydraulicproperties output that can be obtained using HP records. Table 4-28 should be consulted for HP record details. Figure 7-16 shows hydraulic-properties output for the total section for a range of elevations from 138.25 to 140.5 in increments of 0.15 m as reflected by the HP record. IHP has been assigned a value of zero.

```
т1
          Example 3: Examples of Input & Output for WSPRO Computer Model
т2
          FHWA/USGS Model for Water-surface Profile
                                                        Computations
Т3
          <<<<< Transfer Known Rating to Upstream Sections >>>>
SI 1
            127.426
Q
            139.394
WS
     SURVY 762.000
ΧТ
            0.000,141.732
                            0.000,140.513
                                              7.010,139.842
                                                                10.973,139.598
GR
                            16.764,139.69030.175,139.72040.538,138.83643.586,138.775
            13.716,139.537
                                                                33.528,138.958
GR
GR
            36.271,138.928
                                                                45.720,138.806
                             47.244,137.770 48.768,137.251
            46.939,138.379
                                                                51.206,137.221
GR
GR
           57.302,137.312
                             58.826,137.617 60.960,137.983
                                                               62.484,138.532
                             69.799,138.867
GR
            64.008,138.775
                                              78.638,138.775
                                                                81.077,139.080
                             92.964,139.537 104.851,139.598
                                                               115.824,140.513
GR
            84.125,139.598
           115.824,141.732
GR
XS
      GAGE 304.800
           -0.671
GT
           0.055,0.050 0.065,0.060 0.040,0.040
Ν
N
                 0.065,0.060 0.055,0.050
            30.175 45.720 64.008 84.125
SA
           0.305,0.914 0.610,1.524 0.000,0.305
ND
ND
                 0.305, 1.219 0.305, 0.914
XS
      XS2 457.200 * * * 0.0020
      XS3 609.600 * * * 0.0012
XS
     EXIT 720.852
FULLV 757.428
XS
XS
     APPR 803.148
XS
HP 0 FULLV 138.379 0.3048 140.818
HP 1 FULLV 138.379 0.3048 140.818
HP 2 FULLV 138.379 0.3048 140.818 127.426
```

* EX ER								
		Secti	lon of d	output de	eleted-			
		ay Admini For Water	stration Surface		. S. Geo le Comp : Units	ologica	l Survey s.	***
	C: EXAMPLES USGS MODEI TRANSFER	G FOR V	VATER-SU	JRFACE I	PROFILE		PUTER MOD FATIONS S >>>>>	EL
*** (	Compute Cros SRD Locatio			erties Fo Hea				* * *
Surface	Cross Section Conveyance	Section					-	Critical Flow
138.684 138.989	320.60 483.93 738.96	19. 29.	$17.4 \\ 47.0$	15.75 18.11 47.76	46.1	63.5	1.082 .611	62.11
139.293 139.598 139.903 140.208 140.512 140.817 140.818	1133.98 1639.75 2368.13 3360.19 4592.25 6096.56 6099.71	60. 89. 121. 155.	50.3 78.4 102.3 109.1 115.7 115.8 115.8	103.15 109.97 116.68 117.29	32.0 10.9 6.3 3.2 .1	82.4 104.9 108.6 112.2 115.8 115.8	.869 1.110 1.342 1.646	129.54 189.61 293.80 426.20 603.61

Figure 7-16. Table of hydraulic properties; total cross-section (IHP=0).

# 7.7.2 Hydraulic Properties by Subarea

By coding IHP = 1, the same data will generate an expanded table that includes a breakdown of the hydraulic properties by subarea. Figure 7-17 shows parts of this expanded table.

F€	ederal Highway Ad Model for W Input Units	ministration ater-Surface : Metric /	CO ************************************	Survey
EXAMPLE FHW	7: EXAMPLES OF A/USGS MODEL FO	INPUT & OUT R WATER-SURF	FPUT FOR WSPRO COMPU FACE PROFILE COMPUTA UPSTREAM SECTIONS	TER MODEL TIONS
* * *			ies For Header Record Header Record Numb	
Water Surface Elevation	S Cross Cr A Section Se # Conveyance Ar	oss ction Top ea(s) Width 	Bank Station Wetted Pmtr Left Right	Hydrlic Critical Depth Flow
138.379	3 320.60 320.60	14. 15.2 14. 15.2	15.75 15.75 46.9 62.1 ctor (alpha): 1.000	.919 41.81 .919 41.81
138.684	3 483.93 483.93 Velocity Head C		18.11 18.11 46.1 63.5 ctor (alpha): 1.000	1.082 61.41 1.082 61.41
138.989	/38.96	29. 47.0	12.36 19.00 16.40 47.76 33.4 80.4 ctor (alpha): 1.285	.132 1.87 1.333 88.18 .167 3.50 .611 62.11
139.293	2 47.46 3 1014.37 4 72.15 1133.98 Velocity Head C	30. 18.3 8. 18.4 44. 50.3	13.73 19.00 18.40 51.13 32.0 82.4 ctor (alpha): 1.529	.409 11.24 1.638 120.09 .438 16.69 .866 102.82
139.598	4 172.75 5 1.52 1639.75	60. 78.4		$\begin{array}{ccccccc} .035 & .08 \\ .664 & 25.49 \\ 1.943 & 155.13 \\ .692 & 36.26 \\ .036 & .45 \\ .769 & 129.54 \end{array}$
139.903	2368.13	15. 15.5 41. 18.3 20. 20.1 8. 24.5 89. 102.3	23.87 15.63 19.00 20.18 24.47 103.15 6.3 108.6 ctor (alpha): 1.872	.2247.92.94544.752.247193.03.99762.70.31313.42.869189.61
140.208	1 150.91 2 365.31 3 2124.61 4 519.03 5 200.34 3360.19 Velocity Head C	13. 27.0 19. 15.5 47. 18.3 26. 20.1 16. 28.1 121. 109.1 orrection Fac	27.02 15.63 19.00 20.18 28.14 109.97 3.2 112.2 ctor (alpha): 1.849	.485 28.57 1.250 68.06 2.552 233.61 1.301 93.56 .557 36.64 1.110 293.80
	$\begin{array}{cccc} 1 & 340.83 \\ 2 & 538.65 \\ 3 & 2564.11 \\ 4 & 737.07 \\ 5 & 411.59 \end{array}$	22.30.124.15.552.18.332.20.125.31.7	30.12 15.63 19.00 20.18 31.74	.72558.141.55594.422.857276.681.606128.29.78268.68

WSPRO User's Manual

SECTION 7. Examples of Model Input and Output

 140.512
 4592.25
 155.
 115.7
 116.68
 .1
 115.8
 1.342
 426.20

 Velocity Head Correction Factor (alpha): 1.749

		ral Highway A Model for Input Unit	dminist Water-S	ration Surface	- U.S Profile	6. Geolo Computa	ogical : ations.		- *
	A/US	EXAMPLES C GGS MODEL F TRANSFER KNC	'OR WAT	TER-SURF	ACE PRO	FILE (	COMPUTA	-	
* * *		mpute Cross-S RD Location:							* * *
Water Surface Elevation	S A #	Cross C Section S Conveyance A	cross ection crea(s)	Top Width	Wetted Pmtr	Bank St Left	ation Right	Hydrlic Depth	Flow
140.817	2 3 4 5	626.70 725.90 3036.06 984.64 723.26 6096.56 elocity Head	29. 58. 38. 34. 191.	15.5 18.3 20.1 31.7 115.8	15.63 19.00 20.18 32.05 117.29	.1	115.8	1.029 1.860 3.162 1.911 1.086	98.41 123.50 322.12 166.49 112.50
140.818	4 5	627.31 726.30 3037.03 985.16 723.92 6099.71	29. 58. 38. 34. 191.	15.5 18.3 20.1 31.7 115.8	15.63 19.00 20.18 32.05 117.29	.1	115.8	1.860 3.162 1.912 1.087	322.21 166.57 112.59
	Ve 	elocity Head	Correct	ion Fac	tor (alp	oha): 1 	.611		

Figure 7-17. Table of hydraulic properties; subarea breakdown (IHP=1).

# 7.7.3 Velocity and Conveyance Distribution

Velocity and conveyance distribution(s) for the same data can be obtained by coding IHP = 2. Sample results are illustrated in Figure 7-18.

Fede	ral Highw Model Input	way Adminis for Water- Units: Met	tration - Surface Pro ric / Out	U. S. Geo ofile Compu cput Units:	*********** logical Sur tations. Metric	vey
EXAMPLE 7: FHWA/U	EXAMPLE SGS MODE	ES OF INP EL FOR WA	UT & OUTPUI TER-SURFACE	FOR WSP PROFILE	RO COMPUTER COMPUTATIO ECTIONS >>	MODEL NS
SR	D Locatio	on: 757.	428 F	leader Reco	Record FUL rd Number	5
Flow: Cross Ba	127. Section . nk Statio	.426 Velc Area: ons -> Lef	city: 9.15 13.92 t: 46.9	5 Hydrauli Conveya 922 Right	Element c Depth: nce: 3 : 62.07	.919 20.60 6
X STA.	46.9	48.4	49.0	49.6	50.1	50.6
A( I )		1.0	.7	.6	.6	.6
V( I )		6.45	9.00	9.91	10.44	10.44
D( I )		.69	1.09	1.14	1.15	1.15
X STA.	50.6	51.2	51.7	52.2	52.7	53.2
A( I )		.6	.6	.6	.6	.6
V( I )		10.52	10.63	10.65	10.47	10.54
D( I )		1.16	1.16	1.15	1.15	1.14
X STA.	53.2	53.8	54.3	54.9	55.5	56.1
A( I )		.6	.6	.6	.6	.7
V( I )		10.42	10.17	10.29	10.03	9.75
D( I )		1.13	1.12	1.11	1.10	1.10
X STA. A( I ) V( I ) D( I )						
Water Flow:	Surface	Elevation: .426 Velc	138.684 city: 6.76	1 5 Hydrauli	Element c Depth: nce: 4 : 63.47	# 2 1.082
X STA.	46.1	48.3	49.0	49.6	50.2	50.7
A(I)		1.5	1.0	.9	.8	.8
V(I)		4.24	6.70	7.21	7.65	7.72
D(I)		.66	1.39	1.45	1.45	1.46
X STA.	50.7	51.3	51.8	52.4	52.9	53.5
A( I )		.8	.8	.8	.8	.8
V( I )		8.01	7.99	8.16	8.01	8.06
D( I )		1.47	1.46	1.46	1.45	1.44
X STA.	53.5	54.0	54.6	55.2	55.8	56.4
A(I)		.8	.8	.8	.8	.9
V(I)		7.95	7.75	7.83	7.62	7.40
D(I)		1.43	1.42	1.41	1.40	1.40
X STA.	56.4	57.0	57.7	58.6	59.7	63.5
A( I )		.9	.9	1.0	1.2	1.7
V( I )		7.17	6.92	6.15	5.45	3.65
D( I )		1.39	1.35	1.21	1.02	.46

	************* Federal High Model Input	way Adminis for Water- Units: Met	tration - Surface Pro ric / Out	U. S. Geol file Comput put Units:	ogical Surv ations. Metric	уеу
EXAMI I	PLE 7: EXAMPL FHWA/USGS MOD <<<< TRANSFER	ES OF INP EL FOR WA	UT & OUTPUT TER-SURFACE	FOR WSPR PROFILE	O COMPUTER	MODEL IS
* * *		Velocity Di on: 757.	stribution 428 H	For Header eader Recor	Record FULI d Number 5	JV ***
	Water Surface Flow: 127 Cross-Section Bank Statio	.426 Velo	citv: 4.43	Hvdraulic	Depth:	.611
X STA. A( I V( I D( I	33.4 ) )	47.9 3.3 1.92 .23	48.8 1.4 4.61 1.60	49.4 1.1 5.55 1.75	50.1 1.1 5.55 1.76	50.7 1.1 5.92 1.76
X STA. A( I V( I D( I	50.7 ) )	51.3 1.1 5.86 1.77	51.9 1.0 6.08 1.77	52.5 1.0 6.08 1.76	53.1 1.1 5.97 1.75	53.7 1.1 6.00 1.74
X STA. A( I V( I D( I	53.7 ) )	54.4 1.1 5.85 1.73	55.0 1.1 5.88 1.72	55.6 1.1 5.84 1.71	56.3 1.1 5.68 1.70	57.0 1.2 5.51 1.69
X STA. A( I V( I D( I	57.0 ) )	57.7 1.2 5.40 1.66	58.5 1.2 5.10 1.52	59.5 1.4 4.54 1.35	61.0 1.7 3.82 1.13	80.4 4.3 1.48 .22
	Water Surface Flow: 127 Cross-Section Bank Statio	.426 Velo	city: 2.92	Hydraulic	Depth:	.866
X STA. A( I V( I D( I	32.0 ) )	46.6 6.2 1.03 .42	48.2 2.2 2.85 1.43	49.0 1.7 3.82 1.98	49.8 1.5 4.19 2.06	50.5 1.5 4.23 2.07
X STA. A( I V( I D( I		51.2 1.5 4.39 2.07	51.9 1.5 4.37 2.07	52.6 1.4 4.46 2.06	53.3 1.5 4.38 2.05	54.0 1.4 4.40 2.04
X STA. A( I V( I D( I	54.0 ) )	54.7 1.5 4.34 2.03	55.5 1.5 4.37 2.02	56.2 1.5 4.19 2.01	57.0 1.5 4.17 2.00	57.8 1.6 4.10 1.96
X STA. A( I V( I D( I	) )	58.7 1.7 3.79 1.80	59.8 1.8 3.48 1.61	61.3 2.0 3.13 1.37	67.3 3.7 1.70 .63	82.4 6.4 .99 .43

	*************** Federal High Model Input	way Adminis for Water- Units: Met	tration - Surface Pro ric / Out	U. S. Geol file Comput put Units:	ogical Surv ations. Metric	vey
EXAMI I	PLE 7: EXAMPL FHWA/USGS MOD <<<< TRANSFER	ES OF INP EL FOR WA	UT & OUTPUT TER-SURFACE	' FOR WSPR PROFILE	O COMPUTER COMPUTATION	MODEL IS
* * *	Beginning SRD Locati	Velocity Di on: 757.	stribution 428 H	For Header leader Recor	Record FULI d Number 5	JV ***
	Water Surface Flow: 127 Cross-Section Bank Stati	.426 Velo	city: 2.11	Hvdraulic	Depth:	.641
X STA. A( I V( I D( I	10.9 ) )	42.4 7.4 .86 .24	47.4 4.7 1.35 .94	48.5 2.3 2.72 2.08	49.4 2.1 3.03 2.35	50.2 1.9 3.31 2.37
X STA. A( I V( I D( I	50.2 ) )	51.1 1.9 3.28 2.38	51.9 1.9 3.39 2.38	52.6 1.9 3.39 2.37	53.5 1.9 3.33 2.36	54.3 1.9 3.35 2.34
	54.3 ) )					
X STA. A( I V( I D( I	58.7 ) )	59.8 2.2 2.83 1.91	61.3 2.5 2.55 1.68	65.0 3.8 1.69 1.02	72.9 6.0 1.06 .77	104.9 7.8 .81 .24
	Water Surface Flow: 127 Cross-Section Bank Stati	.426 Velo	city: 1.43	Hydraulic	Depth:	.869
	6.3 ) )					
X STA. A( I V( I D( I	49.9 ) )	50.9 2.6 2.47 2.68	51.8 2.5 2.55 2.68	52.7 2.5 2.55 2.67	53.7 2.5 2.51 2.66	54.6 2.5 2.52 2.64
X STA. A( I V( I D( I	54.6 ) )	55.6 2.5 2.50 2.63	56.6 2.5 2.51 2.62	57.6 2.6 2.42 2.59	58.7 2.8 2.32 2.43	60.0 2.9 2.17 2.20
X STA. A( I V( I D( I	)	61.8 3.3 1.94 1.90	65.8 5.0 1.28 1.24	72.3 6.9 .92 1.07	78.7 7.0 .91 1.10	108.6 11.8 .54 .39

