

A COMPUTER PROGRAM TO ANALYZE BENDING OF BENT CAPS

by

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Research Report Number 56-2

Development of Methods for Computer Simulation
of Beam-Columns and Grid-Beam and Slab Systems

Research Project 3-5-63-56

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The Texas Highway Department

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by the

CENTER FOR HIGHWAY RESEARCH
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PREFACE

This report is one of a series of developments planned to facilitate the use of computers in the analysis of highway bridge structures. It specifically concerns a computer program for the bending analysis of bent caps.

The development of this program began in June 1963 under sponsorship of the Texas Highway Department, Research Project 3-5-63-56, in cooperation with the U. S. Department of Commerce, Bureau of Public Roads. A preliminary report on basic concepts was furnished the Texas Highway Department in December 1963 for a period of trial use. Following this period of trial use, revisions were made to the program based on comments and criticisms of the users. In March 1964 a version of the program was furnished to the Texas Highway Department for use on their CDC 1604A computer. Further comments from the users have resulted in the present program.

Although the program is written for the CDC 1604 computer, it is in FORTRAN language and only very minor changes would be required for it to be compatible with IBM 7090 systems. Duplicate copies of the program deck and test data cards for the example problems in this report may be obtained from the Center for Highway Research at The University of Texas.

The cooperation and support of Texas Highway Department personnel are gratefully acknowledged. Particular thanks are given to Mr. Larry G. Walker who has acted as project contact representative and has provided much helpful advice. The use of the computer and facilities of The University of Texas Computation Center is also gratefully acknowledged. A library subroutine written by the Computation Center Staff is used to operate the digital plotter used in conjunction with this program.

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LIST OF REPORTS

Report No. 56-1, "A Finite-Element Method of Solution for Linearly Elastic Beam-Columns" by Hudson Matlock and T. Allan Haliburton, presents a finite-element solution for beam-columns that is a basic tool in subsequent reports.

Report No. 56-2, "A Computer Program to Analyze Bending of Bent Caps" by Hudson Matlock and Wayne B. Ingram, describes the application of the beam-column solution to the particular problem of bent caps.

Report No. 56-3, "A Finite-Element Method of Solution for Structural Frames" by Hudson Matlock and Berry Ray Grubbs, describes a solution for frames with no sway.

Report No. 56-4, "A Computer Program to Analyze Beam-Columns under Movable Loads" by Hudson Matlock and Thomas P. Taylor, describes the application of the beam-column solution to problems with any configuration of movable non-dynamic loads.

Report No. 56-5, "A Finite-Element Method for Bending Analysis of Layered Structural Systems" by Wayne B. Ingram and Hudson Matlock, describes an alternating-direction iteration method for solving two-dimensional systems of layered grids-over-beams and plates-over-beams.

Report No. 56-6, "Discontinuous Orthotropic Plates and Pavement Slabs" by W. Ronald Hudson and Hudson Matlock, describes an alternating-direction iteration method for solving complex two-dimensional plate and slab problems with emphasis on pavement slabs.

Report No. 56-7, "A Finite-Element Analysis of Structural Frames" by T. Allan Haliburton and Hudson Matlock, describes a method of analysis for rectangular plane frames with three degrees of freedom at each joint.

Report No. 56-8, "A Finite-Element Method for Transverse Vibrations of Beams and Plates" by Harold Salani and Hudson Matlock, describes an implicit procedure for determining the transient and steady-state vibrations of beams and plates, including pavement slabs.

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ABSTRACT

The problem of analyzing the cap of a highway bridge bent is handled by digital computer simulation. The solution is based on the beam-column method described in detail in Reference 1.

Any particular problem is solved in three phases. The first phase determines initial curves of bending moment and shear from dead loads only. The second phase applies a single movable load and expands the initial curves to positive and negative envelopes as this load is moved according to instructions from the user. The third phase duplicates the movable load configuration at the proper location in as many lanes as required to produce maximums of each design variable at each design-control point.

The program has numerous optional features including the ability to solve caps that are skewed, the ability to retain data or certain results from problem to problem, as well as the ability to give automatically plotted envelopes of maximums.

Three example solutions of one problem illustrate the various capabilities and uses of the program, including input data techniques and program control options.

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NOMENCLATURE

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
E	kips/ft ²	Modulus of elasticity
F	kip-ft ²	Flexural stiffness = EI
h	ft	Increment length
h _n	ft	Normal value of increment length
h _s	ft	Skewed value of increment length
i	-	Station number
I	ft ⁴	Moment of inertia of the cross section
Q	kips	Concentrated applied transverse load
θ	degrees	Skew angle of cap

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CHAPTER 1. INTRODUCTION

In the design of highway bridge structures, prescribed configurations of live load are placed on the structure in such locations that maximum stresses are reached at each of many critical points. To analyze complete structures or structural systems under all possible loading conditions is at present a hopelessly complex and time-consuming problem. In current design practice it is therefore necessary (1) to make some severe simplifications in the structure and in the way the loads are transferred, so that a piecemeal analysis may be performed, and (2) to reduce the complexity of loadings by the application of considerable individual judgment. Even with these short-cuts, a great amount of effort is still required by manual methods.

As initial steps in overcoming some of the difficulties, two computer programs have been developed.

- (1) The BMCOL program (Ref 1) is a finite-element representation of a beam-column that may be subjected to continuous or freely discontinuous loads and restraints. The approach constitutes direct numerical simulation of the physical system in the digital computer.
- (2) The CAP program uses a simplified adaptation of the BMCOL method to analyze the bending of a highway bridge bent cap under various controlled loading situations.

The purpose of this report is to describe the CAP program. Sufficient detail is included so that the uninitiated user may understand the workings of the program and may apply it to problems of design. Three example solutions are included which illustrate some of the various uses of the program and its optional features.

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CHAPTER 2. EXPLANATION OF METHOD

Computer Representation of Bent Cap

A wide variety of beam-column problems can be represented in the computer in accordance with the basic BMCOL method described in Ref 1. The more conventional case of a beam on simple supports with varying stiffness and with distributed and concentrated loads would be represented in the method as shown in Fig 1a. The beam is divided into many increments of equal length h .

Figure 1b shows one station along the beam as it is approximated in the computer. The flexural stiffness EI (the product of the modulus of elasticity and the moment of inertia of the beam, which will hereafter be designated by the term F) is represented at each station by a spring-restrained hinge. All stiffness and load values are applied only at station points and may vary in a freely discontinuous manner along the beam.

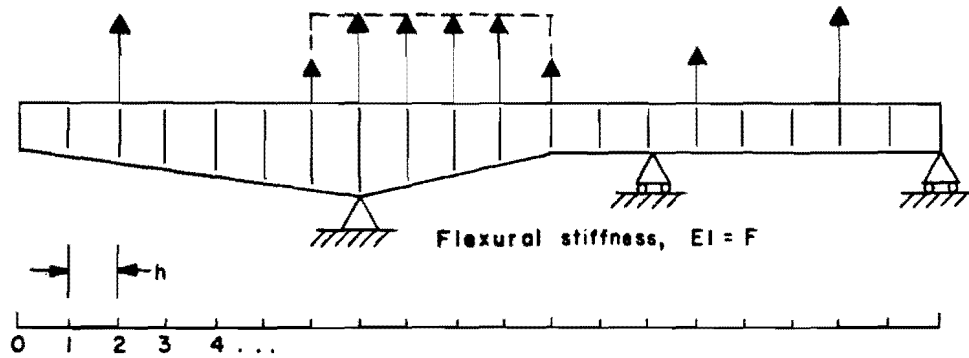
Figure 2a shows a generalized bent cap, simplified so that loads are transmitted through a slab and stringer system in accordance with current design practice. The structure is purposely shown somewhat distorted to illustrate generality. The supports are represented by knife-edges. The ends of the cap need not be at the zero station number nor at the last station number. They are not specifically designated but are established only by proper input of stiffness values; hinges or points of zero bending stiffness are automatically assumed at all stations beyond each end of the cap.

On the cap are stringers and the roadway slab rests in turn on the stringers. The slab may have curbs and a median and may be divided into lanes, usually according to AASHO design specifications (Ref 2). Simple-span distribution of slab loads is accomplished by the assumption of a hinge in the slab at each interior stringer.

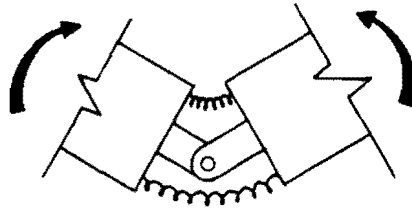
Caps may be skewed with respect to the roadway centerline.

Sign Conventions and Units

Upward deflections and upward loads are considered to be positive in accordance with the BMCOL method. Thus in the normal usage of the CAP program,

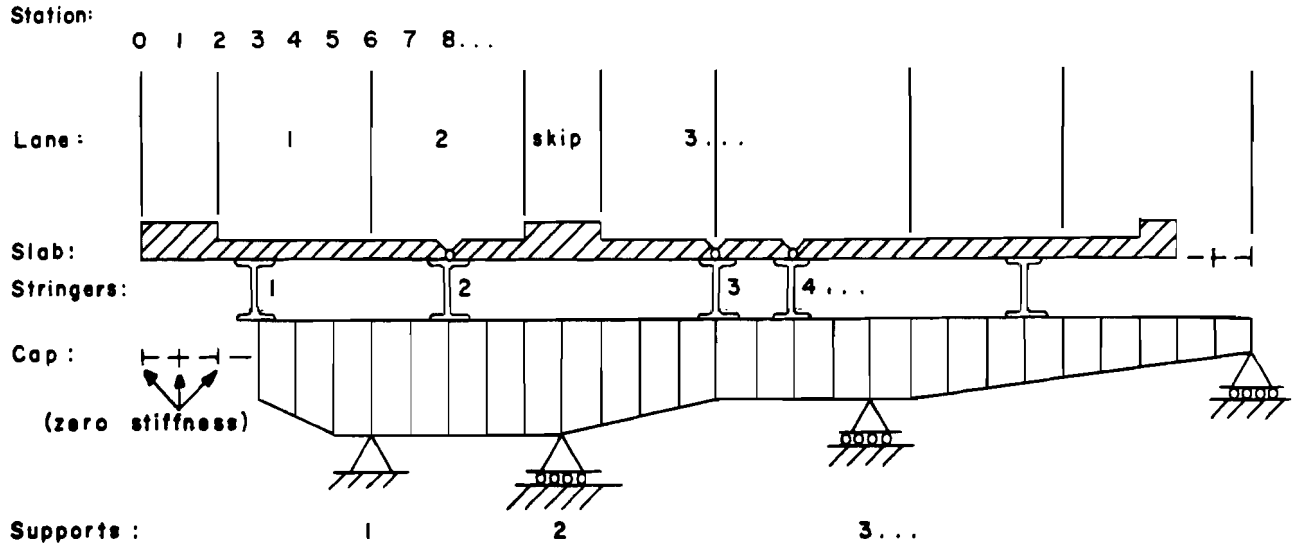


(a)

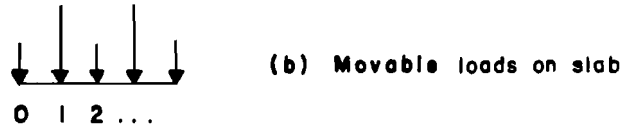


(b)

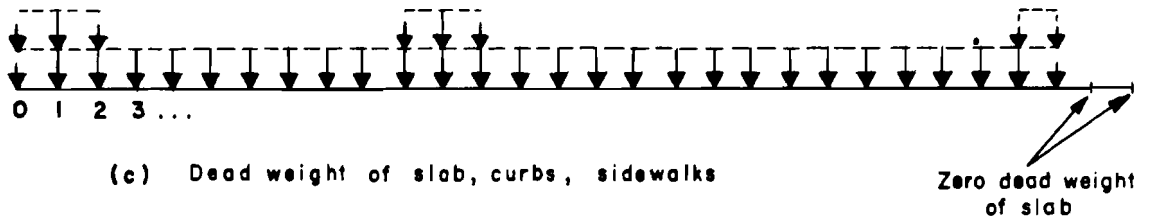
Fig 1. Finite-element representation of a beam. (a) Beam on simple supports, with varying flexural stiffness plus distributed and concentrated loads. (b) Flexural stiffness of the real beam is approximated at each station as a spring-restrained hinge.



(a) Elements of bent cap problem

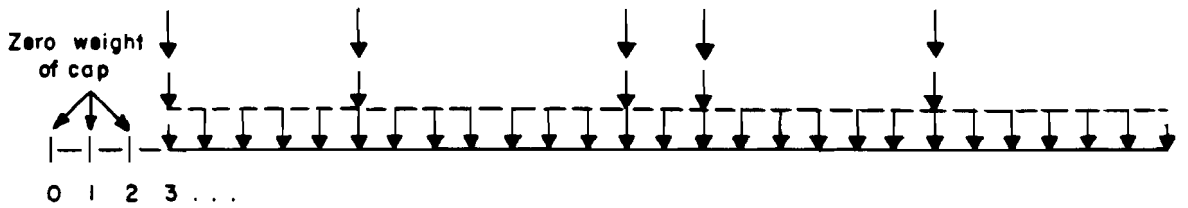


(b) Movable loads on slab



(c) Dead weight of slab, curbs, sidewalks

Zero dead weight of slab



(d) Cap loads (dead weights of cap and stringers plus reactions from slab loads)

Fig 2. Representation of bent cap and loads.

downward loads have a negative sign.

Units of kips and feet have been adopted for the program. The user must conform to this system. All distances along the cap must be in roadway-width dimensions (normal to the roadway centerline). The input skew angle must be in degrees and decimals of degrees.

Loadings

The loads on the cap are of two types.

- (1) Dead loads are developed directly from the dead weight of the stringers and from the dead weight of the cap itself.
- (2) Reactions occur on the cap at the stringer locations from simple-span distribution of slab loads. These slab loads consist of dead weight of the slab, curbs, sidewalks and medians plus live loads that can be moved to represent traffic loadings.

All of the slab loads are transferred by simple-span distribution to the stringers and thence to the cap. If there are no stringers, the slab rests directly on the cap and the loads are transferred station-by-station directly to the cap.

The movable load may be any desired configuration or combination of distributed or concentrated loads. Figure 2b shows one possible movable-load configuration. The stations for the movable load are completely independent of the fixed stations along the slab or cap except that the spacing must be the same for both stationing systems.

A range of movement for the movable load is specified by (1) a start station on the slab where the zero station of the movable load is first placed and (2) a stop station which is the last station on the slab where the zero station of the movable load is to be placed.

The positioning of the movable load within the range of movement may be controlled by specifying the number of increments between each successive position of the movable load. A solution of the cap is made for each such position of the movable load. For this initial series of loadings and solutions, all medians and lane boundaries are disregarded.

Subsequently, in the usual operation of the program, the movable-load configuration is automatically duplicated in several appropriate lanes as may be required to develop critical design conditions. The process is described in the following sections.

Figure 2c shows the dead weights of the slab, curbs, sidewalks, and medians. Figure 2d shows the dead weights of the cap and stringers together with the forces transmitted to the cap from the slab.

Computer Representation of Skew

Regardless of the angle of skew of the bent, all values of load and stiffness are input to the computer in terms of roadway-width dimensions and in quantities per station, as if the cap were normal to the roadway in Fig 3a. In Fig 3b a corresponding cap is shown that has been skewed through an angle θ . Although distances along the cap will vary with skew angle, in accordance with the increase in increment length from the normal value h_n to the skewed value h_s , the number of stations will not change.

The program internally adjusts only two items to properly represent skewed caps: the increment length and the correspondingly increased distributed dead weight of the cap expressed in kips per station. Both are increased in inverse proportion to $\cos \theta$.

In essence, the cap is skewed but the roadway is not. Regardless of skew, loads from the slab are transmitted at the same station points. The only increase in load is that associated with the increase in dead weight of the cap itself. Concentrated loads applied in the input data directly to the cap are not affected, but all distributed loads directly designated as cap loads in the input data are assumed to represent dead weight of the cap and are therefore adjusted. When all loads finally have been stored in the computer at all appropriate stations, the solution for bending of the cap proceeds using the skewed increment length. The effect on bending moments is then properly reflected in the computed results.

Program Operations

The CAP program is guided by design-control variables (moment, shear, and reaction), by design-control points designated by the user (for moment and shear), and by certain loading options. Each support is automatically a design-control point for reaction.

In the usual application of the program, AASHO-type multiple-lane loads will be arranged to produce maximum and minimum values of each design-control variable

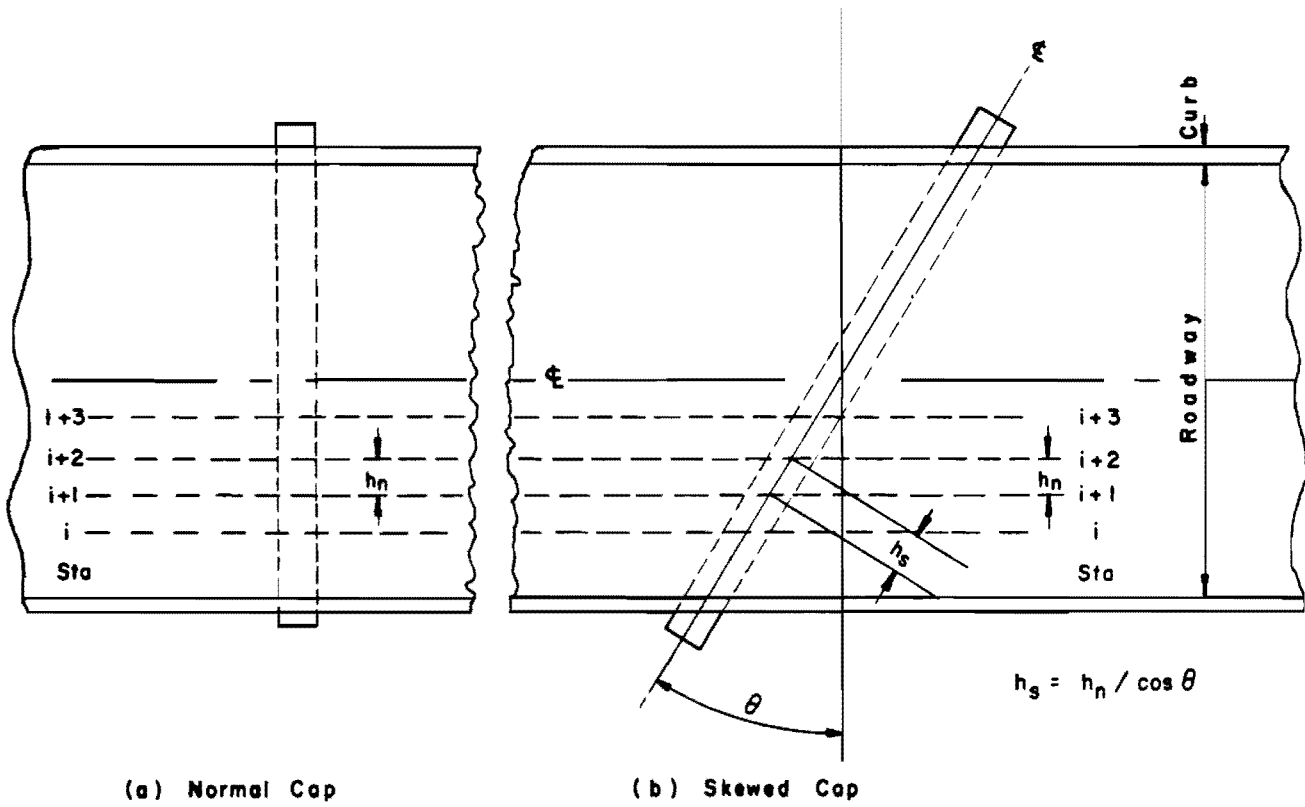


Fig 3. Computer representation of skew.

at each design-control point.

The effects of various dead and live loadings are not obtained separately and then superposed. Instead, each individual BMCOL-type solution constitutes a separate computer simulation of a complete structural system, including all dead and live loads that may be acting at the time.

After a solution is performed with each specific pattern of loads, the resulting design variables (bending moment, shear and support reactions) are compared at every station with the previous maximum and minimum values at that station. Whenever a new value is found that is more extreme than that retained from previous solutions, the new value replaces the old one. The resulting tabulations constitute envelopes of maximum positive and maximum negative effects at each station that have been progressively developed by the complete history of prior loadings. As shown in the simplified flow chart in Fig 4, a complete study for design of a bent cap is accomplished in three principal phases:

- Phase 1. Determination of the initial envelopes of moment and shear from a single solution considering only the dead loads of the system.
- Phase 2. Expanding of the envelopes due to effects of dead load plus the single movable load as it is shifted from position-to-position across the slab.
- Phase 3. Duplication of the movable-load configuration simultaneously in the appropriate number of lanes to produce the maximum of each design-control variable at each design-control point.

For each critical pattern of multiple-lane loading that is established by the program the following operations are performed in Phase 3.

- (1) The movable load is duplicated at the proper location in each of the appropriate lanes.
- (2) The appropriate live-load reduction factors (specified by the user, but usually in accordance with AASHTO recommendations) are applied.
- (3) The dead weights of slab, stringers and cap are added.
- (4) A BMCOL-type solution is performed.
- (5) The computed results at each station are compared against previously stored maximum and minimum values and in each case the more extreme value is retained.

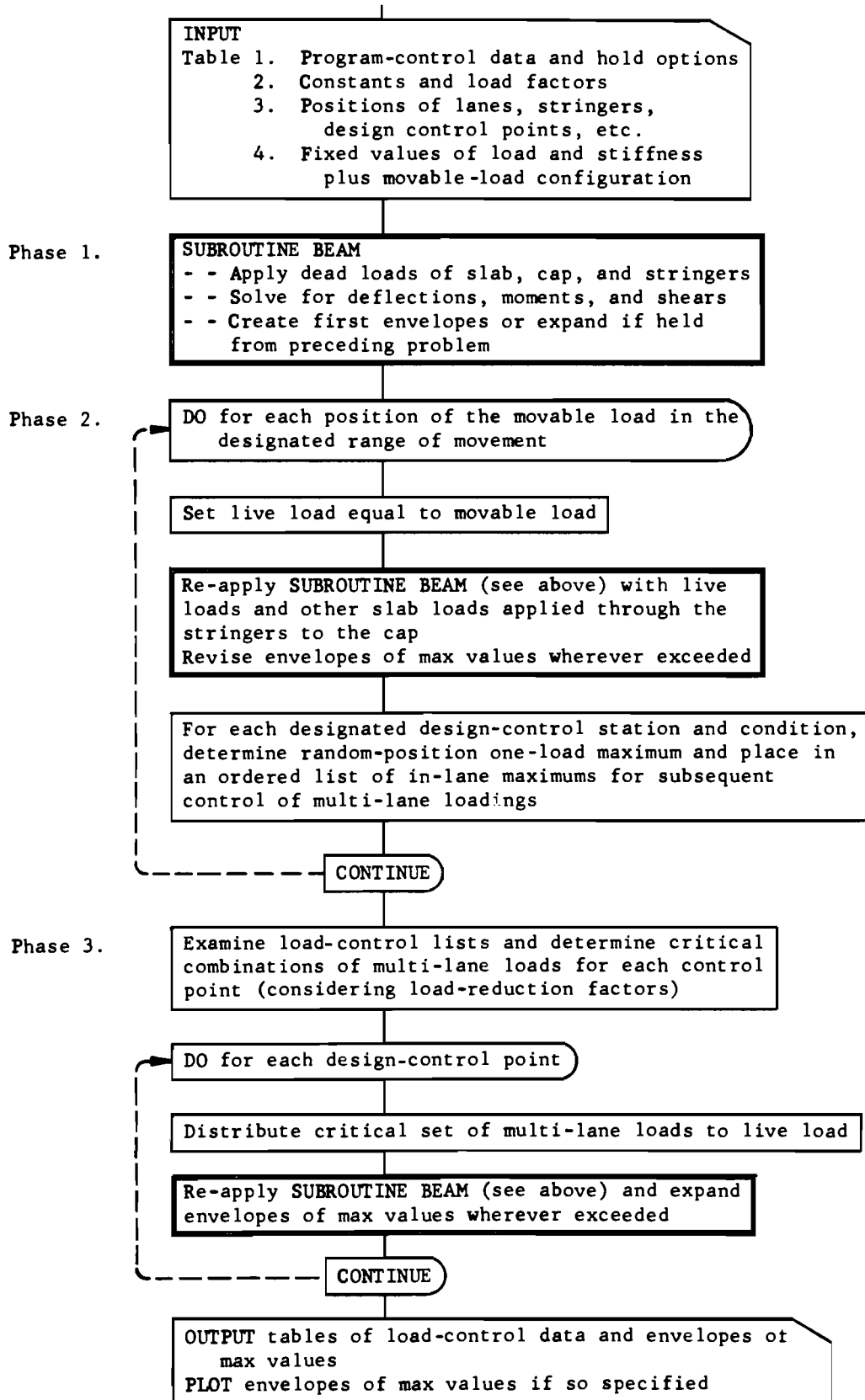


Fig 4. General flow diagram for program CAP 14.

Results Produced by the Program

Both input and output data are arranged and designated in terms of table numbers. Input data are always printed out in four tables which precede the computed results. The usual results of the program include

- (1) Table 5, a summary of movable-load effects;
- (2) Table 6, envelopes of extreme values of moment and shear;
- (3) Table 7, extreme values of reaction at each control point;
- (4) Table 8, scales used for automatic plots; plus
- (5) automatically plotted envelopes of moment and shear.

Included in the movable loading summary in Table 5 is the lane and station number of each significant lane load, arranged in the table in order of the magnitude of the contribution of that lane load to the corresponding moment or shear value. The one most critical loading combination also is indicated. Data in this table are used by the program internally to control the multiple-lane loadings of Phase 3.

A number of options are available for control and operation of the program which allow various loadings to be considered separately and may result in some variation from the usual patterns described above. Details are given in the next chapter and are subsequently illustrated by three variations of an example problem. Included are options that allow the user to exclude either out-of-lane or in-lane loading effects as well as to retain input data and computed results for use in sequences of similar problems.

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CHAPTER 3. DESCRIPTION OF CAP PROGRAM

The specific version of the CAP program that is described in this report is designated as CAP 14. The number indicates only that this version is the fourteenth significant revision in development of the program. It is to be expected that future developments will yield additional modifications.

The complete and detailed flow diagram for CAP 14 is shown in App 1. Appendix 1.1 is similar to Fig 4 and is included as a guide to the remainder of the flow diagram. The flow diagram is annotated at appropriate points to aid in understanding the program.

The program is written in FORTRAN-63 language for the Control Data Corporation 1604 and 1604A computers. With only minor changes it should be compatible with the IBM 7090-94 systems. Appendix 2 consists of a list of FORTRAN notation used and a complete listing of the program is in Appendix 3. One binary subroutine is included for plotting purposes. Storage requirement is approximately fourteen thousand words and the compile time is approximately two minutes and 30 seconds.

A guide for filling out coding forms is given at the end of this chapter of the report. It is designed so that additional copies may be furnished as separately bound extracts to be used in routine analysis with the program. Understanding of many of the following comments will be facilitated by a parallel study of the guide.

Standard Features of the CAP Program

Input of load, stiffness, and position data to the program is normally done according to station numbers. One exception is that stringer locations may be specified optionally in tenths of stations. This exception is discussed under optional features. It is permissible to use up to three hundred increments, ten lanes, twenty supports, thirty stringers, thirty design points for moment and thirty design points for shear. Each support is automatically a design point for reaction. There is a maximum number of five multiple-lane load-reduction factors and the solution is limited to five or fewer simultaneous lane loads. The load-reduction factors are left to the discretion of the user but ordinarily will be

in accordance with AASHO recommendations (Ref 2).

The ends of the slab and cap are not defined except by stiffness and load data. Overhangs of cap or slab are permissible but no load should be entered, or held from a preceding problem, which would cause a force to be applied directly to the cap at the first station beyond either end of the real cap. The zero station for a problem usually should be the left-most extension of either the cap or the slab.

All loads and flexural stiffness values are stored at station points; provision is made for the program to distribute values to storage based on linear interpolation between input values. The cap stiffness values and various types of loads are algebraically added into storage at each designated station. Therefore, superposition of data can be done merely by adding more data into storage or subtracting data from storage.

Reactions from slab loads are transmitted to the cap at the stringer locations unless the optional feature for fractional stringer stations is used. If this option is used, the transmitted load will be proportionally split to each station adjacent to the fractional station. If dead load of any stringer is included, it must go in at even stations. Thus, if the fractional stringer station option is used, the stringer dead load must be proportioned by the user and input as a load at each of the two adjacent stations.

In CAP 14, solutions are done for every position of the movable load in the designated range of movement regardless of lane boundaries or medians. For purposes of studying only non-lane loadings, the user may specify a movable load wider than a lane provided that he also specifies that the number of lanes is zero and therefore that no conventional in-lane loadings are to be done.

Optional Features

There are several options which the user may apply at his discretion and for his particular needs. The principal options are as follows:

- (1) Hold Envelopes. The envelopes of maximums may be retained from problem to problem in order to study cumulative effects of various loading situations. The envelopes will be appropriately expanded when a new maximum or minimum is encountered. Regardless of whether the envelopes are held or not, the multi-lane loading summary and control table is cleared for each new problem and therefore reflects only the movable load effects for the immediate problem considered.

- (2) Data Hold. Table 1 controls the input of data and includes options for holding data or results from the preceding problem. It therefore must be input anew for each problem. Any number of problems may be worked as one run and the hold options enable very economical saving and reuse of data. Tables 2 and 3, giving constants and lists of stations, may be fully retained from the preceding problem or otherwise must be fully input. The load and stiffness data of Table 4 may be retained and modified if desired by additions or subtractions in subsequent problems since all data in Table 4 are algebraically summed into storage at each designated station. Any number of cards may be used in this table.
- (3) Clear Envelopes. The envelopes of maximum and minimum design variables that are produced in Phase 2 by the single movable load may be erased prior to any multiple-lane loadings. The single movable-load option is a design condition which is usually considered by the Texas Highway Department. This allows a single movable load (truck) to be placed anywhere on the structure and not within the AASHO specified lane boundaries. The multiple-lane loading summary table would exhibit the appropriate results from all loadings but the envelopes would be from only the initial dead load solution of Phase 1 and the AASHO-type multiple-lane loading patterns of Phase 3.
- (4) Plot Envelopes. This option is used to cause the envelopes of maximum and minimum moment and shear to be plotted versus distance along the cap. Engineering scales are internally generated to produce the best plot of moment or shear in a 4-inch by 10-inch space. Design-control points are also plotted as well as the identifying problem number. The scales are included in the printed output as Table 8.
- (5) Table 5 Skip. This option allows suppression of printing the multiple-lane loading summary table. No computational advantage is gained through omission of this table; however, numerous pages of output can be saved from each problem. It is important that each user understand fully how the envelopes of maximums are created before arbitrarily electing to omit this information. Table 5 is the only source of this information.
- (6) Skew. The skew capabilities have been previously described. If no skew angle is specified, zero degrees is assumed. To solve a series of similar caps for a given structure, all data may be retained with only the value of skew angle being changed from problem to problem. If the option to hold envelopes is used, the skew angle must not be changed.
- (7) Movable-Load Incrementation. The positioning of the single movable load in Phase 2 of the cap solution may be controlled by specifying the number of increments between successive positions of the zero station of the movable load. This has the advantage of reducing computation time. If the bent cap is divided into a large number of increments (of very small size), this feature may be important. However, an extended period

of use of the BMCOL method has indicated that rarely is an increment length of less than one foot justified and virtually never less than one-half foot and then this feature need not be used. (The user may need to satisfy himself on this point by running trial solutions and it is strongly recommended that he do so.) It should be understood that in developing the critical multiple-lane loading patterns, the only load positions considered are those actually used in the Phase 2 loadings.

- (8) Load Reduction Factors. The lane-load reduction factors may be anything desired by the user. It is expected that they usually will be in accordance with AASHO recommendations.
- (9) Stringer Load Splitting. Stringers are normally specified exactly at stations. They may be specified optionally at the nearest one-tenth of an increment. A noticeable discrepancy in results may occur when a concentrated load, such as a stringer reaction, is improperly placed due to the increment spacing not precisely fitting the stringer positions. Reducing the increment size to match the stringer spacing is a solution, but may cause an unreasonable increase in computation time. To reduce both of these effects, stringer locations may be specified to the nearest one-tenth of an increment and the program automatically splits the stringer reaction in inverse proportion to the distances from the two adjacent stations. If this feature is employed, the user must proportion any dead load of the stringer and input it at the two adjacent stations on the cap. The program will not automatically split the dead load of the stringer itself.

Limitations

There are several things which CAP 14 will not do. A one-legged bent cannot be solved nor should the user attempt to specify two completely separated slabs on the same cap. Impact effects are not directly separated from those of static load, but it is permissible to run one problem with live load only and then one problem with live load plus impact to obtain the desired results.

CAP 14 does not output any of the deflections computed for the cap. Moment and shear diagrams from any particular loading are not directly available, but can be obtained in the form of identical maximum and minimum envelopes if a problem is arranged so that there is only one loading pattern considered. Where such results are desired, the BMCOL program normally should be used.

There is an arbitrary exclusion in the multiple-lane loading summary table of any contribution less than ± 0.001 ft-kips of moment or ± 0.001 kips of shear or reaction.

Error Messages

It is possible to get eight error messages from the main program of CAP 14 and three error messages from Subroutine BEAM. The main program gives error messages when

- (1) non-zero Table 4 data is specified or held from the preceding problem beyond the end of the cap (the problem is abandoned),
- (2) a movable load wider than a lane is specified and the number of lanes is not zero (the problem is abandoned),
- (3) lanes are specified which overlap each other (the problem is abandoned),
- (4) the design-variable values in the plot-control routine are beyond the range of the routine from $\pm 1.0 \times 10^{-100}$ to $\pm 1.0 \times 10^{100}$ (the problem is abandoned),
- (5) a design-control point for shear is specified within one station of a concentrated load (the program will ignore the erroneous design-control point and continue with the problem),
- (6) the number of increments between successive positions of the single movable load is left blank or is specified as zero (the program will assume a value of 1 and continue with the problem),
- (7) data is retained from a problem in which an error occurred (the program continues to search until an independent problem is found), or
- (8) a malfunction has occurred to cause the program to take one of several paths for which specific error messages are not provided (the problem is abandoned).