	Tnnut	way Adminis for Water-	tration - Surface Pro	U. S. Geol file Comput	ogical Surv ations.	vey
E	PLE 7: EXAMPL: FHWA/USGS MOD <<<< TRANSFER	EL FOR WA'	TER-SURFACE	PROFILE	COMPUTATION	IS
***	Beginning SRD Locatio	Velocity Dia on: 757.	stribution 428 H	For Header eader Recor	Record FULL d Number 5	V ***
	Water Surface Flow: 127 Cross-Section Bank Statio	.426 Velo	city: 1.05	Hydraulic	Depth: 1	110
		32.3 14.6 .44 .50				
	49.2 ) )					
X STA. A( I V( I D( I	54.9 ) )	56.0 3.4 1.89 2.93	57.2 3.3 1.90 2.91	58.4 3.5 1.82 2.80	59.9 3.8 1.67 2.54	61.7 4.1 1.57 2.23
X STA. A( I V( I D( I	61.7 ) )	65.6 6.0 1.06 1.56	71.6 8.3 .77 1.37	77.4 8.1 .79 1.40	88.0 10.0 .64 .94	112.2 13.2 .48 .55
	Water Surface Flow: 127 Cross-Section Bank Statio	Elevation: .426 Velo Area: 1 ons -> Lef	140.512 city: .82 55.32 t: .1	Hydraulic Conveyan 00 Right:	Element Depth: 1 ce: 459 115.824	# 8 342 92.25
X STA. A( I V( I D( I	.1 ) )	22.0 15.2 .42 .69	34.9 12.7 .50 .98	41.2 10.3 .62 1.63	46.4 9.0 .71 1.74	48.5 5.5 1.15 2.63
X STA. A( I V( I D( I		49.8 4.4 1.44 3.26	51.1 4.3 1.49 3.29	52.4 4.2 1.51 3.29	53.7 4.3 1.49 3.27	55.0 4.3 1.50 3.25
X STA. A( I V( I D( I	55.0 ) )	56.4 4.3 1.48 3.23	57.7 4.3 1.50 3.20	59.2 4.6 1.39 2.98	61.1 4.9 1.29 2.68	64.0 6.0 1.06 2.05
X STA. A( I V( I D( I	) )	.65	75.5 9.7 .66 1.68	81.8 10.3 .62 1.63	94.4 12.4 .51 .98	115.8 14.9 .43 .70

Mod	ghway Adminis el for Water ut Units: Met	stration – -Surface Pro tric / Out	U. S. Geo ofile Comput cput Units:	logical Surv tations. Metric	vey
EXAMPLE 7: EXAM FHWA/USGS M <<<<< TRANSF	PLES OF IN ODEL FOR WA	PUT & OUTPUT ATER-SURFACE	F FOR WSPE E PROFILE	RO COMPUTER COMPUTATION	MODEL IS
SRD Loca	g Velocity D: tion: 757	.428 H	leader Reco	rd Number S	5
Flow: 1 Cross-Secti Bank Sta	ce Elevation 27.426 Velo on Area: E tions -> Le:	ocity: .65 190.60 ft: .(	7 Hydraulio Conveyar )75 Right	c Depth: 2 nce: 609 : 115.849	L.646 96.56 9
X STA1 A(I) V(I) D(I)					
X STA. 47.6 A(I) V(I) D(I)	49.2 5.7 1.12 3.43	50.7 5.3 1.19 3.59	52.2 5.3 1.21 3.60	53.7 5.4 1.19 3.58	55.2 5.3 1.20 3.55
X STA. 55.2 A(I) V(I) D(I)	56.7 5.3 1.21 3.53	58.3 5.5 1.16 3.45	60.1 5.8 1.09 3.14	62.6 6.7 .96 2.66	67.4 9.8 .65 2.06
X STA. 67.4 A(I) V(I) D(I)	73.4 11.8 .54 1.98	78.9 11.3 .57 2.02	88.2 13.6 .47 1.47	98.8 13.4 .47 1.27	115.8 16.0 .40 .93
Water Surfa Flow: 1 Cross-Secti Bank Sta	ce Elevation 27.426 Velo on Area: : tions -> Le:	: 140.818 ocity: .6 190.67 ft: .(	3 7 Hydraulio Conveyar )74 Right	Element C Depth: 1 nce: 609 : 115.849	#10 L.647 99.71
X STA1 A(I) V(I) D(I)	16.9 16.1 .40 .96	29.4 14.1 .45 1.12	37.3 12.9 .49 1.64	43.3 11.9 .53 1.98	47.6 9.5 .67 2.23
X STA. 47.6 A(I) V(I) D(I)	1.12	50.7 5.3 1.19 3.59	1.21	1.19	1.20
X STA. 55.2 A(I) V(I) D(I)	56.7 5.3 1.21 3.53	1.16 3.46	5.8 1.10 3.15	6.6 .96 2.67	9.8 .65
X STA. 67.3 A(I) V(I) D(I)	73.3 11.7 .54 1.98	79.1 11.7 .55 2.02	88.2 13.4 .48 1.46	98.8 13.4 .47 1.27	115.8 16.0 .40 .94

Figure 7-18. Table of velocity and conveyance distribution (IHP=2).

## 7.8 EXAMPLE #8: MULTIPLE-OPENING BRIDGE

Figure 7-19 shows the input data for a simple, multiple-opening bridge problem. A definition sketch for the problem is shown in Figure 7-20. The valley is 152.4 m wide with two 15.24 m low-flow channels. Each channel bridge is 33.528-m long. Component mode is used for both bridges (BL, BC, and CD records). The geometry (GR records) and roughness (N record) of the downstream match section, DNSTM, is assumed representative of the entire reach. Those data are thus propagated to the full valley, FULLV, and upstream match, UPSTM, sections with elevation adjustments for a valley slope of 0.002 (XS DNSTM record). DNSTM and UPSTM are located at SRDs that satisfy exit and approach section locations for the longest opening. One profile is computed for a discharge of 226.535 m<sup>3</sup>/s using an energy gradient (SK record) equal to the valley slope (assuming uniform flow downstream from DNSTM).

```
т1
           Example 8: Multiple Opening Bridge Example
           Symmetrical Valley
т2
т3
          33.5 And 27.4 Meter Long Bridges
*
SI 1
*
          226.535
Q
SK
            0.0020
     DNSTM 0.000 * * *
XS
                             0.0020
             0.000,6.096 0.000,1.524
                                            30.480,1.524
                                                              30.480,0.000
GR
            45.720,0.000 45.720,1.524 106.680,1.524 106.680,0.000
121.920,0.000 121.920,1.524 152.400,1.524 152.400,6.096
GR
GR
N
            0.040
*
     FULLV 33.528
XS
* Left Channel Bridge (33.5 Meter Bridge)
      LTBO 33.528
BR
ΒL
            33.528
                    30.480 45.720
BC
             3.658
                    15.240
CD
                 1
* Right Channel Bridge (27.4 Meter Bridge)
BR
      RTBO 33.528
BL
            27.432 106.680 121.920
BC
            3.658
                    15,240
CD
                 1
*
XS
     UPSTM 82.296
ΕX
ER
```

Figure 7-19. Input data for multiple-opening bridge of example #8.

SECTION 7. Examples of Model Input and Output

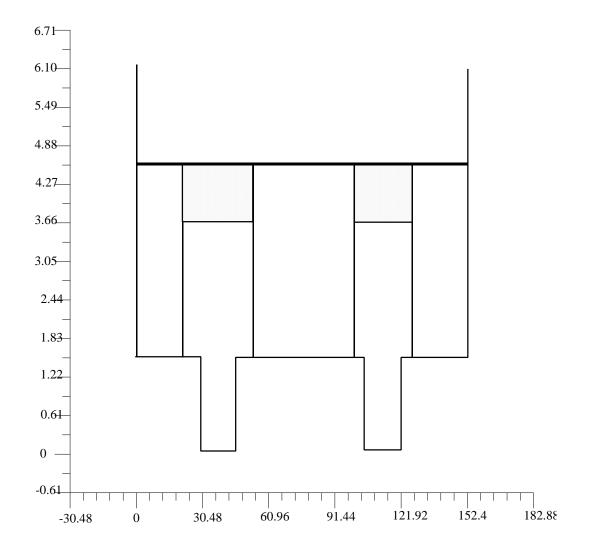


Figure 7-20. Definition sketch: multiple-opening bridge of example #8

Figure 7-21 is part of the profile output. The first lines show the unconstricted profile results. These results are used to make the initial estimates of the valley strips and their properties shown in the next lines. First shown are the first iteration backwater computations. A step-backwater computation is made to determine the elevation at UPSTM for that strip. The valley-strip elevation and conveyance at UPSTM for each opening are then used to compute the conveyance-weighted elevation at UPSTM.

<< Beginning Computations for Profile 1 >>

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: DNSTM	2.428	.077	226.535	184.220	********	.080
Header Type: XS	2.505	*****	1.229	5063.46	*********	152.419
SRD: .000	1.826	*****	.357	*****	1.000	*****
Section: FULLV	2.496	.076	226.535	184.448	33.528	.080
Header Type: FV	2.573	.066	1.228	5073.87	33.528	152.419
SRD: 33.528	1.893	.000	.356	.0020	1.000	.005

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

Section: UPSTM	2.595	.076	226.535	184.635	48.767	.080
Header Type: A	S 2.672	.097	1.226	5082.34	48.767	152.419
SRD: 82.29	5 1.993	.000	.356	.0020	1.000	.005

<<< The Preceding Data Reflect The "Unconstricted" Profile >>>

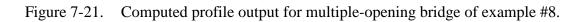
<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

Input	l for Wate t Units: N	er-Suri Metric	face Prof: / Outpu	ile Compu ut Units:	utation Metri	s.	- *
EXAMPI	SYI	MMETRIC	OPENING D CAL VALLD METER LONG	EY			
Multiple Bridg	ge Opening	g Calcı	ulations		It	eration 1	
Stagnation Po Bridge 1 Section LTBO	int ***** A3 QS	* * * * H	BOLEW BOREW	CA3 CJ	CDF CRF	CDFn QSn	
AS 94.829 KS 2597.8							
Stagnation Po Bridge 2 Section RTBO	int 79 A3 QS	.017 H	BOLEW BOREW	CA3 CJ	CDF CRF	CDFn QSn	
AS 89.807 KS 2484.7	48.0 107	024 1 .11 1	100.640 128.060	42.334 .882	.967 2.305	.972 107.63	
Stagnation Po Total Values All Sections	int ***** A3 QS	* * * * I I	BOREW BOLEW	CA3 CJ	CDF CRF	CDFn QSn	
AS 184.635 KS 5082.5	******	* * * * * * * * * * * * * * * * * 	* * * * * * * * * * * * * * * * * * 	89.194 ******* 	****	**********	
KS 5082.5	****** WSEL EGEL CRWS	*** ***  VHD HF HO	******* ** Q V FR #	*******  ARE K SE	*****  CA	**********  SRDL	LEW REW ERR
KS 5082.5 	******** WSEL EGEL CRWS 2.356 2.749	*** *** VHD HF HO . 392 . 123	Q V FR # 	********  K SF  02 48 33 144	*****  2A 	**************************************	REW ERR 21.398 54.900
KS 5082.5 	******** WSEL EGEL CRWS 2.356 2.749 1.985	VHD HF HO .392 .123 .117	Q V FR # 	******** ARE K SE  02 48 33 144 44 **	***** 	**************************************	REW ERR 21.398 54.900 .000
KS 5082.5 	******** WSEL EGEL CRWS 2.356 2.749 1.985 formation ow Type 1	VHD HF HO .392 .123 .117 C	Q V FR # 118.90 2.4 .60	********  K SE  02 48 33 144 44 ** PFELEV	***** CA 3.854 8.88 **** BLEN	**************************************	REW ERR 21.398 54.900 .000 XRAB
KS 5082.5 	WSEL EGEL CRWS 2.356 2.749 1.985 formation fow Type 1 Full Valle	VHD HF HO .392 .123 .117 C 	Q V FR # 118.90 2.4: 	******** ARF K SE 02 48 33 144 44 ** PFELEV  3.657 r Surface	***** EA 5 .854 8.88 **** BLEN .33.52 .3.52 	**************************************	REW ERR 21.398 54.900 .000 XRAB 6 54.80
KS 5082.5 	WSEL EGEL CRWS 2.356 2.749 1.985 formation bw Type 1  Full Valle dge Section Distance WSEL EGEL CRWS	VHD HF HO .123 .117 C .8777 	Q V FR # 118.90 2.4: 	******** ARE K SE 02 48 33 144 44 ** PFELEV 3.657  r Surface e Elevati ARE K SE	***** EA 3.854 48.88 **** BLEN 33.52  2 Eleva con:  EA	**************************************	REW ERR 21.398 54.900 .000 XRAB 6 54.80 2.496 2.356
KS 5082.5 	WSEL EGEL CRWS 2.356 2.749 1.985 formation ow Type 1 Full Valle dge Section Distance WSEL EGEL CRWS 2.893	VHD HF HO .392 .123 .117 C  ey Secton Wate e:  VHD HF HO  051 .092	Q V FR # 118.90 2.4: 	**************************************	***** EA 5 8.854 8.88 **** BLEN 33.52 9 2 Eleva con: 2 2 2 2 3 3 5 2 3 3 5 2 3 3 5 2 3 3 5 2 3 3 5 2 3 5 2 5 2	**************************************	REW ERR 21.398 54.900 .000 XRAB 6 54.80 2.496 2.356 .140 LEW REW ERR .073
KS 5082.5 	WSEL EGEL CRWS 2.356 2.749 1.985 formation ow Type 1 Full Valle dige Section Distance WSEL EGEL CRWS 2.893 2.944 2.008	VHD HF HO .392 .123 .117 C .877 .877 .877	Q V FR # 	ARF K SE 02 48 33 144 44 ** PFELEV 3.657 	***** EA BLEN 3.854 48.88 **** BLEN 33.52 Eleva on: EA CON: EA CON: CON: EA CON: EA CON: EA CON: EA CO	**************************************	REW ERR 21.398 54.900 .000 XRAB 6 54.80 2.496 2.356 .140 LEW REW ERR ERR .073 79.017

<<< End of Bridge Hydraulics Computations >>>							
Section: UPSTM Header Type: XS SRD: 82.295	2.894 2.945 2.008	.051 .000 .000	118.902 1.004 .262	118.406 3853.15 .0010	.000 .000 1.000	.073 79.017 .002	
_	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR	
Section: RTBO Header Type: BR SRD: 33.528	2.343 2.738 1.907	.395 .097 .127	2.455 .620	43.826 1360.02 *****	27.432 1.284	128.052 .000	
Specific Bridge Information C P/A PFELEV BLEN XLAB XRA Bridge Type 1 Flow Type 1						XRAB	
Pier/Pile Code **		.8824	.000 3.	657 27.4	32 100.58	34 128.016	
Unconstricted Full Valley Section Water Surface Elevation: 2.496 Downstream Bridge Section Water Surface Elevation: 2.343 Bridge DrawDown Distance: .153							
	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR	
Section: SLICE Header Type: AS SRD: 76.199	2.876 2.924	.047 .090	107.632 .966 .251	111.348 3446.77 .0020	33.528 36.485 1.000	79.017 152.426 .001	
Approach Section SLICE Flow Contraction Information M(G) M(K) KQ XLKQ XRKQ OTEL							
			8.3 100.8			-	
<<< End of Bridge Hydraulics Computations >>>							
Section: UPSTM Header Type: XS SRD: 82.295	2.882 2.930 1.974	.048 .005 .000	107.632 .970 .252	110.901 3423.93 .0010	6.096 6.096 1.000	79.017 152.425 .001	
Section: UPSTM Header Type: XS SRD: 82.295						0.5.0	

******************* Federal Highv Model Input	vay Admini for Water Units: Me	stratio -Surfac tric /	n - U e Profi Outpu	. S. Geo le Comp t Units	ologica utation : Metri	l Survey s. c		
EXAMPLE	E 8: MULT	IPLE OP ETRICAL	ENING B VALLE	RIDGE E: Y	XAMPLE		_ ^	
Multiple Bridge	e Opening	Calcula	tions		It	eration 2		
Stagnation Poir Bridge 1 Section LTBO	nt ****** A3	** BOL	EW	CA3	CDF CRF	CDFn QSn		
AS 118.947 KS 3674.2	48.85 119.4	5 21 3 54	.399 .901	42.661 .873	1.024 2.301	1.023 118.82		
Stagnation Poir Bridge 2 Section RTBO	nt 79.0 A3 QS	15 BOL BOR	EW EW	CA3 CJ	CDF CRF	CDFn QSn		
AS 112.239 KS 3492.4	43.82 107.1	6 100 1 128	.648 .052	38.616 .881	.975 2.304	.976 107.71		
Stagnation Poir Total Values All Sections	QS	BOL	EW	CJ	CRF	CDFn QSn		
AS 231.185 KS 7166.6	* * * * * * * * * * * * * * * * * * * *	* ***** * *****	* * * * * * * * * * *	81.277 ******	* * * * * * * * * *			
	WSEL V EGEL CRWS	HD HF HO	Q V FR #	ARI K SI	EA F	SRDL FLEN ALPHA	LEW REW ERR	
Section: LTBO Header Type: BR SRD: 33.528	2.357 2.749	.392 .123	118.82 2.43	2 43 1 14	8.872 49.72 ****	33.528 33.528 1.300	21.3 54.9	398 900 000
Specific Bridge Info Bridge Type 1 Flow	ormation v Type 1 -	C	P/A P	FELEV	BLEN	XLAB	2	XRAB
Pier/Pile Code **		.8771 	.000	3.657	33.52	8 21.336	5 5	54.863
Unconstricted Fu Downstream Bridg Bridge DrawDown	je Section	Sectio Water	n Water Surface	Surface Elevat:	e Eleva ion:	tion:	2.496 2.35 .139	/
	WSEL V EGEL CRWS	HF HO	Q V FR #	ARI K SI		SRDL FLEN ALPHA	LEW REW ERR	
Section: SLICE	2.893	.051	118.82	2 11	8.338	33.528 36.672 1.000	.( 79.( .(	073 015 001
M(G)	ach Section M(K)	KQ	XL	KQ		ormation OTEL		
.575	. 384				55.33	5 2.893 		

<<< End of Bridge Hydraulics Computations >>>							
Section: UPSTM Header Type: XS SRD: 82.295	2.894 2.945 2.008	.051 .000 .000	118.822 1.003 .262	118.395 3838.26 .0010	.000 .000 1.000	.073 79.015 .002	
	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR	
Section: RTBO Header Type: BR SRD: 33.528	2.343 2.739 1.907	.395 .096 .127	107.712 2.458 .621	43.812 1359.33 *****	27.432 27.432 1.284	100.647 128.052 .000	
Specific Bridge In Bridge Type 1 Fl	formation	C	P/A PFE	LEV BLEN	XLAB	XRAB	
Pier/Pile Code **		.8826	.000 3	8.657 27.43	32 100.58	4 128.016	
Pier/Pile Code **.8826.0003.65727.432100.584128.01Unconstricted Full Valley Section Water Surface Elevation:2.496Downstream Bridge Section Water Surface Elevation:2.343Bridge DrawDown Distance:.153							
	WSEL EGEL CRWS	VHD HF HO	Q V FR #	AREA K SF	SRDL FLEN ALPHA	LEW REW ERR	
Section: SLICE Header Type: AS SRD: 76.199	2.876 2.924 1.963	.047 .090 .095	107.712 .967 .251	111.360 3460.98 .0020	33.528 36.485 1.000	79.015 152.426 .001	
M(G)	M(K)	KQ	XLKÇ	ntraction Inf	OTEL		
	6.40	3 206	6.6 100.	850 128.25	5 2.876		
<<<	End of B:	ridge Hy	draulics (	Computations	>>>		
Section: UPSTM Header Type: XS SRD: 82.295	2.882 2.931 1.975	.048 .005 .000	107.712 .971 .252	110.911 3437.92 .0010	6.096 6.096 1.000	79.015 152.425 .001	
Section: UPSTM Header Type: XS SRD: 82.295	2.888 2.938 1.993	.049 ***** *****	226.535 .987 .257	229.322 * 7275.96 * *****	********* ********* 1.000	.073 152.426 .005	
<< Completed Computations of Profile 1 >>							
ER							
**************************************							
Remainder of output deleted							



\_

WSPRO User's Manual

## 7.9 EXAMPLE #9: CULVERTS

WSPRO was originally designed to perform culvert computations only in multipleopening analyses but was later "patched" to analyze "stand-alone" culverts (both single- and multi-barrel installations). This version of WSPRO removes the limitations of previous versions in computing continuous culvert profiles.

A culvert installation is required for a design discharge of 135.94 m<sup>3</sup>/s. Site characteristics and other factors suggest a battery of six 2.7-m, circular-concrete pipes might suffice. Parts of the culvert analysis output are shown in Figure 7-22.

```
CONTINUOUS WATER SURFACE PROFILE WITH CULVERT IN REACH
т1
т2
          METRIC
SI 1
0
              22.6536
             139.2140
WS
XT
   SURVY 762.000
             0.000,141.732
                              0.000,140.513
                                                7.010,139.842
                                                                  10.973,139.598
GR
                             16.764,139.69030.175,139.72040.538,138.83643.586,138.775
            13.716,139.537
                                                                  33.528,138.958
GR
            36.271,138.928
                                                                  45.720,138.806
GR
                             47.244,137.770 48.768,137.251
            46.939,138.379
                                                                  51.206,137.221
GR
                             58.826,137.61760.960,137.98369.799,138.86778.638,138.775
GR
            57.302,137.312
                                                                  62.484,138.532
            64.008,138.775
                                                                  81.077,139.080
GR
                              92.964,139.537 104.851,139.598
GR
            84.125,139.598
                                                                 115.824,140.513
GR
           115.824,141.732
XS
      EXIT 720.852 * * * 0.0012
GΤ
Ν
           0.055,0.050 0.065,0.060 0.040,0.040
                 0.065,0.060 0.055,0.050
N
             30.175 45.720 64.008 84.125
SA
           0.305,0.914 0.610,1.524 0.000,0.305
ND
ND
                 0.305,1.219 0.305,0.914
     FULLV 757.428
XS
CV
     BOXCV 757.428 51.206 30.48 137.21 137.24 1
CG
           115 3048 3658
     APPR 803.148
XS
ΕX
ER
```

Figure 7-22. Input data for culvert analysis.

A convenient way to estimate the road overflow is to substitute an "equivalent" bridge opening for the culvert. This should not be considered as an alternative for culvert analysis (culvert hydraulics and bridge hydraulics are different) but can be used to obtain a valid "rating" for the road overflow. This example uses a bridge opening having an area equivalent to a battery of culverts. Part of the profile output is shown in Figure 7-23. It should be noted that the headwater elevation from culvert computations is for a point immediately upstream from the culvert, 19.8 m downstream from the approach section. The water-surface elevation difference between these points is less than 0.03 m for "normal" flow. If adjustment is made for total loss of velocity head for ponded conditions the difference may be about 0.06 m. The elevation immediately upstream from the embankment used for road-overflow computations in WSPRO is the approach elevation minus an "estimated" friction loss. Thus, we have a road overflow of 3.716 m<sup>3</sup>/s at a 183.279 HWE.

* * *	Federal N Run Da	Highway Admi Model for Wat ate & Time:	** W S P R O ****** nistration - U. S. ( er-Surface Profile Con 9/11/97 8:30 am DAT Output File	Geological Survey mputations. Version V070197
T1 T2 SI 1	CON] METF		SURFACE PROFILE WITH	CULVERT IN REACH
UT T		Metric	(SI) Units Used in W	SPRO
		Quantity	SI Unit	Precision
	-	Length Depth Elevation Widths	meters meters	0.001 0.001 0.001 0.001 0.001
		Discharge	meters/second cubic meters/second meter/meter	
	-	Angles	degrees	0.01
Q		22.6536		
* * *	Process	sing Flow Dat	a; Placing Information	n into Sequence 1 ***

WS 139.2140

Federa	l Highway Adr Model for Wa	**** W S P R ninistration ater-Surface : : Metric /	- U. S. Geo Profile Compu	ological Sur utations.	******* rvey *
		SURFACE PROFI			I
* * *		To Process H			- * * - *
GR       13         GR       36         GR       46         GR       57         GR       64         GR       84	62.000 .000,141.732 .716,139.537 .271,138.928 .939,138.379 .302,137.312 .008,138.775 .125,139.598 .824,141.732	16.764,139 40.538,138 47.244,137 58.826 137	.690 30.175 .836 43.586 .770 48.768 .617 60.960 .867 78.638	0,139.842 5,139.720 5,138.775 3,137.251 0,137.983 3,138.775 1,139.598	10.973,139.598 33.528,138.958 45.720,138.806 51.206,137.221 62.484,138.532 81.077,139.080 115.824,140.513
*** Comple +++072 NOTICE: +++072 NOTICE: *** SRD Location	X-coordinat X-coordinat Storing Temp Data Su	plate Header i ummary For He	sed to elimir sed to elimir Record Data J	nate vertica nate vertica In Memory SURVY	al segment. al segment. ***
Х	Х, Y- Y	-coordinates X	(29 pairs) Y	х	Y
$\begin{array}{c} .000\\ 10.973\\ 30.175\\ 40.538\\ 46.939\\ 51.206\\ 60.960\\ 69.799\\ 84.125\\ 115.824\\ \end{array}$	141.732 139.598 139.720 138.836 138.379 137.221 137.983 138.867 139.598 140.513	$\begin{array}{c} .100\\ 13.716\\ 33.528\\ 43.586\\ 47.244\\ 57.302\\ 62.484\\ 78.638\\ 92.964\\ 115.924\end{array}$	140.513 139.537 138.958 138.775 137.770 137.312 138.532 138.775 139.537 141.732	$\begin{array}{c} 7.010\\ 16.764\\ 36.271\\ 45.720\\ 48.768\\ 58.826\\ 64.008\\ 81.077\\ 104.851 \end{array}$	
Maximum X-S Minimum Y-B	Station: Station: Elevation:	and Maximum 1 .000 ( a 115.924 ( a 137.221 ( a 141.732 ( a	ssociated Y-E ssociated Y-E ssociated X-S	Elevation: Elevation: Station:	141.732 ) 51.206 )
*					

Federal	Highway Admir Model for Wate Input Units: M	nistration – er-Surface Pr Metric / Ou	U. S. Geol ofile Comput tput Units:	ogical Surve ations. Metric	У
CONTI	NUOUS WATER SUF	FACE PROFILE METRIC	WITH CULVER	RT IN REACH	
* * *	Starting To	Process Hea			
GT	20.852 * * * 0.				
N SA	.055,0.050 0.0 0.065,0.060 30.175 45.720 .305,0.914 0.6 0.305,1.219	0.055,0.050 64.008 84	.125 000,0.305		
	eted Reading Da g X-Section Dat				
Valley Slope	Data Summ n: 721. e: .00120 Coefficients -	Averaging Co	n Skew: . nveyance By	0 Error Co Geometric Me	an.
Х	Y	oordinates (2 X	9 pairs) Y	Х	Y
.000 10.973 30.175 40.538 46.939	141.683 139.549 139.671 138.787 138.330	.100 13.716 33.528 43.586 47.244		36.271 45.720	139.793 139.641 138.879 138.757 137.202
51.206 60.960 69.799 84.125 115.824	137.172 137.934 138.818 139.549 140.464	62.484 78.638	138.483	48.768 58.826 64.008 81.077 104.851	138.726
Minimum X-S Maximum X-S Minimum Y-I Maximum Y-I SubAre	Station: 11 Station: 13 Slevation: 14 Roughne Transition	nd Maximum X, .000 ( ass 5.924 ( ass 7.172 ( ass 1.683 ( ass ess Data ( 5 Roughness Coefficient	ociated Y-El ociated Y-El ociated X-St ociated X-St SubAreas ) Hydraulic	es evation: 14 evation: 14 ation: 5 ation: Horizontal Breakpoint	1.683 ) 1.683 ) 1.206 ) .000 )
 1	Bottom	.055	.305		
2	Top  Bottom Top	.050  .065 .060 	.914  .610 1.524 	30.175   45.720	
3	Bottom Top	.040	.000 .305	45.720	
4	Bottom Top 	.065 .060 	.305 1.219	64.008   84.125	

5	Bottom Top	.055	-	.305 .914	
*		Processing	Header		*
*					 *

M I	Nodel for Wat	nistration - er-Surface Prof Metric / Outp	file Comput put Units:	ations. Metric	-
		RFACE PROFILE W METRIC			
		'o Process Head			
XS FULLV 757	7.428				
*** No Roug	ghness Data I	oata Associated nput, Propagat ta In Temporary	ing From Pr	evious Sect	ion ***
*** SRD Location: Valley Slope: Energy Loss C	757. .00120	mary For Header Cross-Section Averaging Conv -> Expansion	Skew: . veyance By	0 Error Co Geometric Mo	ean.
Х	Y	oordinates (29 X	Ŷ		Y
.000 10.973 30.175 40.538 46.939 51.206 60.960	141.727 139.592 139.715 138.831 138.374 137.216 137.978	.100 13.716 33.528 43.586 47.244	140.508 139.532 138.952 138.770 137.765	7.010 16.764 36.271 45.720 48.768	138.923 138.801 137.246
04.125	140.508	115.924	138.770 139.532 141.727	58.826 64.008 81.077 104.851	139.075 139.592
Minimum X-St Maximum X-St Minimum Y-El Maximum Y-El	ation:	nd Maximum X,Y- .000 ( assoc 15.924 ( assoc 37.216 ( assoc 41.727 ( assoc	riated V-El	evation: 1	41.727 ) 41.727 ) 51.206 ) .000 )
SubArea	Transition	ess Data ( 5 S Roughness Coefficient	Hydraulic	Horizontal Breakpoint	
1	Bottom Top 	.055 .050 	.305 .914	  30.175	
2	Bottom Top 	.065 .060	.610 1.524 	45.720	
3	Bottom Top 	.040	.000 .305	  64.008	
4	Bottom Top 	.065	.305 1.219	  84.125	
5	Bottom Top	.055	.305		

\* Finished Processing Header Record FULLV \*

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \*\_\_\_\_\_ ----\* CONTINUOUS WATER SURFACE PROFILE WITH CULVERT IN REACH METRIC \_\_\_\_\* \_\_\_\_\_ \* Starting To Process Header Record BOXCV \* \* CV BOXCV 757.428 51.206 30.48 137.21 137.24 1 115 3048 3658 CG \* \* \* \* \* \* Completed Reading Data Associated With Header Record BOXCV \* \* \* Notice - Program Assuming All Culvert Input Complete \* \* \* \* \* \* Storing Culvert Data In Temporary File As Record Number 3 \* \* \* \* \* \* \* \* \* Data Summary For Header Record BOXCV Culvert Code: 115 SRD Location: 757. Error Code 0 Culvert Information: Material: Concrete Shape: Box Snape: Box Fattering 1 Inlet Code: 5 Length: 30.480 Rise: 3048.000 Span: 3658.000 # Barrel(s): 1 Roughness: .0120 Horizontal Stationing: 51.206 Entrance Loss Coefficient: .20 Alpha: 1.0000 Invert Elevations -> Upstream: 137.240 Downstream: 137.210 \*\_\_\_\_\_\* \* Finished Processing Header Record BOXCV

\*\_\_\_\_\_

Fedei	at Highway Adr Model for Wa Input Units	ninistration ater-Surface : Metric /	- U. S. Ge Profile Comp Output Units	eological Surv putations. : Metric	vey
	SINUOUS WATER S		ILE WITH CULV		
	Starting				* * *
XS APPR	803.148				
*** No H	oleted Reading Roughness Data .ng X-Section I	Input, Propa	agating From	Previous Sect	tion ***
Valley Slo	Data Su Lon: 803. Ope: .00120 ss Coefficients	Averaging	Conveyance E	By Geometric I	Mean.
X	Х, Ү- Ү		(29 pairs) Y	Х	Y
.000 10.973 30.175	$141.781 \\ 139.647 \\ 139.769 \\ 138.885 \\ 138.428 \\ 137.270 \\ 138.032 \\ 138.916 \\ 139.647 \\ \end{array}$	.100 13.716 33.528	140.562 139.586 139.007 138.824 137.361 138.581 138.824 139.586 141.781	7.010 16.764 36.271	139.891 139.739 138.977
	X-Station: X-Station: Z-Elevation: Z-Elevation: Rough Transitic	.000 ( a 115.924 ( a 137.270 ( a 141.781 ( a nness Data ( on Roughnes	5 SubAreas ss Hvdrauli	Elevation: Elevation: Station: Station:	1
	Area Point Bottom Top		.305	·	_
2	2 Bottom Top	.065 .060	 .610 1.524		
3	Bottom Top	.040 .040	.000 .305		
4	Bottom Top	.065 .060	.305 1.219 		
Į	5 Bottom Top	.055 .050	.305 .914		