The Subroutine BEAM gives error messages when

- (1) only one stringer is specified (the problem is abandoned),
- (2) load is placed at any hinge location or point of zero bending stiffness (the problem is abandoned), or
- (3) a malfunction has occurred to cause the program to take one of several paths for which specific error messages are not provided (the problem is abandoned).

When a problem is abandoned, the program begins to search through the remaining data cards until a new problem designation is encountered. If the new problem is found to be completely independent from the erroneous problem (no hold-options exercised) a new solution will be started; otherwise, another new problem will be sought. It is partly for this purpose that the problem designation is repeated on every data card as shown in the input forms. The general arrangement allows any number of problems from any number of users to be stacked

for running in sequence. It reduces the danger that errors by one user will delay completion of subsequent sets of problems.

Guide for Data Input

On the following pages a condensed guide is given for preparing input data for the program. It is intended that separate copies be furnished for convenience in routine use.

One basic example problem is discussed in the next chapter. Three variations of the problem are solved to aid in learning to use the input data forms and to illustrate a variety of coding situations. Input and output listings for the examples are given in App 4 and App 5 and should be used to check practice coding.

GUIDE FOR DATA INPUT FOR CAP 14

with Supplementary Notes

extract from

A COMPUTER PROGRAM TO ANALYZE BENDING OF BENT CAPS

by

Hudson Matlock and Wayne B. Ingram

1 October 1966

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PROGRAM CAP 14 GUIDE FOR DATA INPUT --- Card forms

IDENTIFICATION OF PROBLEM (2 cards each problem; program stops if Prob designation is left blank)

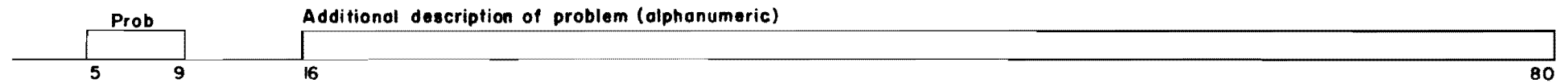
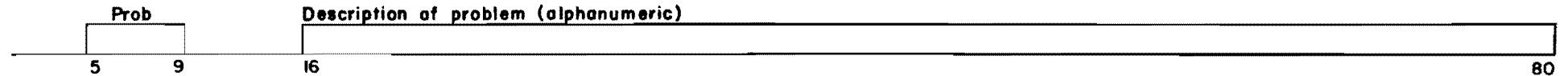


TABLE 1. PROGRAM - CONTROL DATA (1 card each problem)

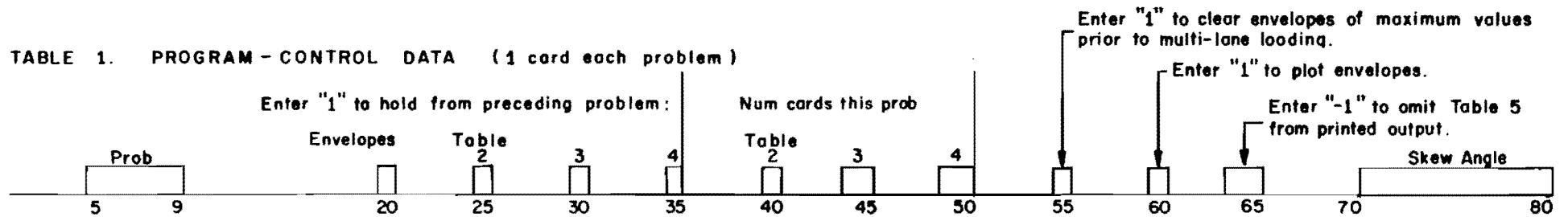
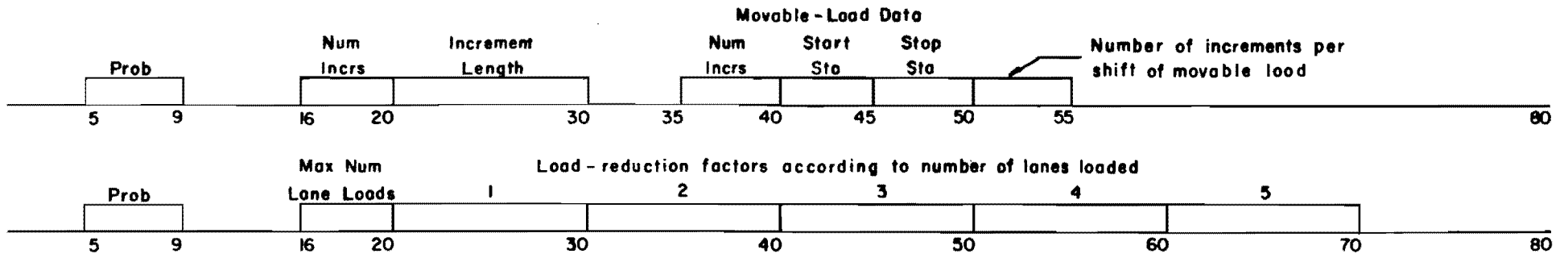


TABLE 2. CONSTANTS (2 cards unless data held from preceding problem)



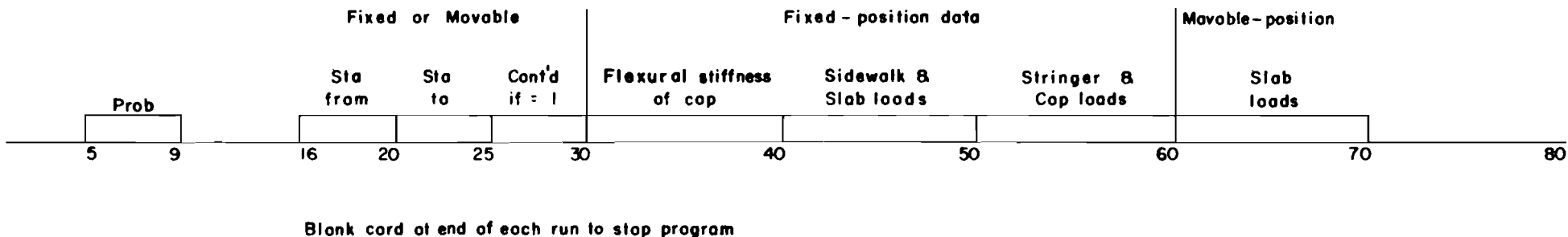
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TABLE 3. LISTS OF STATIONS (number of cards must be entered in Table 1 as either none or 14)

Prob		Lanes	Strs	Sups	Number of Moment-control Points		Number of Shear-control Points							80					
5	9	20	25	30	35	40			1	2	3	4	5	6	7	8	9	10	
Sta of Left of Lane																			
Sta at Right of Lane																			
Sta at Stringers (Fractional tenths of increments permitted)																			
II																		20	
2I																		30	
Sta at Supports																			
II																		20	
Sta at Design-control Points for Moment																			
II																		20	
2I																		30	
Sta at Design-control Points for Shear																			
II																		20	
2I																		30	
5	9	16	20	25	30	35	40	45	50	55	60	65						80	

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TABLE 4. STIFFNESS AND LOAD DATA (number of cards must be entered in Table 1; all data added to storage)



80

GENERAL NOTES

Two cards containing any desired alphabetic or numerical information are required (for identification purposes only) at the beginning of the data for each new problem.

All data cards for any particular problem must contain the same problem designation as letters or numbers in columns 5 through 9.

Any input error found by the program will generally cause the problem and any subsequent problem dependent on the erroneous problem to be abandoned at that time. The program will skip to the next independent problem.

The data cards must be stacked in proper order for the program to run.

Blank spaces are interpreted as zeros.

Units of feet and kips are to be used throughout. Skew angle must be input in degrees.

All data words must be justified completely to the right in the spaces provided.

All data words of 5 spaces or less are to be whole integer numbers: - 1 2 3

The only exception is that stringer stations may be expressed either as integers or decimally to the nearest one-tenth of an increment: 1 2 3 . 4

All data words of 10 spaces are to be entered as floating-point decimal numbers including a multiplier expressed in terms of an exponent of 10: - 1 . 2 3 4 E + 0 2

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TABLE 1. PROGRAM CONTROL DATA

Exercising the option to hold the envelopes of maximum and minimum design values from the preceding problems has no influence on the multiple-lane load-control table which is cleared prior to beginning each new problem.

For each of Tables 2 and 3, the user must decide whether (1) to hold all data in that table from the preceding problem or (2) to read in an entirely new set of data. If the option to hold is exercised, no data cards may be added for that table in the current problem.

In Table 4, the data are accumulated into storage by adding algebraically to any previously stored values, including data which may be held from the preceding problem. Thus any number of new cards may be input regardless of the hold option.

If the clear option is exercised, all the envelopes of maximum and minimum design values will be cleared to zero prior to the multiple-lane loadings. However, dead load effects will be properly restored.

If the plot option is exercised, all envelopes of moment and shear will be plotted.

The option to skip Table 5 has no effect on computations but simply reduces the amount of output when not needed.

A zero skew angle is assumed unless the user specifies otherwise.

All values in Table 1, particularly the card counts, should be carefully checked upon completion of coding of each problem (the card counts should be zero or 2 for Table 2, zero or 14 for Table 3, any number for Table 4).

TABLE 2. CONSTANTS

The maximum number of increments into which the cap may be divided is 300.

The number of increments for the movable load is not restricted except that if any multiple-lane loadings are to be done, the movable load must fit within the width of the narrowest lane designated in Table 3.

The start station is the first station of the slab where the zero station of the movable load is to be placed.

The stop station is the last station on the slab where the zero station of the movable load is to be placed.

A movable-load solution will be performed for each station in the interval if the movable-load shift value is blank, zero or 1 increment. If the value is 2, a solution will be performed every two stations, and similarly if it is 3 or 4.

The maximum number of lanes that may be loaded simultaneously is 5.

The load-reduction factors will usually be those recommended by AASHO design specifications.

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TABLE 3. LISTS OF STATIONS

There will be a total of either zero or fourteen cards. Cards with only a problem number must be inserted where necessary to complete the fourteen cards.

There is a maximum number of 10 lanes (one data card for left of lane and one data card for right of lane), 30 stringers (three data cards), 20 supports (two data cards), 30 design control points for moment (three data cards), and 30 design control points for shear (three data cards).

Design points for shear must be at least 2 increments from any support station, stringer station or from any station receiving load from a stringer specified between stations.

TABLE 4. STIFFNESS AND LOAD DATA

Units for flexural stiffness F are kips \times ft² and for input loads Q are kips per station.

For convenience in input coding, stiffness or load data may be distributed to storage by interpolation. There are four variations in the station numbering and in referencing for continuation to a succeeding data card. These variations are explained and illustrated on the following page by cases a through d.

For input and data storage purposes the interpolation and distribution process for movable-position data is the same as for fixed-position data.

There is no restriction on the order of cards in Table 4 except that within a distribution sequence the stations must be in regular order. There is no limit to the number of cards that may be used in this table.

If stringer stations are specified at fractional increments, the user must proportion the dead load of the stringer as two concentrated loads at the two adjacent stations on the cap.

Any concentrated loads applied as "stringer and cap loads" are not affected by skew. However, any such loads entered as distributed loads are assumed to represent dead weight per station of the cap itself and are therefore increased according to the angle of skew of the bent.

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Fixed - position Data

Individual - card Input

- Case a.1. Data concentrated at one sta.....
- Case a.2. Data uniformly distributed.....

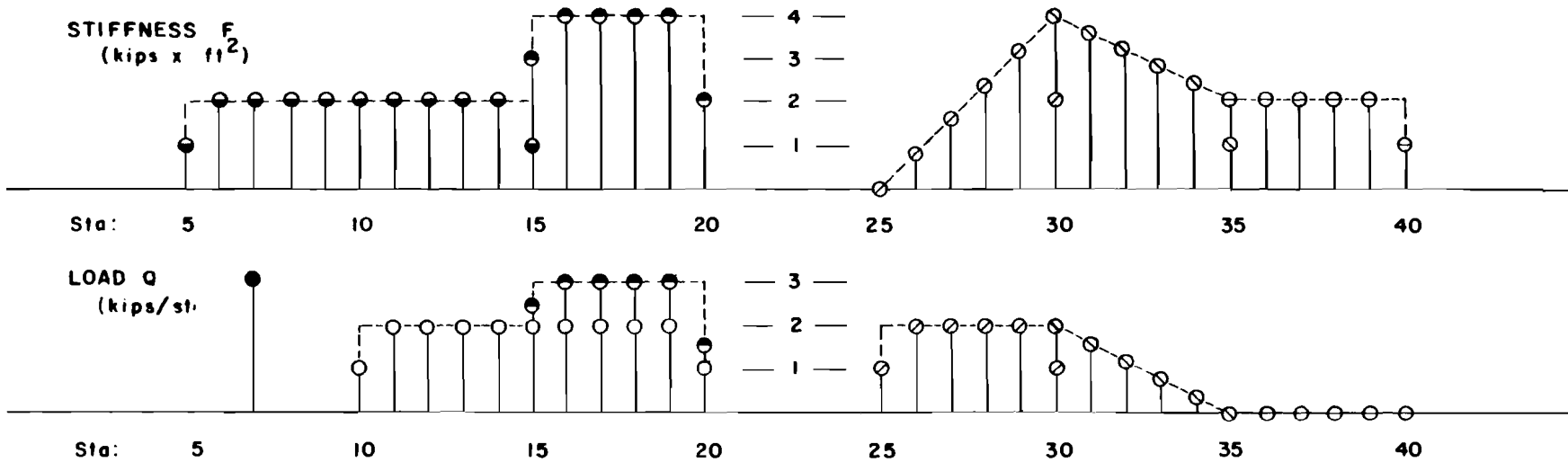
FROM STA	TO STA	CONT'D TO NEXT CARD?	F BENDING STIFFNESS OF CAP	Q TYPICAL LOAD (FIXED OR MOVABLE)
7	7	0 = NO		3.0
5	15	0 = NO	2.0	
15	20	0 = NO	4.0	1.0
10	20	0 = NO		2.0

Multiple - card Sequence

- Case b. First - of - sequence
- Case c. Interior - of - sequence
- Case d. End - of - sequence

25		1 = YES	0.0	2.0
	30	1 = YES	4.0	2.0
	35	1 = YES	2.0	0.0
	40	0 = NO	2.0	

Resulting Distributions of Data



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CHAPTER 4. EXAMPLE PROBLEM

One basic problem has been selected to illustrate the principal features of CAP 14. Three variations of the problem will be solved to show typical uses of the program, including input data techniques and control options.

The problem is shown in Fig 5a. The structure consists of a slab, eight stringers, a cap, and four supports. An increment length of 0.5 ft was selected and the corresponding station numbers are shown directly below the structure. Fig 5b shows the structure simplified for representation in the computer. In accordance with conventional design practice, knife-edge supports are assumed and hinges are put in the slab at each interior stringer. The stations of moment and shear control points are indicated by asterisks.

The ability to vary flexural stiffness is illustrated in these problems only by the tapered ends of the cap. However, haunches or any other variations could have been described anywhere along the cap.

Figures 6a, b, and c give the various loads for the problem. The movable load is 20 increments wide and its zero station has a range of movement from Station 4 to Station 112. The movable load will be placed at each station within the range of movement. All loads are downward and therefore have a negative sign.

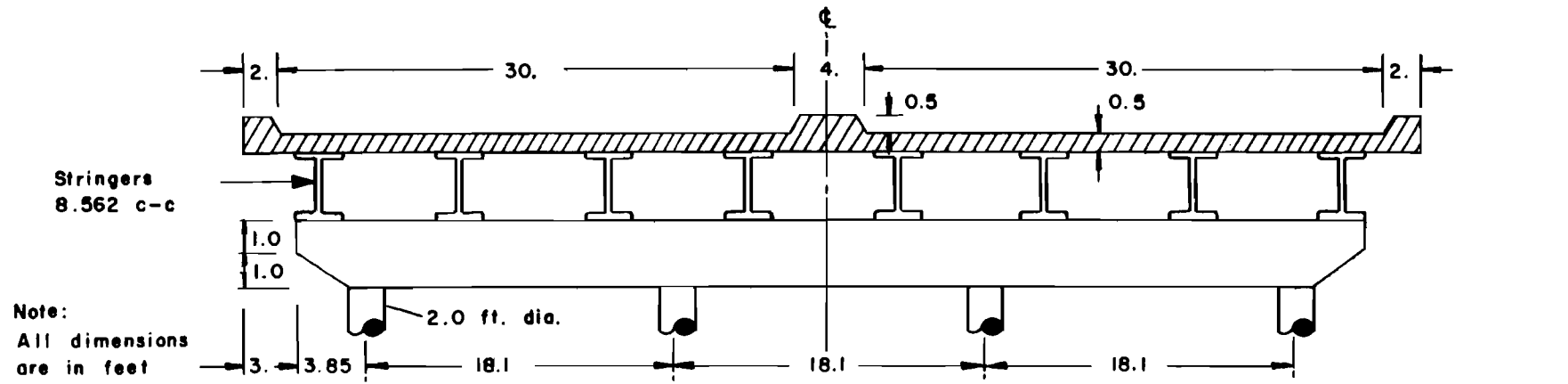
The actual input data listings are reproduced in App 4 and the computed results are in App 5.

Solution 1.

In this solution all stringers have been shifted to the nearest whole station. The station numbers are shown at each stringer position in Fig 5b.

A tabulation of the input data for this solution is given in App 4. No tables of data are retained from a preceding problem, the option to exclude the out-of-lane loadings is not exercised, and it is desired to have plots of the results.

The printed output from this solution is given as App 5.1. The items of primary interest are the multiple-lane loading summary in Table 5, the envelopes of extreme values of moment and shear in Table 6 and the support reactions given in Table 7. The following tabulation is an extract of one typical set of output for one design-control variable at one design-control point from the multiple-lane loading summary.



(a) The structure

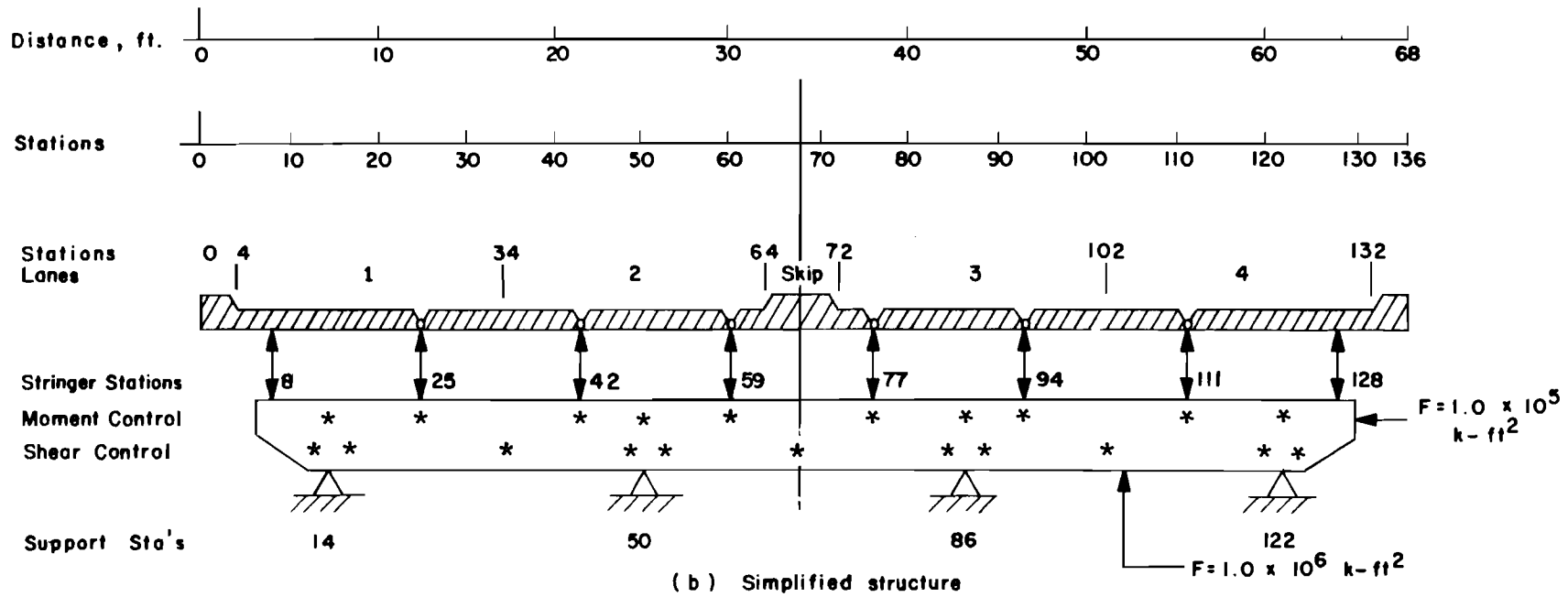


Fig 5. The real and the simplified structure of the example problems.

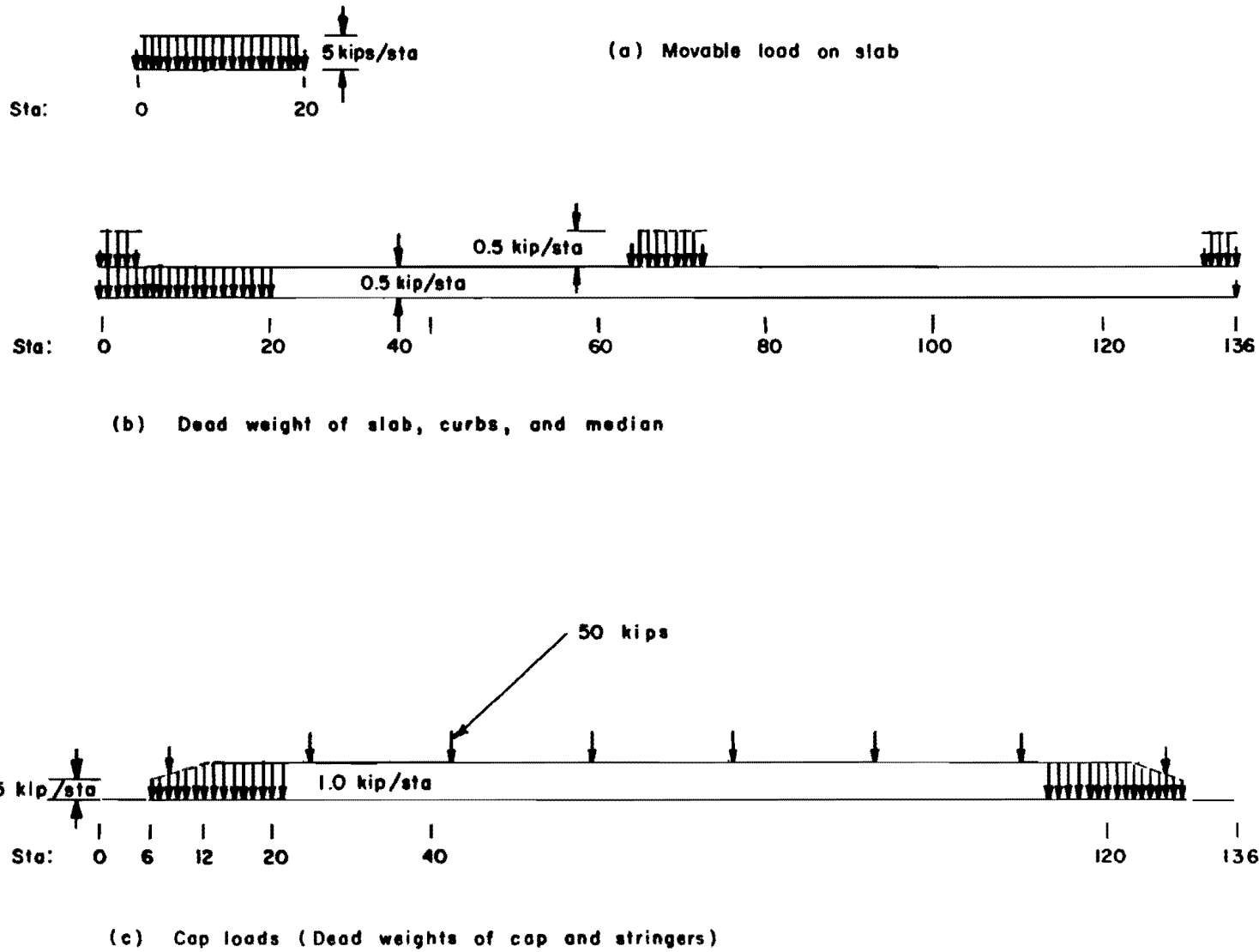


Fig 6. Loads of Example Solution 1.

Extract from Multi-Lane Loading Summary

TABLE 5 -- MULTI-LANE LOADING SUMMARY (*--CRITICAL NUMBER OF LANE LOADS)

MOMENT (FT-K)		LANE ORDER	POSITIVE MAXIMUM	LOAD AT		LANE ORDER	NEGATIVE MAXIMUM	LOAD AT	
AT STA	DEAD LC EFFECT			LANE STA	LANE STA			LANE STA	LANE STA
50	-2.697E C2								
		0	3.443E 01	0	93	0	-1.427E 02	0	31
		1	2.416E 01	4	102	1	-1.422E 02	2	34
		2	1.319E 01	3	82	2	-9.676E 01	1	14
		3	4.790E 00	1	4	3	-3.871E 01	3	72
		4	0			4	-1.196E 00	4	112
		3*				3*			

The design-control variable for the above extract is the bending moment and the design-control point is Station 50. The maximum moment at Station 50 due to all dead load effects is -2.697×10^2 ft-kips as shown in the second column. The next four columns give positive-moment effects and the last four columns give corresponding negative-moment values.

Consider the two columns marked "lane order". Lane order "0" represents the maximum effect of the single movable load on the design variable without regard to lane boundaries. Lane order "0" may be out-of-lane or in-lane. Lane order "1" represents the single in-lane movable-load position which contributes the largest effect on the particular design variable. The corresponding lane and station number are shown. (It is possible for this position and result to be the same as for lane order "0".) Lane order "2" represents the second largest in-lane contribution to the design variable and so on for the other lane orders.

The number of lane loads that will be critical is signified by the number with an asterisk at the bottom of the lane-order column. This represents the number of lane loads which will be applied simultaneously, with application of the proper load-reduction factors, to create the maximum or minimum design value at the particular design point. In the case considered, 3 lane loads will be applied simultaneously to produce maximum positive moment greater than the single-movable-load maximum at Station 50 and a different set of 3 lanes will be

loaded to produce maximum negative moment. Nowhere in Lane 2 could the movable load be placed to produce positive moment and a Lane 2 loading effect therefore does not appear in the positive moment portion of the table. Notice that for negative moment, a fourth loaded lane contributed some negative moment but considering load reduction factors, only three lanes were loaded to develop the most critical value.

Table 6 in App 5.1 gives the envelopes of moment and shear for Solution 1. Some very small extraneous numbers are listed in the output but should be ignored as they are insignificant remnants from arithmetic operations in the computer. Table 7 is the summary of maximum support reactions. In the problem all maximum negative reactions are zero since negative contributions from the live loadings were never large enough to overcome the dead-load positive reactions. This can be seen by comparing Tables 5 and 7 for reactions.

Table 8 gives the engineering scales used with the plotted envelopes of moment and shear shown in Figs 7 and 8. The design-control points are automatically indicated by the row of points plotted above and below the curves. The station numbers and scales have been added by hand.

Solution 2.

The second variation to be illustrated is that of using fractional values for stringer stations. In this solution, the stringer positions have been rounded to the nearest one-tenth of an increment. The fractional stringer station numbers are shown in Fig 9. The dead weights on the cap for this solution are also in Fig 9; other loads are the same as for Solution 1.

A tabulation of the input data is given in App 4. The input stringer stations are listed to the nearest one-tenth of a station in Table 3. In Table 4 the stringer loads have been proportionately shared to each adjacent whole station. This requires two input cards for each stringer load instead of one.

The output from this solution is given in App 5.2 and the corresponding plots of moment and shear envelopes are in Figs 10 and 11. To simplify study of effects of the more precise stringer placement in Solution 2, the key results from both Solution 1 and Solution 2 are summarized in Table A.

For the particular problem considered, the largest effects on bending moment occur at Stations 42 and 94 where the change is approximately 6 percent. Effects on shears and reactions are quite small.

Table A. Comparison of maximum values at design-control points.

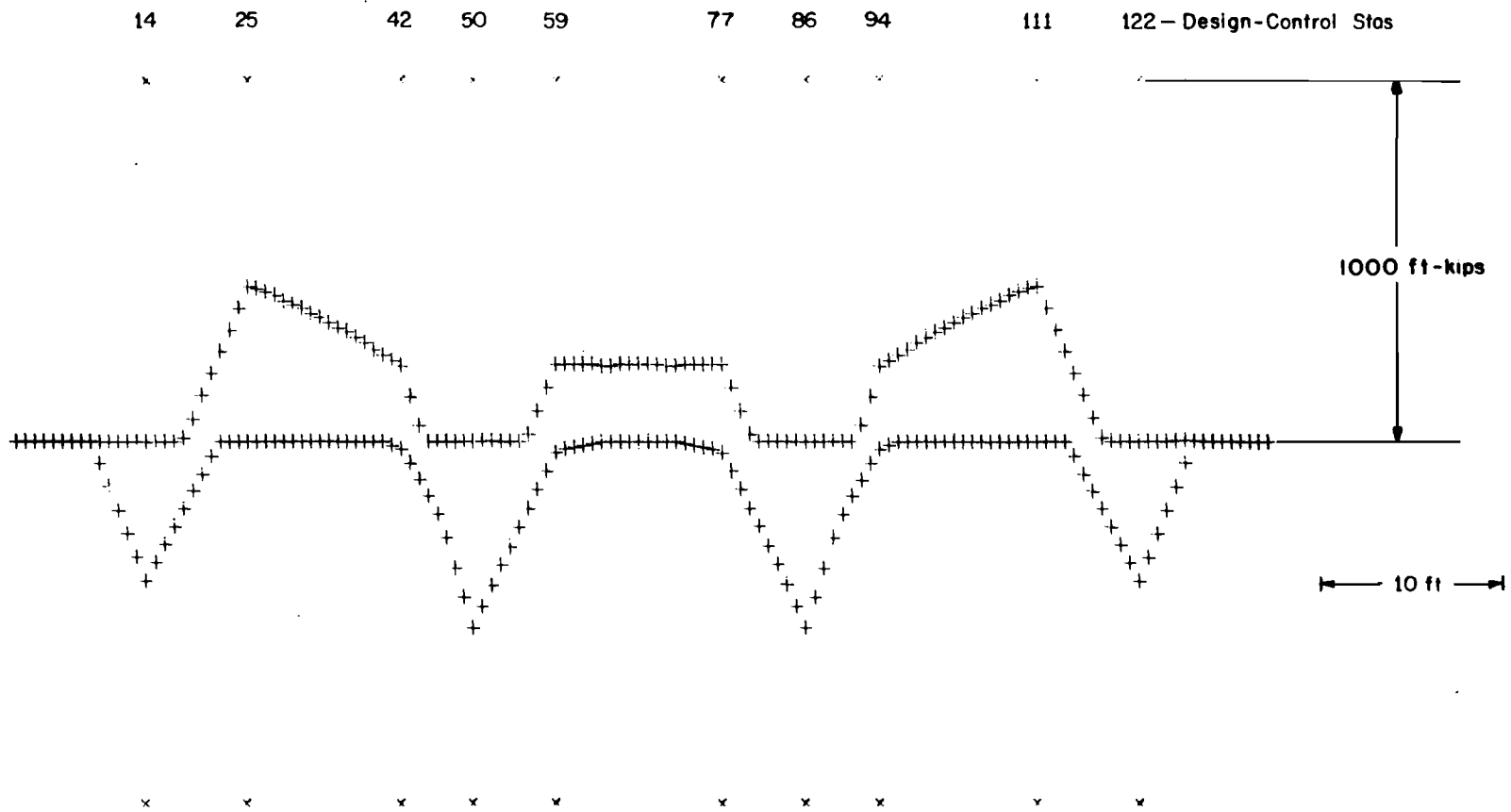
	Design-Control Stations	Solution 1	Solution 2
Bending moment (ft-kips)	14, 122	-391.3	-387.6
	25, 111	431.8	424.9
	42, 94	208.2	195.8
	50, 86	-519.7	-521.4
	59, 77	217.1	206.8
Shear (kips)	12, 124	131.1	132.0
	16, 120	127.6	125.9
	34, 102	33.8	35.5
	48, 88	180.8	183.1
	52, 84	143.7	141.9
	68 (center)	17.8	18.1
Support Reaction (kips)	14, 122	249.2	248.2
	50, 86	295.2	296.1

Solution 3.

The third variation is intended to demonstrate the various data-hold options plus the effect of an input skew angle of 30 degrees, which is the only real change from Solution 2.

A tabulation of the input data is given at the end of the listing in App 4. Notice that only three input cards are required, the two problem identification cards and the control card (Table 1) on which the hold options and the skew angle are entered.

The output from this solution is in App 5.3 and the plotted envelopes are in Figs 12 and 13. All moments, shears and reactions have changed significantly with the skew angle due to the increased dead weight of the cap. The values of moment have changed also because of the increased dimensions along the cap. However, in Table 5, the movable-load reactions from both the out-of-lane and in-lane loadings have not changed since the magnitude of the transferred load and its relative position on the cap remain the same. Notice that precisely the same lanes are loaded at precisely the same station numbers in both Solutions 2 and 3.



PROB 1

Fig 7. Envelopes of bending moment diagrams for Solution 1.