Bridge datum projection(s): XREFLT XREFRT FDSTLT FDSTRT

		Model	********* W S P ay Administration for Water-Surface Units: Metric /	n – U.S.Ge e Profile Com	putations.						
EX	~	CONTINUOUS WATER SURFACE PROFILE WITH CULVERT IN REACH METRIC									
		** Summary of Boundary Condition Information *									
	#	Reach Discharge	Water Surface Elevation		Flow Regime						
	1	22.65	139.214	*****	Sub-Critical						
			ginning 1 Profi								

Federal Highway Administration - U. S. Geological Survey Model for Water-Surface Profile Computations. Input Units: Metric / Output Units: Metric \_\_\_\_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ CONTINUOUS WATER SURFACE PROFILE WITH CULVERT IN REACH METRIC << Beginning Computations for Profile 1 >> WSEL VHD Q AREA SRDL LEW HF 77 K FLEN REW EGEL CRWS HO FR # SF ERR ALPHA 41.806 \*\*\*\*\*\*\*\* Section: EXIT 139.213 .022 22.653 32.183 .541 139.236 \*\*\*\*\* 1081.61 \*\*\*\*\*\*\*\* Header Type: XS 82.156 138.000 \*\*\*\*\* \*\*\*\*\* 720.852 1.513 \*\*\*\*\* SRD: .233 .024 Section: FULLV 139.229 22.653 40.414 36.576 32.305 Header Type: FV 139.253 .016 .560 1041.91 36.576 81.991 .000 SRD: 757.428 138.044 .000 .243 .0005 1.499 <<< The Preceding Data Reflect The "Unconstricted" Profile >>> 22.653 38.784 Section: APPR 139.251 .025 45.719 32.450 .022 .584 Header Type: AS 139.277 996.29 45.719 81.798 SRD: 803.147 138.098 .000 .256 .0005 1.481 .001 <<< The Preceding Data Reflect The "Unconstricted" Profile >>>

<<< The Following Data Reflect The "Constricted" Profile >>> <<< Beginning Bridge/Culvert Hydraulic Computations >>>

	WSEL HWic HWoc	TWD Dc Dn	Q/BL VELIN VELOUT	AIN AOUT CVSLPE	OTFULL Ac An	
Section: BOXCV Header Type: CV SRD: 757.428	139.844 2.456 2.604		22.654 3.033 3.066	7.469 7.389 .0010	069 5.761 8.539	
	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: APPR	139.849	.007	22.653	77.845	45.719	7.692
Header Type: AS	139.857	.005	.291	2074.98	45.719	107.274
SRD: 803.147	138.098	.000	.142	.0001	1.821	.000

Approach Section APPR Flow Contraction Information M(G) M(K) KQ XLKQ XRKQ OTEL

<<< End of Culvert Hydraulics Computations >>>

<< Completed Computations of Profile 1 >>

ER

Figure 7-23. Output for "culvert" analysis.

#### 7.10 EXAMPLE #10: SCOUR COMPUTATIONS

Estimates of contraction scour, pier scour, and abutment scour can be computed with WSPRO by using the DC, DP, and DA records. The methods used in WSPRO to compute scour are presented in HEC-18, Evaluating Scour at Bridges (add reference here). If more information is required about the techniques used to evaluate scour at bridges, the user should refer to HEC-18. The following example illustrates the procedure that is used to compute scour with WSPRO.

#### 7.10.1 Estimating Scour at Bridges - Computer Solution

A 198.12 meter long type 3 (spill-through abutments and sloping embankments) bridge is to be constructed over a channel that has an estimated design flow of 849.51  $m^3/s$ . The right abutment is to be fixed at the right bank of the main channel and the left abutment extends into the overbank area. The elevation of the bridge deck is 6.71 meters and the girder depth is 1.22 meters. Six round nose piers that are 1.52 meters thick and 12.19 meters long will be placed in the channel. Four piers are in the main channel and two are in the overbank area. The abutments are on 1V:2H slopes. The abutments and piers are designed to be aligned with the flow. Long term aggradation and degradation have been considered and are assumed to be negligible. Using the scour computing functions in WSPRO, estimate the contraction scour and the local scour depths at the abutments and piers. The  $D_{50}$  of the bed material is 1 millimeter. Assume that the main channel is stable laterally and will not migrate into the overbank area of the bridge opening. This implies that pier scour can be estimated with reduced depths and velocities in the overbank area. Also, assume that clear-water contraction scour occurs in the overbank area of the bridge opening. Figures 7-24 and 7-25 show the geometry and configuration of the bridge and approach cross-sections. Figures 7-26 and 7-27 show the left and right locations of the bridge opening and approach cross-section live-bed and clear-water contraction scour boundaries that have been assumed. Figure 7-28 shows the input data file that can be used to accomplish the scour computations. Once the magnitude of each of the scour components is determined, the magnitude of total scour can be computed and plotted on a figure similar to Figure 7-29.

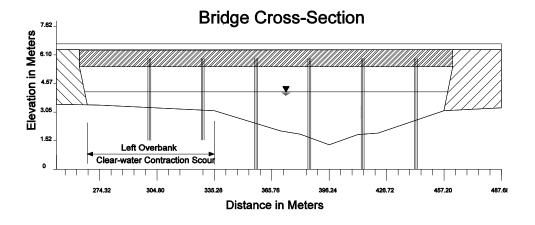


Figure 7-24. Bridge Cross-Section Configuration

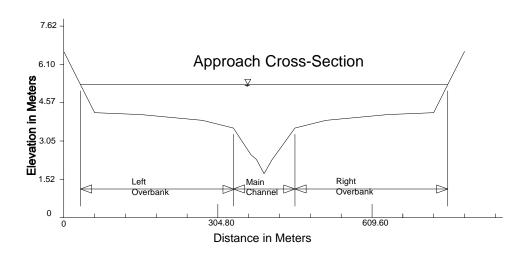


Figure 7-25. Approach Cross-Section Configuration.

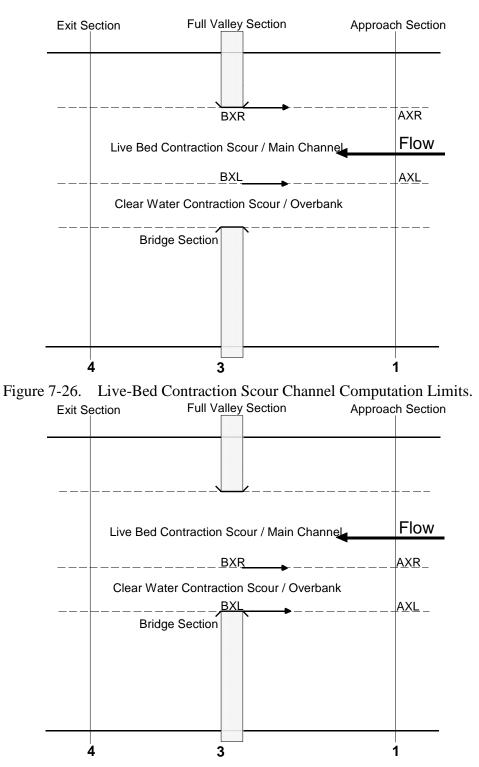


Figure 7-27. Clear-Water Contraction Scour Channel Computation Limits.

WSPRO User's Manual

т1 WORKSHOP PROBLEM - SCOUR CREEK - METRIC т2 ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION т3 CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS \* SI 1 849.51 Q SK 0.002 \* EXIT 228.60 \* \* \* .002 0.00,5.79 30.48,4.57 XS 60.96,3.35 152.40,3.28 274.32,3.05 GR 335.28,2.74 370.33,1.68 381.00,1.49 396.24,0.93 411.48,1.48 GR 422.15,1.55 457.20,2.74 518.16,3.05 640.08,3.28 731.52,3.35 762.00,4.57 792.48,5.79 0.042 0.032 0.042 GR GR Ν 335.28 457.20 SA \* XS FULLV 426.72 \* BR BRDG 426.72 BL 1 198.12 335.28 457.20 BC 5.49 3 15.24 2 6.71 CD AB 2 1.72 9.14 6 0.042 0.032 PD Ν 335.28 SA \* APPR 640.08 XS \* \* SECID K1 K2 YL YR Q FS DA BRDG .55 \* \* SECID BXL BXR PW YΒ QΒ К1 K2 KЗ K4 V1M D1M DP BRDG 335.28 \* 1.52 \* \* SECID BXL BXR PW YΒ QΒ К1 K2 KЗ ĸ4 V1M D1M 335.28 1.52 DP BRDG \* \* \* 0 SECID BXL BXR AXL AXR К1 PW ΥB YA DC 0 BRDG 335.28 \* \* \* \* 6.10 \* \* 1 SECID BXL BXR D50 PW AXL AXR ΥB YA \* 335.28 DC 1 BRDG \* \* 1.0 3.05 ΕX ER

Figure 7-28. WSPRO Input Data File For "Scour Computation" Example.

#### 7.10.2 Abutment Scour Computations

The abutment scour computations are accomplished by using the DA record. The program is allowed to default in all data fields except for the K1 value where the assumed default is 1.00. Since K1 for spill through abutments is 0.55, this value is placed in the field where the K1 value is read. Figure 7-30 presents the WSPRO output for the abutment scour computations.

#### 7.10.3 Pier Scour Computations

Since the assumption is made that over time the main channel will not migrate into the overbank portion of the bridge opening, pier scour is be computed for the piers in the main channel using hydraulic properties for flow in that portion of the bridge opening. Using similar logic, pier scour in the overbank portion of the bridge opening is computed using hydraulic properties for the flow in that portion of the opening. By default, WSPRO will compute worst case (by using maximum depths and velocities) pier scour for the opening using hydraulic properties based on equal conveyance "tubes" computed using the entire opening. Since in this example we are to compute scour in the main channel and overbank portions of the bridge openings using their respective hydraulic properties, the user needs to use multiple DP records. The first DP record is used to compute pier scour in the main channel. Since the defaults for BXL and BXR are the left and right edges of water in the bridge cross-section, overrides are necessary. Because the main channel begins at station 335.28 and ends where water intersects the right abutment, an override is necessary for BXL. The second DP record works similarly except that an override is required for BXR which will limit the calculations to the portion of the bridge opening between the intersection of the water surface and the left abutment and the boundary between the overbank and the main channel which occurs at station 335.28. If the user desires, overrides can be input on the DP record for velocities and depths. These values will be used in the scour computations. Figures 7-31 and 7-32 present the WSPRO output for the main channel and overbank pier scour computations.

#### 7.10.4 Contraction Scour Computations

The contraction scour computations are made assuming that clear-water contraction scour will occur in the overbank and live-bed contraction scour will occur in the main channel portion of the bridge opening. The boundary between clear-water contraction scour and live-bed contraction scour is assumed to occur at station 335.28 in the bridge opening. By using multiple DC records an estimate of contraction scour can be computed for each portion of the bridge opening. The live-bed contraction scour is computed using a DC record with the option code set equal to 0. The default value of BXL is overwritten with the value 335.28 which is the left boundary of the portion of the bridge opening where live-bed sediment transport occurs. All other values with the exception of PW are allowed to default. The cumulative pier width (PW) is equal to 6.10 meters (4 piers each 1.52 meters wide), with is equal to the summed width of piers in the portion of the main channel.

Clear-water contraction scour is computed by setting the option code on the DC record equal to 1 and allowing the value of BXL to default to the intersection of the left edge of water in the bridge opening. The default value of BXR is overwritten with 335.28 which is the right boundary of the portion of the bridge opening where clear-water sediment transport occurs. Since clear-water contraction scour is estimated using different variables than live-bed contraction scour, the format of the DC record is different when estimating clear-water contraction scour. The  $D_{50}$  value of the bed material is assumed to be 1 millimeter and the cumulative pier width is 3.05 meters (2 piers each 1.52 meters wide). Figures 7-33 and 7-34 present the WSPRO output for the live-bed and clear-water contraction scour computations.

#### 7.10.5 Summary

Once the estimates of local scour at the piers and abutments have been determined, they need to be added to the contraction scour and long term aggradation or degradation to produce a total scour depth. The estimate of scour at the left abutment (5.86 meters) is added to the clear-water contraction scour (-0.63 meters) to produce the total scour. Note that if the estimated contraction scour is negative it should be assumed equal to zero. If this is the case, the user should re-examine their assumption of clear-water contraction scour (-0.63) to produce the total scour at the piers in the overbank (2.18 meters) is added to the clear-water contraction scour (-0.63) to produce the total scour at the piers in the overbank (2.18 meters). The total scour at the piers the main channel is estimated by adding the local scour at the piers (3.57 meters) to the live-bed contraction scour (2.08 meters) to obtain a value of 5.65 meters. The total scour at the right abutment is obtained by adding the estimate of scour at the right abutment (6.77 meters) to the live-bed computations the long aggradation or degradation was assumed negligible. Once the various estimates of scour have obtained, the user can plot them on a figure similar to Figure 7-29.

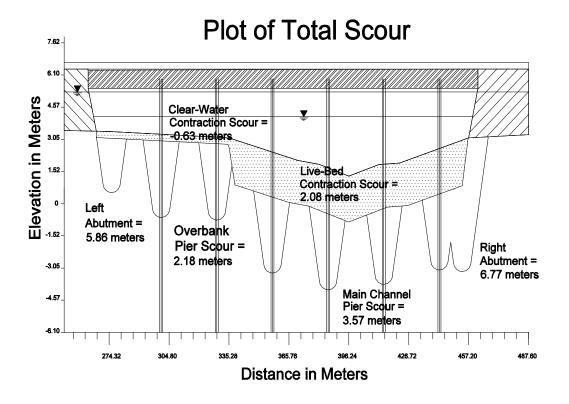


Figure 7-29. Schematic Plot of Scour Components.

	Federal	Highway Model fo Input Un	Administ or Water-S uits: Metr	WSPRO Tration - Surface Pros Tic / Out	U. S. Geo file Comp put Units	ological utations.				
	** WORKSHOP PROBLEM - SCOUR CREEK - METRIC ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS									
	*** Ab	utment S	cour Calc	ulations f	or Header	Record B	RDG ***			
	Constants and Input Variables									
		*				*				
				or						
				tack Facto						
				<b>,</b>						
		*				*				
	Abtmnt	Scour					Overbank			
#			X-Statn	A-Prime	Ya	Qe				
1				268.077						
	Right	6.768	457.200	335.280	1.140	221.506	.173			

Figure 7-30. Printed Output From Abutment Scour Computations.

	Feder	**************************************									
	WORKSHOP PROBLEM - SCOUR CREEK - METRIC ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS										
	* * *	Pier Scour Calculations for Header Record BRDG ***									
		Constants and Input Variables									
		Pier Width: 1.520									
		Pier Shape Factor(K1):1.10Flow Angle of Attack Factor(K2):1.00Bed Condition Factor(K3):1.00Bed Material Factor(K4):1.00Velocity Multiplier(VM):1.00Depth Multiplier(YM):1.00									
#		Localized Hydraulic Properties X-Stations Flow WSE Depth Velocity Froude # Left Righ									
 1 	3.57	759.074 4.261 2.935 3.647 .679 335.280 459.	450								

Figure 7-31. Printed Output From Main Channel Pier Scour Computations.

	Feder	************** al Highway A Model for Input Unit	dministr Water-Su s: Metri	ration Irface P .c / O	- U. rofil utput	S. Geo e Compu Units:	logica utation Metri	al Surve ns. Lc	У	
	** WORKSHOP PROBLEM - SCOUR CREEK - METRIC ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS									
	* * *	Pier Scour	Calculat	ions fo	r Hea	der Red	cord BH	RDG **	*	
		Con	stants a	ind Inpu	t Var	iables				
		*		dth:				- *		
		Pier Shap Flow Angl Bed Condi Bed Mater Velocity Depth Mul	e Factor e of Att tion Fac ial Fact Multipli tiplier	ack Fac tor or er	tor	(K1): (K2): (K3): (K4): (VM): (YM):	1.10 1.00 1.00 1.00 1.00 1.00			
#		Localiz Flow W								
 _1 	2.18	90.436	4.261 1	.125	1.566	.4	171 1	266.381	335.280	

Figure 7-32. Printed Output From Overbank Pier Scour Computations.

**************************** W S P R O *********************************								
** WORKSHOP PROBLEM - SCOUR CREEK - METRIC ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS								
*** Live-Bed Contraction Scour Calculations for Header Record BRDG ***	•							
Constants and Input Variables								
** Bed Material Transport Mode Factor (k1): .64 Total Pier Width Value (Pw): 6.100 **								
Scour Flow Width X-Limits # Depth Contract Approach Contract Approach Side Contract Approach								
1 2.079 770.268 406.497 115.820 121.920 Left: 335.280 335.280 Approach Channel Depth: 2.664 Right: 457.200 457.200								

Figure 7-33. Printed Output From Live-Bed Contraction Scour Computations.