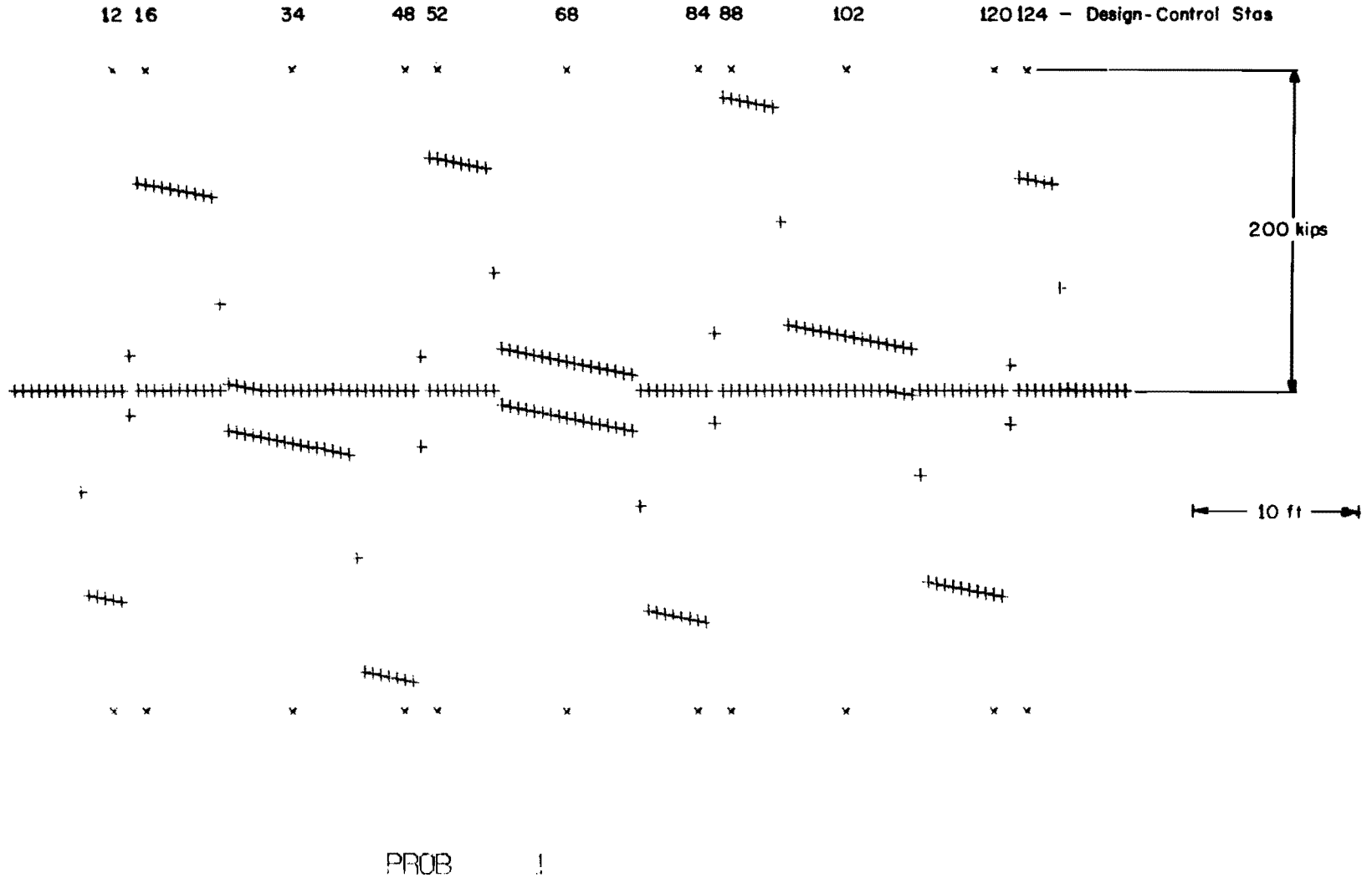


Fig 8. Envelopes of maximum shear values from Solution 1.

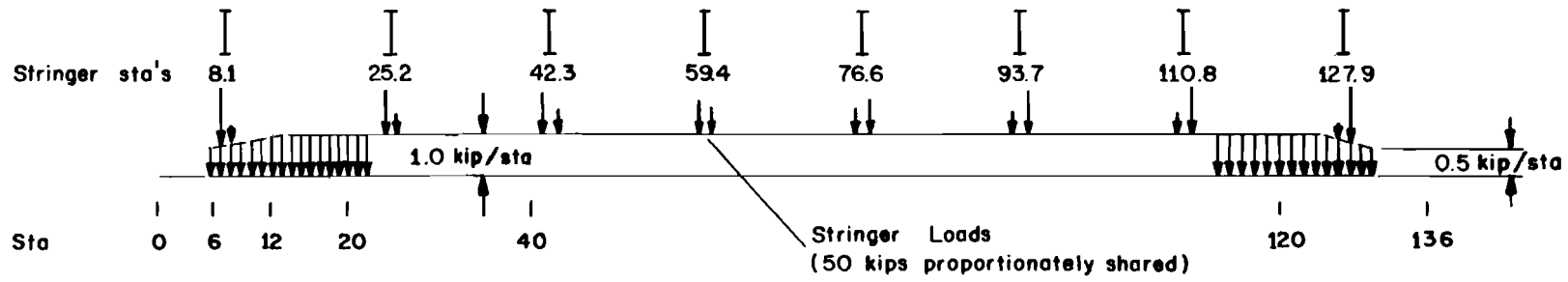


Fig 9. Dead loads of Example Solution 2.

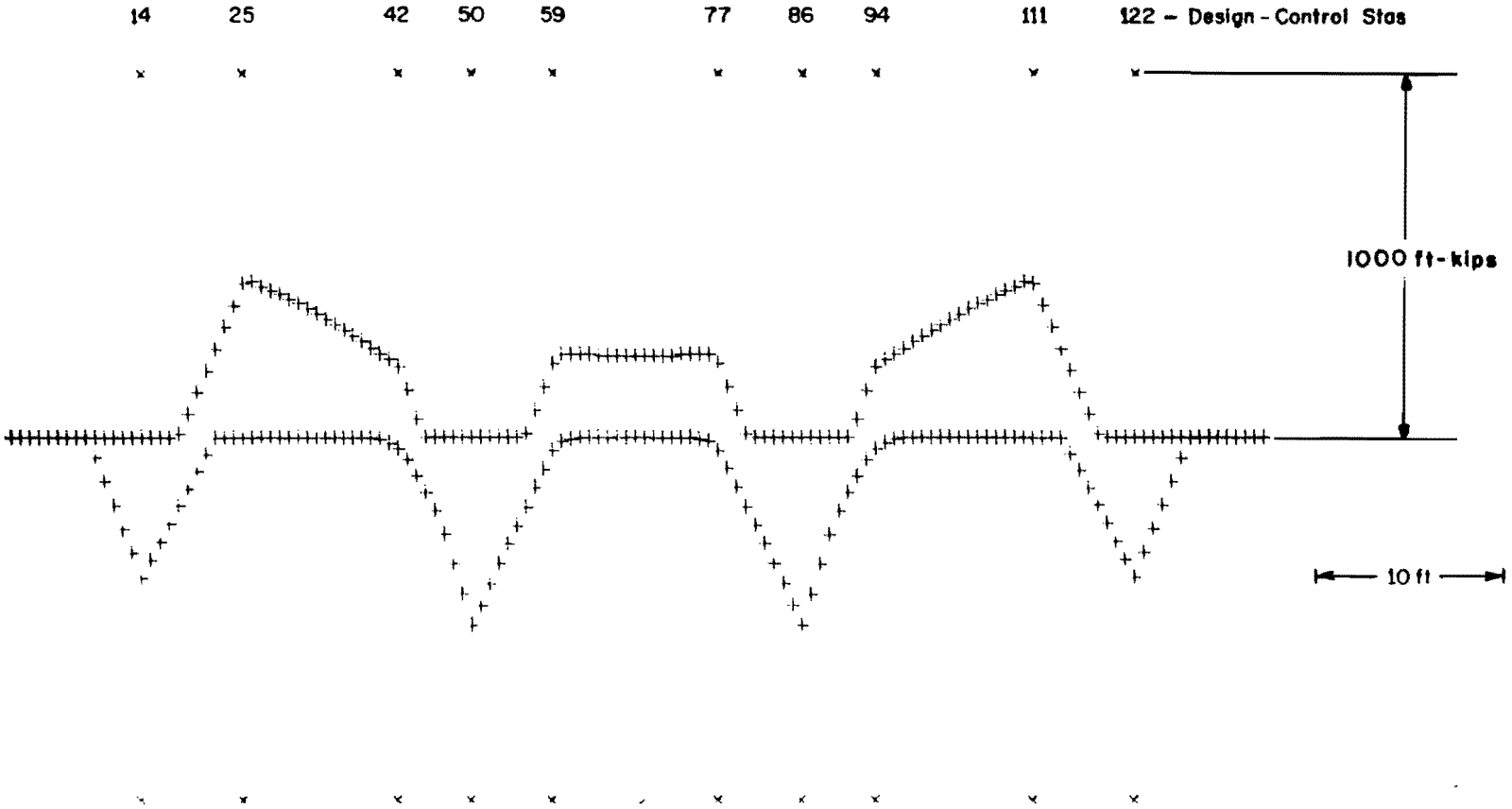
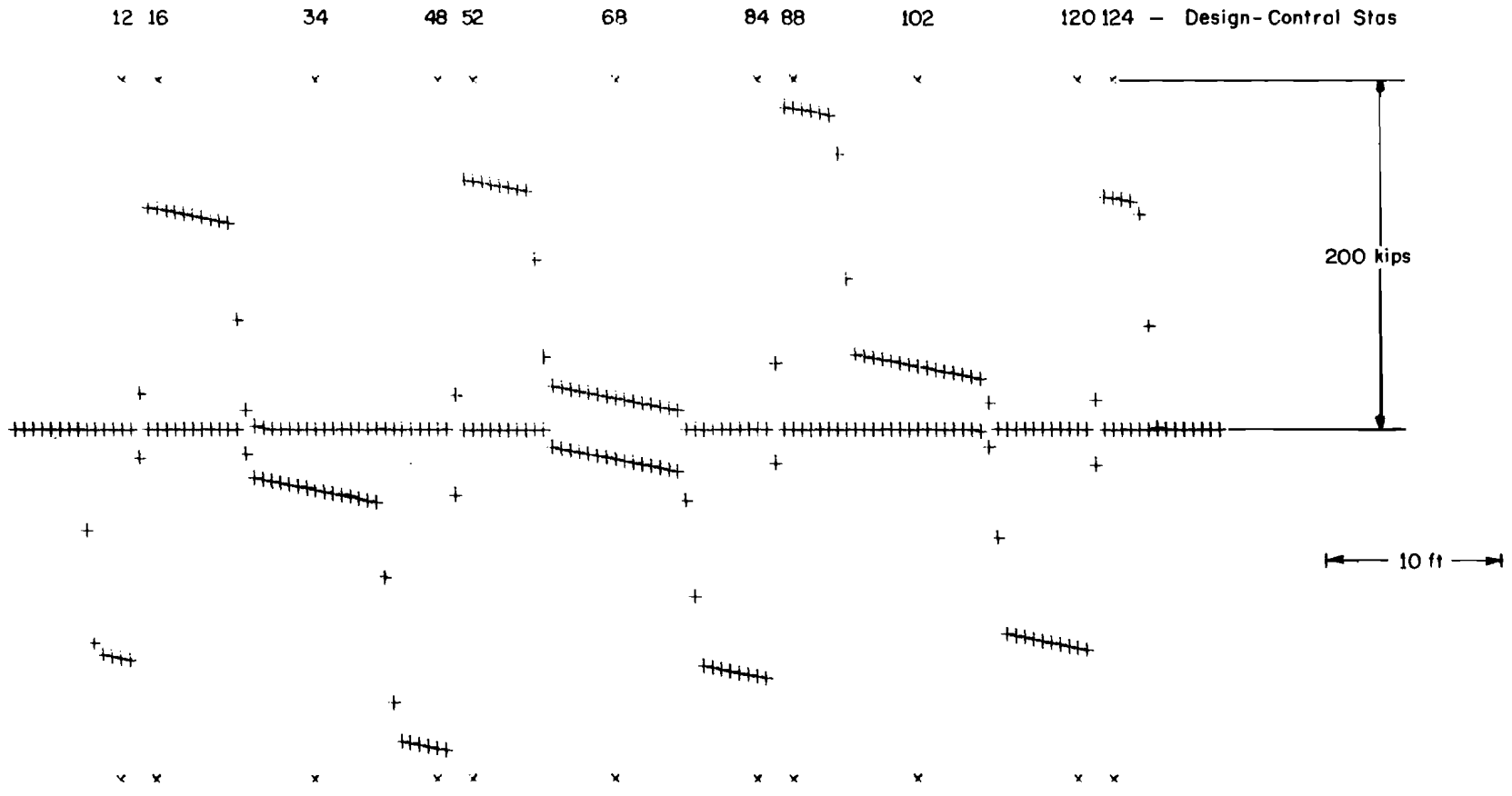


Fig 10. Envelopes of bending moment diagrams for Solution 2.



PROB 2

Fig 11. Envelopes of maximum shear values from Solution 2.

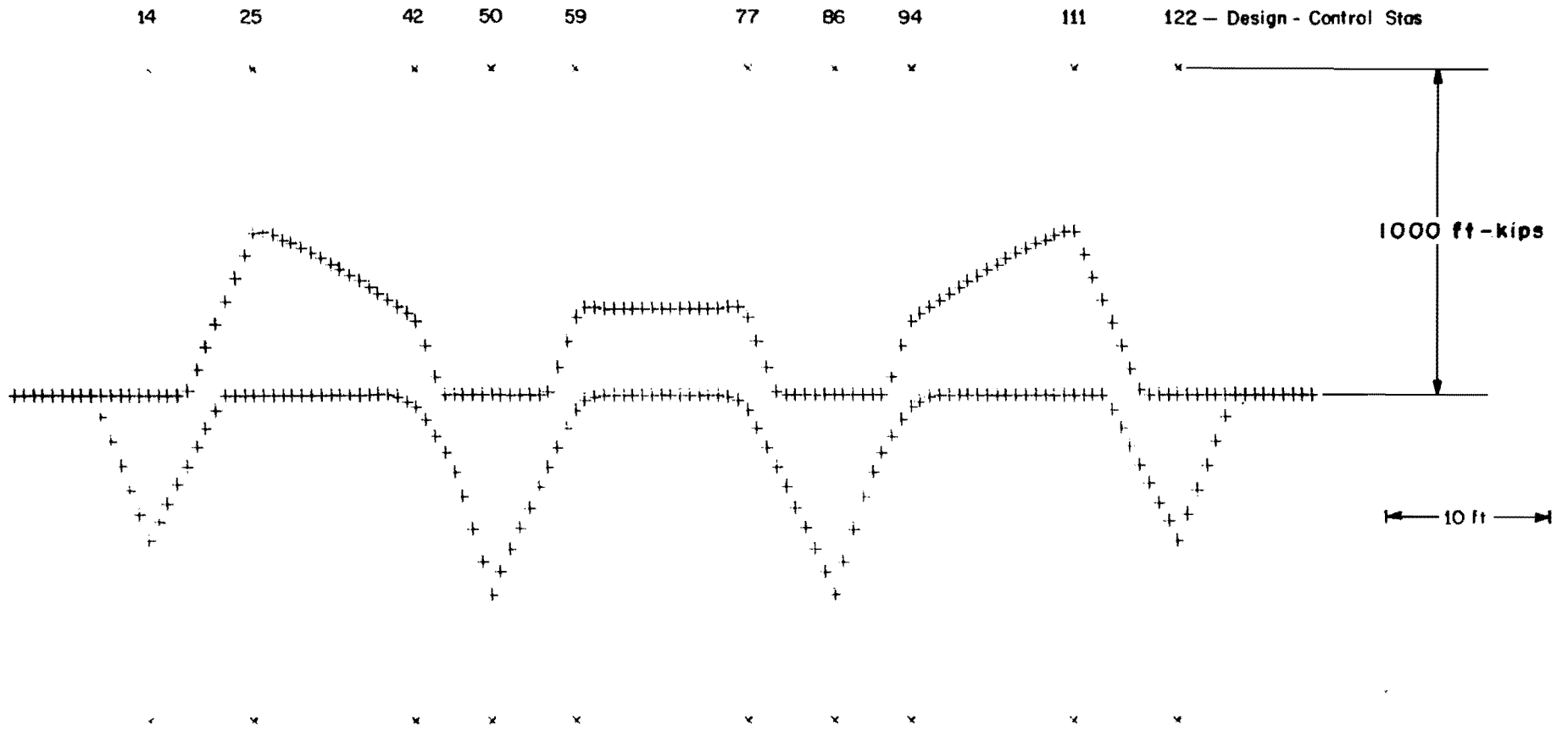
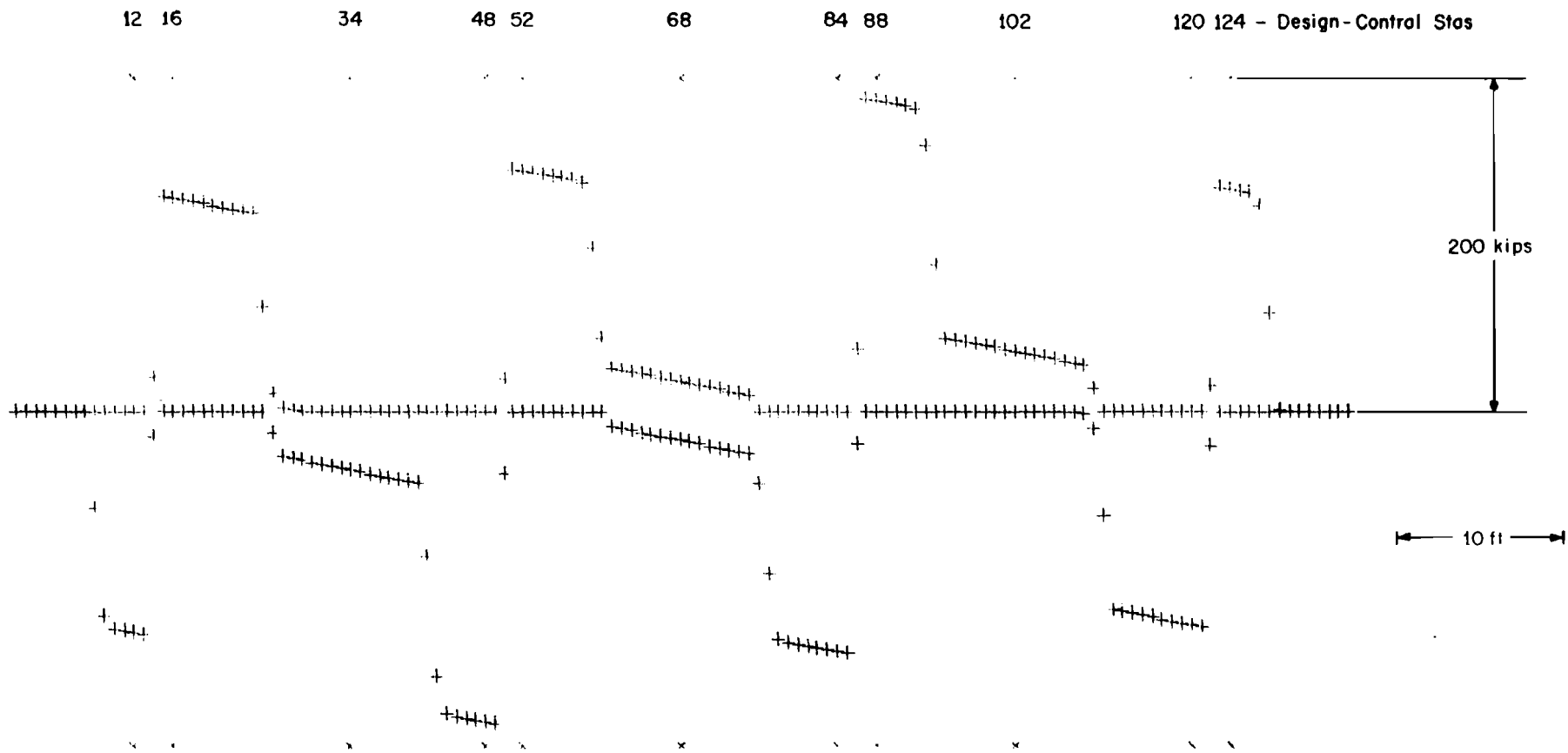


Fig 12. Envelopes of bending moment diagrams for Solution 3.



PROB 3

Fig 13. Envelopes of maximum shear values from Solution 3.

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CHAPTER 5. SUMMARY AND RECOMMENDATIONS

Principal Assumptions and Approximations

In any general-purpose computer method every effort should be made to avoid burying items of engineering judgment and decision within the program. However, it is often necessary as a temporary expedient to include such items, as is the case with some aspects of the program described in this report. These items should be recognized, and eliminated wherever possible, as more realistic analyses of the total structure are developed.

The assumptions and approximations in the basic BMCOL method (Ref 1) should be clearly recognized by the users of the CAP program. There are various additional assumptions and approximations in the CAP analysis which are dictated by design practice and also must be clearly understood.

In the BMCOL method the beam is assumed to be replaced by a series of rigid bars, pin-connected and spring-restrained. These bars form the finite-element beam, a portion of which was shown in Fig 1b. All loads and stiffnesses are concentrated at regularly spaced station points.

The CAP analysis includes additional simplifications which are justified primarily by current design practice. A hinge is assumed in the slab at each interior stringer so that simple-span distribution of slab loads may be used. All columns are replaced by knife-edge supports acting at the nearest available station points.

Recommendations

The following recommendations are considered applicable to the use of program CAP 14.

- (1) Each user should satisfy himself as to the most desirable practice in regard to (a) size of increment length, (b) the stringer load splitting option, and (c) the use of the variable incrementation in positioning the single movable load in the Phase 2 solutions. It has been found that rarely is an increment length of less than 0.5 ft necessary and if used in conjunction with the stringer load splitting technique 1.0 ft should be adequate for most cases.
- (2) The user should run test solutions of cases for which independent hand solutions are also made, especially when

entering a new or uncertain area of analysis. Such trials also help to guard against improper use of input forms.

- (3) CAP 14 should be used to study ways of improving or simplifying the more or less arbitrary loading rules that are currently used. Specifically, it is felt that through use of CAP 14, a fixed pattern of movable loads may be evolved that would give equally satisfactory results when compared to those based on the intricacies of the AASHO rules. A fixed pattern of movable loads would greatly reduce the amount of logic and computation and in turn might considerably reduce the overall costs of analysis. This will be increasingly important as larger and larger aggregations of structural elements are considered in more sophisticated computer programs of the future.

If properly and efficiently used, modern high-speed computers offer a tremendous advantage in the performance of engineering work. By freeing the engineer from many tedious hand computations, the computer greatly enhances the opportunities for creative improvements in design. Furthermore, many problems can be attacked that could not be considered by conventional methods. In this concept, the computer is used as an aid but never as a substitute for the application of sound engineering judgment and decision.

REFERENCES

1. Matlock, Hudson, and Haliburton, T. A., "A Finite-Element Method of Solution for Linearly Elastic Beam-Columns," a report to the Texas Highway Department, February 1, 1965.
2. American Association of State Highway Officials Standard Specifications for Highway Bridges, Eighth Edition, General Offices, 917 National Press Building Washington, D. C., 1961.

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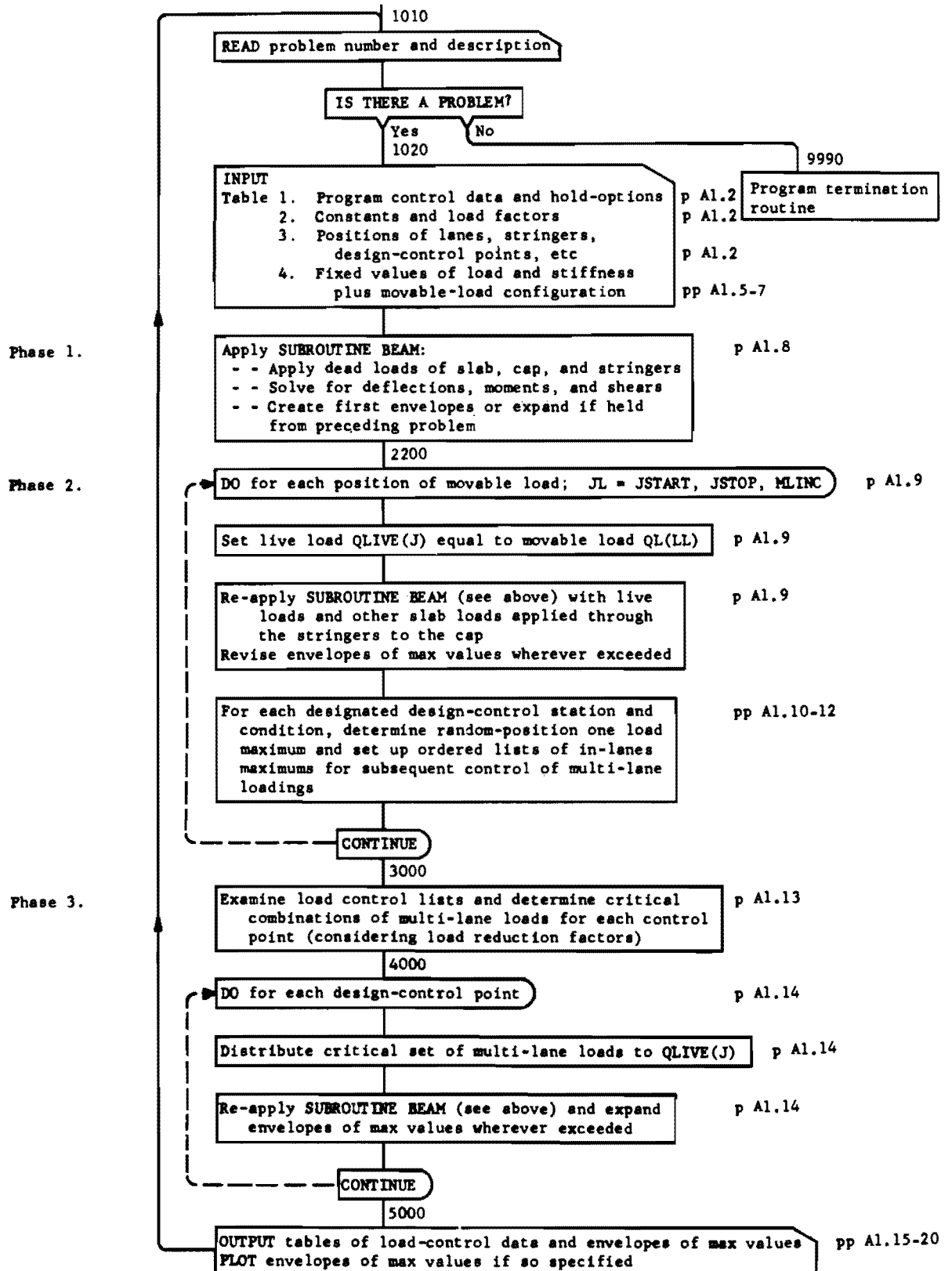
APPENDIX 1

FLOW DIAGRAMS FOR PROGRAM CAP 14

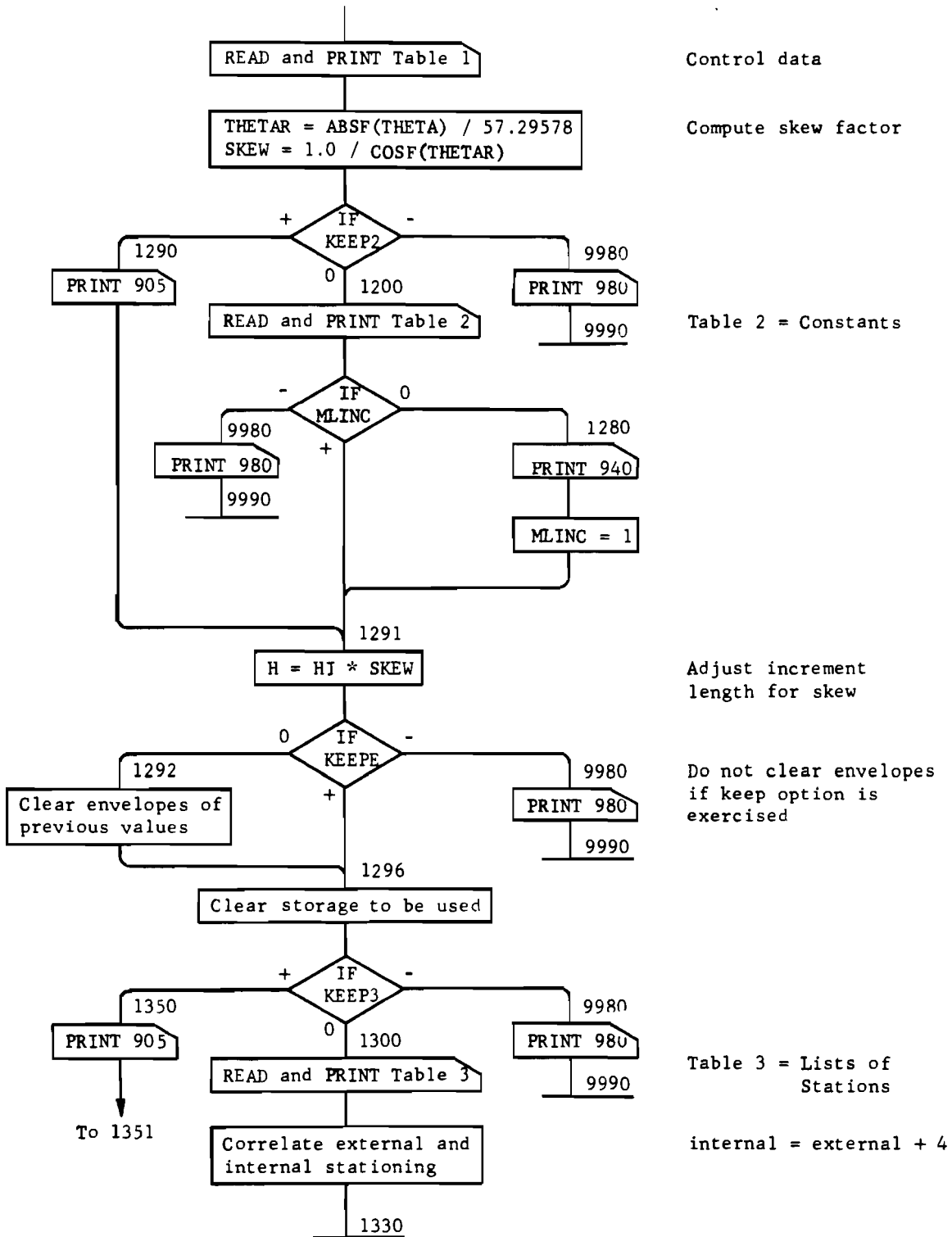
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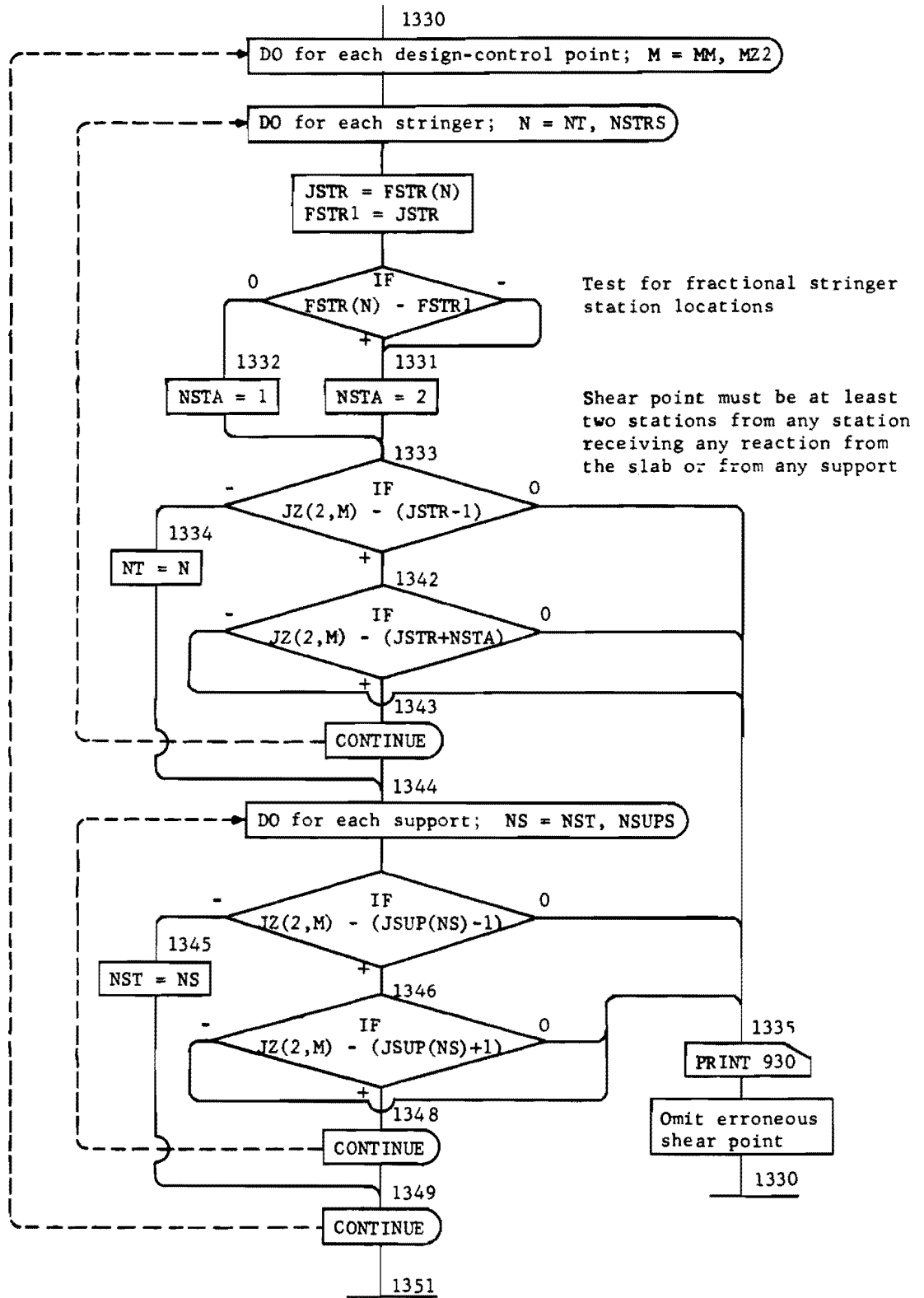
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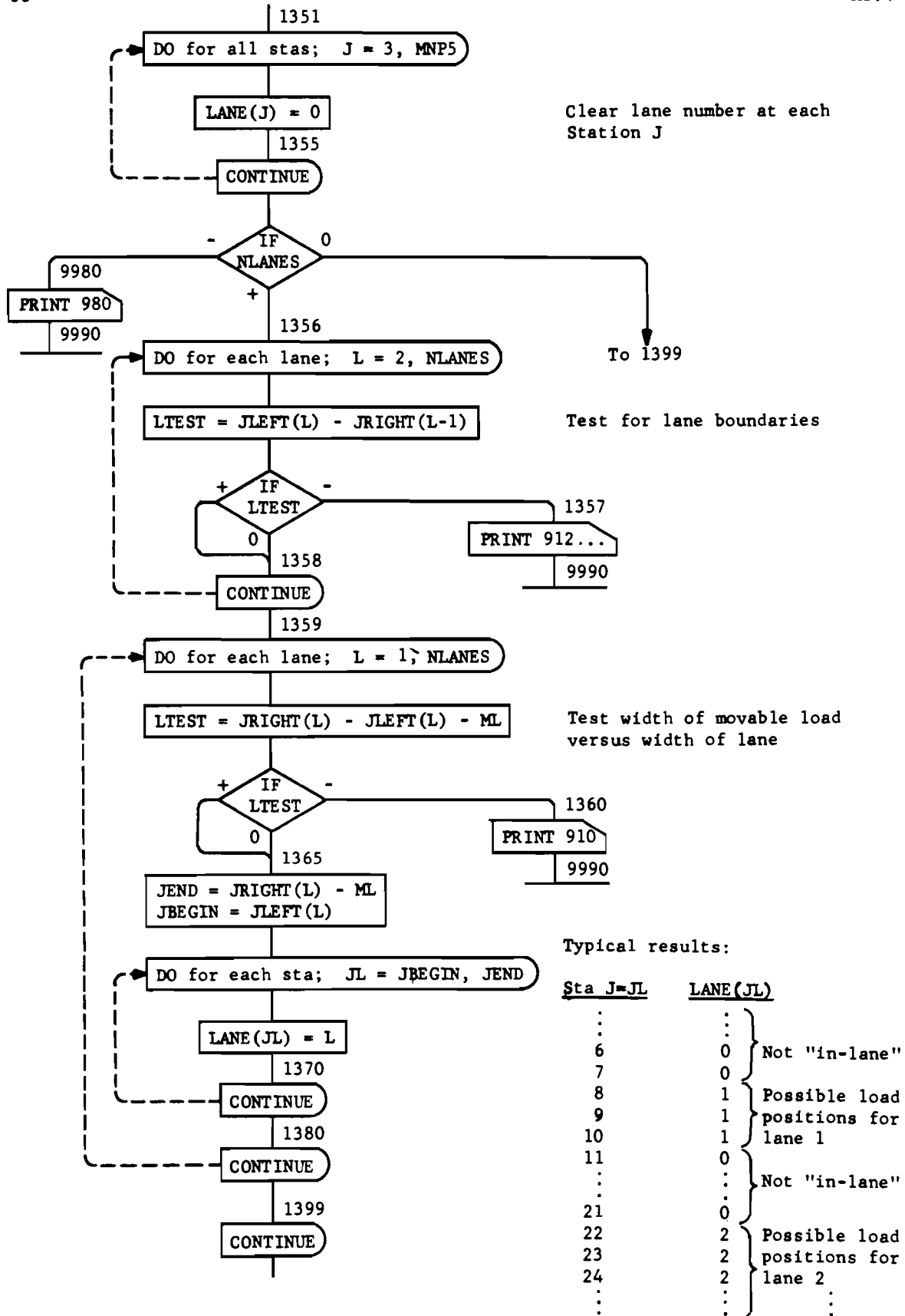
SUMMARY FLOW DIAGRAM FOR CAP 14

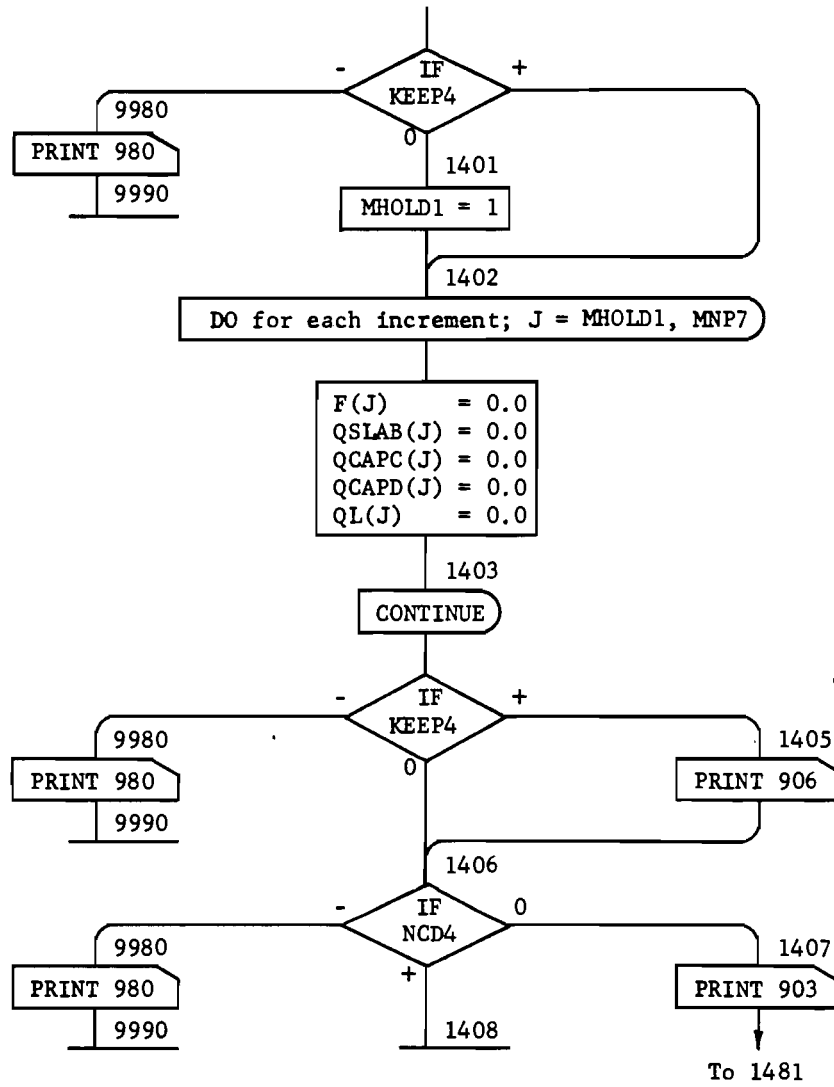


COMPLETE FLOW DIAGRAM FOR PROGRAM CAP 14

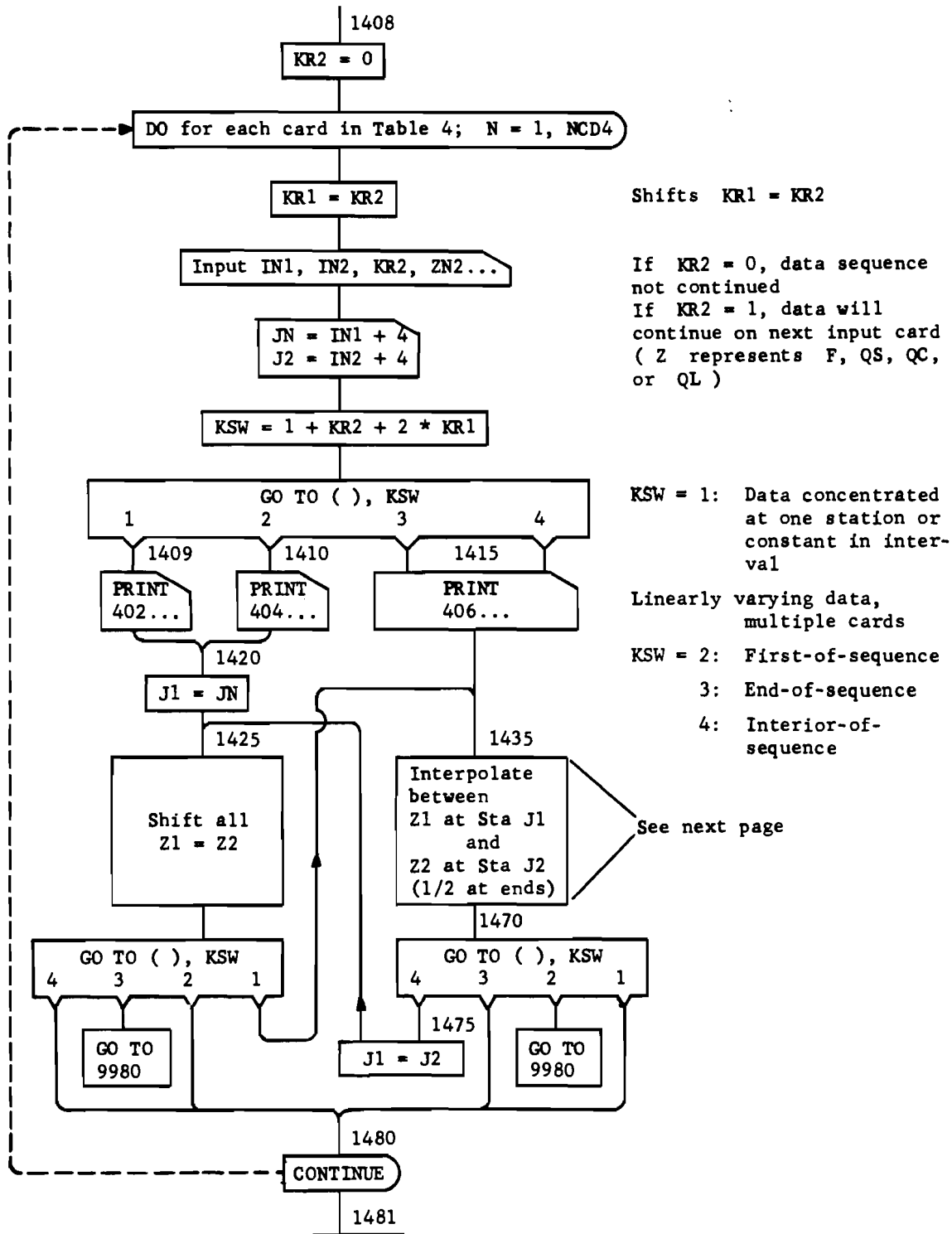






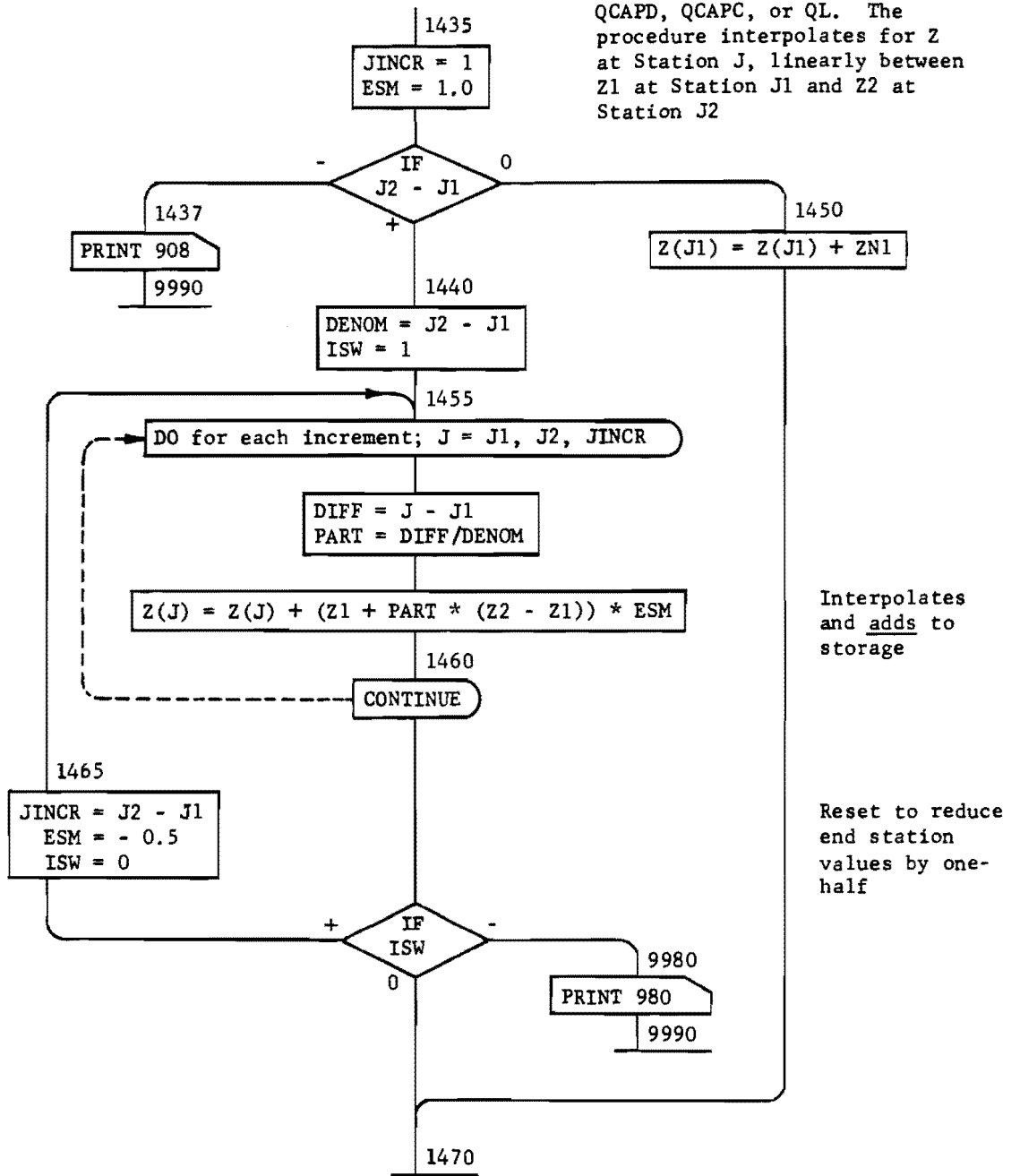


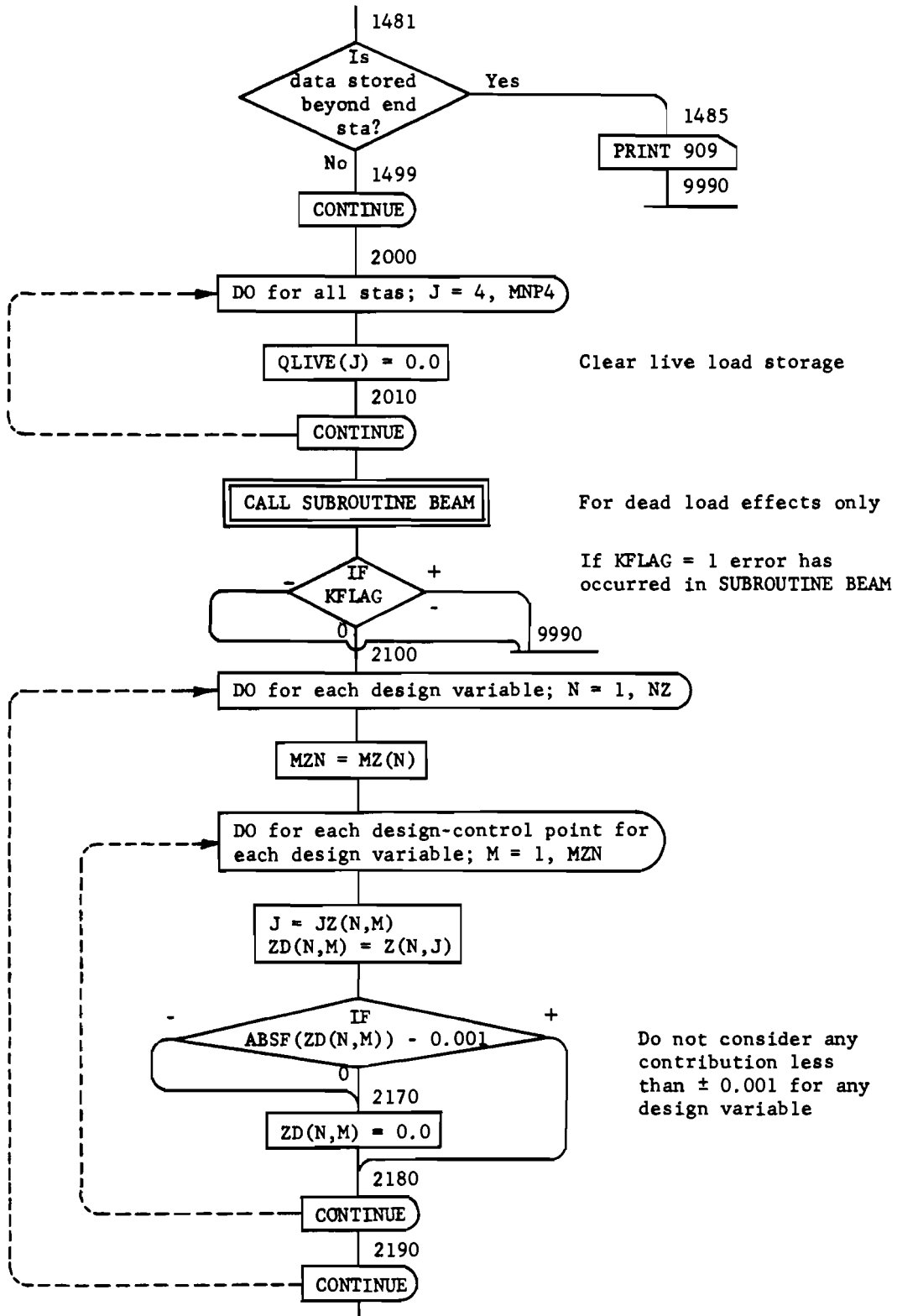
Use data from previous problem or clear previous data from storage

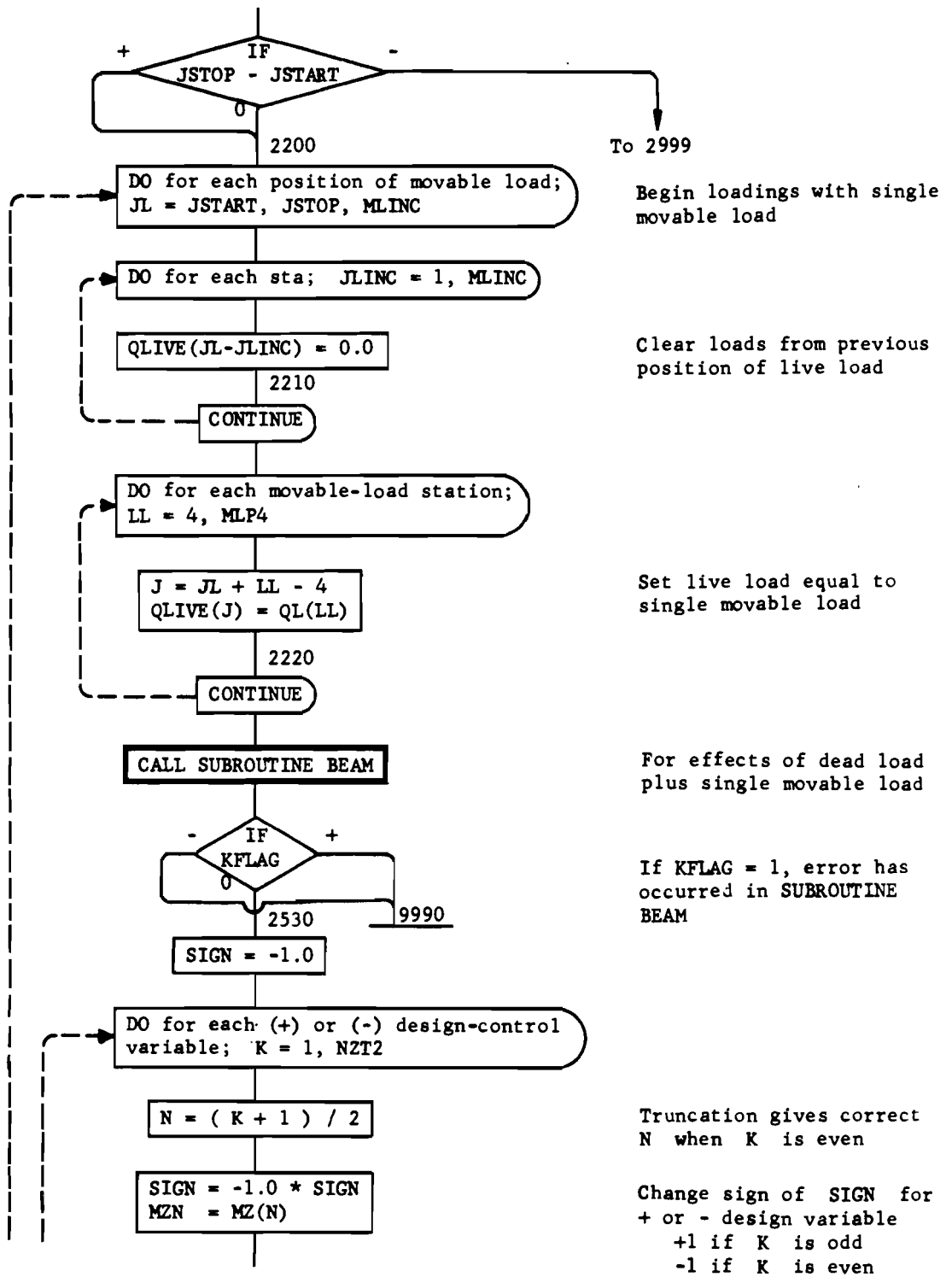


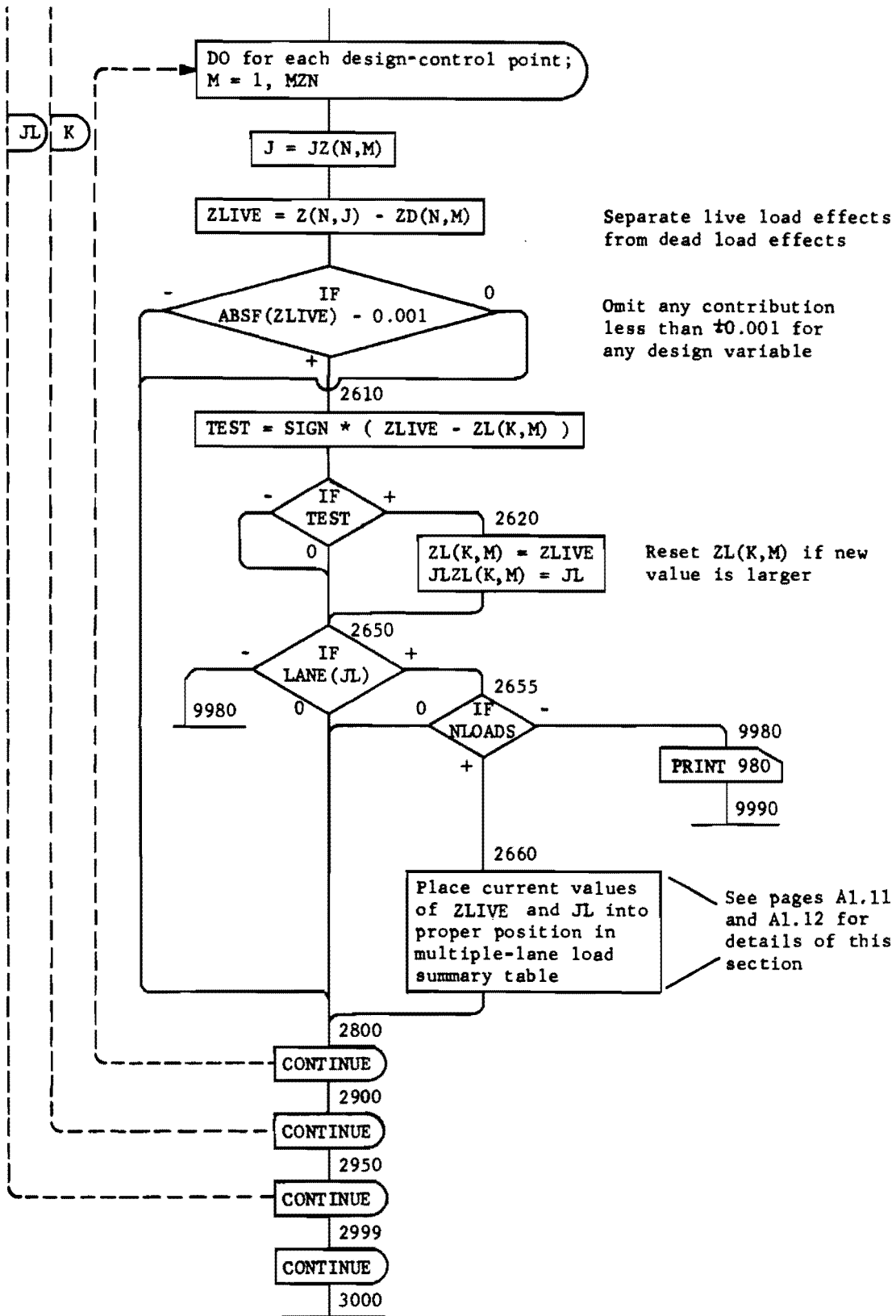
Expansion of statements
1435 through 1470 on page A1.6

Let Z represent F, QSLAB, QCAPD, QCAPC, or QL. The procedure interpolates for Z at Station J, linearly between Z1 at Station J1 and Z2 at Station J2

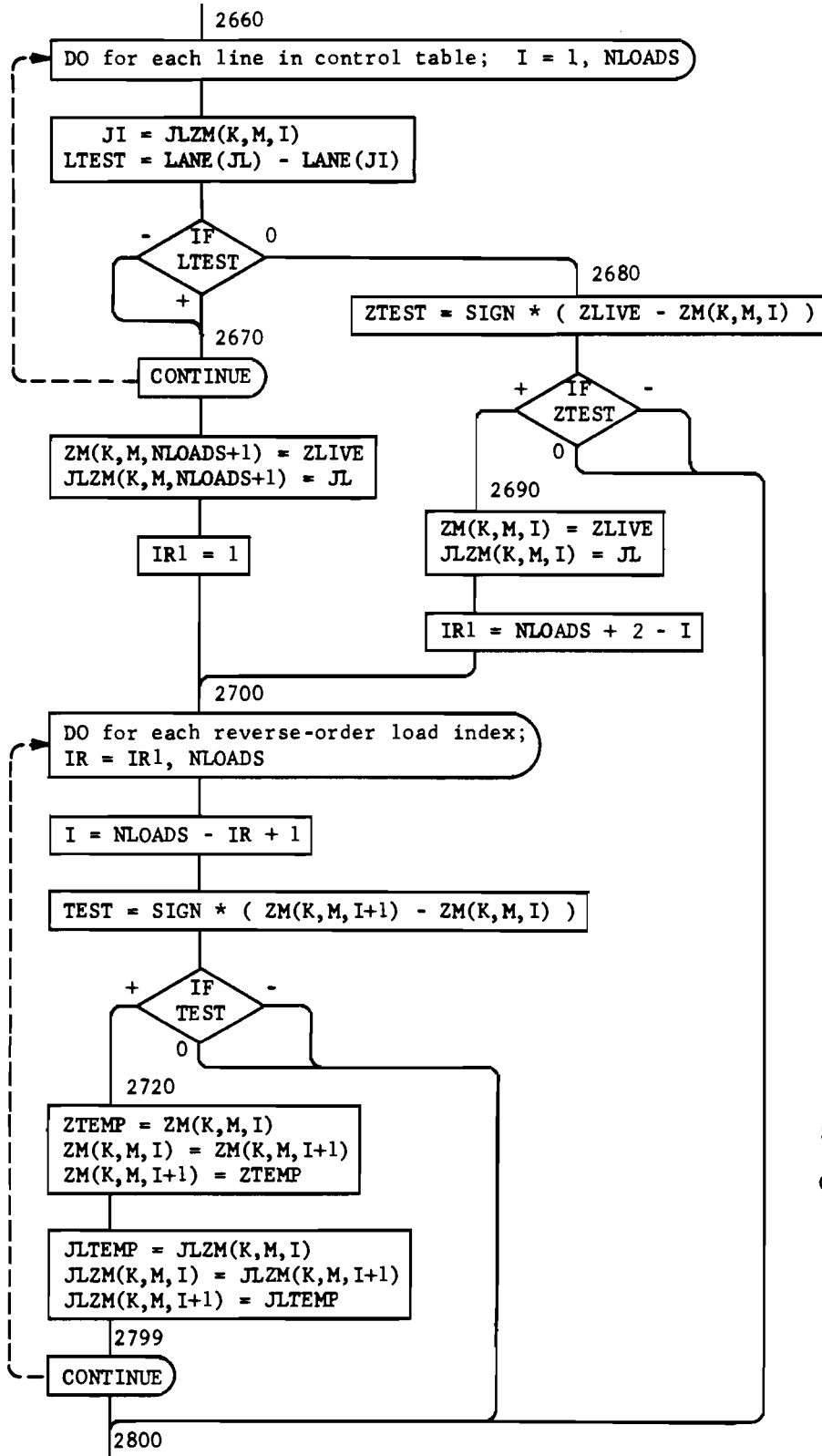








Expansion of statements 2660 through 2800 on page A1.10

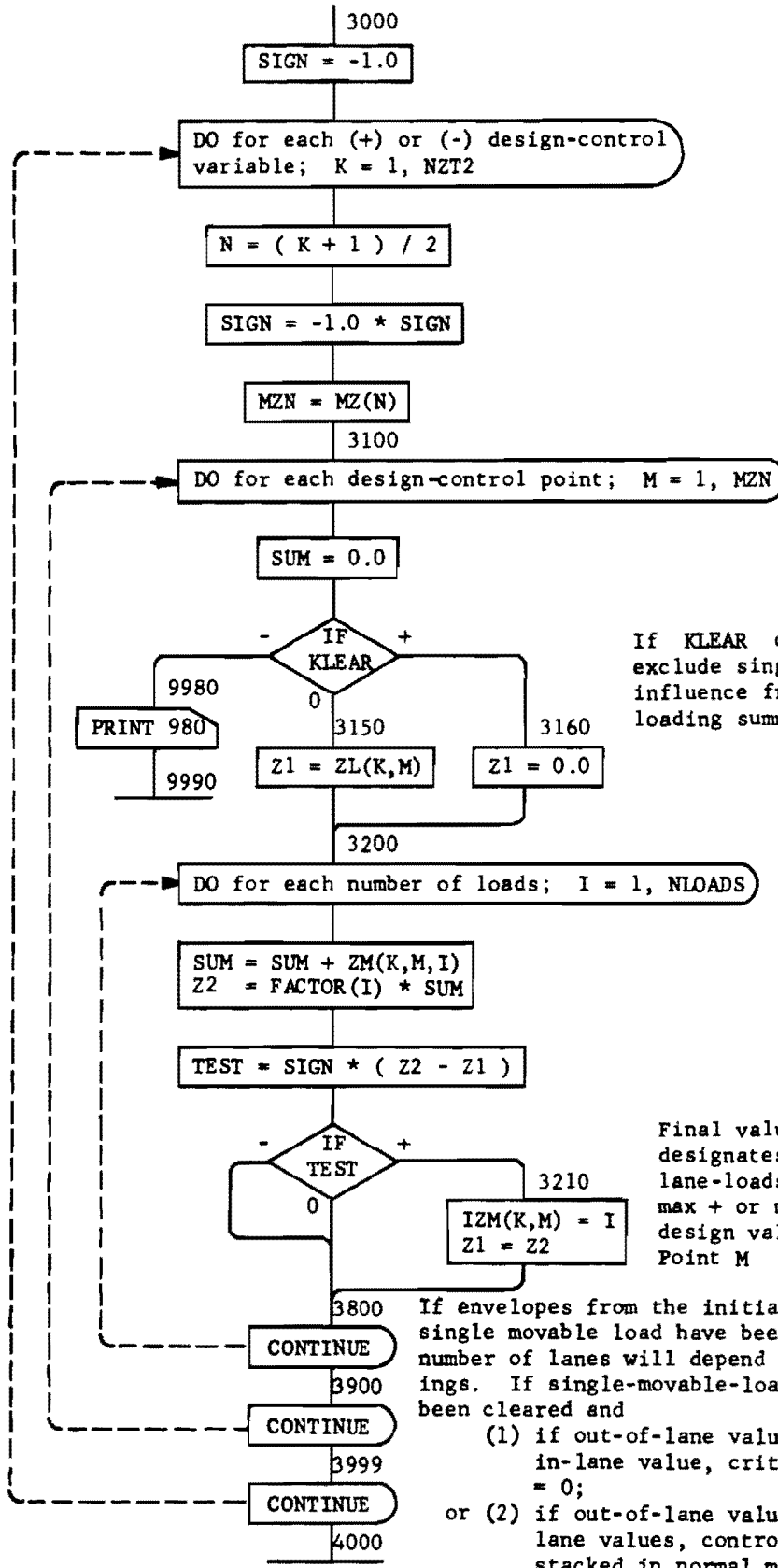


See page A1.12
for the Load
Control Table

Multiple-Lane Load Summary Table

LOAD TYPE	LANE ORDER I	± DESIGN VARIABLE AT POINT M	LOAD PLACED AT	
			Lane Number	Station Number
Single Movable Load (A)	I = 0	ZL(K,M)	LANE(JL)	JL = JLZL(K,M)
In-Lane (B)	I = 1	ZM(L,M,1)	LANE(JL)	JL = JLZM(K,M,1)
	2	.	.	.
	⋮	⋮	⋮	⋮
	I	ZM(K,M,I)	LANE(JL)	JL = JLZM(K,M,I)
	⋮	⋮	⋮	⋮
	NLOADS	.	.	.
Position to Enter New Values (C)	NLOADS + 1	ZLIVE	LANE(JL)	JL = JLZM(K,M,NLOADS + 1)

- (A) "Single Movable Load" refers to the effect of the most critical, single movable load, placed without regard to lane boundaries. This loading will always have Order $I = 0$.
- (B) "In-Lane" refers to the effect of the most critical, single movable load which is entirely within a lane. Therefore, $ZL(K,M)$ for $I = 0$ will be the same value as $ZM(K,M,I)$ for $I = 1$ when the most critical "single-movable-load" falls entirely within a lane.
- (C) New values of live-load $ZLIVE$ in $LANE(JL)$ at Station JL enter the table at the bottom and are compared to successive values in the table until the new values have progressed upwards to a value which is equal to or larger than the entering value. The values which are less than the new value are displaced one position downward with the least one (of previous order $NLOADS$) being discarded at the bottom.