**************************************	
WORKSHOP PROBLEM - SCOUR CREEK - METRIC ESTIMATING SCOUR AT BRIDGES - COMPUTER SIMULATION CONTRACTION, PIER, AND ABUTMENT SCOUR CALCULATIONS	
*** Clear-Water Contraction Scour for Header Record BRDG ***	
Constants and Input Variables	
** Bed Material D50 Value (D50): 1.0000 Pier Width Value (Pw): 3.050 **	
Scour Flow Width X-Limits # Depth Contract Approach Contract Approach Side Contract Approac	h
1633 83.192 71.425 64.153 67.203 Left: 268.077 268.07 Approach Channel Depth: 1.579 Right: 335.280 335.28 * Negative Scour Depth Encountered - Check If Variables Are Reasonable	0

Figure 7-34. Printed Output From Overbank Contraction Scour Computations.

### 7.11 EXAMPLE #11: FLOODWAY COMPUTATIONS

A floodway encroachment analysis is required for a simple channel. It is desired that the encroachment occur equally from both sides of the channel. Figure 35 illustrates input data for a simple reach. A surcharge of 0.3048 m has been applied throughout the reach by using a FW record at each cross-section. The results of the analysis are presented in Figure 36.

```
т1
               WORKSHOP PROBLEM - FLOOD CREEK - METRIC
т2
               FLOODWAY ENCROACHMENT ANALYSIS - COMPUTER SIMULATION
Т3
               HYPOTHETICAL CHANNEL
*
SI 1
*
Q
               849.51
               0.002
SK
*
        XS#1 0 * * * .002

0.00,5.79 30.48,4.57 60.96,3.35 152.40,3.28 274.32,3.05

335.28,2.74 370.33,1.68 381.00,1.49 396.24,0.93 411.48,1.48

422.15,1.55 457.20,2.74 518.16,3.05 640.08,3.28 731.52,3.35

762.00,4.57 792.48,5.79

0.042 0.032 0.042

225 200 457 20
XS
GR
GR
GR
GR
Ν
                          335.28 457.20
SA
                 0.3048
FW
*
XS
        XS#2 457.20
FW
                0.3048
*
         XS#3 685.80
XS
                 0.3048
FW
XS
         XS#4 914.40
                0.3048
FW
*
ΕX
ER
```

Figure 7-35. WSPRO Input Data File For Floodway Computation Example.

	Federal Highway Admi Model for Wat Run Date & Time:	nistration - U. er-Surface Profile	Version V070197
	FLOODWAY ENCROAC HYPOTHETICAL CHA Metric	: - FLOOD CREEK - M HMENT ANALYSIS - C NNEL (SI) Units Used i	COMPUTER SIMULATION
	Quantity	SI Unit	Precision
	Length Depth Elevation Widths	meters	0.001 0.001 0.001 0.001 0.001
	Velocity Discharge Slope	meters/second cubic meters/seco meter/meter	0.001 ond 0.001 0.001
	Angles	degrees	0.01
Q	849.51		

\*\*\* Processing Flow Data; Placing Information into Sequence 1 \*\*\*

SK 0.002

-	Highway Adm Model for Wa Input Units:	*** WSPRC inistration - ter-SurfacePr Metric / Ou	- U. S. Geol cofile Comput utput Units:	logical Sur tations. Metric	vey
	WORKSHOP PR NAY ENCROACH	OBLEM - FLOOD MENT ANALYSIS YPOTHETICAL CH	CREEK - METH - COMPUTER S	RIC	
		To Process Hea			*
					*
GR ( GR 335.2 GR 422.3	28,2.74 370 15,1.55 457 00,4.57 792 0.042 0. 335.28	30.48,4.57 6 .33,1.68 381. .20,2.74 518.	.00,1.49 396	5.24,0.93	411.48,1.48
		Data Associate ata In Tempora			
*** SRD Location Valley Slope Energy Loss (	0. .00200	mmary For Head Cross-Sectic Averaging Co -> Expansic	on Skew: . onveyance By	.0 Error Geometric	Mean.
	etails: Encr Init Maxin X,Y- Y	utations Selec oachment from ial floodway k mum Surcharge: coordinates (1 X	left and rig boundary sect : .305	ght banks	* Y
.000		30.480 274.320	4.570	60.960	3.350 2.740
152.400	3.280	274.320	3.050	335.280	2.740
152.400 370.330	3.280 1.680	274.320 381.000 422 150	3.050 1.490 1.550	335.280 396.240 457 200	. 930
152.400	3.280 1.680 1.480 3.050 4.570	274.320 381.000 422.150 640.080 792.480	1.490 1.550	396.240 457.200	.930 2.740
152.400 370.330 411.480 518.160 762.000	1.680 1.480 3.050 4.570 Minimum tation: tation: levation: levation: Rough SubArea	381.000 422.150 640.080 792.480 and Maximum X, .000 ( ass 792.480 ( ass .930 ( ass 5.790 ( ass ness Data ( 3 Roughness Coefficient	1.490 1.550 3.280 5.790 	396.240 457.200 731.520  es Levation: Levation: cation: cation:	.930 2.740 3.350
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\* Finished Processing Header Record XS#1 \*

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	AY ENCROACHM	BLEM - FLOOD ENT ANALYSIS POTHETICAL CH	- COMPUTER :		
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XS XS#3 685 FW 0.3					
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* * *	Starting T		ader Record	XS#4	* * *
XS XS#4 91 FW 0.					
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*** SRD Locatior Valley Slope Energy Loss		mary For Hea Cross-Secti Averaging C -> Expansi	on Skew: onveyance By	.0 Error Geometric	Mean.
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]	Model	ay Administration for Water-Surface Units: Metric /	n - U.S.Ge Profile Comp Output Units	putations.
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	* Summa *=======	ry of Boundary Co	ondition Info	rmation *
#	Reach Discharge	Water Surface Elevation		Flow Regime
 1	849.51	******	.0020	Sub-Critical
		ginning 1 Profi	le Calculation	

	WSEL	VHD	Q	AREA	SRDL	LEW
	EGEL	HF	V	K	FLEN	REW
	CRWS	HO	FR #	SF	ALPHA	ERR
Section: XS#1	3.833	.173	849.510	622.719	********	48.886
Header Type: XS	4.006	*****	1.364	18992.98	*********	743.592
SRD: .000	3.611	*****	.622	*****	1.829	*****
Section: XS#2	4.747	.173	849.510	623.075	457.200	48.874
Header Type: XS	4.921	.913	1.363	19006.80	457.200	743.605
SRD: 457.200	4.525	.000	.622	.0020	1.829	.002
Section: XS#3	5.205	.173	849.510	623.175	228.600	48.870
Header Type: XS	5.378	.456	1.363	19010.66	228.600	743.609
SRD: 685.800	4.983	.000	.622	.0020	1.829	.002
Section: XS#4	5.662	.173	849.510	623.170	228.600	48.870
Header Type: XS	5.835	.456	1.363	19010.47	228.600	743.609
SRD: 914.400	5.440	.000	.622	.0020	1.829	.002

<< Completed Computations of Profile 1 >>

Mode	ghway Adm: el for Wat	inistrati ter-Surfa	PRO **** on - U.S ace Profile / Output U	5. Geologio Computatio	cal Survey	* * *
	ENCROACHI	MENT ANAL	'LOOD CREEK LYSIS – COMI CAL CHANNEL		LATION	1
* ======= * * ========	Flood	way Analy	vsis Computa	ations	*	
	EGEL		V	K	SRDL FLEN ALPHA	REW
Section: XS#1 Header Type: XS SRD: .000	4.402	* * * * * *	2.074	18917.72	* * * * * * * * *	284.920 507.302 004
Section: XS#2 Header Type: XS SRD: 457.200	5.057 5.321 4.441	.263 .918 .000	849.510 2.071 .534	409.998 18983.40 .0020	457.200 457.200 1.204	285.882 507.559 .001
Section: XS#3 Header Type: XS SRD: 685.800	5.515 5.779 4.898	.457	849.510 2.070 .534	18997.30		507.560
Section: XS#4 Header Type: XS SRD: 914.400		.457	2.070	19001.47		

			lghway . Mel for	Administr Water-Su		U. S. Ge file Comp	ological utations.	-	*	
				ACHMENT A	- FLOOD C NALYSIS - TICAL CHA	COMPUTER	-	ON		
#	SECID	SRD	d	FW-WSEL	UN-WSEL	WIDTH	LEW	REW	AREA	VEL
1 2 3 4 		.000 457.200 685.800 914.400	.304 .310 .310 .311	4.137 5.057 5.515 5.973	3.833 4.747 5.205 5.662	222.382 221.676 221.679 221.678	284.920 285.882 285.881 285.881			2.074 2.071 2.070 2.070
		*******	IN IN	ormal end sed Time:	of WSPRO 0 Minu	executio tes 1 Se	11.	* * * * * * * * * *		

Figure 7-36. WSPRO Output Data File For Floodway Computation Example.

# **SECTION 8 - COMPUTER CONSIDERATIONS**

Implementation of WSPRO can range from a quite simple procedure on a microcomputer to a relatively major task on some minicomputers and (or) large mainframe computers. Discussions in this section are intentionally general because details of implementation may vary greatly depending on where, from whom, and in what form the user obtains the model.

The microcomputer version in its executable form (i.e., .EXE file) is quite easily implemented. Some modification of the CONFIG.SYS file relative to BUFFERS and FILES may be required. The first personal computer (PC) implementation of WSPRO used an IBM PC/AT<sup>1</sup> but the model has since been implemented on several different brands and models of personal computers. WSPRO requires less than 400 kb of memory. Execution time, which is greatly reduced by a math coprocessor, varies considerably depending on the model of PC used. Typical run times of from one-half to two min on a math-coprocessor-equipped IBM PC/AT (or compatible model) may be five to seven min on a similarly equipped IBM PC/XT (or compatible model) and one-half h or more on less powerful IBM PC's (or compatible models) without a math coprocessor.

Implementation also depends somewhat on how the user operates (i.e., with an internal hard disk, one or two external disks, or some combination thereof). Regardless of the system, WSPRO execution results in the user being "prompted" for the input and output file names or other information depending on the user's intentions. To execute the WSPRO program, the user needs to enter the name of the WSPRO executable from a DOS prompt. The first response must be to enter the file name of an existing file that contains the input data. If the user is willing to accept the default output file name extension (\*.lst), the second prompt can be responded to with a simple "yes" command. If the user desires a different name for the output file, one can be selected at this time. On second, and subsequent, executions of the WSPRO executable, the file names are read from a "master" data file. Accepting the file names to be used for the current program execution. All file names must conform to standard MS/DOS conventions.

The output can also be redirected to the computer screen or a printer. Specifying a file name provides an opportunity to "preview" the results prior to generating a hard copy.

<sup>&</sup>lt;sup>1</sup> Reference to trade names, commercial products, manufacturers, or distributors in this manual is for identification purposes only and does not constitute endorsement by the Federal Highway Administration or the U.S. Geological Survey and are not necessarily recommended for use.

Many minicomputers and mainframe computers require additional detail regarding all files to be used by a program. Some systems require only that the user specify how each file is to be used (i.e., input, output, or both input and output). Other systems need information (for at least some files) regarding record length, record format, etc., and perhaps the amount of space to be allocated for each file. Such information is included in the following descriptions of each "unit" used by WSPRO.

Unit 5:	Input file ("card reader") requires 80 characters per record; may be blocked; space required depends on the total number of individual data records.
Unit 6:	Output file ("printer") requires 133 characters per record (includes carriage-control character); may be blocked; space required depends on the number of cross-sections and the number of profiles computed.
Unit 7:	"obsolete" output file (can be "dummied") requires 80 characters per record; may be blocked; space required depends on the total number of individual data records.
Unit 8:	"obsolete" output file (can be "dummied") requires 80 characters per record; may be blocked; space required depends on the volume of bridge computations.
Unit 9:	"obsolete" output file (can be "dummied") requires 133 characters per record (includes carriage-control character); may be blocked; space required depends on the number of cross-sections and the number of profiles computed.
Unit 13:	Input and output file (direct-access) requires 1680 characters per record; unformatted, variable block size; requires one record per cross-section.
Unit 14:	Input and output file (direct-access) requires 400 characters per record; unformatted, variable block size; requires one record per cross-section per profile plus overhead of about 10 records per profile.

Executable code generally cannot be transferred between different brands (and frequently not between different installations of like brand) of minicomputers and large mainframe computers. Users of such systems will need to acquire WSPRO source code, which will more than likely require some minor revisions. This can easily be accomplished, but the pertinent instructions are beyond the scope of this user's manual.

# **SECTION 9 - APPENDIX A / GLOSSARY**

This section, although referred to as a glossary, actually has characteristics of both a glossary and an index while not totally conforming to the normal content or structure of either. The tables in Section 4, which describe the individual data records, provide complete definitions of the input parameters associated with each data record. Those definitions are not repeated in this section. Instead a very brief definition is provided along with cross references to: (1) table(s) in which they are defined; (2) figure(s) in which they are illustrated; and (3) section(s) of the report where additional discussion or examples of them may be found.

Many output parameters appear in several places in the output and those are simply labeled as "output parameters" in this section. Single- (and some dual-) purpose output parameters are defined in terms of "culvert output," "profile output," etc. Most input parameters double as output parameters but are not doubly defined as such in this section.

Individual data records are not re-defined in this section. The user can refer to: (1) Table 2-1 which lists all of the data records in groups by function; (2) Table 4-2 which lists all of the data records with a brief statement of the purpose of each record (in alphabetical order of record identifiers); and (3) to the individual tables in Section 4 which define each record and all of its associated parameters in detail (the individual tables are alphabetically ordered). Also, the detail in the table of contents and the lists of figures and tables should be very useful in locating specific items of interest.

The remainder of this section is devoted to the alphabetical list of parameters and abbreviations with associated definitions and cross-reference information.

A3	-	Multiple-opening output, flow area in each opening.
ABSLPL, ABSLPR	-	Input parameters, left and right abutment slopes [see Table 4-3; Figure 4-9].
AIN	-	Culvert output, flow area at culvert inlet.
ALPH	-	Cross-sectional properties and profile output, velocity head correction factor for nonuniform velocity distribution.
AOUT	-	Culvert output, flow area at culvert outlet.
AREA	-	Cross-sectional properties and profile output, flow area of a cross-section.
ASUBC	-	Culvert output, flow area in culvert at critical depth.

ASUBN	-	Culvert output, flow area in culvert at normal depth.
BCELEV	-	Input parameter, bridge-deck elevation [see Table 4-5; Figure 4-1].
BCSLP	-	Input parameter, bridge-deck slope [see Table 4-5; Figure 4-1].
BCSTA	-	Input parameter, bridge-deck station [see Table 4-5; Figure 4-1].
BETA	-	User-defined table parameter, momentum correction factor for non- uniform velocity distribution, used in computing expansion loss downstream from bridge.
BLEN	-	Output for bridge section, bridge length output for component mode (also see BRLEN).
BOLEW, BOREW	-	Multiple-opening output, left and right edge of water in each opening.
BOTD	-	Input parameter, hydraulic-depth breakpoint for BOTN [see Tables 4-31, 4-32; Figure 4-15].
BOTN	-	Input parameter, roughness coefficient for BOTD breakpoint [see Tables 4-31, 4-32; Figure 4-15].
BOTRAD	-	Input parameter, bottom radius of pipe-arch culvert [see Table 4-12].
BRLEN	-	Input parameter, bridge length measured between tops of abutments, [see Table 4-6; Figures 4-3, 4-7, 4-8, 4-9, 4-10].
BRTYPE	-	Input parameter, indicates type of bridge opening [see Table 4-11; Figures 4-7, 4-8, 4-9, 4-10].
BRWDTH	-	Input parameter, width of bridge longitudinal to the flow [see Table 4-11; Figures 4-7, 4-8, 4-9, 4-10].
BSUBD	-	Input parameter, offset for straight grade banks; perpendicular distance between abutment toes and dikes [see Table 4-25].
С	-	Output for bridge or roadway weir section, coefficient of discharge for bridge opening or roadway weir [see Tables 4-8, 4-41].
CA3	-	Multiple-opening output, "live" flow area (product of CJ and A3) for each opening.
CAVG	-	Road overflow output, average of the weir coefficient(s) used for computing the flow over the road.
CDF	-	Multiple-opening output, conveyance distribution factor for each valley strip (see Shearman and others, 1986).
CJ	-	Multiple-opening output, coefficient of discharge for each opening.

СК	-	Input parameter, contraction coefficient for open-channel computations [see Tables 4-4, 4-8, 4-25, 4-41, 4-42].
CKE	-	Input parameter, entrance-loss coefficient for culvert [see Table 4-9].
CN	-	Input parameter, culvert roughness coefficient [see Table 4-9].
CODE	-	Label used in output headings for record identifiers.
CORRAD	-	Input parameter, corner radii for pipe-arch culvert [see Table 4-12].
CRF	-	Multiple-opening output, channel resistance ratio for each valley strip (see Shearman and others, 1986).
CRWS	-	Profile output, water-surface elevation for critical flow [see Figure 4-11; Section 4.3.5].
CVALPH	-	Input parameter, velocity head correction factor for culvert [see Table 4-9].
CVLENG	-	Input parameter, culvert length [see Table 4-15].
DAF	-	Output parameter, abbreviation for direct-access file.
DAVG	-	Road overflow output, average weir-flow depth.
DELTAY	-	Input parameter, elevation increment used in search for energy balance [see Table 4-29].
DMAX	-	Road overflow output, maximum weir-flow depth.
DMIN	-	Road overflow output, minimum weir-flow depth.
DSINV	-	Input parameter, downstream culvert invert elevation [see Table 4-15].
DSUBC	-	Culvert output, critical flow depth in the culvert.
DSUBN	-	Culvert output, normal flow depth in the culvert.
EGL	-	Profile output, elevation of the energy-grade line.
ЕК	-	Input parameter, expansion coefficient for open-channel computations [see Tables 4-4, 4-8, 4-25, 4-41, 4-42].
ELMAX	-	Input parameter, maximum elevation for cross-sectional properties computations [see Table 4-28, Section 7.7].
ELMIN	-	Input parameter, minimum elevation for cross-sectional properties computations [see Table 4-28; Section 7.7].
EMBELV	-	Input parameter, embankment elevation [see Table 4-11].

EMBSS	-	Input parameter, embankment side slope [see Table 4-11].
EMBWID	-	Input parameter, embankment width [see Table 4-41].
ENTRND	-	Input parameter, radius of entrance rounding [see Table 4-11; Figure 4-7].
ERR	-	Profile output, discrepancy in balancing energy and (or) discharge.
ERR-CODE	-	Input data summary output, indicates +++xxx message for that section (magnitude of ERR-CODE not important), consult Table 6-1.
FDSTLT, FDSTRT	-	Input parameters, permit adjustment of effective flow length upstream from a bridge [see Table 4-7; Figure 4-6].
FLEN	-	As input parameter(s), permits specification of variable flow lengths across the valley [see Table 4-22; Figure 4-13]. In the profile output it indicates the effective flow length computed for: (1) the FLEN input for open-channel computations; and (or) (2) the approach reach in the bridge-backwater computations.
FLOW	-	Output for bridge section (see Shearman and others, 1986), indicates FLOW class as follows:
		<ul> <li>FLOW = 1 - free-surface flow through bridge, no road overflow.</li> <li>FLOW = 2 - unsubmerged pressure flow through bridge, no road overflow.</li> <li>FLOW = 3 - submerged pressure flow through bridge, no road overflow.</li> <li>FLOW = 4 - free-surface flow through bridge, with road overflow.</li> <li>FLOW = 5 - unsubmerged pressure flow through bridge, with road overflow.</li> <li>FLOW = 6 - submerged pressure flow through bridge, with road overflow.</li> </ul>
FNTEST	-	Input parameter, test value of Froude number for approximate check for possibility of critical flow [see Table 4-29].
FR#	-	Profile output, computed value of Froude number for approximate check for possibility of critical flow.
GBOFF	-	Input parameter, offset distance for elliptical guidebanks [see Table 4-25; Figure 4-14].
GBTYPE	-	Input parameter, indicates type of guidebanks [see Table 4-25].
GIRDEP	-	Input parameter, vertical distance between top of bridge deck and low- chord elevation of the opening [see Table 4-5; Figure 4-1].
HAVG	-	Road overflow output, average total head for weir-flow computations.

HF	-	Profile output, friction loss.
НО	-	Profile output, losses other than friction loss.
HWE	-	Culvert output, headwater elevation.
HWIC, HWOC	-	Culvert output, these two variables represent the number of meters to be added to the DSINV to obtain the HWE for inlet and outlet control, respectively. WSPRO assumes the maximum of the two is applicable. If, in the user's judgement, this assumption is invalid for a given flow situation, simply add the other quantity to DSINV to obtain the HWE for the alternate controlling condition.
ICODE	-	Input parameter, indicates culvert characteristics [see Table 4-12].
IDIR	-	Input parameter, indicates computational direction [see Table 4-21].
IEX	-	Input parameter, indicates computational direction [see Table 4-21].
IHFNO	-	Input parameter, indicates averaging method(s) for friction-loss computations [see Table 4-22].
IHFNOJ	-	Input parameter, indicates averaging method for friction-loss computations [see Table 4-29].
IHP	-	Input parameter, option code for selecting cross-sectional properties output [see Table 4-28; Section 7.7].
IPAVE	-	Input parameter, indicates road-surface condition [see Table 4-41].
Κ	-	Cross-sectional properties and profile output, cross-sectional conveyance.
KQ	-	Profile output, conveyance of the KQ segment of the approach section.
KS	-	Multiple-opening output, valley-strip conveyance at the upstream match section.
K1,K2,K3,K4	-	Input parameters, adjustment/correction factors applied to local pier scour estimation equation.
LEW	-	Cross-sectional properties and profile output, left edge of water.
LOCOPT	-	Input parameter, option code controlling bridge-opening location [see Table 4-6; Figure 4-2].
M[G]	-	Bridge-backwater output, geometric contraction ratio (see Shearman and others, 1986).
M[K]	-	Bridge backwater output, flow contraction ratio (see Shearman and others, 1986) [Table 4-30; Figure 4-15; Section 7.2].

NBBL	-	Input parameter, number of barrels for multi-barrel culverts [see Table 4-15].
NGP	-	Output parameter, number of x,y-coordinates in GR data for a cross- section [see Table 4-26; Figure 4-15].
NPD	-	Output parameter, number of data pairs in PD record(s) [see Table 4-33].
NPROF	-	Output parameter, number of profiles to be computed [see Tables 4-34, 4-37, 4-40].
NRD	-	Output parameter, number of x,y-coordinates in GR data for a road- grade section.
NSA	-	Output parameter, number of subareas in the cross-section [see Tables 4-31, 4-32, 4-35; Figure 4-15].
OTEL	-	Profile output, minimum elevation at which road grade could be built without being subjected to overtopping.
OTFULL	-	Culvert output, a "flag" to indicate the existence of full-pipe flow; positive value indicates full-pipe flow.
P/A	-	Output for bridge section, ratio of pier (pile) area to gross area in the bridge opening.
PD	-	Input parameter, pier width used in pier scour calculations.
PDTH	-	Input parameter, gross pier (pile) width for an associated PELV [see Table 4-33; Figure 4-17].
PELV	-	Input parameter, elevation at which gross pier (pile) width, PDDTH, is input [see Table 4-33; Figure 4-17].
PFELEV	-	Input parameter, test value for low-chord elevation in a bridge used to test for possible pressure flow [see Table 4-8; Figures 4-1, 4-2, 4-3; Section 4.3.7].
PPCD	-	Input parameter, option code to distinguish between piers and piles [see Table 4-33; Figure 4-17].
Q	-	Input parameter, discharge specified for each profile [see Table 4-34] and velocity and conveyance distribution [see Table 4-28].
QBBL	-	Culvert output, discharge per barrel (discharge in Q record divided by NBBL in CV record).
QB	-	Input parameter, discharge through the bridge opening (used in pier scour analysis).

QS	-	Multiple-opening output, discharge apportioned to each valley strip and its associated opening (see Shearman and others, 1986).
QTOL	-	Input parameter, allowable tolerance for balancing discharge in a combined bridge flow and road overflow situation [see Table 4-29].
RECORD NO	-	Input data processing output, indicates the "sequence" number of each cross-section as it is stored on the direct-access file.
REW	-	Cross-sectional properties and profile output, right edge of water.
RISE	-	Input parameter, vertical dimension of culvert [see Tables 4-12, 4-14].
SCALE	-	Input parameter, scaling factor for fabricating cross-sections [see Table 4-27].
SECID	-	Input parameter, unique identifier for each cross-section [see Tables 4- 4, 4-8, 4-15, 4-16, 4-17, 4-18, 4-25, 4-28, 4-41, 4-42, 4-43; Section 4.1].
SKEW	-	Input parameter, angle at which the cross-section is skewed to the flow [see Tables 4-4, 4-8, 4-25, 4-41, 4-42; Section 4.3.6].
SKSL	-	Input parameter, energy gradient [see Table 4-36; Figure 4-17].
SLEN	-	User-defined table parameter, difference between adjacent SRD's (same as SRDL).
SPAN	-	Input parameter, horizontal dimension of culvert [see Tables 4-12, 4-14].
SRD	-	Input parameter, section reference distance [see Tables 4-4, 4-8, 4-15, 4-25, 4-41, 4-42, 4-43; Section 4.3.4].
SRDL	-	Profile output, difference between adjacent SRD's (same as SLEN).
TOPD	-	Input parameter, hydraulic-depth breakpoint for TOPN [see Tables 4-31, 4-32; Figure 4-15].
TOPN	-	Input parameter, roughness coefficient for TOPD breakpoint [see Tables 4-31, 4-32; Figure 4-15].
TOPRAD	-	Input parameter, top radius for pipe-arch culvert [see Table 4-12].
TWDEP	-	Culvert output, tailwater depth (elevation on WS record minus DSINV).
TYPE	-	Output for bridge section, type of bridge opening (same as BRTYPE).
USERCD	-	Input parameter, to override C-value for bridge [see Table 4-8].

USERCF	-	Input parameter, to override weir coefficient [see Table 4-41].
USINV	-	Input parameter, upstream culvert invert elevation [see Table 4-15].
V1	-	Input parameter, used as safety factors in pier scour depth calculations.
VARNOS	-	List of parameter codes for user-defined Tables [see Table 4-39; Sections 5, 7.5].
VAVG	-	Road overflow output, estimated average velocity of road overflow.
VEL	-	Output parameter, flow velocity.
VELIN	-	Culvert output, flow velocity at the culvert inlet.
VELOT	-	Culvert output, flow velocity at the culvert outlet.
VHD	-	Profile output, velocity head.
VMAX	-	Road overflow output, estimated maximum velocity of road overflow.
VSLOPE	-	Input parameter, valley slope for data propagation [see Tables 4-4, 4-25, 4-41, 4-42].
WLEN	-	Road overflow output, length of weir section for road overflow.
WS1	-	Profile output, computed water-surface elevation at approach section [see Figures 4-8, 4-9, 4-11].
WS3N	-	Profile output, computed water-surface elevation at the full-valley section for unconstricted flow [see Figures 4-8, 4-10].
WSEL	-	Profile output, computed or assumed water-surface elevation.
WSI	-	Profile output, assigned or computed water-surface elevation at the initial cross-section [see Table 4-40; Figure 4-11; Section 4.3.2].
WWANGL	-	Input parameter, wingwall angle [see Table 4-11; Figures 4-7, 4-10].
WWWID	-	Input parameter, wingwall width [see Table 4-11; Figure 4-7].
X	-	Incremental distance(s) added to BRWDTH to compute total flow length through bridges with sloping embankments [see Table 4-11; Figures 4-8, 4-9, 4-10, 4-11].
Х	-	Input parameter, x-coordinate(s) of GR data [see Table 4-26; Figure 4-15].
XCONLT, XCONRT	-	Input parameter, left and right horizontal constraints on bridge location [see Table 4-6; Figures 4-2, 4-3].

XCTR	-	Input parameter, x-coordinate at centerline of culvert(s) [see Table 4-15].
XFL	-	Input parameter, x-coordinate(s) of breakpoint(s) for FLEN input data [see Table 4-22; Figure 4-13].
XLAB	-	Bridge section output, x-coordinate at toe of left abutment (component mode only).
XLIML, XLIMR	-	Input parameter, left and right limits for fabricated cross-section [see Table 4-27; Figures 3-6, 3-7; Section 3-1].
XLKQ	-	Input parameter, left limit of Kq-section [see Table 4-30; Figure 4-16; Section 7.2].
XLT	-	Input parameter, left limit of channel segment to be analyzed for pier scour depth estimations.
XMAX, XMIN	-	Output parameter, maximum and minimum cross-section stations.
XORIG	-	Input parameter, to fix a point in a scaled, fabricated cross-section [see Table 4-26].
XRAB	-	Bridge-section output, x-coordinate at toe of right abutment (component mode only).
XREFLT	-	Input parameter, for horizontal datum correction(s) between approach section and bridge and (or) road-grade section(s) [see Table 4-7; Figures 5-4, 5-5; Section7.2].
XREFLT, XREFRT	-	Input parameters, permit adjustment of effective flow length for curvilinear flow upstream from a bridge [see Table 4-7; Figure 4-6].
XRKQ	-	Input parameter, right limit of Kq-section [see Table 4-30; Figure 4-16; Section 7.2].
XS	-	Multiple-opening output, flow area in each valley strip at the upstream match section.
XSA	-	Input parameter, x-coordinate(s) at subarea breakpoints [see Table 4-35; Figure 4-15].
XSID	-	Output parameter, column heading for SECIDs.
XSTW	-	Output parameter, cross-sectional top width.
XSWP	-	Output parameter, cross-sectional wetted perimeter.
Y1M	-	Input parameter, depth multiplier used in pier scour analysis.
Y	-	Input parameter, elevation(s) of GR data [see Table 4-26; Figure 4-15].

YABLT, YABRT	-	Elevations at toes of left and right abutments [see Table 4-3; Figure 4-8].
YINC	-	Input parameter, elevation increment for cross-sectional properties computations [see Table 4-28; Section 7.7].
YMAX	-	Output parameter, maximum cross-section elevation [see Figure 4-12; Section 4.3.5].
YMIN	-	Output parameter, minimum cross-section elevation [see Figure 4-12].
YSHIFT	-	Input parameter, for vertical shift of GR data [see Table 4-27; Section 7.3.1].
YTOL	-	Input parameter, allowable tolerance for balancing energy equation [see Table 4-29].

# SECTION 10 - APPENDIX B / THE HYDRAIN WSPRO INPUT / OUTPUT PROGRAM

The WSPRO Input/Output Program is a Graphical User Interface (GUI) program and set of supporting files which allow a user to interface with the WSPRO model in a Windows-like environment. This facilitates viewing and editing input data, output data, cross-sections and other aspects of interaction with WSPRO. The Input/Output Program is not intended to replace familiarity with the WSPRO model. Its objective is to facilitate interaction with the model. Use of the WSPRO Input/Output Program requires a VGA monitor or better. See Section 8 for further information on computer configuration considerations. The purpose of this section is to familiarize WSPRO users with the GUI and help associate the imagery of the GUI with the WSPRO functionality it facilitates.

The WSPRO Input/Output Program can be run by itself or through the HYDRAIN program. To run the program standalone, the name of the executable (WSHLL.EXE) needs to be entered from a DOS prompt. To run the WSPRO Input/Output Program through HYDRAIN, access the Analyze pull down menu and choose WSPRO. HYDRAIN also has an editor that provides long and short help which can be used to enter data interactively into an ASCII data file. A more comprehensive overview of the HYDRAIN editor can be found in the HYDRAIN User's Manual (1998).<sup>15</sup>

Much of the user's interaction with the program is done with the mouse. Actions which can be taken with the mouse are pointing, clicking, left-clicking, right-clicking, double-clicking, dragging and "dragging and dropping." The user gets visual feedback of actions taken by movements of the arrow-shaped cursor on the screen.

- C Pointing is done by moving the mouse so the cursor points at a desired location on the screen. As the mouse is moved, the cursor tracks its movements.
- C Left- and right-clicking are done by pointing to a desired location on the screen and then operating the mouse's left or right button, respectively, by pressing and then releasing the appropriate mouse button. Right-clicking on an icon button displays a short help message about that object at the bottom of the screen.
- C Double-clicking is done by pointing at the desired screen object and then pressing and releasing the left mouse button twice in quick succession. Pointing and left-clicking is the standard action to take to select and object for use. Double-clicking both selects the object for use and causes a logical action to be taken on it.