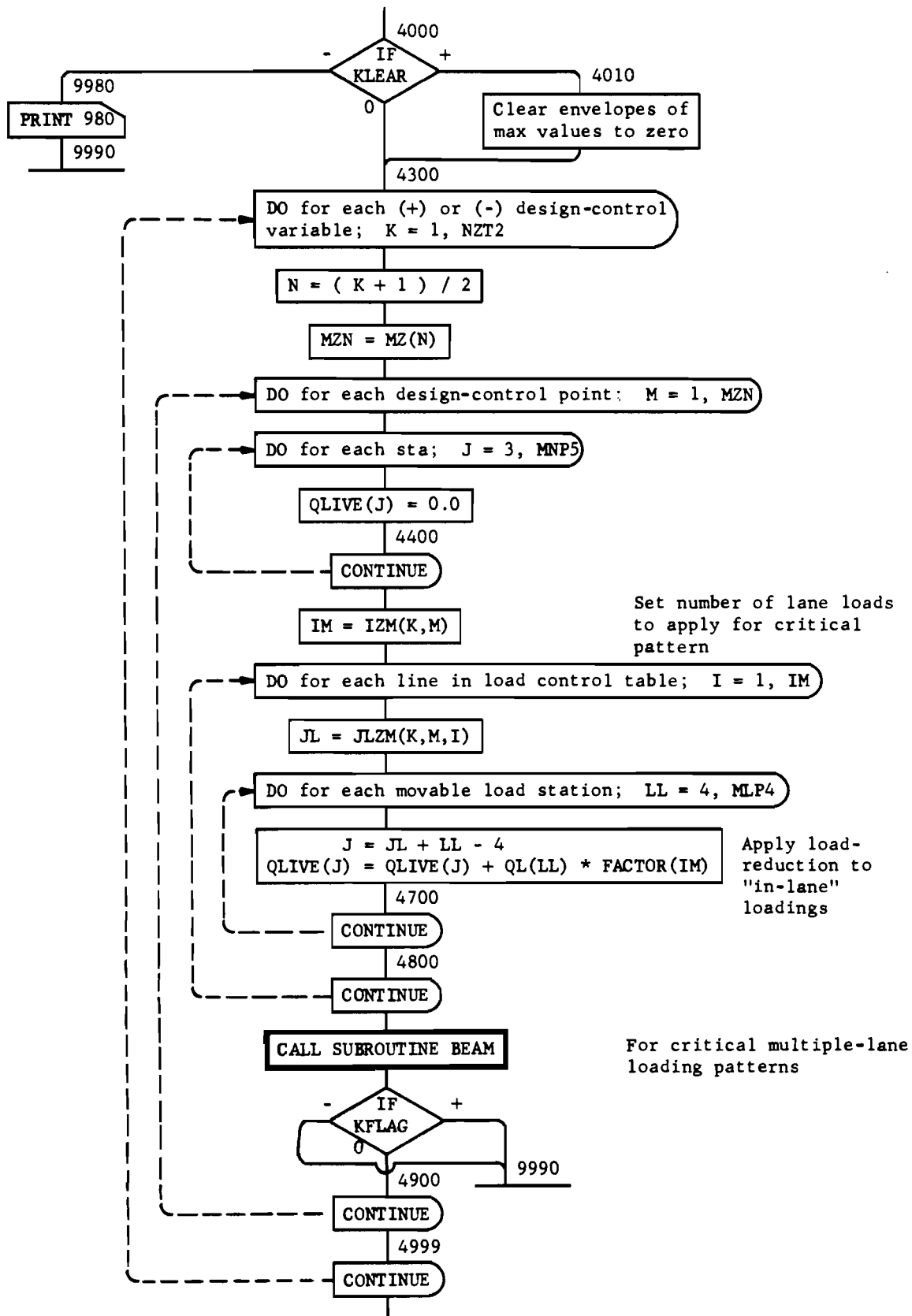


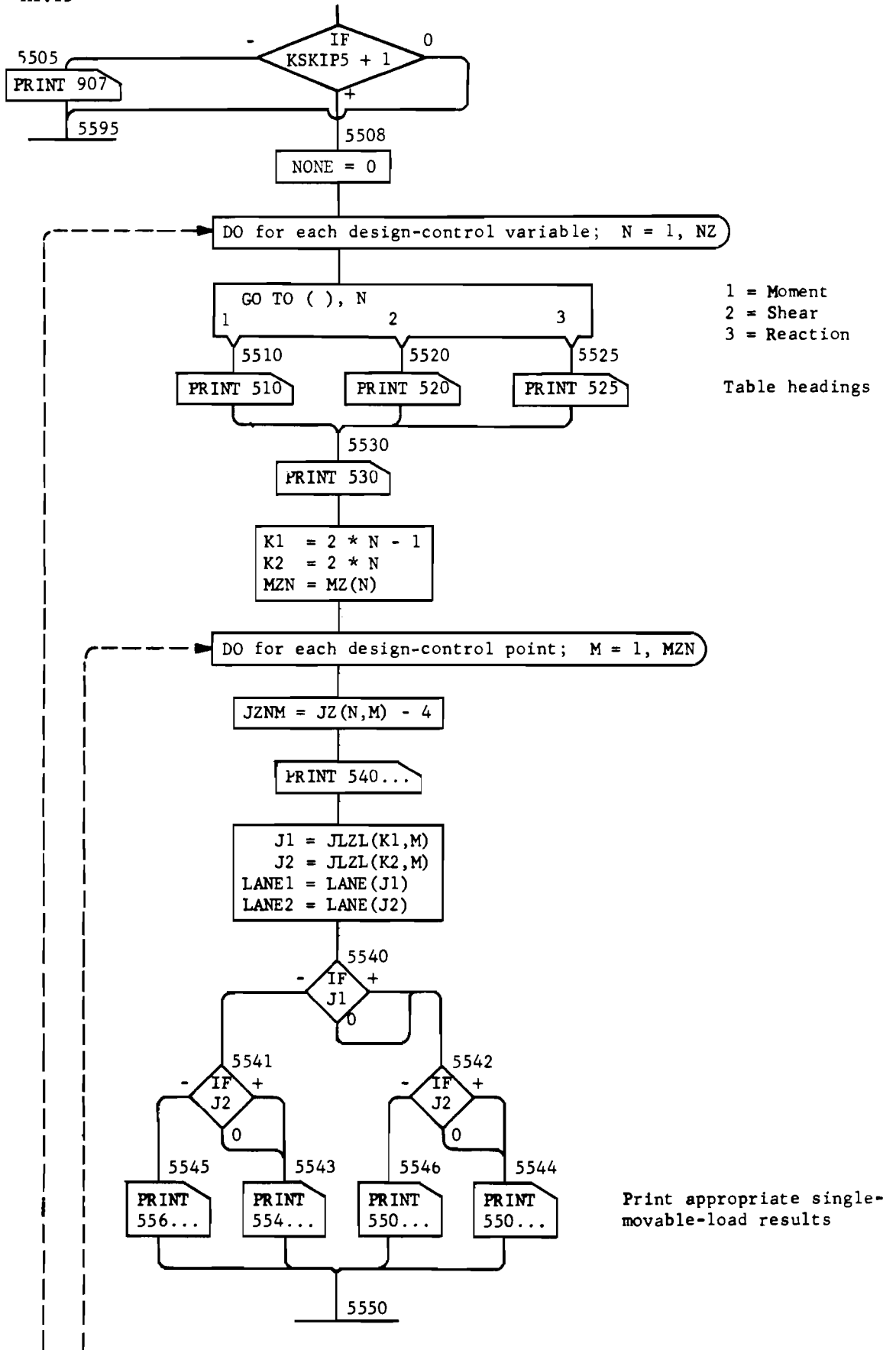
If KLEAR option is exercised, exclude single-movable-load influence from multiple-lane loading summary

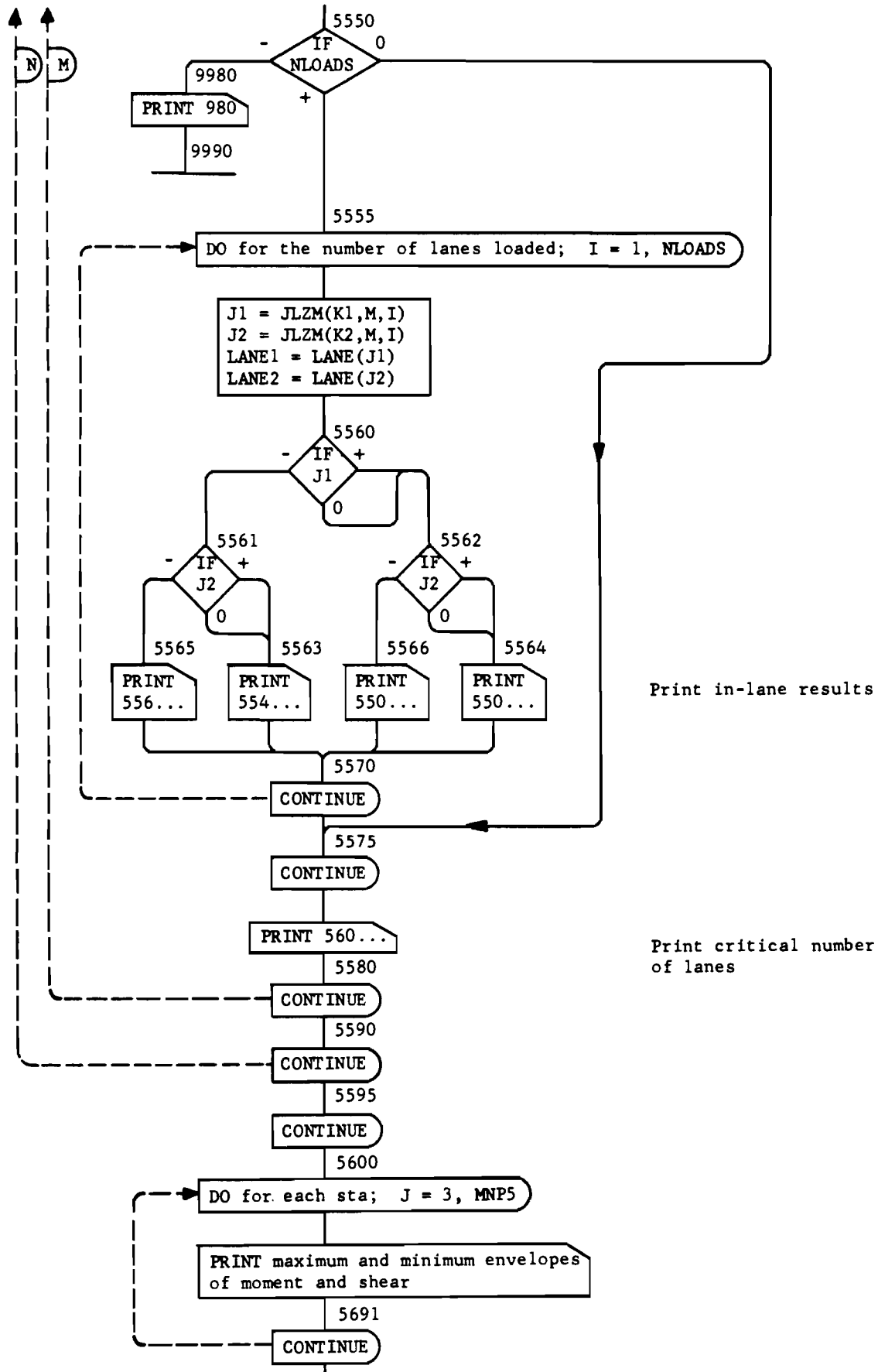
Final value of IZM(K,M) designates the number of lane-loads to apply for max + or max - critical design value at Control Point M

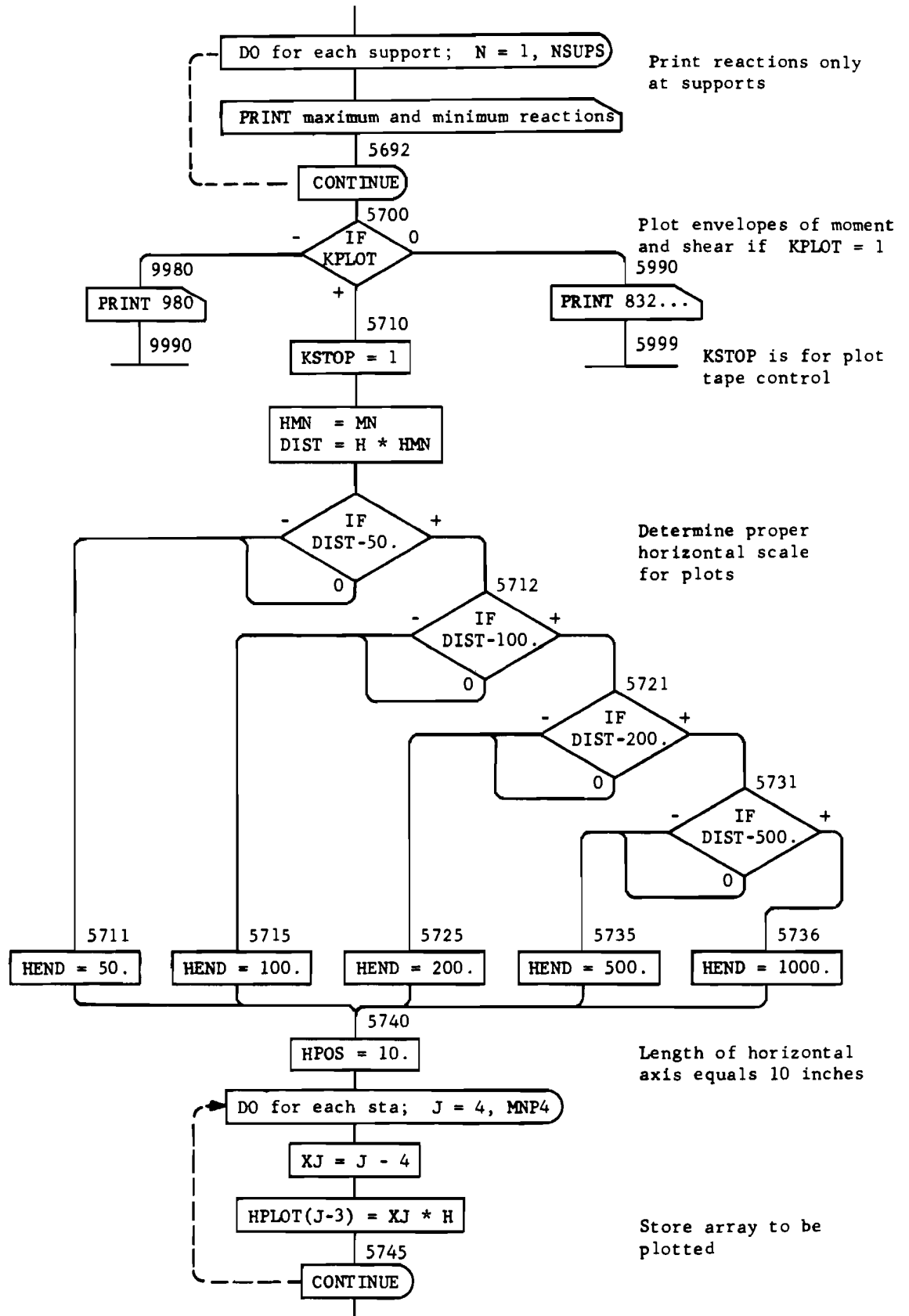
If envelopes from the initial solutions with the single movable load have been cleared, critical number of lanes will depend only on in-lane loadings. If single-movable-load envelopes have not been cleared and

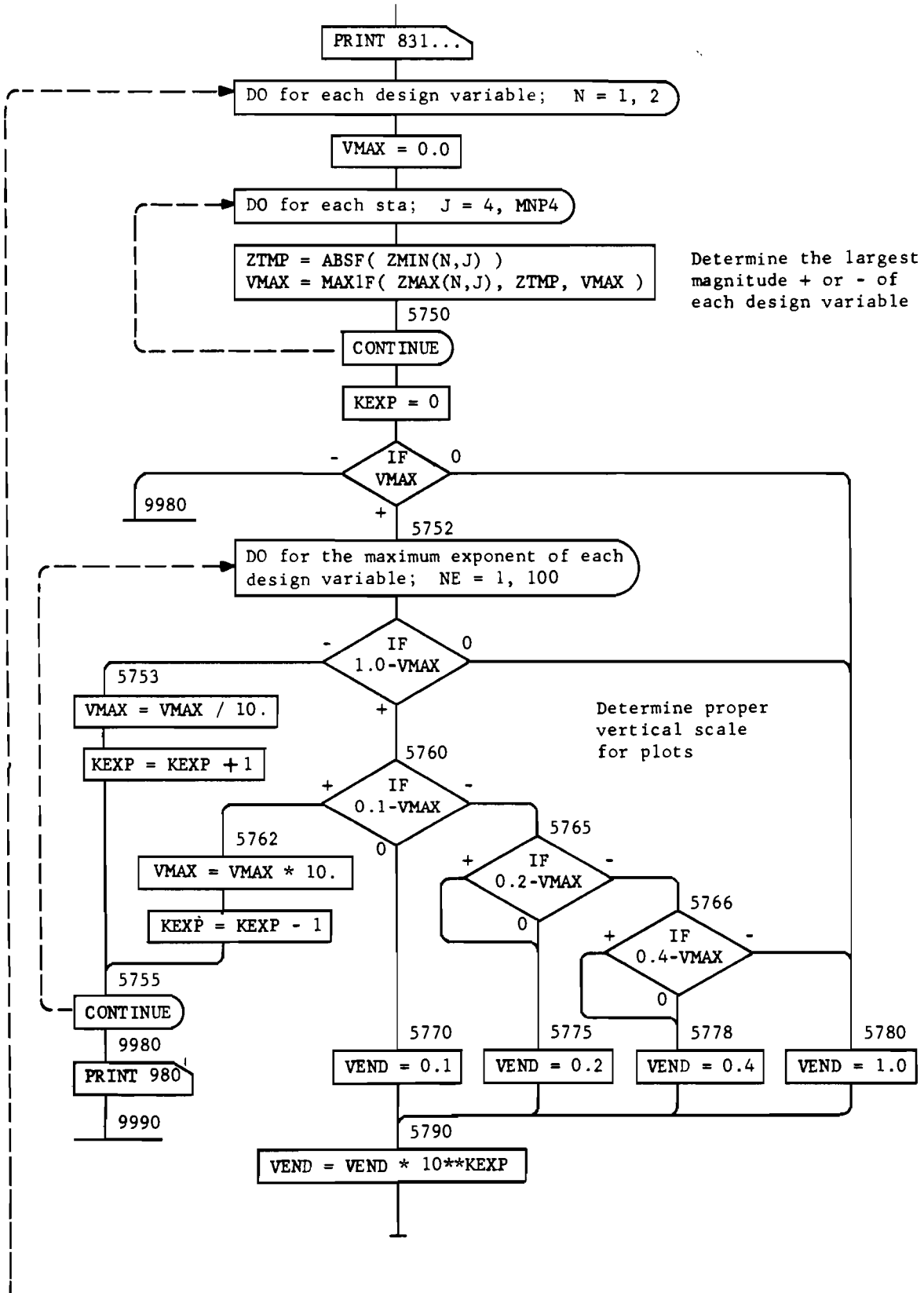
- (1) if out-of-lane value is larger than any in-lane value, critical number IZM(K,M) = 0;
- or (2) if out-of-lane value is exceeded by in-lane values, control table will be stacked in normal manner.

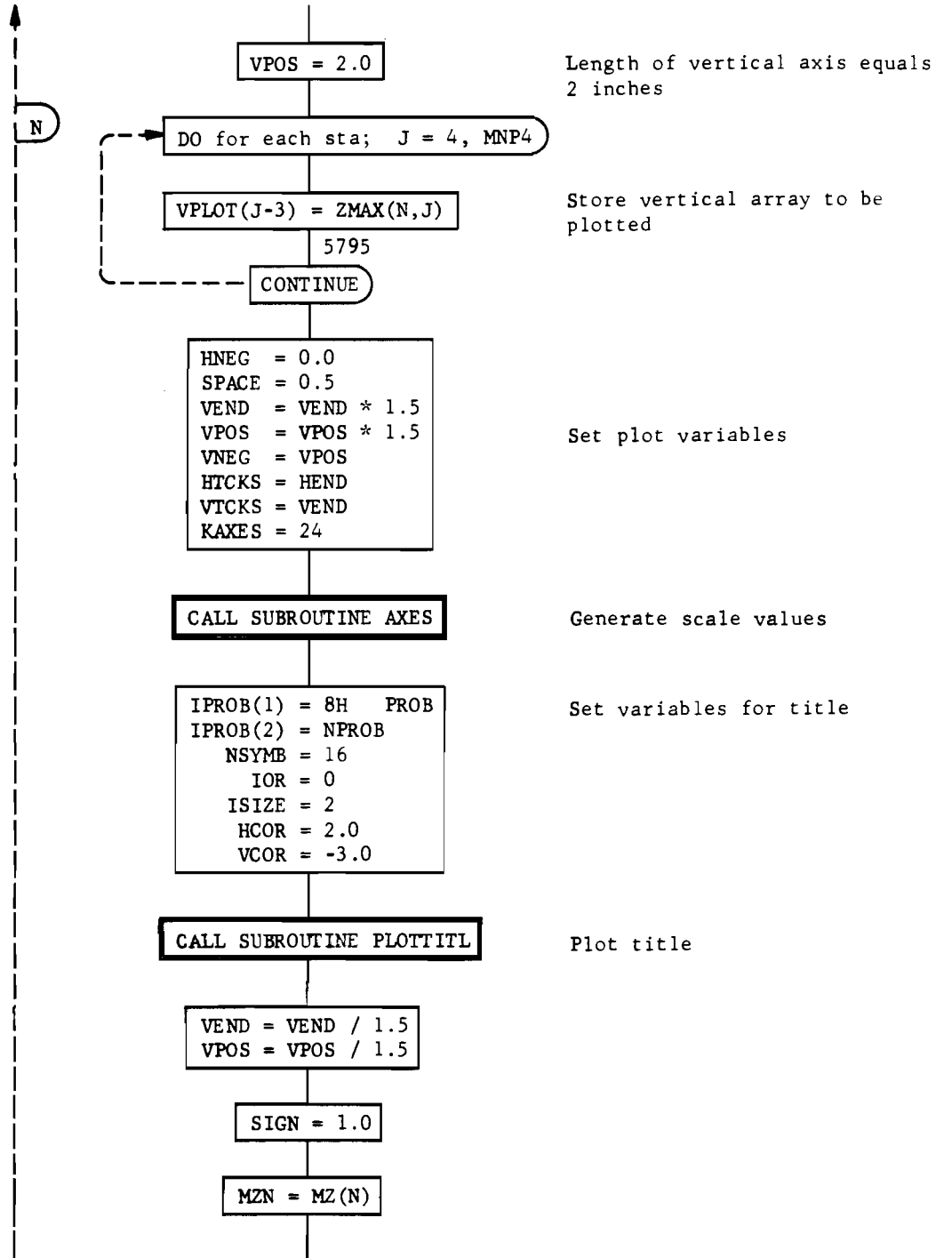


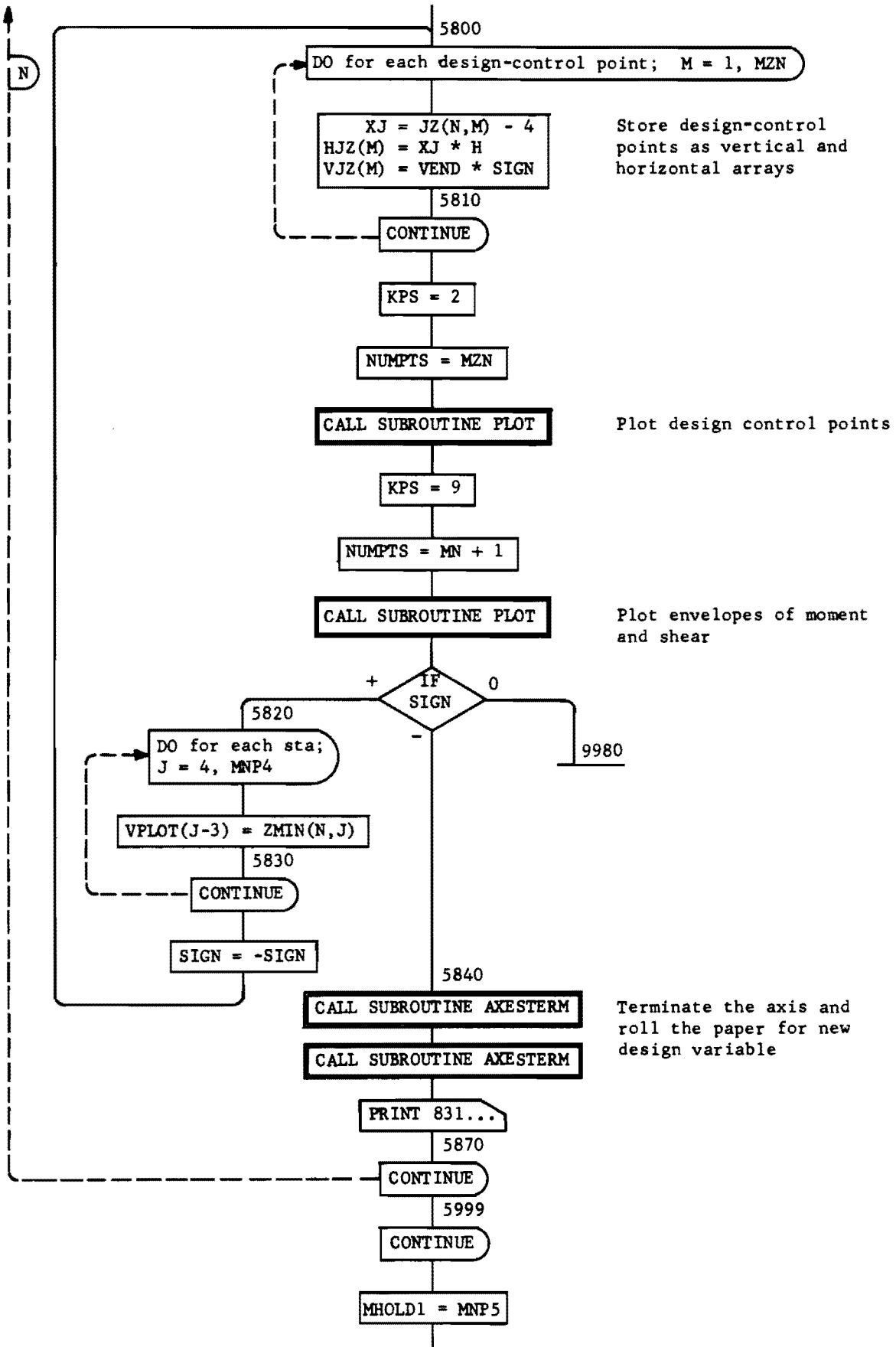


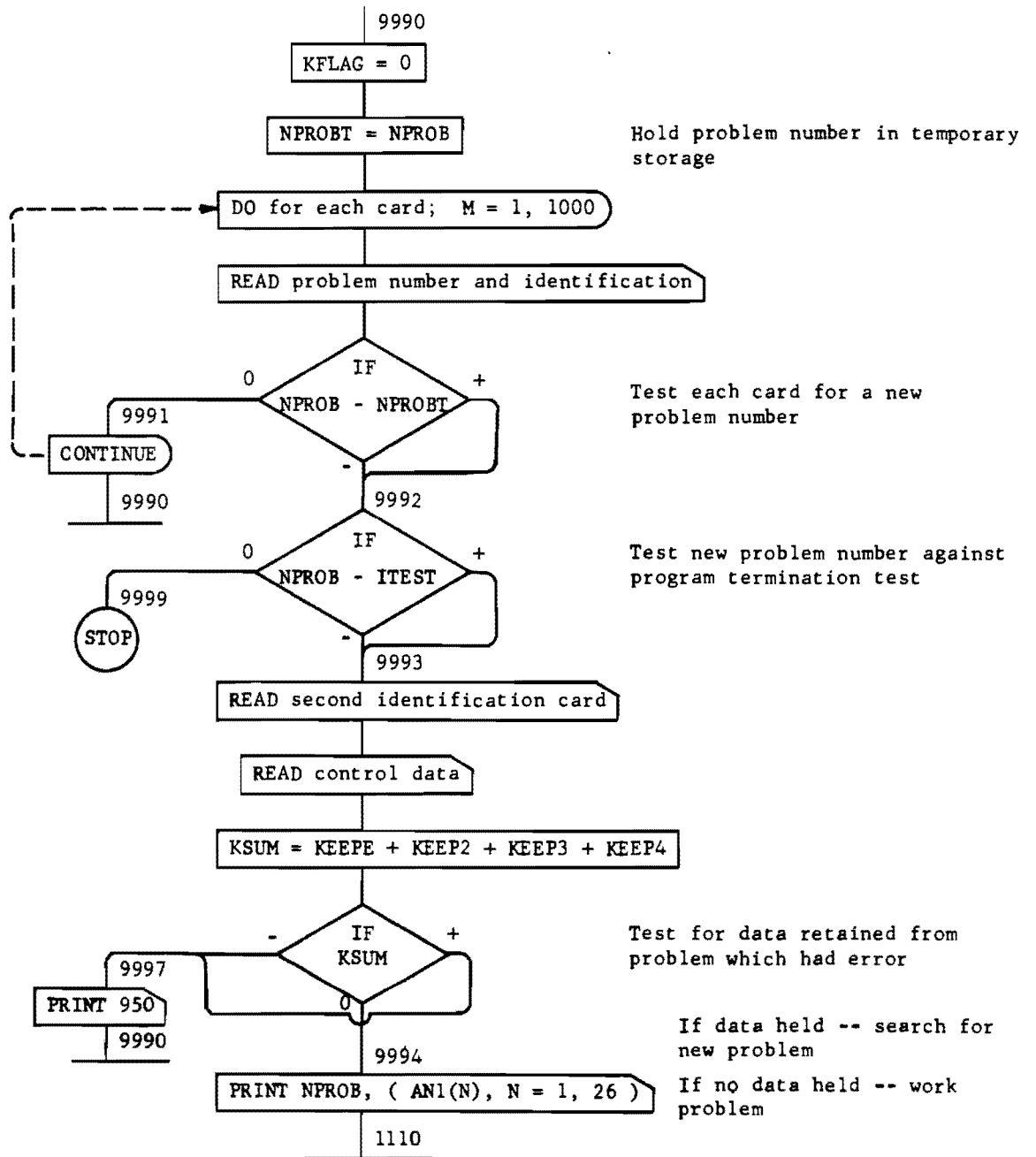


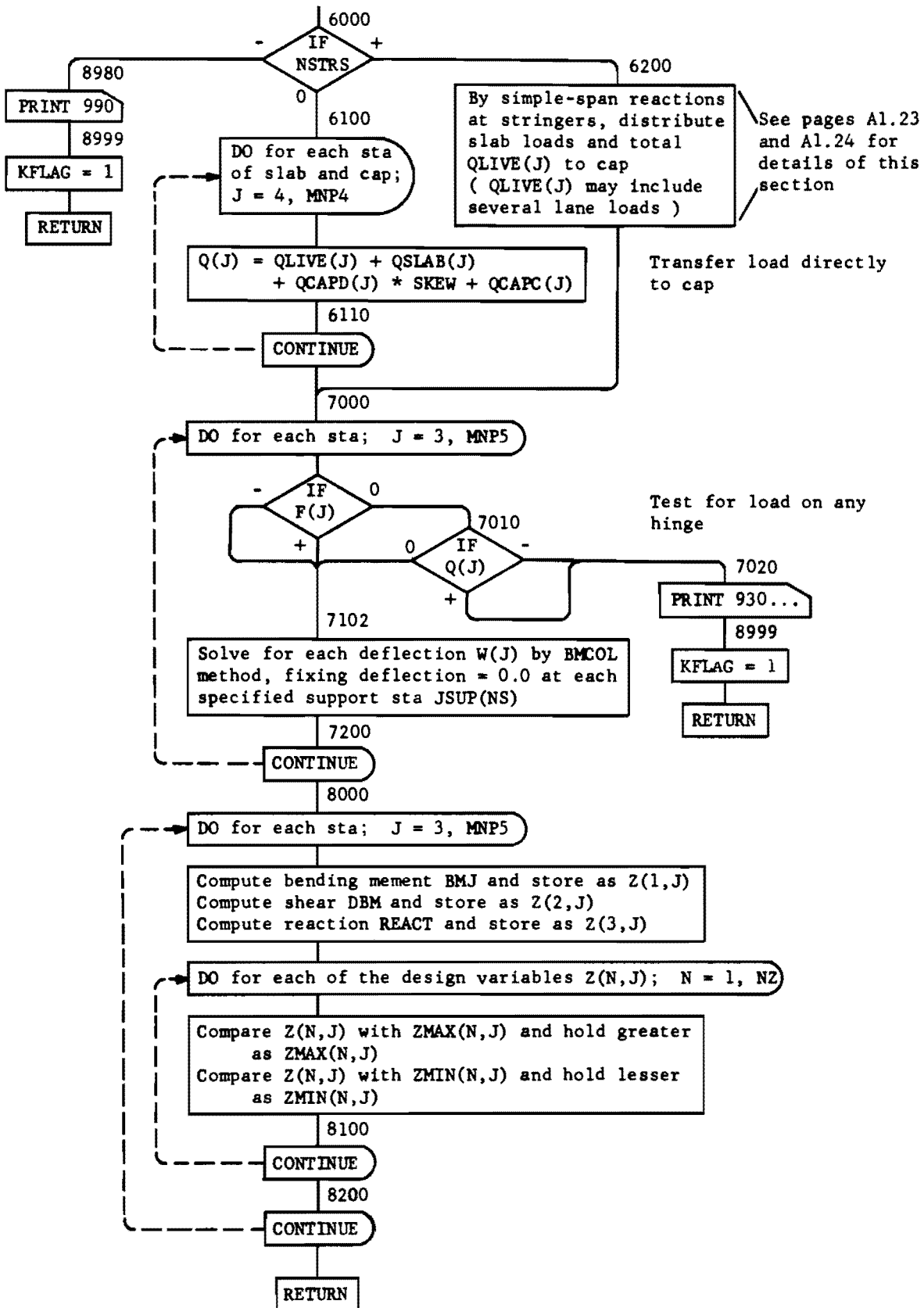




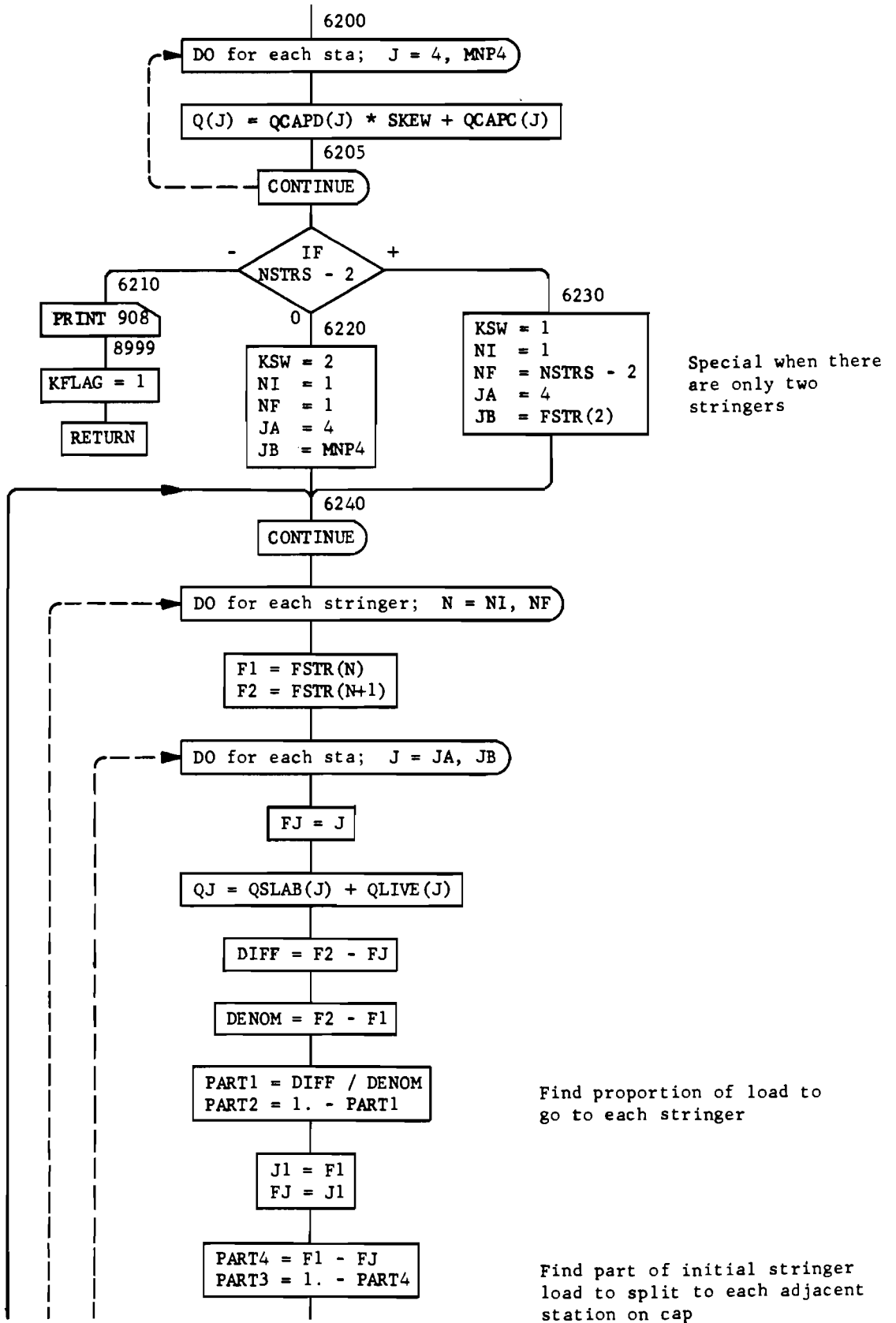




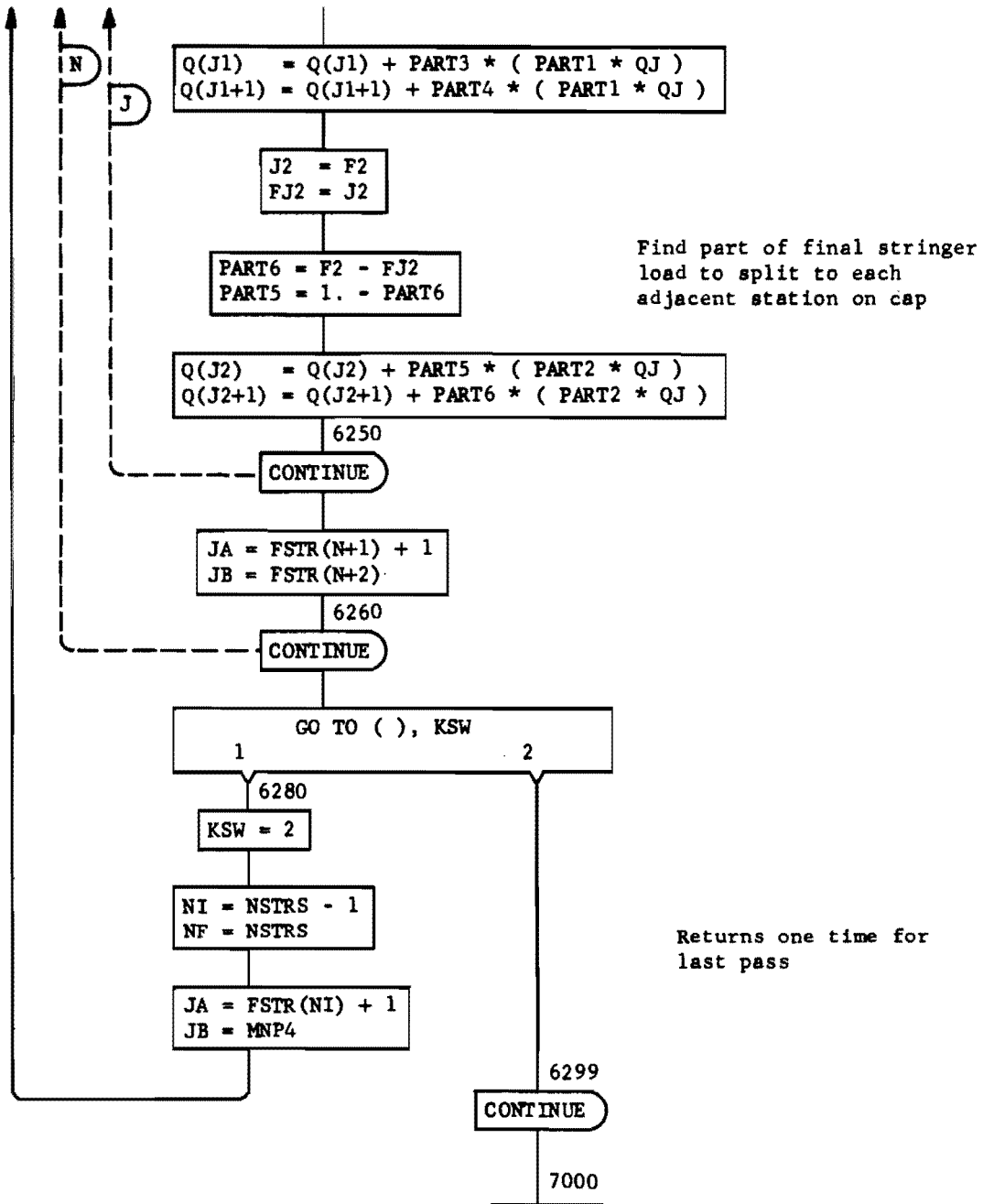




SUBROUTINE BEAM cont'd



SUBROUTINE BEAM cont'd



APPENDIX 2

GLOSSARY OF NOTATION FOR CAP 14

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C	-----NOTATION FOR CAP 14	* DENOTES INPUT	18NO4
C	A(J)	CONTINUITY COEFFICIENT	20AG3
C	AA	COEFF IN STIFFNESS MATRIX	12JE3
C	AN1()	IDENTIFICATION AND REMARKS (ALPHA-NUM)	20AG4
C	B(J)	CONTINUITY COEFFICIENT	20AG3
C	BB	COEFF IN STIFFNESS MATRIX	12JE3
C	BMI	BENDING MOMENT AT STA J-1	04JE3
C	BMJ	BENDING MOMENT AT STA J	07JE3
C	BMK	BENDING MOMENT AT STA J+1	04JE3
C	C(J)	CONTINUITY COEFFICIENT	20AG3
C	CC	COEFF IN STIFFNESS MATRIX	12JE3
C	D	MULTIPLIER IN CONTINUITY COEFF EQS	20AG3
C	DBM	FIRST DERIV OF BENDING MOMENT, DM/DX	07JE3
C	DD	COEFF IN STIFFNESS MATRIX	12JE3
C	DENOM	DENOMINATOR	07JE3
C	DIFF	DIFFERENCE	12JE3
C	DIST	LENGTH OF CAP IN FEET	15JL4
C	E	TERM IN CONTINUITY COEFF EQS	12JE3
C	EE	COEFF IN STIFFNESS MATRIX	12JE3
C	ESM	MULTIPLIER FOR HALF VALUES AT END STAS	07JE3
C	* FACTOR(1)	MULTIPLE-LANE LOAD-REDUCTION FACTOR	20AG3
C	FF	COEFF IN LOAD MATRIX	12JE3
C	F1, F2	INITIAL AND FINAL DISTRIBUTION POINTS IN SEQUENCE	03JN4
C	FJ, FJ2	FLOATING POINT STA NUM IN STRINGER LOAD SPLIT ROUTINE	15JL4
C	* FN1, FN2, F(J)	FLEXURAL STIFFNESS (EI) (INPUT AND TOTAL)	20AG3
C	FSTR(N)	STATION FOR STRINGER N	03JN4
C	FSTR1	FLOATING POINT EQUIVALENT OF JSTR	15JL4
C	H	INTERNAL INCREMENT LENGTH	30JE4
C	HCOR	HORIZONTAL COORDINATE OF THE FIRST SYMBOL IN THE PLOT TITLE	15JL4
C	HEND	VALUE ASSIGNED TO END OF HORIZONTAL-AXIS.	16AP4
C	HE2	H SQUARED	12JE3
C	HE3	H CUBED	12JE3
C	* HI	INCREMENT LENGTH	30JE4
C	HJZ()	HORIZONTAL VALUE OF DESIGN CONTROL POINT	15JL4
C	HMN	FLOATING POINT EQUIVALENT OF MN	15JL4
C	HNEG	LENGTH, IN INCHES, OF THE NEGATIVE H-AXIS. THIS VARIABLE MUST NOT ITSELF BE NEGATIVE.	14AP4
C	HPL0T()	NAME OF ARRAY TO BE PLOTTED ON H-AXIS	27AP4
C	HPOS	LENGTH, IN INCHES, OF THE POSITIVE H-AXIS	14AP4
C	HTCKS	INCREMENT LENGTH BETWEEN TICK MARKS ON H-AXIS IN TERMS OF HEND	14AP4
C	HT2	H TIMES 2	12JE3
C	I	INDEX IN LOAD-CONTROL TABLE	31JA4
C	IM	NUM OF CRITICAL LOAD REDUCTION FACTOR	26NO3
C	IN1, IN2, I STA	EXTERNAL STA NUMBER = J - 4	18DE3
C	IOR	ORIENTATION OF THE TITLE, = PARALLEL TO THE AXIS	15JL4
C	IPROB()	VARIABLE USED TO PLOT TITLES ON PLOTS	15JL4
C	IR	REVERSE-ORDER INDEX IN LOAD-CONTROL TABLE	22AG3
C	IR1	INITIAL INDEX IN LOAD CONTROL TABLE	26NO3
C	ISIZE	SIZE OF THE LETTERS IN THE PLOT TITLE	15JL4
C	ISW	ROUTING SWITCH FOR TABLE 4	18DE3

C	ITEST	= 5 ALPHANUMERIC BLANKS, USED TO TERMINATE	15JL4
C		THE PROGRAM	15JL4
C	IUNIT()	UNITS OF OUTPUT TERMS, TABLE 7	15JL4
C	IZM(K,M)	CRITICAL NUMBER OF SIMULTANEOUS LANE LOADS	22AG3
C	J, JN	INTERNAL STA ALONG SLAB AND CAP = I + 4	20AG3
C	JA, JB	STRINGER DISTRIBUTION STATIONS	31JA4
C	J1, J2	INITIAL AND FINAL STATIONS IN SEQUENCE	05JE3
C	JBEGIN	INITIAL IN-LANE POSITION OF MOVABLE LOAD	26NO3
C	JEND	FINAL IN-LANE POSITION OF MOVABLE LOAD	20AG3
C	JI	NON-SUBSCRIPTED VALUE OF JLZM(K,M,I)	20AG4
C	JINCR	INCREMENTATION INDEX	12JE3
C	JL	POSITION OF MOVABLE-LOAD STA ZERO (LL=0)	20AG3
C	* JLEFT(L)	STATION AT LEFT SIDE OF LANE L	20AG3
C	JLINC	INDEX USED IN MOVING MOVABLE LOAD	15JL4
C	JLTEMP	TEMPORARY HOLD FOR CONTROL-TABLE ROUTINE	22AG3
C	JLZL(K,M)	LOAD STA FOR RANDOM-POS MIN OR MAX AT PT M	22AG3
C	JLZM(K,M,I)	LOAD POS FOR I-TH MAX OR MIN AT DES PT M	22AG3
C	* JRIGHT(L)	STATION AT RIGHT SIDE OF LANE L	20AG3
C	* JSTART	INITIAL POSITION OF MOVABLE-LOAD STA ZERO	20AG3
C	* JSTOP	FINAL POSITION OF MOVABLE-LOAD STA ZERO	09JA4
C	JSTR	FIXED POINT EQUIVALENT OF FSTR(N)	15JL4
C	* JSUP(NS)	STATION FOR SUPPORT NS	20AG3
C	* JZ(N,M)	STA AT DESIGN CONTROL POINT M FOR Z(N,J)	22AG3
C	JZNM	TEMPORARY VALUE OF JZ(N,M)	15JL4
C	K	DO LOOP INDEX	15JL4
C	K1, K2	INTERNAL VALUE OF NZ	15JL4
C	KAXES	INDEX USED TO DESCRIBE MANNER IN WHICH	10JL4
C		AXES SHOULD BE PLACED ON PAPER	14AP4
C	* KEEP2 THRU KEEP4	IF = 1, KEEP PRIOR DATA, TABLES 2-4	20AG4
C	* KEEPE	IF = 1, KEEP PRIOR ENVELOPES OF EXTREMES	18DE3
C	KEXP	EXPONENT OF 10 TIMES DESIGN VARIABLE	20AG4
C		REDUCED TO ONE SIGNIFICANT FIGURE	24AP4
C	KFLAG	ERROR FLAG	20AG4
C	* KLEAR	IF = 1, CLEAR ENVELOPES BEFORE LANE LOADS	18DE3
C	* KPLOT	IF = 1, EXCLUDE PLOTS OF ENVELOPES	18DE3
C	KPS	PLOT SYMBOL TO BE USED	14AP4
C		1 DENOTES SMALL PLUS SIGN	14AP4
C		2 DENOTES SMALL X	14AP4
C		3 DENOTES SMALL SQUARE	14AP4
C		4 DENOTES SMALL STAR (COMBINATION OF 1, 2)	14AP4
C		8 DENOTES NO SYMBOL TO BE PLOTTED, BUT	14AP4
C		PEN TO BE MOVED	16AP4
C		9 DENOTES LARGE PLUS SIGN	14AP4
C		10 DENOTES LARGE X	14AP4
C		11 DENOTES LARGE SQUARE	14AP4
C		12 DENOTES LARGE STAR (COMBINATION OF 9	16AP4
C		AND 10)	16AP4
C		A NEGATIVE NUMBER WILL MAKE THE SAME	14AP4
C		SYMBOLS AS THE POSITIVE NUMBERS, BUT THE	14AP4
C		POINTS WILL BE CONNECTED WITH STRAIGHT	14AP4
C		LINE. THUS IF KPS = -8 THE POINTS WILL	16AP4
C		BE CONNECTED BY STRAIGHT LINES WITHOUT	16AP4
C		THE POINTS BEING MARKED BY PLOT SYMBOLS	16AP4
C	KR1	PRIOR VALUE OF KR2	05AG3
C	KR2	IF = 1, REFER TO NEXT CARD	05AG3

C	KSKIP5	OPTION (IF= -1) TO OMIT OUTPUT TABLES	18N04
C	KSTOP	ROUTING SWITCH TO SKIP PLOT TAPE (IF=0)	23JL4
C	KSUM	SUM OF ALL KEEP OPTIONS	20AG4
C	KSW	ROUTING SWITCH FOR TABLE 4 AND OTHERS	19AG4
C	LANE(JL)	LANE FOR MOVABLE LD, ZERO IF NOT IN-LANE	20AG3
C	LANE1, LANE2	TEMPORARY LANE RANK OUTPUT DEVICES	26N03
C	L, LLL	LANE NUMBER IN LANE ROUTINE	31JA4
C	LL	MOVABLE-LOAD STATION	20AG3
C	LTEST	TEMPORARY TEST IN LANE ROUTINE	26N03
C	M	DO LOOP INDEX	15JL4
C	MHOLD	M FROM PREVIOUS PROBLEM	15JL4
C	* ML	TOTAL MOVABLE-LOAD WIDTH IN INCREMENTS	20AG3
C	* MLINC	NUMBER OF STAS BETWEEN EACH POSITION OF MOVABLE LOAD	16JL4
C	MLP4	ML + 4	31JA4
C	MM	DO LOOP INDEX	20AG4
C	* MN	TOTAL NUM INCREMENTS FOR SLAB AND CAP	20AG3
C	MNP4, MNP5, MNP7	MN + 4, MN + 5, MN + 7	31JA4
C	MP1	M + 1	20AG4
C	MT	VARIABLE DO LOOP INDEX	20AG4
C	* MZ(N), MZN	TOTAL NUM DES CONTROL POINTS FOR Z(N,J)	15MY4
C	MZ1, MZ2, MZ3	NON-SUBSCRIPTED VALUES OF DESIGN POINTS	20AG4
C	N	NUMBER OF THE PARTICULAR DESIGN VARIABLE	31JA4
C	NAME(N)	AXIS NAME - OUTPUT IN TABLE 7	15MY4
C	* NCD2 THRU NCD4	NUM CARDS IN TABLES 2 THRU 4, THIS PROB	20AG3
C	NE	INDEX IN SCALE DETERMINATION ROUTINE	24AP4
C	NI, NF	STRINGER DISTRIBUTION INDICES	03FE4
C	* NLANES	TOTAL NUMBER OF LANES	20AG3
C	* NLOADS	MAX NUM LANES TO BE LOADED SIMULTANEOUSLY	20AG3
C	NONE	OUTPUT CONSTANT EQUAL TO ZERO	26N03
C	* NPROB	PROBLEM NUMBER (PROG STOPS IF BLANK)	15JL4
C	NPROBT	TEMPORARY HOLD OF NPROB	20AG4
C	NS	INDEX NUM FOR SUPPORT POINT	18DE3
C	NSTA	NUM OF STA REQUIRED BETWEEN A SUPPORT AND A DESIGN CONTROL POINT	15JL4
C	* NSTRS	TOTAL NUMBER OF STRINGERS	20AG3
C	* NSUPS	TOTAL NUMBER OF SUPPORTS	20AG3
C	NSYMB	NUM OF SYMBOLS TO BE PLOTTED IN THE TITLE	15JL4
C	NT, NST	INDICES IN SHEAR DESIGN POINT CHECK	06MY4
C	NUMPTS	NUMBER OF POINTS TO BE PLOTTED	14AP4
C	NZ (*3FOR CAP 14)	TOTAL NUM OF DES VARIABLES (INTERNAL)	20AG4
C	NZT2	NZ TIMES 2	15JL4
C	PART, PART1-6	FRACTIONS	15JL4
C	* QCAPC(J)	CONCENTRATED DEAD LOAD FROM CAP AT STA J	03JN4
C	* QCAPD(J)	DISTRIBUTED DEAD LOAD FROM CAP AT STA J	03JN4
C	QJ	TEMP VALUE OF LOADS IN SUBROUTINE BEAM	15JL4
C	* QL(LL)	FORCE AT MOVABLE-LOAD STA LL	20AG3
C	QLIVE(J)	TOTAL LIVE-LOAD DISTRIBUTION	06MY4
C	Q(J)	TRANSVERSE FORCE (INPUT, TOTAL)	20AG4
C	QS1, QC1, QL1	TRANSVERSE FORCE (INTERNAL DISTRIBUTION)	26N03
C	QS2, QC2, QL2	TRANSVERSE FORCE (INPUT)	26N03
C	* QSLAB(J)	DEAD LOAD FROM SLAB OR SIDEWALK AT STA J	20AG3
C	REACT	SECOND DERIV OF BENDING MOMENT	20AG4
C	SIGN	SWITCH CONTROL ON + OR - DESIGN VARIABLE	26N03
C	SKEW	1.0 DIVIDED BY COSINE OF THETAR	15JL4

C	SPACE	DISTANCE, IN INCHES, FROM LEFT EDGE OF	14AP4
C		PAPER TO END OF NEGATIVE H-AXIS	14AP4
C	SUM	CUMULATIVE EFFECT OF MULTI-LANE LOADING	22AG3
C	TEST, ZTEST	TEMPORARY TEST IN CONTROL TABLE ROUTINE	26NO3
C	* THETA	ANGLE OF SKEW, DEGREES AND DECIMALS	15JL4
C	THETAR	ABSOLUTE VALUE OF THETA IN RADIANS	15JL4
C	VCOR	VERTICAL COORDINATE OF THE BOTTOM OF THE	15JL4
C		PLOT TITLE	15JL4
C	VEND	VERTICAL AXIS VALUE AT 2 INCHES	24AP4
C	VJZ()	VERTICAL PLOT VALUE OF DES CONTROL POINT	15JL4
C	VMAX	ABSOLUTE MAXIMUM VALUE OF DESIGN VARIABLE	24AP4
C	VNEG	LENGTH, IN INCHES, OF THE NEGATIVE V-AXIS.	14AP4
C		THIS VARIABLE MUST NOT ITSELF BE NEGATIVE.	14AP4
C	VPLOT()	NAME OF ARRAY TO BE PLOTTED ON V-AXIS	27AP4
C	VPOS	LENGTH, IN INCHES, OF THE POSITIVE V-AXIS	14AP4
C	VTCKS	INCREMENT LENGTH BETWEEN TICK MARKS ON	14AP4
C		V-AXIS IN TERMS OF VEND	16AP4
C	W(J)	LATERAL DEFLECTION OF BMCOL AT STA J	12JE3
C	X	DISTANCE ALONG THE BMCOL	30MY3
C	XJ	FLOATING POINT EQUIVALENT OF THE STA NUM	15JL4
C	Z(N,J)	DESIGN VARIABLE NUM N AT STA J	22AG3
C	Z1,Z2	SUCCESSIVE VALUES IN CRITICAL-LOAD STUDY	22AG3
C	ZD(N,M)	DESIGN VARIABLE N AT DESIGN POINT M	10JA4
C	ZL(K,M)	MAX OR MIN ONE-LOAD INFLUENCE AT DES PT M	22AG3
C	ZLIVE	DESIGN VARIABLE FROM LIVE LOAD ONLY	31JA4
C	ZM(K,M,I)	I-TH VALUE IN IN-LANE LOAD-CONTROL TABLE	22AG3
C	ZMAX(N,J)	MAX Z(N,J) FROM ALL PRIOR LOADINGS	22AG3
C	ZMIN(N,J)	MIN Z(N,J) FROM ALL PRIOR LOADINGS	22AG3
C	ZTEMP	TEMPORARY HOLD FOR CONTROL-TABLE ROUTINE	22AG3
C	ZTMP	TEMPORARY HOLD FOR SCALE DETERMINATION	24AP4

APPENDIX 3

LISTING OF PROGRAM DECK OF CAP 14

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PROGRAM CAP 14
1 FCRMAT ( 51H PROGRAM CAP 14 - DECK 2 - MATLOCK - INGRAM
1 28H REVISION DATE = 3AUG 66 ) 25JL6
DIMENSION AN1(27), 04AG4
1 JLEFT(10), JRIGHT(10), JSUP(20), FSTR(30), F(307), 04AG4
2 QL(307), LANE(307), FACTOR(5), QLIVE(307), CSLAB(307), 10AP4
3 QCAPD(307), Z(3,307), ZMAX(3,307), ZMIN(3,307), QCAPC(307), 03AG4
4 MZ(3), JZ(3,30), ZM(6,30,6), JLZM(6,30,6), ZL(6,30), 03AG4
5 JLZL(6,30), IZM(6,30), ZD(3,30), 03AG4
6 VPLCT(307), HPLCT(307), VJZ(30), HJZ(30), NAME(3), 30JE4
7 IPRCB(2), ILNIT(3) 02JL4
COMMON QLIVE, CSLAB, QCAPD, QCAPC, FSTR, JSUP, F, ZMAX, ZMIN, Z 05JE4
11 FCRMAT ( 1H1 ) 14AG4
12 FCRMAT ( 4X, A5, 6X, 13A5 ) 04AG4
13 FCRMAT ( 5X, 16A5 ) 26AG3 ID
14 FCRMAT ( 15X, 13A5 ) 04AG4
15 FCRMAT (///10H PRCB , /, 5X, A5, 5X, 13A5, /, 15X, 13A5 ) 04AG4
16 FCRMAT (///17H PROP (CCNTD) / 5X, A5, 5X, 13A5, /, 15X, 13A5) 04AG4
100 FCRMAT (// 36H TABLE 1 -- PROGRAM-CONTROL DATA / 29AP4
1 50X, 30H ENVELOPES TABLE NUMBER / 22OC3
2 50X, 30H OF MAXIMUMS 2 3 4 / 22OC3
3 52H OPTIONS TO HOLD (IF=1) FROM PRECEDING PROB 22OC3
4 , I10, 3X, 3I5, / 15MY4
5 52H NUMBER OF ADDITIONAL CARDS FOR CURRENT PROB 22OC3
6 1M2, 12X, 3I5, // 03FE4
7 52H OPTION (IF=1) TO CLEAR ENVELOPES BEFORE LA 29OC3
8 11M2 LOADINGS, 14X, I3 ) 07N03
101 FCRMAT ( 52H OPTION (IF=1) TO PLOT DESIGN VARIABLE ENVE 29AP4
1 5HLOPES, 2CX, I3, // 20MY4
2 46H OPTION (IF=-1) TO OMIT OUTPUT TABLE 5, 31X, 18N04
3 I3, // 17N04
4 32H ANGLE OF SKEW, DEGREES, 38X, 8I0.3 ) 18N04
105 FCRMAT ( 15X, 10I5, 5X, E10.3 ) 17N04
200 FCRMAT (// 25H TABLE 2 -- CONSTANTS / ) 29AP4
205 FCRMAT ( 15X, 15, E10.3, 5X, 4I5 ) 03AG4
206 FCRMAT ( 15X, 15, 5E10.3 ) 03AG4
220 FCRMAT ( 47H NUMBER OF INCREMENTS FOR SLAB AND CAP, 22OC3
1 28X, I5, / 22OC3
2 30H INCREMENT LENGTH, FT, 40X, E10.3, / 08JL4
3 47H NUMBER OF INCREMENTS FOR MOVABLE LOAD, 22OC3
4 28X, I5, / 22OC3
5 51H INITIAL POSITION OF MOVABLE-LOAD STA ZERO, 22OC3
6 24X, I5, / 22OC3
7 49H FINAL POSITION OF MOVABLE LOAD STA ZERO, 22OC3
8 26X, I5 ) 22OC3
225 FCRMAT ( 52H NUMBER OF INCREMENTS BETWEEN EACH POSITION 08JL4
1 16H OF MOVABLE LOAD, 7X, I5 ) 08JL4
230 FCRMAT ( 52H MAXIMUM NUMBER OF LANES TO BE LOADED SIMUL 22OC3
1 9HTANECUSLY, 14X, I5, / 25OC3
2 52H LIST OF LOAD COEFFICIENTS CORRESPONDING TO 22OC3
3 23H NUMBER OF LANES LOADED /, 26X, 2H 1, 10X, 2H 2, 10X 22OC3
4 , 2H 3, 10X, 2H 4, 10X, 2H 5, / 22OC3
5 , 2CX, 5(2X, E10.3) ) 23MR4
300 FCRMAT (// 35H TABLE 3 -- LISTS OF STATIONS / ) 12N03
305 FCRMAT ( 15X, 13I5 ) 03AG4
308 FCRMAT ( 50H NUM OF NUM OF NUM OF NUM 23OC3

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1      15HPCP      NUM SHEAR      ,/      230C3
2      50H      LANES      STRINGERS      SUPPORTS      CONTR      29AP4
3      16H PTS      CCNTR PTS      ,/      06MY4
4      15H      TCTAL, 5(15, 5X) )      28JA4
310 FCRMAT ( / 52H      1      2      3      4      13JL4
1      33H5      6      7      8      9      10 )      13JL4
312 FCRMAT ( / 20H      LANE LEFT      3X, 10I6, /, 23X, 5I6 )      13JL4
314 FCRMAT ( / 20H      LANE RIGHT      3X, 10I6, /, 23X, 5I6 )      13JL4
315 FCRMAT ( 15X, 10I5 )      03AG4
316 FCRMAT ( / 20H      STRINGERS      5X, 10F6.1, /, 25X, 10F6.1, 14JL4
1      /, 25X, 10F6.1 )      14JL4
318 FCRMAT ( 15X, 10F5.0 )      03AG4
320 FCRMAT ( / 20H      SUPPORTS      3X, 10I6, /, 23X, 5I6 )      13JL4
322 FCRMAT ( / 20H      PCP CONTR      3X, 10I6, /, 23X, 10I6, /, 13JL4
1      23X, 10I6 )      13JL4
324 FCRMAT ( / 22H      SHEAR CONTR      1X, 10I6, /, 23X, 10I6, /, 14JE4
1      23X, 10I6 )      13JL4
400 FCRMAT ( // 72H      TABLE 4 -- CAP STIFFNESS, AND DATA FOR BOTH FIXED AND MOVABLE LOADS / )      05SE3
12NO3
401 FCRMAT ( 15X, 3I5, 4E10.3 )      03AG4
402 FCRMAT ( 10X, 3I5, 4(5X, E10.3) )      23MR4
404 FCRMAT ( 10X, 15, 5X, 15, 4(5X, E10.3) )      23MR4
406 FCRMAT ( 15X, 2I5, 4(5X, E10.3) )      23MR4
410 FCRMAT ( / 85H      FIXED-OR-MOVABLE      - - - - - FIXED-POSITION      1290C3
1CN DATA - - - - -      MOVABLE-      )      07NO3
420 FCRMAT ( / 85H      STA STA      CCNTR      CAP BENDING      SIDWAL      120SE3
1K,      STRINGER,      POSITION      )      05SE3
430 FCRMAT ( / 85H      FROM TC      IF=1      STIFFNESS      SLAB LG      120SE3
1ACS      CAP LCADS      SLAB LOADS      / 83H      02JL4
2      ( K-FT*FT )      ( K )      ( K )      ( K )      / ) 02JL4
440 FCRMAT ( 10X, 3I5, 4(5X, E10.3) / )      23MR4
500 FCRMAT ( // 85H      TABLE 5 -- MULTI-LANE LOADING SUMMARY      ( 18NO3
1---CRITICAL NUMBER OF LANE LOADS )      120C3
510 FCRMAT ( / 25H      PCMENT ( FT-K ) )      02JL4
520 FCRMAT ( / 20H      SHEAR ( K ) )      02JL4
525 FCRMAT ( / 24H      REACTION ( K ) )      04AG4
530 FCRMAT ( / 50H      AT      DEAD LC      LANE      POSITIVE      LCA      18NO4
1      35HC AT      LANE      NEGATIVE      LOAD AT /      18NO4
2      50H      STA      EFFECT      ORDER      MAXIMUM      LAN      17JA4
3      36HE STA      CRDER      MAXIMUM      LANE STA )      17JA4
540 FCRMAT ( / 113, 2X, E10.3 )      23MR4
550 FCRMAT ( 25X, 2(17, E13.3, 2I5) )      23MR4
554 FCRMAT ( 25X, 17, E13.3, 10X, 17, E13.3, 2I5 )      23MR4
556 FCRMAT ( 25X, 2(17, E13.3, 10X) )      23MR4
560 FCRMAT ( 31X, 11, 1H*, 28X, 11, 1H* )      18NO3
600 FCRMAT ( / 45H      TABLE 6 -- ENVELOPES OF MAXIMUM VALUES      //      19NO3
1      52H      STA      DIST X      MAX + MCP      MAX - MCP      11SE3
2      35HCP      MAX + SHEAR      MAX - SHEAR      /      02JL4
3      52H      ( FT )      ( FT-K )      ( FT-K )      ( FT-K ) 02JL4
4      35H )      ( K )      ( K )      / )      02JL4
610 FCRMAT ( 10X, 13, 2X, 5(E10.3, 5X) )      23MR4
700 FCRMAT ( // 45H      TABLE 7 -- MAXIMUM SUPPORT REACTIONS      //      03AG4
1      52H      STA      DIST X      MAX + REACT      MAX - REACT      10AG4
2      5HACT      /      FT      K      K / ) 10AG4
3      50H      FT      K      K / ) 10AG4
710 FCRMAT ( 10X, 13, 2X, 3(E10.3, 5X) )      03AG4

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830 FCRMAT (///38H      TABLE 8 -- SCALES FOR PLOT OUTPUT / )      03AG4
831 FCRMAT ( 10X, A8, F5.0, 10H INCHES = , F10.0, A5 )      03AG4
832 FCRMAT (    40H      NO PLOTS SPECIFIED FOR PROBLEM, 1X, A5 ) 03AG4
903 FCRMAT (    25H      NONE )      28JA4
905 FCRMAT (5X,41H      USING DATA FROM THE PREVIOUS PROBLEM )      28JA4
906 FCRMAT (5X,46H      USING DATA FROM THE PREVIOUS PROBLEM PLUS / ) 03FE4
907 FCRMAT(5X,45H      TABLE 5 OUTPUT OMITTED FROM THIS PROBLEM // ) 17NO64
908 FCRMAT (// 40H      ERRCR STOP -- STATIONS NOT IN ORDER )      10JE6
909 FCRMAT ( //46H      ERRCR -- NON-ZERO TABLE 4 DATA BEYOND END ) 21JL6
910 FCRMAT (///50H      LANE TOC NARROW FOR WIDTH OF MOVABLE LOAD ) 13DE3
912 FCRMAT (///18H      LANE NUMBERS , 15, 3HAND, 15, 8H (OVERLAP ) 03FE4
920 FCRMAT ( //52H      ERRCR IN PLGT ROUTINE, EXPONENT GRBATER THAN E 29AP4
    1      10H+ OR -10C )      18NO4
930 FCRMAT (///52H      SHEAR DESIGN CONTRCL POINT DESIGNATED TOO CLOSE1MY4
    1      / 50H      TO A CCNTRATED LCAD )03AG4
940 FCRMAT (///50H      --- MOVABLE LCAD INCREMENT ASSUMED = 1 --- )18NO4
950 FCRMAT (/ 10H      PRCB A5, 30H HAS RETAINED DATA FROM PRCB A5,10AG4
    1      29H      IN WHICH AN ERROR OCCURRED. )      10AG4
    2      / 10H      PRCB A5, 20H HAS BEEN REJECTED. )      10AG4
980 FCRMAT (///50H      UNDESIGNATED ERROR STOP      )13AG4