- C Dragging is done by pointing to a desired location on the screen, pressing (and holding) the left mouse button, moving the mouse to another desired location on the screen, and the releasing the mouse button.
- C Dragging and dropping is a special capability offered by some screen objects. Physically, it is done exactly like dragging; however, there are additional visual and functional effects. When the mouse button is pressed on an object offering drag-and-drop functionality, the cursor will change to indicate it has be "loaded" and is prepared for the drop. When the mouse is moved to the target area of the screen and the mouse button released, the cursor returns to its normal appearance and the action represented by the dragged icon takes effect.

### 10.1 SCREEN

The WSPRO Input/Output Program divides the screen into four areas: the Command Bar, the Tool Bar, the Schematic Layout Window, and the Browser Window. These areas are used in conjunction with each other to carry out WSPRO-related tasks. Only one of these areas is "active" at a time. The active area is indicated by an orange box which highlights the perimeter of the active area. In addition to the orange box which indicates the active window, the title bars of the Schematic Layout and Browser windows are green when active and white when not.

Some screen control features are common to more than one area. They are discussed below.

• • • • Up, down, left, and right, respectively. These icon buttons are the up, down, left, and right button icons, respectively. There are left and right buttons at the left end of the Command Bar and one of each on appropriate ends of the scroll bars in the Schematic Layout and Browser windows. Clicking on one of these buttons causes the affected bar or window to scroll in the indicated direction revealing additional information or icon buttons.

▲ ▼ *Maximize* and *minimize window*, respectively. One of these buttons can always be found in the upper right-hand corner of the Schematic Layout and Browser windows. When the ▲ button is present and left clicked, the affected window will expand to full-screen size. When the ▼ is present and left-clicked, the window contracts to its normal size and place on the screen.

*Clear.* This button is found in the upper left-hand corner of the Schematic Layout and Browser Windows and in dialog boxes. In the Schematic Layout and Browser windows, left-clicking it causes whatever information is displayed there to be cleared. In dialog boxes, it has the same effect as the Cancel button.

\*

Scroll Bar. A vertical or horizontal

scroll bar appear at the bottom or

right edge, respectively, of the Schematic Layout and Browser windows. Only a horizontal scroll bar is shown here. Clicking on the arrow button at either end of the scroll bar causes the affected display to scroll in increments of one line or character. Dragging the "thumb" (the button icon on the scroll bar between the arrow buttons) causes the display to scroll an amount proportional to the distance the thumb is dragged. Clicking in the gray area between the thumb and the arrow causes the display to scroll an amount equal the one window's width.

# 10.2 COMMAND BAR

The command bar contains 17 button icons which, when left-clicked with the mouse, carry out specific actions. Not all 17 buttons are visible simultaneously. At the left end of the command bar are left and right scroll buttons. Clicking these buttons causes the command bar to "scroll" making the "hidden" buttons "scroll" into view for access. The actions associated with each button is described below.



*Open/Create.* This icon summons a dialog from which the user selects and loads a particular existing input data set or creates a new one.



Save. This icon saves the currently open data set to disk.



*Run.* This icon executes (analyzes) the currently open data set and displays the output in the Browser window.



Quit. This icon quits and exits the WSPRO Input/Output Program.



*Flow Characteristics.* This icon summons a scrollable dialog box where the user enters information from which a starting water-surface elevation can be obtained. This record is also used to insert a Q record in the data set. See Tables 4-33, 4-36, and 4-39 for details about use of this record.



*Title Information.* This icon summons a dialog for entry of up to three lines of textual information inserted into the ASCII data set as T1, T2, and T3 commands. See Table 4-37 for details about this use of this record.



*DA Command.* This icon summons a scrollable dialog box which allows entry of abutment scour information and inserts an appropriate DA command into the ASCII data set. See Table 4-15 for details about use of this record.



*DC Command.* This icon summons a scrollable dialog box for entry of live-bed and clear-water scour information and inserts an appropriate DC command into the ASCII data set. See Table 4-16 for details about use of this record.



*DP Command.* This icon summons a scrollable dialog box for entry of local pier scour computation parameters and inserts an appropriate DP command into the ASCII data set. See Table 4-18 for details about use of this record.



*HP command.* This icon summons a scrollable dialog box where the user provides hydraulic property information and inserts an HP record into the ASCII data set. See Table 4-28 for details about use of this record.



*J1 Command.* This icon summons a dialog for entry of computational control parameters and inserts a J1 record into the ASCII data set. See Table 4-29 for details about use of this record.



*UT command.* This icon summons a scrollable dialog box for entry of parameters specifying user-defined output tables by inserting a UT record into the ASCII data set. See Table 4-39 for details about use of this record.



*Input.* This icon summons the currently established input data set to the Browser window. Whatever was previously displayed in the Browser window is cleared before the input file is displayed. No input data are lost when the Browser window is cleared.



*Output.* This icon summons the current output data file to the Browser window. Whatever was previously displayed in the Browser window is cleared before the output file is shown. No output data are lost when the Browser window is cleared.



*Section.* This icon summons a scrollable dialog in which the names of crosssections available for plotting are displayed. Left-clicking on a cross-section name and the OK button (or double-clicking on the cross-section name) displays the cross-section with water-surface elevation in the Browser window. Note: Since no output cross-sections are available until after the input data set has been analyzed, the dialog will be empty until the input data are analyzed.



*Profile*. This icon summons a scrollable dialog in which the names of available output profiles are displayed. Left-clicking on a profile name and the OK button (or double-clicking on the profile name) displays the cross-section and water-surface elevation in the Browser window. Note: Since no output profile are available until after the input data set has been analyzed, the dialog will be empty until the input data are analyzed.



*Units.* This icon summons a dialog where the user sets the unit system for the input data set and inserts and appropriate SI command in the data set. Note: Four additional buttons are displayed with this dialog. They are for options not implemented in the current version of the program. See Table 4-36 for details about use of this record.



*Paths.* This icon allows the user to change the default path and file name information. This option is not implemented in the current version of the program.



*Device*. This icon allows the user to set the default output device. This options is not implemented in the current version of the program.



*Global Defaults.* This icon summons a dialog in which the user specifies global defaults. This option is not implemented in the current version of the program.



*Graphic Defaults*. This icon summons a dialog in which the user specifies defaults for the graphic devices. This capability is not implemented in the current version of the program.

# 10.3 TOOL BAR

The Tool Bar becomes active when clicked. This is indicated by the orange box surrounding its three icon buttons. The tools in the Tool Bar are used to select and view elements of the WSPRO data set and its output. Icon buttons are used by dragging and dropping them onto the icons attached to the River in the Schematic Layout window. The reverse, dragging and dropping the attached icons from the Schematic Layout window onto these buttons, has the same effect. The three buttons are:



*Edit.* This icon summons the dialog associated with the edit function. This allows the user to edit an existing header. Using this icon button has the same effect as double-clicking an attached icon. Whatever was previously displayed in the Browser window is first cleared. No data are lost by clearing the Browser window.



*Graph.* This icon summons a graph of that cross-section to the Browser window. Whatever was previously displayed in the Browser window is first cleared. No data are lost by clearing the Browser window.

*Print.* This icon sends the current data to the output device.

# 10.4 SCHEMATIC LAYOUT WINDOW

The Schematic Layout window normally occupies the left side of the screen. It becomes "active" when left-clicked. This is indicated when its title bar turns green and the orange box surrounds it. When no header information is available in the data set, most of the window is blank containing only a representation of a channel labeled "RIVER".

To the far left of the Schematic Layout is an area labeled "Header" containing eight icon buttons. The top six of these buttons represent header records. To use one of these icons, drag and drop it on the "RIVER" in the Schematic Layout window. This will summon a dialog box in which the user can specify parameters associated with that header and attach an icon to the River to indicate the header is present in the data set. Initially, the icon will attach to the River at the top, but its location along the River will change when its location is edited in the corresponding dialog box.

Once attached to the River, information associated with these icons can be viewed and modified. Dragging and dropping an attached icon on the Edit, Graph, or Print icons causes and edit, graph, or print action, respectively, to be carried out on the information associated with the dragged icon. The reverse action, dragging and dropping the Edit, Graph, or Print icons on an attached header icon, has the identical effect. Left-clicking on an attached icon selects it as indicated by highlighting around its perimeter. Double-clicking on an attached icon summons its dialog box so data associated with that element can be edited.

The six header icons are:

*XS Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines cross-section information. See Table 4-42 for details about use of this record.

*BR Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines bridge section information. See Table 4-8 for details about use of this record.



*CV Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines culvert section information. See Table 4-15 for details about use of this record.

*GB Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines guidebank section information. See Table 4-25 for details about use of this record.



*XR Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines road-grade section information. See Table 4-41 for details about use of this record.



*XT Header*. This icon when dragged to the River in the Schematic Layout window summons a dialog box where the user defines template section information. See Table 4-43 for details about use of this record.

Two of the icons under the Header title allow the user to edit header records. These icons are also "drag and drop" icons.



*Clipboard.* Dragging and dropping an icon attached to the River onto the Clipboard icon (or vice versa) creates a copy of the dragged header information and summons its dialog box for the user to edit. The two icon button images shown represent an empty and loaded clipboard, respectively.



*Trash can.* Dragging an attached header record icon from the River to the trash can icon (or vice versa) deletes information associated with that header from the data set.

#### **10.5 BROWSER WINDOW**

The Browser window allows the user to view both textual and graphical input and output information. It is a "read only" viewer and does not allow the displayed files or graphics to be edited. When occupying only half the screen, the Browser window can be enlarged to full screen by left-clicking on the button at its upper-right corner. Left-licking the button in the same corner again causes the Browser window to reduce to its normal size. When the information being browsed contains more data than can be displayed in the window, the scroll bars can be used to access the additional data. Graphical data are always displayed in a format which fits within the boundaries of the Browser window, regardless of its size.

#### **10.6 DIALOG BOXES**

Not all interaction is done through the mouse and screen. Text input to the program is entered by the user with the keyboard and accepted by the Input/Output Program through dialog boxes. Dialog boxes may contain circular "radio" buttons, text fields, and scroll bars. When a dialog box is displayed, the user can left-click in a text field to begin data entry in that field. Clicking on a radio button sets the associated property or option. In some cases clicking on a radio button leads to other options which can be set through additional radio buttons. When too many fields are needed to display all the available fields within the space of the dialog box, a vertical scroll bar is available on the right side of the dialog box allowing the user to scroll up and down to reach all the available data fields. Once all the data is entered and the choices made, the user accepts the information in the dialog box by left-clicking on the OK button. If the user left-clicks on the Cancel button, conditions are reset to the state which existed before the dialog was summoned.

## **10.7 COMPUTER CONSIDERATIONS**

The HYDRAIN WSPRO Input/Output Program is designed to run only on a PC/AT (or higher) IBM-compatible using MS/DOS or an equivalent operating system. Computers running the HYDRAIN WSPRO Input/Output Program must have at least 560 Kb free memory. It is also necessary to load the HYMOUSE mouse driver by typing "HYMOUSE" from a DOS prompt. The program can be evoked from the WSPRO directory by typing "WSHLL" on the command line.

# SECTION 11 - APPENDIX C / THE SURFACE WATER MODELING SYSTEM (SMS) WSPRO GRAPHICAL USERS INTERFACE

The information in this appendix is modified from the corresponding chapter in the Surface Water Modeling Systems Reference Manual, Brigham Young University, Engineering Computer Graphics Laboratory (1997)<sup>16</sup>.

WSPRO is a water-surface profile computation model that can be used to analyze onedimensional, gradually-varied, steady flow in open channels. WSPRO also can be used to analyze flow through bridges and culverts, embankment overflow, and multiple-opening stream crossings. WSPRO is supported and maintained by the U.S. Federal Highways Administration (FHWA).

WSPRO categorizes its input data into five general groups:

- Title information. Used for output identification.
- Job parameters. Used to define parameters that pertain to the entirety of the profile computations.
- Profile control data. Information regarding discharge, starting elevation and computation direction.
- Cross-section definition. Information describing physical system (geometry, roughness, etc.).
- Data display commands. Used to control output of tables of cross-sectional properties, velocity, conveyance, etc..

SMS provides graphical tools for defining and editing the data in each of these groups, graphically editing cross-sectional information, and visualization of profiles and cross-sectional properties computed by WSPRO.

This chapter describes the commands used to create and edit the WSPRO specific parameters included in the WSPRO menu. The commands for selecting sections and operating on the river model are described in Chapter 7. SMS also interfaces to WSPRO to invoke analysis of a river model. After the analysis is complete, SMS can import the solution file via the WSPRO Display Options Dialog to allow the user to view cross section and profile plots of all the data

generated by WSPRO (See Lesson 13 of the SMS Tutorial and the WSPRO Users Manual for more about running WSPRO).

## 11.1 NEW SIMULATION

The *New Simulation* command in the WSPRO menu deletes the current river model including all of the WSPRO specific data. This data includes the run control data, the job parameters, and the solution data. Run control data contains the computation directions, profile discharges etc. Job parameters include output tables and tolerances. Solution data includes all variables computed for profile or cross section visualization that is not geometric. The *New Simulation* command also deletes the general river model data (geometric cross sections). To delete all the data currently in SMS, the user should select *New* from the *File* menu which causes all existing data (geometry and model specific data) to be deleted from memory.

## 11.2 OPEN SIMULATION

The *Open Simulation* command in the *WSPRO Menu* reads in a data file that has been previously created and saved. These files typically have the file extension ".dat". The name of the current simulation is displayed at the top of the River Window. The data file contains both the geometric and model control data for a WSPRO analysis. Geometric data consists of both the definition of the shape of the section and the section reference distance locating sections in relation to each other. See Section 4.2 of the WSPRO Users Manual for more information on data record formats. Opening a new simulation file causes all existing river data (geometry and model specific data) to be deleted from memory, however, data in other formats (such as two dimensional mesh data is not affected).

## 11.3 SAVE SIMULATION

The *Save Simulation* command in the *WSPRO Menu* saves a data file so that it can be opened at a later time or used in an analysis.

## 11.4 EDIT SECTION

The *Edit Section* command in the *WSPRO Menu* invokes the *Section Editor* (see Figure 11.1). This editor can also be invoked by double clicking on a section in the *River Window*.

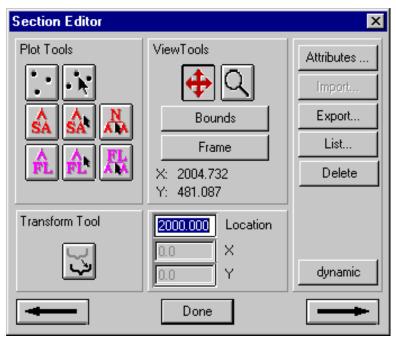


Figure 11-1. WSPRO River Section Editor.

The *Section Editor* is divided into five areas. These include the plot tools in the upper left portion of the dialog, the view tools in the upper center, the section translation tool in the lower left, section edit fields in the lower center, and various graphical buttons on the right side. Each of these areas are described in some detail in the sections to follow. In the bottom corners of the *Section Editor* are arrows that allow the user to step through the selected sections and edit each in turn. If only one section is selected, the arrows cause the current section to be unselected and the next section in the river model to be selected.

#### 11.4.1 Plot Tools

The plot tools area in the upper left portion of the *Section Editor* presents a set of tools that apply to the river section that is currently being edited. For example, if the user is editing a cross- section, up to three rows of tools are available (see Figure 11.1). The first row consists of tools to edit the geometry points (GR records). The second row consists of tools to create and edit the materials data including break points (SA records) and roughness data (N or ND records). The third row includes tools to specify flow lengths (FL records) for different portions of the section. The visible tools change based on the section type, and the section position. For example, the flow length tools are not visible if the section being edited is the outlet of the river model, due to the fact that no flow lengths are applicable to the outlet. They also vanish if the section has a bridge section defined because this is an unsupported combination of records in *WSPRO*.

The tools for bridges, guide banks, culverts and roads are all specific to the section type. The available tools also change based on the attributes of the section. An instance of this is a component mode bridge as opposed to a coordinate mode bridge. Coordinate mode includes tools for geometry point manipulation, while component mode does not.

### 11.4.2 View Tools

The view tools area in the upper center portion of the *Section Editor* presents tools which allow the user to pan, zoom, frame, and specify specific window boundaries for the plots of the cross sections and profiles. These tools function exactly as their counterparts in the display menu for the graphics window (see Sections 2.3.2 & 2.9.4 of the SMS Version 5.0 Reference Manual).