C
1000 CALL TIME      13AG4
C-----PRCGRAM ANC PROBLEM IDENTIFICATION      04MY3 ID
      KFLAG = C      03AG4
      KSTCP = C      29AP4
1010 READ 12, NPRCB, ( AN1(N), N = 1, 13 )      04AG4
      ITEST = 5H      16JE4
      IF ( NPROB - ITEST ) 1C20, 9995, 1020      03AG4
1020 PRINT 11      26AG3
      PRINT 1      04AG4
      READ 14, ( AN1(N), N = 14, 26 )      06AG4
      PRINT 15, NPRCB, ( AN1(N), N = 1, 26 )      06AG4
C-----INPUT TABLE 1 = CCNTRCL DATA      14JA4
1100 READ 105, KEEPE, KEEP2, KEEP3, KEEP4, NCD2, NCD3, NCD4, KLEAR,      06MY4
    1      KPLCT, KSKIP5, THETA      17NO4
1110 PRINT 100, KEEPE, KEEP2, KEEP3, KEEP4, NCD2, NCD3, NCD4, KLEAR      03AG4
      PRINT 101, KPLCT, KSKIP5, THETA      17NO4
      THETAR = ABSF(THETA) / 57.29578      03JE4
      SKEW = 1.0 / CCSF( THETAR )      22MY4
C-----INPUT TABLE 2 = CCNSTANTS      14JA4
      PRINT 200      01NO3
      IF ( KEEP2 ) 9980, 120C, 129C      29AP4
1200 READ 205, MN, HI, ML, JSTART, JSTOP, MLINC      08JL4
1205 READ 206, NLCADS, ( FACTOR(I), I=1, NLOADS )      01OC318M
      NZ = 3      03AG4
      NZT2 = NZ + NZ      29AG3
      PRINT 220, MN, HI, ML, JSTART, JSTOP      22MY4
      PRINT 225, MLINC      08JL4
      IF ( MLINC ) 9980, 128C, 1285      04AG4
1280 PRINT 940      04AG4
      MLINC = 1      04AG4
1285 PRINT 230, NLCADS, ( FACTOR(I), I = 1, NLOADS )      14JL418M
      JSTART = JSTART + 4      09OC3
      JSTCP = JSTCP + 4      09OC3
      GC TO 1291      06DE3
1290 PRINT 905      01OC3

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1291      H = HI * SKEW
          HT2 = H + H
          HE2 = H * H
          HE3 = H * HE2
          MNP4 = MN + 4
          MNP5 = MN + 5
          MNP6 = MN + 6
          MNP7 = MN + 7
          MLP4 = ML + 4
C-----CLEAR ENVELOPES OF PREVIOUS VALUES
          IF ( KEEPE ) 9980, 1292, 1296
1292      CC 1295  N = 1, NZ
          CC 1295  J = 1, MNP7
          ZMAX(N,J) = 0.C
          ZMIN(N,J) = 0.C
1295      CCNTINUE
1296      CC 1298  K = 1, NZT2
          CC 1298  M = 1, 30
C-----IF A CHANGE IS EVER MADE IN THE DIMENSIONED VALUES OF DESIGN
C        POINTS, THIS DC LCCP MUST BE CHANGED APPROPRIATELY
          ZL(K,M) = 0.0
          JLZL(K,M) = -1
          IZM(K,M) = 0
          CC 1298  I = 1, 6
          ZM(K,M,I) = 0.C
          JLZM(K,M,I) = -1
1298      CCNTINUE
1299      CCNTINUE
C
C-----INPUT TABLE 3 = STATION NUMBER LISTS
          PRINT 300
C-----SKIP TO 1390 IF TABLE 3 IS RETAINED FROM PREVIOUS PCRB
          IF ( KEEP3 ) 9980, 1300, 1350
1300      REAC 305, NLANES, NSTRS, NSUPS, MZ(1), MZ(2)
          PRINT 308, NLANES, NSTRS, NSUPS, MZ(1), MZ(2)
          MZ(3) = NSUPS
          MZ3 = MZ(3)
          PRINT 310
          REAC 315, ( JLEFT(L), L = 1, 10 )
          REAC 315, ( JRIGHT(L), L = 1, 10 )
          REAC 318, ( FSTR(N), N = 1, 30 )
          REAC 315, ( JSUP(NS), NS = 1, 20 )
          REAC 315, ( JZ(1,M), M = 1, 30 )
          REAC 315, ( JZ(2,M), M = 1, 30 )
          PRINT 312, ( JLEFT(L), L = 1, NLANES )
          PRINT 314, ( JRIGHT(L), L = 1, NLANES )
          PRINT 316, ( FSTR(N), N = 1, NSTRS )
          PRINT 320, ( JSUP(NS), NS = 1, NSUPS )
          MZ1 = MZ(1)
          PRINT 322, ( JZ(1,M), M = 1, MZ1 )
          MZ2 = MZ(2)
          PRINT 324, ( JZ(2,M), M = 1, MZ2 )
C-----CONVERT ALL EXTERNAL STATIONS TO INTERNAL STATIONS
          DC 1310  L = 1, NLANES
          JRIGHT(L) = JRIGHT(L) + 4
          JLEFT(L) = JLEFT(L) + 4
22MY4
22MY4
30MY3
30MY3
09SE3
09SE3
25JL6
09SE3
29AG3
05MR4
22MY4
06MY4
28JA4
080C3
080C3
06DE3
22MY4
28JAJDIM
13N03
13DE3
020C3
300C3
020C3
27FE4DIM
020C3
300C3
13DE3
010C3
14JA4
290C3
13JA4
15JL4
04SE3
12N03
03AG4
04AG4
010C3
03AG4DIM
03AG4DIM
05JE4
03AG4DIM
30JA4DIM
30JA4DIM
30JA4IBM
30JA4IBM
05JE4
03FE4
04SE3
290C3IBM
04SE3
290C3IBM
13JA4
24SE3IBM
24SE3
24SE3

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1310	CCONTINUE	24SE3
	DC 1315 N = 1, NSTRS	28JA4 IBM
	FSTR(N) = FSTR(N) + 4	05JE4
1315	CCONTINUE	24SE3
	DC 1321 NS = 1, NSLPS	28JA4
	JSUP(NS) = JSUP(NS) + 4	24SE3
	JZ(3,NS) = JSUP(NS)	03AG4
1321	CCONTINUE	25SE3
	DC 1325 M = 1, MZ1	28JA4 IBM
	JZ(1,M) = JZ(1,M) + 4	25SE3
1325	CCONTINUE	24SE3
	DC 1329 M = 1, MZ2	03AG4 IBM
	JZ(2,M) = JZ(2,M) + 4	24SE3
1329	CCONTINUE	03AG4
	C-----CHECK OF PROXIMITY OF SHEAR STA TO CONCENTRATED LOAD FB&LQWS	11MY4
	NT = 1	11MY4
	NST = 1	11MY4
	MM = 1	10AG4
1330	DC 1349 M = MM, MZ2	10AG4
	C-----PRCXIMITY TO STRINGERS	04AG4
	DC 1343 N = NT, NSTRS	14AG4
	JSTR = FSTR(N)	05JE4
	FSTR1 = JSTR	10JL4
	IF (FSTR(N) - FSTR1) 1331, 1332, 1331	10JL4
1331	NSTA = 2	10JL4
	GC TO 1333	10JL4
1332	NSTA = 1	10JL4
1333	IF (JZ(2,M) - (JSTR - 1)) 1334, 1335, 1342	03AG4
1334	NT = N	10JL4
	GC TO 1344	03AG4
1335	PRINT 930	11MY4
	C-----CMIT ONE SHEAR DESIGN CONTROL POINT IF TOO NEAR A CONC LOAD	04AG4
	IF (MZ2 - M) 999C, 1338, 1336	04AG4
1336	MP1 = M + 1	03AG4
	DC 1337 MT = MP1, MZ2	04AG4
	JZ(2,MT-1) = JZ(2,MT)	03AG4
1337	CCONTINUE	03AG4
	C-----REDUCE NUMBER OF SHEAR DESIGN CONTROL POINTS	04AG4
1338	MZ(2) = MZ2 - 1	04AG4
	MZ2 = MZ(2)	04AG4
	MM = M	18ND4
	GC TO 1330	03AG4
1342	IF (JZ(2,M) - (JSTR + NSTA)) 1335, 1335, 1343	03AG4
1343	CCONTINUE	03AG4
	C-----PRCXIMITY TO SUPPCRTS	04AG4
1344	DC 1348 NS = NST, NSLPS	03AG4
	IF (JZ(2,M) - (JSUP(NS) - 1)) 1345, 1335, 1346	03AG4
1345	NST = NS	03AG4
	GC TO 1349	11MY4
1346	IF (JZ(2,M) - (JSUP(NS) + 1)) 1335, 1335, 1348	06JL4
1348	CCONTINUE	11MY4
1349	CCONTINUE	11MY4
	C-----LANE ROUTINE FCLLCWS	17DE3
	PRINT 11	08MY3 ID
	GC TO 1351	15JL4
1350	PRINT 905	15JL4

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1351      CC 1355  J = 3, MNP5
           LANE(J) = 0
1355      CCNTINUE
C-----CMIT LANE ROUTINE IF ZERO LANES ARE SPECIFIED
           IF ( NLANES ) 9980, 1359, 1356
C-----TEST FOR LANE BOUNDARIES
1356      CC 1358  L = 2, NLANES
           LTEST = JLEFT(L) - JRIGHT(L-1)
           IF ( LTEST ) 1357, 1358, 1358
1357      LLL = L - 1
           PRINT 912, LLL, L
           GC TO 999C
1358      CCNTINUE
C-----TEST FOR LOAD WIDER THAN WIDTH OF LANE
1359      CC 1380  L = 1, NLANES
           LTEST = JRIGHT(L) - JLEFT(L) - ML
           IF ( LTEST ) 1360, 1365, 1365
1360 PRINT 910
           GC TO 999C
1365      JEND = JRIGHT(L) - ML
           JBEGIN = JLEFT(L)
           CO 1370  JL = JBEGIN , JEND
           LANE(JL) = L
1370      CCNTINUE
1380      CCNTINUE
1399      CCNTINUE
C
C-----INPUT TABLE 4 = LCADS AND STIFFNESSES
1400 PRINT 400
           IF (KEEP4) 9980, 1401, 1402
1401      MHOLD1 = 1
1402      CO 1403  J = MHOLD1, MNP7
           F(J) = 0.0
           QSLAB(J) = C.C
           QCAPC(J) = C.C
           QCAPC(J) = C.C
           QL(J) = C.C
1403      CCNTINUE
           IF (KEEP4) 9980, 1406, 1405
1405 PRINT 906
1406      IF ( NCC4 ) 9980, 1407, 1408
1407 PRINT 903
           GC TO 1481
1408      KR2 = 0
           PRINT 410
           PRINT 420
           PRINT 430
           CC 1480  N = 1, NCC4
           KR1 = KR2
           READ 401, IN1, IN2, KR2, FN2, QS2, QC2, QL2
           JN = IN1 + 4
           J2 = IN2 + 4
           KSW = 1 + KR2 + 2 * KR1
           GC TO ( 1409, 1410, 1415, 1415 ), KSW
1409 PRINT 402, IN1, IN2, KR2, FN2, QS2, QC2, QL2
           GC TO 142C

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15JL4
29AG3
04SE3
13JA4
29AP4
13JA4
09JA418M
16SE3
03DE3
13DE3
01N03
29AP4
09SE3
13JA4
13DE318M
09SE3
03DE3
30OC3
29AP4
04SE3
29AG3
04SE3
29AG3
04SE3
04SE3
29AG3
-
14JA4
04JE3
17JL6
10JE6
17JL6
30MY3
30AG3
22MY4
22MY4
30AG3
17JL6
17JL6
17JL6
17JL6
17JL6
21JL6
17JL6
05SE3
05SE3
05SE3
28JA4
28MY3
20SE3
28MY3
28MY3
28MY3
17JL6
17JL6
04JE3

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1410 PRINT 404, IN1, KR2, FN2, CS2, QC2, QL2      24SE3
      GC TO 1420                                04JE3
1415 PRINT 406, IN2, KR2, FN2, CS2, QC2, QL2      09SE3
      GC TO 1435                                04JE3
1420      J1 = JN                               04JE3
1425      FN1 = FN2                             04JE3
      QS1 = QS2                                 09SE3
      QC1 = QC2                                 30AG3
      QL1 = QL2                                 30AG3
      GC TO (1435, 1480, 9580, 1480), KSW      10JE6
1435      JINCR = 1                             07JE3
      ESM = 1.0                                 07JE3
      IF (J2 - J1) 1437, 1450, 1440            10JE6
1437 PRINT 908                                  10JE6
      GC TO 9990                                  10JE6
1440      DENCN = J2 - J1                       07JE3
      ISW = 1                                    07JE3
      GC TO 1455                                  07JE3
1450      F(J1) = F(J1) + FN1                   22MY4
      QSLAB(J1) = QSLAB(J1) + QS1              22MY4
      QCAPC(J1) = QCAPC(J1) + QC1              22MY4
      QL(J1) = QL(J1) + QL1                    22MY4
      GC TO 1470                                  22MY4
1455      DC 1460 J = J1, J2, JINCR             04JE3
      DIFF = J - J1                             28MY3
      PART = DIFF / DENCN                       28MY3
      F(J) = F(J) + ( FN1 + PART * ( FN2 - FN1 ) ) * ESM 28MY3
      QSLAB(J) = QSLAB(J) + ( QS1 + PART * ( QS2 - QS1 ) ) * ESM 30AG3
      QCAPC(J) = QCAPC(J) + ( QC1 + PART * ( QC2 - QC1 ) ) * ESM 22MY4
      QL(J) = QL(J) + ( QL1 + PART * ( QL2 - QL1 ) ) * ESM 17SE3
1460      CCNTINUE                               04JE3
      IF ( ISW ) 9980, 1470, 1465              28AG4
1465      JINCR = J2 - J1                       07JE3
      ESM = - 0.5                               07JE3
      ISW = 0                                    28MY3
      GC TO 1455                                  04JE3
1470      GC TO (1480, 9980, 1480, 1475), KSW  10JE6
1475      J1 = J2                                04JE3
      GC TO 1425                                  04JE3
1480      CCNTINUE                               04JE3
C-----TEST FOR DATA ERRONEOUSLY STORED BEYOND END STA 21JL6
1481      IF ( F(MNP5) + F(MNP6) + QSLAB(MNP5) + QCAPC(MNP5)
1          + QCAPC(MNP5) + QL(MNP5) ) 1485, 1499, 1485 21JL6
1485 PRINT 909                                  21JL6
      GC TO 9990                                  21JL6
1499      CCNTINUE                               04JE3
C
C-----CLEAR MOVABLE-LOAD VALUES FROM TEMPORARY LIVE LOAD STORAGE 13JA4
2000      DC 2010 J = 4, MNP4                   09SE3
      QLIVE(J) = 0.0                             29AG3
2010      CCNTINUE                               29AG3
C
C-----COMPUTE EFFECTS OF DEAD LOAD ONLY 160C3
      CALL BEAM ( NSTRS, MN, MNP4, MNP5, H, HE2, HE3, HT2, NZ, SKEW,
1          KFLAG )                               03AG4
      03AG4
C

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C-----	SKIP TO CARD EJECT ROUTINE IF KFLAG = 1 FROM SUBROUTINE BEAM	04AG4
	IF (KFLAG) 999C, 210C, 999C	03AG4
2100	CC 2190 N = 1, NZ	03AG4
	MZN = MZ(N)	160C3
	CC 2180 M = 1, MZN	160C3IBM
	J = JZ(N,M)	160C3
	ZD(N,M) = Z(N,J)	160C3
	IF (ABSF(ZD(N,M)) - C.C01) 2170, 2170, 2180	08JL4
2170	ZD(N,M) = 0.0	08JL4
2180	CCONTINUE	160C3
2190	CCONTINUE	160C3
C-----	CCMPUTE EFFECTS OF DEAD LOAD PLUS SINGLE LOAD MOVED ACROSS SLAB	160C3
	IF (JSTCP - JSTART) 2999, 2200, 2200	01N03
2200	CC 2950 JL = JSTART, JSTOP, PLINC	08JL4
	CC 2210 JLINC = 1, PLINC	13JL4
	QLIVE(JL - JLINC) = 0.0	13JL4
2210	CCONTINUE	13JL4
C-----	SET LIVE LOAD EQUAL TO SINGLE MOVABLE LOAD	13JA4
	CC 2220 LL = 4, MLP4	29JA4
	J = JL + LL - 4	29AG3
	QLIVE(J) = CL(LL)	29AG3
2220	CCONTINUE	160C3
C		
C-----	CCMPUTE EFFECT OF RANDOM POSITION ONE-TRUCK LOADING	14JA4
	CALL BEAM (NSTRS, MN, MNP4, MNP5, H, HE2, HE3, HT2, NZ, SKEW,	03AG4
	1 KFLAG)	03AG4
C		
C-----	SKIP TO CARD EJECT ROUTINE IF KFLAG = 1 FROM SUBROUTINE BEAM	04AG4
	IF (KFLAG) 999C, 2530, 999C	03AG4
C-----	ARRANGE CONTROL TABLE FOR MULTI-LANE LOADINGS	14JA4
C-----	SIGN TAKES CARE OF + AND - DESIGN VARIABLES	29AP4
2530	SIGN = -1.0	20SE3
	CC 2900 K = 1, NZT2	20SE3
	N = (K + 1) / 2	30JA4
	SIGN = -1.0 * SIGN	30AG3
	MZN = MZ(N)	070C3
	CC 2800 M = 1, MZN	28JA4IBM
	J = JZ(N,M)	30AG3
	ZLIVE = Z(N,J) - ZD(N,M)	160C3
	IF (ABSF(ZLIVE) - C.C01) 2800, 2800, 2610	04MY4
2610	TEST = SIGN * (ZLIVE - ZL(K,M))	06MY4
	IF (TEST) 2650, 2650, 2620	030E3
2620	ZL(K,M) = ZLIVE	160C3
	JLZL(K,M) = JL	30AG3
2650	IF (LANE(JL)) 999C, 2800, 2655	28AG4
2655	IF (NLCADS) 9980, 2800, 2660	29AP4
2660	CC 2670 I = 1, NLCADS	28JA4IBM
	JI = JLZM(K,M,I)	110C3
	LTEST = LANE(JL) - LANE(JI)	110C3
	IF (LTEST) 2670, 2680, 2670	030E3
2670	CCONTINUE	110C3
	ZM(K,M,NLCADS+1) = ZLIVE	160C3
	JLZM(K,M,NLCADS+1) = JL	100C3
	IR1 = 1	110C3
	CC TO 270C	110C3
2680	ZTEST = SIGN * (ZLIVE - ZM(K,M,I))	160C3

2690	IF (ZTEST) 2800, 28CC, 269C	03DE3
	ZM(K,M,I) = ZLIVE	160C3
	JLZM(K,M,I) = JL	110C3
	IR1 = NLCADS + 2 - I	110C3
2700	CC 2799 IR = IR1, NLCADS	28JA418M
	I = NLOACS - IR + 1	30AG3
	TEST = SIGN * (ZM(K,M,I+1) - ZM(K,M,I))	30AG3
2720	IF (TEST) 2800, 2800, 2720	29JA4
	ZTEMP = ZM(K,M,I)	30AG3
	ZM(K,M,I) = ZM(K,M,I+1)	30AG3
	ZM(K,M,I+1) = ZTEMP	30AG3
	JLTEMP = JLZM(K,M,I)	30AG3
	JLZM(K,M,I) = JLZM(K,M,I+1)	30AG3
	JLZM(K,M,I+1) = JLTEMP	30AG3
2799	CCNTINUE	30AG3
2800	CCNTINUE	30AG3
2900	CCNTINUE	30AG3
2950	CCNTINUE	13DE3
2999	CCNTINUE	30AG3
C		
C-----ESTABLISH CRITICAL PATTERN OF MULTI-LANE LOADING		14JA4
3000	SIGN = -1.0	30AG3
	CC 3999 K = 1, NZT2	28JA4
	N = (K + 1) / 2	30JA4
	SIGN = -1.0 * SIGN	30AG3
	MZN = MZ(N)	30AG3
3100	CC 3900 M = 1, MZN	28JA418M
	SUM = 0.C	30AG3
C-----IF KLEAR = 1, EXCLUDE ONE-TRUCK INFLUENCE		14JA4
	IF (KLEAR) 998C, 315C, 316C	29AP4
3150	Z1 = ZL(K,M)	160C3
	GC TO 3200	160C3
3160	Z1 = 0.0	160C3
3200	CC 3800 I = 1, NLCADS	28JA418M
	SUM = SUM + ZM(K,M,I)	30AG3
	Z2 = FACTOR(I) * SUM	30AG3
	TEST = SIGN * (Z2 - Z1)	30AG3
	IF (TEST) 3800, 38CC, 3210	090C3
3210	I ZM(K,M) = I	30AG3
	Z1 = Z2	160C3
3800	CCNTINUE	30AG3
3900	CCNTINUE	30AG3
3999	CCNTINUE	30AG3
C		
C-----APPLY MULTI-LANE LOADINGS		14JA4
4000	IF (KLEAR) 998C, 430C, 401C	29AP4
4010	CC 4200 J = 3, MNP5	09SE3
	CC 4100 N = 1, NZ	09SE3
	ZMAX(N,J) = 0.C	30AG3
	ZMIN(N,J) = 0.C	30AG3
4100	CCNTINUE	30AG3
4200	CCNTINUE	30AG3
4300	CC 4999 K = 1, NZT2	30AG3
	N = (K + 1) / 2	30AG3
	MZN = MZ(N)	210C3
	CC 4900 M = 1, MZN	210C318M

	CC 4400 J = 3, MNP5	02JA4
	QLIVE(J) = 0.0	30AG3
4400	CCONTINUE	30AG3
	IM = IZP(K,P)	30AG3
	CC 4800 I = 1, IM	30AG3
	JL = JLZP(K,M,I)	30AG3
	CC 4700 LL = 4, MLP4	30AG3
	J = JL + LL - 4	30AG3
	QLIVE(J) = QLIVE(J) + QL(LL) * FACTOR(IM)	210C3
4700	CCONTINUE	30AG3
4800	CCONTINUE	30AG3
C		
C-----	COMPUTE EFFECTS OF CRITICAL MULTI-LANE LOADING PATTERNS	13JA4
	CALL BEAM (NSTRS, MN, MNP4, MNP5, H, HE2, HE3, HT2, NZ, SKEW,	03AG4
1	KFLAG)	03AG4
C		
C-----	SKIP TO CARD EJECT ROUTINE IF KFLAG = 1 FROM SUBROUTINE BEAM	04AG4
	IF (KFLAG) 999C, 450C, 999C	03AG4
4900	CCONTINUE	30AG3
4999	CCONTINUE	30AG3
C		
	PRINT 11	08MY3 ID
	PRINT 16, NPRCB, (AN1(N), N = 1, 26)	06AG4
C-----	CLTPLT TABLE 5 - MULTI-LANE LOADING CONTROL	18N03
	PRINT 500	21JL6
	IF(KSKIP5 + 1) 5595, 5505, 5508	17N064
5505	PRINT 907	17N064
	GC TO 5595	17N064
5508	NCNE = 0	17N064
	CC 5590 N = 1, NZ	28JA4
	GC TO (5510, 5520, 5525), N	03AG4
5510	PRINT 510	18N03
	GC TO 5530	130C3
5520	PRINT 520	18N03
	GC TO 5530	03AG4
5525	PRINT 525	03AG4
5530	PRINT 530	18N03
	K1 = 2 * N - 1	130C3
	K2 = 2 * N	130C3
	MZN = MZ(N)	130C3
	CC 5580 M = 1, MZA	130C3IBM
	JZNM = JZ(N,M) - 4	130C3
	PRINT 540, JZNP, ZD(N,P)	18N03
	J1 = JLZL(K1,P)	130C3
	J2 = JLZL(K2,P)	130C3
	LANE1 = LANE(J1)	130C3
	LANE2 = LANE(J2)	130C3
5540	IF (J1) 5541, 5542, 5542	280C3
5541	IF (J2) 5545, 5543, 5543	280C3
5542	IF (J2) 5546, 5544, 5544	280C3
5543	J2 = J2 - 4	300C3
	PRINT 554, NCNE, ZL(K1,P), NCNE, ZL(K2,M), LANE2, J2	18N03
	GC TO 5550	280C3
5544	J1 = J1 - 4	01N03
	J2 = J2 - 4	300C3
	PRINT 550, NCNE, ZL(K1,P), LANE1, J1, NCNE, ZL(K2,M), LANE2, J2	18N03


```

NAME(3) = B+S+EAR
IUNIT(1) = 5H FT
IUNIT(2) = 5H FT-K
IUNIT(3) = 5H K
PRINT 830
5700 IF ( KPLCT ) 5980, 5990, 5710
C-----SNIP TO 5990 IF NC PLOTS ARE DESIRED
5710 KSTCP = 1
      HMN = MN
      DIST = H * HMN
5711 IF ( DIST - 50. ) 5711, 5711, 5712
      HEND = 50.
GC TO 5740
5712 IF ( DIST - 100. ) 5715, 5715, 5721
5715 HEND = 100.
GC TO 5740
5721 IF ( DIST - 200. ) 5725, 5725, 5731
5725 HEND = 200.
GC TO 5740
5731 IF ( DIST - 500. ) 5735, 5735, 5736
5735 HEND = 500.
GC TO 5740
5736 HEND = 1000.
5740 HPOS = 10.
CC 5745 J = 4, MNP4
      XJ = J - 4
      HPLCT(J-3) = XJ * H
5745 CCNTINUE
PRINT 831, NAME(1), HPCS, HEND, IUNIT(1)
CC 5870 N = 1, 2
      VMAX = 0.0
C-----FIND LARGEST VALUE OF DESIGN VARIABLE
CC 5750 J = 4, MNP4
      ZTMP = ABSF( ZMIN(N,J) )
      VMAX = MAXIF( ZMAX(N,J), ZTMP, VMAX )
5750 CCNTINUE
C-----DETERMINE EXPONENT FOR DESIGN VARIABLE EQUAL OR LESS THAN UNITY
      KEXP = 0
5752 IF ( VMAX ) 9980, 5780, 5752
      CC 5755 NE = 1, 100
5753 IF ( 1.0 - VMAX ) 5753, 5780, 5760
      VMAX = VMAX / 10.
      KEXP = KEXP + 1
5755 CCNTINUE
PRINT 920
GC TO 9980
5760 IF ( 0.1 - VMAX ) 5765, 5770, 5762
5762 VMAX = VMAX * 10.0
      KEXP = KEXP - 1
GC TO 5755
5765 IF ( 0.2 - VMAX ) 5766, 5775, 5775
5766 IF ( 0.4 - VMAX ) 5766, 5778, 5778
5770 VENC = 0.1
GC TO 5790
5775 VENC = 0.2
GC TO 5790

```

```

14MY4
02JL4
02JL4
02JL4
03AG4
04MY4
17JA4
20AP4
30JE4
30JE4
30JE4
30JE4
30JE4
25MY4
30JE4
30JE4
16JA4
06JL4
06JL4
20MR4
30JE4
30JE4
30JE4
30JE4
30JE4
01JL4
02JL4
02JL4
30JE4
04AG4
03AG4
16AP4
17JA4
20AP4
28JA4
16AP4
16JA4
29AP4
16AP4
29AP4
23MR4
16AP4
16AP4
16AP4
16JA4
18ND4
29AP4
16AP4ARB
16AP4
16AP4
23MR4
16AP4ARB
16AP4ARB
16AP4
23MR4
16AP4
16JA4

```

```

5778      VENC = 0.4                                16AP4
      GC TO 5790                                    16JA4
5780      VENC = 1.0                                16AP4
C-----SET VALUE OF DESIGN VARIABLE AT END OF VERTICAL AXIS 29AP4
5790      VENC = VENC * 1C**KEXP                    16AP4
C-----SET DESIGN VARIABLE AXIS LENGTH ( IN INCHES )        29AP4
      VPOS = 2.0                                    16AP4
C-----TRANSFER POSITIVE DESIGN VARIABLE TO PLOT NOTATION 17JA4
      DC 5795 J = 4, MNP4                            16AP4
      VPLCT(J-3) = ZMAX(N,J)                        16AP4
5795      CCNTINUE                                  16JA4
      HNEG = 0.0                                    16AP4
      SPACE = C.5                                  16AP4
      VEND = VENC * 1.5                             16JE4
      VPOS = VPOS * 1.5                             16JE4
      VNEG = VPCS                                    16AP4
      HTCKS = HENC                                   16AP4
      VTCKS = VENC                                   16AP4
      KAXES = 24                                    16AP4
C-----TRANSFER AXES INFCRMATION -- DO NOT DRAW AXES      04AG4
C
      CALL AXES ( HEND, VPCS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS, 16AP4
1          VTCKS, KAXES )                            16AP4
C
      IPRCB(1) = 8H  PROB                            04AG4
      IPRCB(2) = NPRCB                               01JL4
      NSYMB = 16                                     01JL4
      ICR = 0                                         01JL4
      ISIZE = 2                                       01JL4
      HCOR = 2.0                                     01JL4
      VCCR = -3.0                                    01JL4
C-----PRINT PROBLEM NUMBER BELCW EACH PLOT                04AG4
C
      CALL PLOTTITL ( IPRCB, NSYMB, ICR, ISIZE, HCOR, VCCR ) 04AG4
C
      VENC = VENC / 1.5                               16JE4
      VPOS = VPOS / 1.5                               16JE4
C-----CCMPLTE DESIGN CONTRCL PCINTS                       17JA4
      SIGN = +1.0                                    16JA4
      MZN = MZ(N)                                    16JA4
5800      DC 5810 M = 1, MZN                          30JE4
      XJ = JZ(N,M) - 4                               02JL4
      HJZ(M) = XJ * F                                02JL4
      VJZ(M) = VENC * SIGN                           16AP4
5810      CCNTINUE                                  16JA4
C-----PLCT DESIGN CONTRCL POINTS AND DESIGN VARIABLES    17JA4
      KPS = 2                                         20AP4
      NUMPTS = MZN                                   29AP4
      CALL PLOT ( HJZ, VJZ, NUMPTS, KPS )            29AP4
      KPS = 9                                         20AP4
      NUMPTS = MN + 1                                17AP4
      CALL PLOT ( HPLCT, VPLCT, NUMPTS, KPS )        17AP4
C
      IF ( SIGN ) 5E40, 55E0, 5820                    06MY4
C-----TRANSFER NEGATIVE DESIGN VARIABLE TO PLOT NOTATION 17JA4
5820      DC 5830 J = 4, MNP4                        16AP4

```

	VPLCT(J-3) = ZMIN(N,J)	16AP4
5830	CCONTINUE	27JA4
	SIGN = -SIGN	16JA4
	GC TO 5800	16JA4
C-----	RGLL PAPER FOR NEW DESIGN VARIABLE	17JA4
5840	CALL AXESTERM (1)	23MR4
	CALL AXESTERM (1)	15MY4
C-----	PRINT LINES OF OUTPUT IN TABLE 7	15MY4
	PRINT 831, NAME(N+1), VPCS, VEND, IUNIT(N+1)	03AG4
5870	CCONTINUE	27JA4
	GC TO 5999	17JA4
5990	PRINT 832, NPRCB	03AG4
5999	CCONTINUE	17JA4
C		
	CALL TIME	17AP4
	MHOLD1 = MNP5	10JE6
C-----	RETURN FOR NEW PROBLEM	14JA4
	GC TO 1010	26AG3 ID
9980	PRINT 980	15MY4
9990	KFLAG = 0	03AG4
	NPROBT = NPRCB	04AG4
	CC 9991 M = 1, 1000	03AG4
	READ 12, NPROB, (AN1(N), N=1,13)	04AG4
	IF (NPROB-NPRCBT) 9992, 9991, 9992	04AG4
9991	CCONTINUE	03AG4
	GC TO 999C	10AG4
9992	IF (NPRCB - ITEST) 9993, 9999, 9993	04AG4
9993	READ 14, (AN1(N), N = 14, 26)	06AG4
	READ 105, KEEPE, KEEP2, KEEP3, KEEP4, NCD2, NCD3, NCD4, KLEAR,	06AG4
1	KPLCT, KSKIP5, THETA	18N04
	KSUM = KEEPE + KEEP2 + KEEP3 + KEEP4	03AG4
	IF (KSUM) 9997, 9994, 9997	10AG4
9994	PRINT 15, NPRCB, (AN1(N), N = 1, 26)	06AG4
	GC TO 1110	06AG4
9997	PRINT 950, NPRCB, NPRCBT, NPROB	10AG4
	GC TO 999C	10AG4
9995	IF (KSTGP) 9995, 9995, 9996	03AG4
9996	CALL AXESTERM (0)	03AG4
9999	CCONTINUE	03AG4
	CALL TIME	17AP4
	END	04MA3 ID
C		

```

SUBROUTINE BEAM ( NSTRS, MN, MNP4, MNP5, H, HE2, HE3, HT2, NZ, 03AG4
1          SKEW, KFLAG ) 03AG4
C
  DIMENSION A(307), R(307), C(307), F(307), W(307), 10AP4
1          Q(307), QLIVE(307), QSLAB(307), QCAPD(307), CCAPC(307), 22MY4
2          Z(3,307), ZMAX(3,307), ZMIN(3,307), 03AG4
3          FSTR(30), JSUP(20) 04AG4
  COMMON QLIVE, QSLAB, CCAPC, QCAPC, FSTR, JSUP, F, ZMAX, ZMIN, Z 03JE4
908 FCRMAT (///45H  ERRRCR -- LESS THAN 2 STRINGERS INPUT  /// )06SE3
930 FCRMAT (///31H  ILLEGAL LCAD PLACED AT STA, I4, 18MY4
1          25H  PRCBLEM TERMINATED ) 14AG4
990 FCRMAT (///5CH  UNDESIGNATED ERROR STOP IN SUBROUTINE )18NO4
C-----DISTRIBUTE LCAD DIRECTLY TO CAP IF NUM STRINGERS = 0 14JA4
          Q(3) = 0.0 13MY4
          Q(MNP5) = 0.0 15MY4
6000 IF ( NSTRS ) 8980, 6100, 6200 04MY4
6100 DC 6110 J = 4, MNP4 29JA4
          Q(J) = QLIVE(J) + QSLAB(J) + QCAPD(J) * SKEW + QCAPC(J) 22MY4
6110 CCNTINUE 29AG3
          GC TO 6299 29AG3
C-----LCAD DISTRIBUTION THRU STRINGERS 34MY4
6200 DC 6205 J = 4, MNP4 29JA4
          Q(J) = QCAPC(J) * SKEW + QCAPC(J) 22MY4
6205 CCNTINUE 010C3
          IF ( NSTRS - 2 ) 6210, 6220, 6230 29JA4
6210 PRINT 908 10SE3
          GC TO 8999 12SE3
6220 KSW = 2 29AG3
          NI = 1 29AG3
          NF = 1 29AG3
          JA = 4 29AG3
          JB = MNP4 10SE3
          GC TO 6240 29AG3
6230 KSW = 1 10JE4
          NI = 1 03JE4
          NF = NSTRS - 2 03JE4
          JA = 4 03JE4
          JB = FSTR(2) 03JE4
6240 CCNTINUE 03JE4
          DC 6260 N = NI, NF 03JE4
          F1 = FSTR(N) 03JE4
          F2 = FSTR(N+1) 03JE4
          CC 6250 J = JA, JB 03JE4
          FJ = J 03JE4
          QJ = QSLAB(J) + QLIVE(J) 03JE4
          DIFF = F2 - FJ 03JE4
          DENCM = F2 - F1 03JE4
          PART1 = DIFF / DENCM 03JE4
          PART2 = 1. - PART1 03JE4
          J1 = F1 03JE4
          FJ = J1 03JE4
          PART4 = F1 - FJ 03JE4
          PART3 = 1. - PART4 03JE4
          Q(J1) = Q(J1) + PART3 * ( PART1 * QJ ) 03JE4
          Q(J1+1) = Q(J1+1) + PART4 * ( PART1 * QJ ) 09JE4
          J2 = F2 09JE4