*SMS* tracks the location of the cursor in the plot window, both in cross sections and profile plots. This information is reported below the view tools.

### **11.4.3 Section Translation Tools**

The section translation tool is located in the lower left portion of the *Section Editor*. When this tool is selected, the user can translate the entire section either graphically by clicking on the cross section and dragging the geometric data. This is useful for general placement of one section with respect to another. For example, a bridge can be dragged to an approximate location with respect to the full valley cross section.

If a specific translation is desired, such as would be the case if the datum for one section was different than the other sections in the model, the user can specify the translation in the X direction and/or the Y direction and click on the bottom graphics button on the right side of the editor to invoke a translation. This bottom button reads "translate" while the section translation tool is selected.

## 11.4.4 Section Edit Fields

The lower middle region of the *Section Editor* includes three edit fields that are used to enter specific numbers for editing the section. The top edit field allows the user to specify a new section reference distance (SRD) for the current section. The lower two edit fields change function based on the current tool. For example, if the current tool is the edit geometry point tool

, it edit fields represent the X and Y location of the currently selected geometry point (GR record ). This allows the user to edit the point graphically in the plot window, or explicitly using the edit fields. These fields are also used for specific section translations as described in Section 12.4.3 of the SMS Version 5.0 Reference Manual, as well as specific values for SA or FL

breakpoints for the section. The label to the right of the edit field updates to reflect the current purpose of the edit field.

#### 11.4.5 Section Editor

The right side of the *Section Editor* includes six buttons to operate on the section. The *Attribute* button allows the user to access additional dialogs to edit the current section. Each section type has its own set of attributes. For each section type, the attribute dialog is described below.

#### 11.4.5.1 Cross-Section Attributes

Cross- sections require only a name and section reference distance (SRD). The SRD is specified in the *Section Editor*, the name can be specified at the top of the *Cross Section* dialog. The user can access other optional attributes including skew, loss coefficients, valley slope and friction slope averaging method. Default values are filled into these attributes when they are turned on. Any unspecified attribute is entered as a blank in the *WSPRO* data file, causing *WSPRO* to assume a default value.

Cross Section	×
Section Parameters	
EXIT Name	HP Record
	•
Skew (Degrees)	_
Skew	0.00
Coefficient of expansion loss	0.000
Coefficient of contraction loss	0.000
🗵 Valley slope	0.001200
Eriction slope averaging method	,
geometric mean conveyant	се
O arithmetic average convey.	ance
O arithmetic average friction :	slope
O harmonic mean friction slop	)e
OK	Cancel

Figure 11-2. Cross-Section Attributes Dialog.

*WSPRO* also includes an option to create tables of cross sectional properties (HP record). The user defines the desired tables using the HP Record button in the upper right corner of the attribute dialog. This invokes the *WSPRO HP Tables* dialog (see Figure 11.3). Tables can be generated for the entire cross section or for sub areas. The user may also define one velocity conveyance table for each section. If the user desires additional velocity conveyance tables the extra records must be managed by hand.

WSPRO HP Tables		×
🗵 Entire cross sec	tion	1
× 453.000	Minimum elevation	
1.000	Elevation increment	
463.838	Maximum elevation	
🕱 Sub areas		-
× 453.000	Minimum elevation	
× 1.000	Elevation increment	
463.838	Maximum elevation	
🗵 Velocity and co	nveyance	-2
5000.0	Discharge	
<b>×</b> 453.000	Minimum elevation	
1.000	Elevation increment	
463.838	Maximum elevation	
Reset Defaults		
OK	Cancel	

Figure 11-3. HP Record Dialog.

#### 11.4.5.2 Bridge Section Attributes

Bridge sections have many attributes. The *Bridge Section* dialog is divided into four parts (see Figure 11-4).

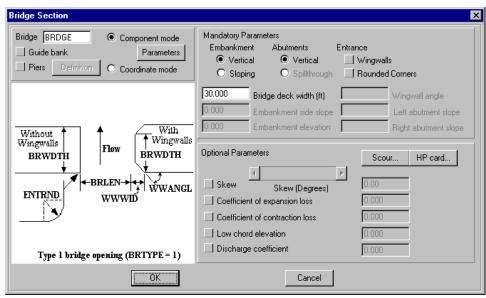


Figure 11-4. Bridge Section Attributes Dialog.

The upper left portion allows the user to change the section name, add piers and/or guidebanks, and specify the geometry mode for the bridge. *WSPRO* allows bridges to be defined explicitly (coordinate mode) using points (GR record), or explicitly using parameters (component mode). If component mode is used, the parameters defining the bridge are defined using the *Bridge Component Mode Parameters* dialog (see Figure 11-5), which is invoked using the *Parameters* button. If the *Pier* toggle is selected, *SMS* will create a pier record for the bridge (PD record). The *Definition* button allows the user to define the widths of the piers and the number of piers at different elevation.

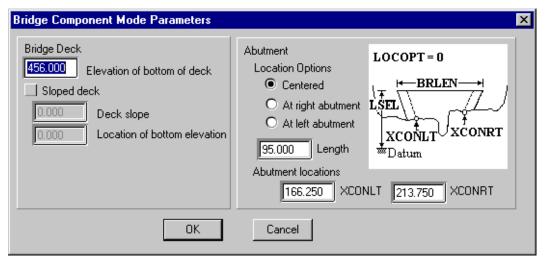


Figure 11-5. Bridge Component Mode Parameters Dialog.

The upper right portion of the dialog includes the mandatory parameters for the bridge. These include the embankment type, the abutment type, the deck width and several slopes and angles that may or may not be required depending on the bridge type and mode. The elements which don't apply to the currently selected type are dimmed. The lower left portion displays the parameters for the currently selected bridge type.

The lower right portion includes optional parameters including skew, discharge and loss coefficients., cross sectional, and scour parameters. The scour is defined using the *Bridge Scour Options* dialog (see Figure 11.6). Three types of scour records are defined. These include abutment scour (DA records), live-bed/clear-water scour (DC records), and pier scour (DP records).

Bridge Scour Op	ions	×
🔀 Abutment Sco	ır	
<b>X</b> 1.000	K1 🗆 0.000 yL 💷	0.000 Q
<b>×</b> 1.000	K2 🗵 0.000 yr 🗷	1.000 FS
🗵 Live-Bed / Cle	ar-Water Scour	
1	Number of DC records	
D	Current DC record	▶
Live-b	ad scour O Clear-water scour	1
0.000	bxL 🗵 0.000 axL 🗌	0.000 ҮЬ
0.000	bxR 🕱 0.000 axR 🕱	0.000 Ya
0.590	K1 0.000 PW	
Local Pier Sco		
D D D D D D D D D D D D D D D D D D D	Number of DP records	
0	Current DP record	Þ
	1	1
	bxL 📕 K1 📕	PW
	bxR  K2 L D1m K3	ЧЬ
	V1m K4	Qb
	OK Car	ncel

Figure 11-6. Bridge Scour Options Dialog.

#### 11.1.5.3 Road Section Attributes

Road grade sections require a name, SRD, road type and embankment top width. Optional parameters include a skew and weir flow coefficient. These are all specified in the *Road Grade Section* dialog shown in Figure 11-7.

Road Grade Section	×
Road Section Parameters Road type Name O Paved O Rough	
Skew (Degrees)	
Specify unsubmerged weir flow	
0.000 Unsubmerged weir flow coefficient	
OK Cancel	

Figure 11-7. Road Section Attributes Dialog.

## 11.1.5.4 Culvert Section Attributes

Culvert sections can be used to model simple culverts, or culverts in combinations with one or more bridge openings. Culvert section attributes are specified using the *Culvert Section* dialog shown in Figure 11-8. The left side of the dialog allows for definition of culvert parameters, while the right allows the users to define the type and size of culvert.

Culvert Section		×
Placement and Elevation         Name         190.000       Distance from left bank (ft)         4.000       Culvert length (ft)         456.582       Downstream invert elevation         456.582       Upstream invert elevation         1       Number of barrels         User Specified Coefficients       Entrance loss (Ke)         0.000       Velocity head (alpha)         0.000       Manning's roughness (n)	Geometry Shape Material Box Concrete Circular Corrugated steel pipe Arch Aluminum 48.000 Maximum barrel height (in) 48.000 Maximum barrel width (in) Arch parameters D.000 Bottom radius (ft) D.000 Corner radius (ft) D.000 Corner radius (ft)	Inlet    Wingwalls (30-75 degree flare)    Headwall (square edge)    Wingwalls (15 degree flare)    Wingwalls (extended w/ no flare)    Headwall (1:1 bevels)    Wingwalls top bevel; (Ke = 0.2)    Headwall (bevel on 3 sides)
	OK Cancel	

Figure 11-8. Culvert Section Attributes Dialog.

### 11.1.5.5 Guide Bank Section Attributes

Guide bank sections are bound to a specific bridge section. They are created by selecting the *Guide Bank* toggle in *the Bridge Section* dialog (see Figure 11-4). Once a guide bank exists, its parameters can be modified using the *Guide Bank Section* dialog (see Figure 11-9) accessed through the *Section Editor*. This dialog allows the user to define the name of the section, loss coefficients, skew, cross sectional tables, and the type of guide bank.

Guide Bank Section	X
Guide Bank Parameters	
GB Name	HP record
4	
Skew Skew (Degrees)	0.00
Coefficient of expansion loss	0.000
Coefficient of contraction loss	0.000
Valley slope	0.000000
Type Elliptical guide bank, no skew	
O Elliptical guide bank, skewed	0.000 Offset
O Straight guide bank, no offset	
O Straight guide bank, offset	
	ancel

Figure 11-9. Guidebank Section Attributes Dialog.

# 11.5 VIEW DATA FILE

The *View Data File* command in the *WSPRO* menu asks the user to select a text editor and then a file. The default editor is Notepad on PC systems and VI on UNIX systems. *SMS* then launches the specified text editor with the specified file. This allows the user to look at the input files used to run *WSPRO*, and the output files generated by *WSPRO*.

## 11.6 ROUGHNESS PARAMETERS

*WSPRO* uses roughness parameters or Manning's n values to simulate the different types of bed conditions in the river. In the *Section Editor* (Section 11.4), the user assigns materials to

the different regions in a cross section. The *WSPRO Material Editor* (see Figure 11-10) allows the user to associate Manning's n values to the materials used by each section. Materials can use a single roughness value, or have a roughness that varies based on the depth of the flow. This simulates situations such as tall grass that resists flow (high roughness) until the flow reaches a depth at which the grass is pushed over and lays down.

WSPR0 Material Editor	×	
Name: n_0.065_0.060 ID: 4 n_0.065_0.060 n_0.040 n_0.065_0.060 n_0.055_0.050	$ \begin{array}{c c}  & & & \\ \hline \hline & & & \\ \hline \hline & & & \\ \hline & & \\ \hline & & & \\ \hline \hline & & \\ \hline & & \\ \hline \hline & & \\ \hline \hline & & \\ \hline \hline \hline \\ \hline \hline \\ \hline \hline \hline \\ \hline \hline \hline \\ \hline \\$	
	Use depth breakpoints	
	0.0650 Top Manning n (n1)	
	0.0000 Top Depth Breakpoint (d1)	
General material properties	0.0650 Bottom Manning n (n2)	
	0.0000 Bottom Depth Breakpoint (d2)	
Close		

Figure 11-10. WSPRO Material Editor Dialog To Specify Roughness Parameters.

# 11.7 WSPRO RUN CONTROL

*WSPRO* requires the user to specify the flow rates for which profiles are to be computed. The *WSPRO Run Control* dialog (see Figure 11-11), which can be accessed from the *WSPRO* pull down menu, allows the user to specify the flow rates, and boundary conditions as well as assign a title to the simulation. Multiple profiles can be computed at once, and branches in the river may be simulated by changing the flow rate at various sections. The user also controls the direction of computation. Upstream is usually used for subcritical situations while downstream is applied for critical and supercritical flow.

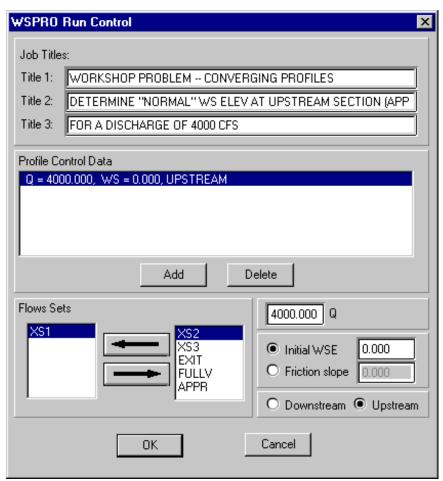


Figure 11-11. WSPRO Run Control Dialog.

#### **11.8 JOB PARAMETERS**

The *Job Parameters* command allows the user to specify the optional parameters associated with a numerical analysis. These include computational step size and tolerances to be used during computation. The *WSPRO Job Parameters* dialog (see Figure 11-12) also allows the user to specify the units to be used in *WSPRO*, and to specify any output tables desired from the analysis.

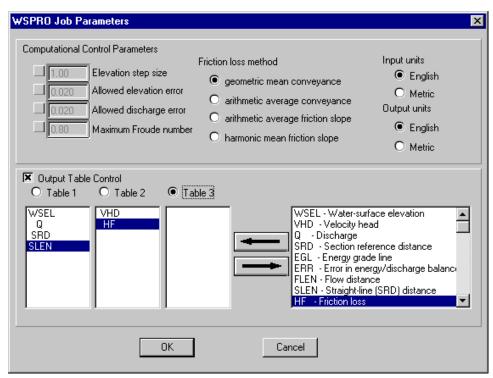


Figure 11-12. WSPRO Job Parameters Dialog.

## **11.9 DISPLAY OPTIONS**

*SMS* provides tools for both *WSPRO* model constructions and visualization of the results of a *WSPRO* analysis. The *Display Options* menu item allows you to specify what can be displayed, and how it will look. The top portion of the *Display Options* dialog (see Figure 11-13) allow the user to specify what colors will be used to display each type of section. The lower portion of the dialog includes tools to import the solution file created by *WSPRO* and specify which variables will be plotted in the *Plot* window.

Display Options		
Section Types  Cross sections  K Guide bank se  K Bridge sections  K Roadway sect		
Variables Contracted Uncontracted a K • K Critical Water-Surf (CRWS) a K • K Water-Surface Elev (WSEL) a K • K Energy gradeline (EGL) • Velocity head (VHD) a • Velocity (V) a • Area (A) a • Conveyance (K) a • Froude number (FR) • Flow (Q) • K Thalweg	Profiles View Import Delete View Options Show cross-section Options Show profiles Options % window for profiles	
 ОК		

Figure 11-13. WSPRO Display Options Dialog.

Each variable may be plotted for both the constricted and unconstricted flow cases. If no bridge or culvert sections exist, only the unconstricted case is available. The user also controls whether the plot window will include profiles, cross sections or both. If the user invokes the section editor, cross section display is turned on.

## **11.10 MODEL CHECK**

A *Model Check* should be performed on all *WSPRO* models before attempting an analysis. The model check will perform a basic check to insure that all of the needed information to run the analysis is present. The *Model Check* command in the *WSPRO* menu causes the *Model Check* dialog to appear.

Selecting the *Checker Options* button will cause the *WSPRO Model Checking Options* dialog to appear. This dialog lists the checks that may be performed during the model checking procedure. By default all supported checks are enabled. The checks include:

• Check Profiles. This check assures that at least one profile has been specified and that the appropriate number of discharges and boundary conditions are specified for each profile.

- Check SRD Values. This option checks the SRD of each section in the model. SRD values should increase monotonically from the exit to the most upstream section. There are some exceptions for multiple opening situations. This option also checks the placement of bridge approach and exit sections to assure correct placement.
- Check GR Points. Cross sections geometry should proceed from left to right. Coordinate mode bridge openings should proceed across the bottom of the opening, then change direction one time and define the bottom of the bridge deck.
- After running the model check, messages are generated to aid in the correction of the problems. To save this information to a text log file, click the *Save Messages* button and choose a file to save the information in. To close the *Model Checker*, click the *Done* button.

## 11.11 RUN WSPRO

Once the data for an analysis has been defined, *WSPRO* may be invoked by selecting the *Run WSPRO* menu item. This command checks the status of the model in *SMS*. If edits have been made, the user is prompted to save his files before running. It then launches *WSPRO* using the data files loaded into *SMS*. If multiple runs are desired, the user should go to the *Display Options* and delete previous solutions before reading in new one. numerical model requires several user specified parameters to control the analysis.

# **SECTION 12 - REFERENCES**

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