```

```

        FJ2 = J2
        PART6 = F2 - FJ2
        PART5 = 1. - PART6
        Q(J2) = C(J2) + PART5 * ( PART2 * QJ )
        Q(J2+1) = Q(J2+1) + PART6 * ( PART2 * QJ )
6250    CCNTINUE
        JA = FSTR(N+1) + 1
        JB = FSTR(N+2)
6260    CCNTINUE
        GC TO ( 6280, 6299 ), KSW
6280    KSW = 2
        NI = NSTRS - 1
        NF = NSTRS
        JA = FSTR(NI) + 1
        JB = MNP4
        GC TO 624C
6299    CCNTINUE
7000    NS = 1
        CC 7160 J = 3, MNPE
C-----PREVENT LOADING ANY F-INGE
        IF ( F(J) ) 7102, 7C1C, 7102
7010    IF ( Q(J) ) 702C, 7102, 702C
7020    ISTA = J - 4
        PRINT 930, ISTA
        GC TO 8999
C-----ZERO INITIAL VALUES AND SET CONTINUITY COEFFS AT SUPPORT STAS
7102    A(1) = 0.0
        A(2) = 0.0
        B(1) = 0.0
        B(2) = 0.0
        C(1) = 0.0
        C(2) = 0.0
        IF ( J - JSUP(NS) ) 7104, 7103, 7104
7103    C(J) = 0.0
        B(J) = 0.0
        A(J) = 0.0
        NS = NS + 1
        GC TO 716C
C-----COMPLETE MATRIX COEFFS
7104    AA = F(J-1)
        BB = - 2.0 * ( F(J-1) + F(J) )
        CC = F(J-1) + 4.0 * F(J) + F(J+1)
        DD = - 2.0 * ( F(J) + F(J+1) )
        EE = F(J+1)
        FF = F3 * C(J)
        E = AA * B(J-2) + BB
        DENCM = E + B(J-1) + AA * C(J-2) + CC
        IF ( DENCM ) 711C, 7105, 711C
7105    D = 0.0
        GC TO 7115
C-----COMPLETE CONTINLITY CCEFFS
7110    D = - 1.0 / DENCM
7115    C(J) = D * EE
        B(J) = C * ( E * C(J-1) + DC )
        A(J) = D * ( E * A(J-1) + AA * A(J-2) - FF )
7160    CCNTINUE

```



```

C-----CGMPUTE DEFLECTION W(J)
      W(MNPS + 1) = C.C
      W(MNPS + 2) = C.C
      CC 7200 L = 3, MNPS
      J = PA + 8 - L
      W(J) = A(J) + B(J) * W(J+1) + C(J) * W(J+2)
7200  CCNTINUE
      BMJ = 0.C
      BMK = 0.C
C-----CCMPUTE BENDING MOMENTS SHEARS AND REACTIONS AT EACH STA J
8000  CC 8200 J = 3, MNPS
      BMI = BMJ
      BMJ = BMK
      BMK = F(J+1) * ( h(J) - 2.0 * W(J+1) + W(J+2) ) / HE2
      DBM = ( - BMI + BMK ) / HT2
      REACT = ( BMI - 2.0 * BMJ + BMK ) / H - Q(J)
      Z(1,J) = BMJ
      Z(2,J) = CBM
      Z(3,J) = REACT
C-----HCLC MAX AND MIN DESIGN VARIABLES AT EACH STATION J
      CC 8100 N = 1, NZ
      ZMAX(N,J) = MAX1F( Z(N,J), ZMAX(N,J) )
      ZMIN(N,J) = MIN1F( Z(N,J), ZMIN(N,J) )
8100  CCNTINUE
8200  CCNTINUE
      RETURN
8580 PRINT 990
8559 KFLAG = 1
      RETURN
      END
      FINIS
-EXECUTE,,,1.

```

Add data cards here. The first card of data is the first card described in the Guide for Data Input for CAP 14.

When plot capabilities are not available by library tape routine, a plot subroutine must be properly added. A binary deck of Subroutine PLOT 63 is to be provided with all CAP 14 decks.

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APPENDIX 4

LISTING OF INPUT DATA FOR EXAMPLE PROBLEMS

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SOL 1          EXAMPLE CAP - NO SKEW,
SOL 1          STRINGERS POSITIONED AT NEAREST WHOLE STATIONS
SOL 1          0      0      0      0      2      14      17      0      1      0      0.000E+00
SOL 1          136 0.500E+00          20      4      112      1
SOL 1          4 1.000E+00 1.000E+00 0.900E+00 0.750E+00
SOL 1          4      8      4      10      11
SOL 1          4      34      72      102
SOL 1          34      64      102      132
SOL 1          8      25      42      59      77      94      111      128
SOL 1
SOL 1
SOL 1          14      50      86      122
SOL 1
SOL 1          14      25      42      50      59      77      86      94      111      122
SOL 1
SOL 1
SOL 1          12      16      34      48      52      68      84      88      102      120
SOL 1          124
SOL 1
SOL 1          6          1 1.000E+05          -5.000E-01
SOL 1          12          1 1.000E+06          -1.000E+00
SOL 1          124          1 1.000E+06          -1.000E+00
SOL 1          130          0 1.000E+05          -5.000E-01
SOL 1          8      8      0          -5.000E+01
SOL 1          25      25      0          -5.000E+01
SOL 1          42      42      0          -5.000E+01
SOL 1          59      59      0          -5.000E+01
SOL 1          77      77      0          -5.000E+01
SOL 1          94      94      0          -5.000E+01
SOL 1          111     111      0          -5.000E+01
SOL 1          128     128      0          -5.000E+01
SOL 1          0      136      0          -5.000E-01
SOL 1          0      4      0          -5.000E-01
SOL 1          64      72      0          -5.000E-01
SOL 1          132     136      0          -5.000E-01
SOL 1          0      20      0          -5.000E+00
SOL 2          EXAMPLE CAP - NO SKEW, SAME AS SOLUTION 1 EXCEPT
SOL 2          STRINGERS INPUT AT FRACTIONAL STATIONS
SOL 2          0      0      0      0      2      14      25      0      1      0      0.000E+00
SOL 2          136 0.500E+00          20      4      112      1
SOL 2          4 1.000E+00 1.000E+00 0.900E+00 0.750E+00
SOL 2          4      8      4      10      11
SOL 2          4      34      72      102
SOL 2          34      64      102      132
SOL 2          8.1 25.2 42.3 59.4 76.6 93.71110.8127.9
SOL 2
SOL 2
SOL 2          14      50      86      122
SOL 2
SOL 2          14      25      42      50      59      77      86      94      111      122
SOL 2
SOL 2
SOL 2          12      16      34      48      52      68      84      88      102      120
SOL 2          124
SOL 2

```

SOL 2	6	1	1.000E+05		-5.000E-01	
SOL 2		12	1	1.000E+06	-1.000E+00	
SOL 2		124	1	1.000E+06	-1.000E+00	
SOL 2		130	0	1.000E+05	-5.000E-01	
SOL 2	8	8	0		-4.500E+01	
SOL 2	9	9	0		-5.000E+00	
SOL 2	25	25	0		-4.000E+01	
SOL 2	26	26	0		-1.000E+01	
SOL 2	42	42	0		-3.500E+01	
SOL 2	43	43	0		-1.500E+01	
SOL 2	59	59	0		-3.000E+01	
SOL 2	60	60	0		-2.000E+01	
SOL 2	76	76	0		-2.000E+01	
SOL 2	77	77	0		-3.000E+01	
SOL 2	93	93	0		-1.500E+01	
SOL 2	94	94	0		-3.500E+01	
SOL 2	110	110	0		-1.000E+01	
SOL 2	111	111	0		-4.000E+01	
SOL 2	127	127	0		-5.000E+00	
SOL 2	128	128	0		-4.500E+01	
SOL 2	0	136	0	-5.000E-01		
SOL 2	0	4	0	-5.000E-01		
SOL 2	64	72	0	-5.000E-01		
SOL 2	132	136	0	-5.000E-01		
SOL 2	0	20	0			-5.000E+00
SOL 3	EXAMPLE CAP - SAME AS SOLUTION 2 EXCEPT					
SOL 3	SKEW = 30.0 DEGREES					
SOL 3	0	1	1	1	0	0
SOL 3					0	0
SOL 3						1
SOL 3						0
SOL 3						3.000E+01

APPENDIX 5
COMPUTED RESULTS FOR EXAMPLE PROBLEMS

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TABLE 4 -- CAP STIFFNESS, AND DATA FOR BOTH FIXED AND MOVABLE LOADS

FIXED-CR-MOVABLE			FIXED-POSITION DATA			MOVABLE-POSITION	
STA FROM	STA TO	CONT IF=1	CAP BENDING STIFFNESS (K-FT*FT)	SIDEWALK, SLAB LCADS (K)	STRINGER, CAP LCADS (K)	SLAB LOADS (K)	
6		1	1.000E C5	0	-5.000E-C1	0	0
	12	1	1.000E C6	0	-1.000E C0	0	0
	124	1	1.000E C6	0	-1.000E C0	0	0
	130	0	1.000E C5	0	-5.000E-C1	0	0
8	8	0	0	0	-5.000E C1	0	0
25	25	0	0	0	-5.000E C1	0	0
42	42	0	0	0	-5.000E C1	0	0
59	59	0	0	0	-5.000E C1	0	0
77	77	0	C	0	-5.000E C1	0	0
94	94	0	C	0	-5.000E C1	0	0
111	111	0	C	0	-5.000E C1	0	0
128	128	0	0	0	-5.000E C1	0	0
0	136	0	C	-5.000E-01	0	0	0
0	4	0	C	-5.000E-01	0	0	0
64	72	0	0	-5.000E-01	0	0	0
132	136	0	0	-5.000E-01	0	0	0
0	20	0	C	0	0	-5.000E 00	0

PROE (CCATC)

SCL 1

EXAMPLE CAP - NO SKEW,
STRINGERS POSITIONED AT NEAREST WHOLE STATIONS

TABLE 5 -- MULTI-LANE LOADING SUMMARY

(*---CRITICAL NUMBER OF LANE LOADS)

MOMENT (FT-K)	LANE	POSITIVE	LOAD AT	LANE	NEGATIVE	LOAD AT
AT DEAD LD	ORDER	MAXIMUM	LANE STA	ORDER	MAXIMUM	LANE STA
STA EFFECT						
14	-1.971E 02					
	C	0		0	-1.941E 02	1 4
	1	0		1	-1.941E 02	1 4
	2	0		2	0	
	3	0		3	0	
	4	0		4	0	
	C*			0*		
25	1.381E 02					
	C	2.378E 02	0 18	0	-3.110E 01	0 57
	1	2.164E 02	1 14	1	-1.183E 01	3 72
	2	7.733E 01	2 34	2	-3.655E-01	4 112
	3	7.383E 00	4 102	3	-3.095E-01	2 44
	4	4.029E 00	3 82	4	0	
	2*			0*		
42	5.388E 01					
	0	1.305E 02	0 29	0	-7.918E 01	0 57
	1	1.168E 02	2 34	1	-3.010E 01	3 72
	2	3.585E 01	1 14	2	-5.304E-01	4 112
	3	1.875E 01	4 102	3	-7.878E-01	2 44
	4	1.026E 01	3 82	4	0	
	3*			0*		
50	-2.697E 02					
	C	3.443E 01	0 93	0	-1.427E 02	0 31
	1	2.416E 01	4 102	1	-1.422E 02	2 34
	2	1.319E 01	3 82	2	-5.676E 01	1 14
	3	4.790E 00	1 4	3	-3.871E 01	3 72
	4	0		4	-1.196E 00	4 112
	3*			3*		
59	6.439E 01					
	0	1.527E 02	0 52	0	-5.478E 01	0 23
	1	1.083E 02	2 44	1	-6.653E 01	1 14
	2	2.072E 01	3 72	2	-3.190E 01	2 34
	3	3.294E 00	1 4	3	-6.069E 00	4 102
	4	3.004E-01	4 112	4	-1.840E 00	3 82
	C*			2*		
77	6.439E 01					
	C	1.527E 02	0 64	0	-5.478E 01	0 93
	1	1.083E 02	3 72	1	-6.653E 01	4 102
	2	2.072E 01	2 44	2	-3.190E 01	3 82
	3	3.294E 00	4 112	3	-6.069E 00	1 14

		4	3.004E-01	1	4	4	-1.840E 00	2	34
		C*				2*			
86	-2.697E 02								
		C	3.443E 01	0	23	0	-1.427E 02	0	85
		1	2.416E 01	1	14	1	-1.422E 02	3	82
		2	1.319E 01	2	34	2	-9.676E 01	4	102
		3	4.790E 00	4	112	3	-3.871E 01	2	44
		4	0			4	-1.196E 00	1	4
		3*				3*			
94	5.388E 01								
		C	1.309E 02	0	87	0	-7.518E 01	0	59
		1	1.168E 02	3	82	1	-3.010E 01	2	44
		2	3.585E 01	4	102	2	-9.304E-01	1	4
		3	1.879E 01	1	14	3	-7.878E-01	3	72
		4	1.026E 01	2	34	4	0		
		3*				0*			
111	1.381E 02								
		C	2.378E 02	0	98	0	-3.110E 01	0	59
		1	2.164E 02	4	102	1	-1.183E 01	2	44
		2	7.733E 01	3	82	2	-3.655E-01	1	4
		3	7.383E 00	1	14	3	-3.095E-01	3	72
		4	4.029E 00	2	34	4	0		
		2*				0*			
122	-1.971E 02								
		0	0			0	-1.941E 02	4	112
		1	0			1	-1.941E 02	4	112
		2	0			2	0		
		3	0			3	0		
		4	0			4	0		
		C*				0*			
SHEAR (K)									
AT	DEAD LD	LANE	POSITIVE	LOAD AT	LANE	NEGATIVE	LOAD AT		
STA	EFFECT	ORDER	MAXIMUM	LANE STA	ORDER	MAXIMUM	LANE STA		
12	-6.640E 01								
		C	0		0	-6.471E 01	1	4	
		1	0		1	-6.471E 01	1	4	
		2	0		2	0			
		3	0		3	0			
		4	0		4	0			
		0*			0*				
16	6.445E 01								
		C	4.905E 01	1	14	0	-5.655E 00	0	57
		1	4.905E 01	1	14	1	-2.150E 00	3	72
		2	1.406E 01	2	34	2	-6.646E-02	4	112
		3	1.342E 00	4	102	3	-5.627E-02	2	44
		4	7.326E-01	3	82	4	0		
		2*				0*			
34	-1.040E 01								
		0	5.165E 00	2	36	0	-2.166E 01	0	16
		1	5.165E 00	2	36	1	-2.124E 01	1	14
		2	1.342E 00	4	102	2	-2.150E 00	3	72
		3	7.326E-01	3	82	3	-6.646E-02	4	112

	4	2.661E-01	1	4	4	-5.627E-02	2	44
	3*				2*			
48	-8.290E 01							
	C	1.913E 00	0	93	0	-6.838E 01	0	30
	1	1.342E 00	4	102	1	-6.476E 01	2	34
	2	7.326E-01	3	82	2	-3.315E 01	1	14
	3	2.661E-01	1	4	3	-2.150E 00	3	72
	4	0			4	-6.646E-02	4	112
	3*				2*			
52	7.675E 01							
	C	6.058E 01	0	50	0	-9.571E 00	0	93
	1	5.371E 01	2	44	1	-6.718E 00	4	102
	2	1.321E 01	3	72	2	-3.339E 00	3	82
	3	6.718E 00	1	14	3	-3.326E-01	1	4
	4	3.326E-01	4	112	4	0		
	2*				2*			
68	0							
	C	1.124E 01	0	68	0	-1.124E 01	0	48
	1	9.733E 00	3	72	1	-9.733E 00	2	44
	2	6.718E 00	1	14	2	-6.718E 00	4	102
	3	3.339E 00	2	34	3	-3.339E 00	3	82
	4	3.326E-01	4	112	4	-3.326E-01	1	4
	3*				3*			
84	-7.675E 01							
	C	9.571E 00	0	23	0	-6.058E 01	0	66
	1	6.718E 00	1	14	1	-5.371E 01	3	72
	2	3.339E 00	2	34	2	-1.321E 01	2	44
	3	3.326E-01	4	112	3	-6.718E 00	4	102
	4	0			4	-3.326E-01	1	4
	2*				2*			
88	8.290E 01							
	C	6.838E 01	0	86	0	-1.913E 00	0	23
	1	6.476E 01	3	82	1	-1.342E 00	1	14
	2	3.315E 01	4	102	2	-7.326E-01	2	34
	3	2.150E 00	2	44	3	-2.661E-01	4	112
	4	6.646E-02	1	4	4	0		
	2*				3*			
102	1.040E 01							
	C	2.166E 01	0	100	0	-5.165E 00	3	80
	1	2.124E 01	4	102	1	-5.165E 00	3	80
	2	2.150E 00	2	44	2	-1.342E 00	1	14
	3	6.646E-02	1	4	3	-7.326E-01	2	34
	4	5.628E-02	3	72	4	-2.661E-01	4	112
	2*				3*			
120	-6.445E 01							
	C	5.655E 00	0	59	0	-4.905E 01	4	102
	1	2.150E 00	2	44	1	-4.905E 01	4	102
	2	6.646E-02	1	4	2	-1.406E 01	3	82
	3	5.627E-02	3	72	3	-1.342E 00	1	14
	4	0			4	-7.326E-01	2	34
	C*				2*			
124	6.640E 01							

C	6.471E 01	4	112	0	0
1	6.471E 01	4	112	1	0
2	0			2	0
3	0			3	0
4	0			4	0
C*				0*	

REACTIGN (K) AT STA	DEAD LD EFFECT	LANE ORDER	POSITIVE MAXIMUM	LOAD AT LANE STA	LANE ORDER	NEGATIVE MAXIMUM	LOAD AT LANE STA
14	1.348E 02	C	1.003E 02	1 4	0	-5.655E 00	0 57
		1	1.003E 02	1 4	1	-2.150E 00	3 72
		2	1.406E 01	2 34	2	-6.646E-02	4 112
		3	1.342E 00	4 102	3	-5.627E-02	2 44
		4	7.326E-01	3 82	4	0	
		2*			0*		
50	1.637E 02	C	9.095E 01	2 38	0	-1.148E 01	0 93
		1	9.095E 01	2 38	1	-8.061E 00	4 102
		2	3.987E 01	1 14	2	-4.072E 00	3 82
		3	1.536E 01	3 72	3	-5.987E-01	1 4
		4	3.990E-01	4 112	4	0	
		3*			2*		
86	1.637E 02	C	9.095E 01	3 78	0	-1.148E 01	0 23
		1	9.095E 01	3 78	1	-8.061E 00	1 14
		2	3.987E 01	4 102	2	-4.072E 00	2 34
		3	1.536E 01	2 44	3	-5.987E-01	4 112
		4	3.990E-01	1 4	4	0	
		3*			2*		
122	1.348E 02	C	1.003E 02	4 112	0	-5.655E 00	0 59
		1	1.003E 02	4 112	1	-2.150E 00	2 44
		2	1.406E 01	3 82	2	-6.646E-02	1 4
		3	1.342E 00	1 14	3	-5.627E-02	3 72
		4	7.326E-01	2 34	4	0	
		2*			0*		

TABLE 6 -- ENVELOPES OF MAXIMUM VALUES

STA	DIST X (FT)	MAX + MOM (FT-K)	MAX - MOM (FT-K)	MAX + SHEAR (K)	MAX - SHEAR (K)
-1	-5.000E-01	0	0	0	0
0	0	0	0	0	0
1	5.000E-01	0	0	0	0
2	1.000E 00	0	0	0	0
3	1.500E 00	0	0	0	0
4	2.000E 00	0	0	0	0
5	2.500E 00	0	0	4.547E-C8	-9.095E-08
6	3.000E 00	4.547E-C8	-9.095E-08	0	-1.250E-01
7	3.500E 00	0	-1.250E-01	0	-5.417E-01
8	4.000E 00	0	-5.417E-01	0	-6.447E 01
9	4.500E 00	0	-6.459E 01	0	-1.285E 02
10	5.000E 00	0	-1.290E 02	0	-1.293E 02
11	5.500E 00	0	-1.939E 02	0	-1.301E 02
12	6.000E 00	0	-2.592E 02	0	-1.311E 02
13	6.500E 00	0	-3.250E 02	0	-1.321E 02
14	7.000E 00	0	-3.913E 02	2.168E 01	-1.555E 01
15	7.500E 00	0	-3.405E 02	1.286E 02	0
16	8.000E 00	0	-2.903E 02	1.276E 02	0
17	8.500E 00	0	-2.405E 02	1.266E 02	0
18	9.000E 00	1.072E 01	-1.912E 02	1.256E 02	0
19	9.500E 00	6.712E 01	-1.425E 02	1.246E 02	0
20	1.000E 01	1.291E 02	-9.424E 01	1.236E 02	0
21	1.050E 01	1.907E 02	-4.649E 01	1.226E 02	0
22	1.100E 01	2.517E 02	0	1.216E 02	0
23	1.150E 01	3.122E 02	0	1.206E 02	0
24	1.200E 01	3.723E 02	0	1.196E 02	0
25	1.250E 01	4.318E 02	0	5.429E 01	0
26	1.300E 01	4.226E 02	0	4.112E 00	-2.580E 01
27	1.350E 01	4.128E 02	0	3.112E 00	-2.680E 01
28	1.400E 01	4.026E 02	0	2.112E 00	-2.780E 01
29	1.450E 01	3.918E 02	0	1.112E 00	-2.880E 01
30	1.500E 01	3.806E 02	0	1.120E-01	-2.980E 01
31	1.550E 01	3.688E 02	0	0	-3.080E 01
32	1.600E 01	3.566E 02	0	0	-3.180E 01
33	1.650E 01	3.438E 02	0	0	-3.280E 01
34	1.700E 01	3.306E 02	0	0	-3.380E 01
35	1.750E 01	3.168E 02	0	0	-3.480E 01
36	1.800E 01	3.026E 02	0	0	-3.580E 01
37	1.850E 01	2.878E 02	0	0	-3.680E 01
38	1.900E 01	2.726E 02	0	0	-3.780E 01
39	1.950E 01	2.568E 02	0	0	-3.880E 01
40	2.000E 01	2.406E 02	-2.237E 00	0	-3.980E 01
41	2.050E 01	2.240E 02	-1.352E 01	0	-4.080E 01
42	2.100E 01	2.082E 02	-2.530E 01	0	-1.049E 02
43	2.150E 01	1.261E 02	-6.743E 01	0	-1.758E 02
44	2.200E 01	4.339E 01	-1.101E 02	0	-1.768E 02
45	2.250E 01	0	-1.537E 02	0	-1.778E 02
46	2.300E 01	0	-2.054E 02	0	-1.788E 02
47	2.350E 01	0	-2.706E 02	0	-1.798E 02
48	2.400E 01	0	-3.526E 02	0	-1.808E 02
49	2.450E 01	0	-4.350E 02	0	-1.818E 02
50	2.500E 01	0	-5.197E 02	2.139E 01	-3.642E 01
51	2.550E 01	0	-4.605E 02	1.447E 02	0
52	2.600E 01	0	-4.019E 02	1.437E 02	0

53	2.650E 01		0	-3.460E 02	1.427E 02	0
54	2.700E 01		0	-2.928E 02	1.417E 02	0
55	2.750E 01		0	-2.400E 02	1.407E 02	0
56	2.800E 01	2.017E C1		-1.878E 02	1.397E 02	0
57	2.850E 01	8.629E C1		-1.360E 02	1.387E 02	0
58	2.900E 01	1.519E C2		-8.478E 01	1.377E 02	0
59	2.950E 01	2.171E C2		-3.404E 01	7.283E 01	0
60	3.000E 01	2.167E C2		-2.476E 01	2.581E 01	-9.812E 00
61	3.050E 01	2.158E C2		-1.865E 01	2.481E 01	-1.081E 01
62	3.100E 01	2.147E C2		-1.390E 01	2.381E 01	-1.181E 01
63	3.150E 01	2.133E C2		-9.649E 00	2.281E 01	-1.281E 01
64	3.200E 01	2.119E C2		-5.897E 00	2.181E 01	-1.381E 01
65	3.250E 01	2.114E C2		-2.644E 00	2.081E 01	-1.481E 01
66	3.300E 01	2.127E C2		0	1.981E 01	-1.581E 01
67	3.350E 01	2.134E C2		0	1.881E 01	-1.681E 01
68	3.400E 01	2.137E C2		0	1.781E 01	-1.781E 01
69	3.450E 01	2.134E C2		0	1.681E 01	-1.881E 01
70	3.500E 01	2.127E C2		0	1.581E 01	-1.981E 01
71	3.550E 01	2.114E C2		-2.644E 00	1.481E 01	-2.081E 01
72	3.600E 01	2.119E C2		-5.897E 00	1.381E 01	-2.181E 01
73	3.650E 01	2.133E C2		-9.649E 00	1.281E 01	-2.281E 01
74	3.700E 01	2.147E C2		-1.390E 01	1.181E 01	-2.381E 01
75	3.750E 01	2.158E C2		-1.865E 01	1.081E 01	-2.481E 01
76	3.800E 01	2.167E C2		-2.476E 01	9.812E 00	-2.581E 01
77	3.850E 01	2.171E C2		-3.404E 01	0	-7.283E 01
78	3.900E 01	1.519E C2		-8.478E 01	0	-1.377E 02
79	3.950E 01	8.629E C1		-1.360E 02	0	-1.387E 02
80	4.000E 01	2.017E C1		-1.878E 02	0	-1.397E 02
81	4.050E 01		0	-2.400E 02	0	-1.407E 02
82	4.100E 01		0	-2.928E 02	0	-1.417E 02
83	4.150E 01		0	-3.460E 02	0	-1.427E 02
84	4.200E 01		0	-4.019E 02	0	-1.437E 02
85	4.250E 01		0	-4.605E 02	0	-1.447E 02
86	4.300E 01		0	-5.197E 02	3.642E 01	-2.139E 01
87	4.350E 01		0	-4.350E 02	1.818E 02	0
88	4.400E 01		0	-3.526E 02	1.808E 02	0
89	4.450E 01		0	-2.706E 02	1.798E 02	0
90	4.500E 01		0	-2.054E 02	1.788E 02	0
91	4.550E 01		0	-1.537E 02	1.778E 02	0
92	4.600E 01	4.339E C1		-1.101E 02	1.768E 02	0
93	4.650E 01	1.261E C2		-6.743E 01	1.758E 02	0
94	4.700E 01	2.082E C2		-2.530E 01	1.049E 02	0
95	4.750E 01	2.240E C2		-1.352E 01	4.080E 01	0
96	4.800E 01	2.406E C2		-2.237E 00	3.980E 01	0
97	4.850E 01	2.568E C2		0	3.880E 01	0
98	4.900E 01	2.726E C2		0	3.780E 01	0
99	4.950E 01	2.878E C2		0	3.680E 01	0
100	5.000E 01	3.026E C2		0	3.580E 01	0
101	5.050E 01	3.168E C2		0	3.480E 01	0
102	5.100E 01	3.306E C2		0	3.380E 01	0
103	5.150E 01	3.438E C2		0	3.280E 01	0
104	5.200E 01	3.566E C2		0	3.180E 01	0
105	5.250E 01	3.688E C2		0	3.080E 01	0
106	5.300E 01	3.806E C2		0	2.980E 01	-1.120E-01
107	5.350E 01	3.918E C2		0	2.880E 01	-1.112E 00
108	5.400E 01	4.026E C2		0	2.780E 01	-2.112E 00
109	5.450E 01	4.128E C2		0	2.680E 01	-3.112E 00
110	5.500E 01	4.226E C2		0	2.580E 01	-4.112E 00
111	5.550E 01	4.318E C2		0	0	-5.429E 01
112	5.600E 01	3.723E C2		0	0	-1.196E 02

113	5.650E 01	2.122E C2	0	0	-1.206E 02
114	5.700E 01	2.517E C2	0	0	-1.216E 02
115	5.750E 01	1.907E C2	-4.649E 01	0	-1.226E 02
116	5.800E 01	1.251E C2	-9.424E 01	0	-1.236E 02
117	5.850E 01	6.712E 01	-1.425E 02	0	-1.246E 02
118	5.900E 01	1.072E C1	-1.912E 02	0	-1.256E 02
119	5.950E 01	0	-2.405E 02	0	-1.266E 02
120	6.000E 01	0	-2.903E 02	0	-1.276E 02
121	6.050E 01	0	-3.405E 02	0	-1.286E 02
122	6.100E 01	0	-3.913E 02	1.555E 01	-2.168E 01
123	6.150E 01	0	-3.250E 02	1.321E 02	0
124	6.200E 01	0	-2.592E 02	1.311E 02	0
125	6.250E 01	0	-1.939E 02	1.301E 02	0
126	6.300E 01	0	-1.290E 02	1.293E 02	0
127	6.350E 01	0	-6.459E 01	1.285E 02	0
128	6.400E 01	0	-5.417E-01	6.447E 01	0
129	6.450E 01	0	-1.250E-01	5.417E-01	0
130	6.500E 01	4.547E-C8	-4.547E-08	1.250E-01	0
131	6.550E 01	0	0	4.547E-08	-4.547E-08
132	6.600E 01	0	0	0	0
133	6.650E 01	0	0	0	0
134	6.700E 01	0	0	0	0
135	6.750E 01	0	0	0	0
136	6.800E 01	0	0	0	0
137	6.850E 01	0	0	0	0

TABLE 7 -- MAXIMUM SUPPORT REACTIONS

STA	DIST X FT	MAX + REACT K	MAX - REACT K
14	7.000E 00	2.492E C2	0
50	2.500E 01	2.952E C2	0
86	4.300E 01	2.952E C2	0
122	6.100E 01	2.492E C2	0

TIME = 5 MINUTES, 27 AND 11/60 SECONDS

TABLE 8 -- SCALES FOR PLOT CUTPLT

DISTANCE	10 INCHES =	100 FT
PGMENT	2 INCHES =	1000 FT-K
SHEAR	2 INCHES =	200 K

TIME = 5 MINUTES, 51 AND 43/60 SECONDS

TABLE 4 -- CAP STIFFNESS, AND DATA FOR BOTH FIXED AND MOVABLE LOADS

FIXED-CR-MOVABLE			FIXED-POSITION DATA			STRINGER,		MOVABLE-
STA	STA	CONTD	CAP BENDING	SIDEWALK,	SLAB LCADS	CAP LCADS	POSITION	SLAB LOADS
FRCM	TC	IF=1	STIFFNESS	SLAB LCADS	(K)	(K)	(K)	(K)
			(K-FT*FT)					
6		1	1.000E C5		0	-5.000E-01		0
	12	1	1.000E C6		0	-1.000E 00		0
	124	1	1.000E C6		0	-1.000E 00		0
	130	0	1.000E C5		0	-5.000E-01		0
8	8	0	0		0	-4.500E 01		0
9	9	0	0		0	-5.000E 00		0
25	25	0	0		0	-4.000E 01		0
26	26	0	0		0	-1.000E 01		0
42	42	0	0		0	-3.500E 01		0
43	43	0	0		0	-1.500E 01		0
59	59	0	0		0	-3.000E C1		0
60	60	0	0		0	-2.000E 01		0
76	76	0	0		0	-2.000E 01		0
77	77	0	0		0	-3.000E 01		0
93	93	0	0		0	-1.500E 01		0
94	94	0	0		0	-3.500E 01		0
110	110	0	0		0	-1.000E C1		0
111	111	0	C		0	-4.000E 01		0
127	127	0	0		0	-5.000E 00		0
128	128	0	0		0	-4.500E 01		0
0	136	0	0		-5.000E-01	0		0
0	4	0	0		-5.000E-01	0		0
64	72	0	0		-5.000E-01	0		0
132	136	0	0		-5.000E-01	0		0
0	20	0	0		0	0		-5.000E 00

PROB (CCATC)
SCL 2

EXAMPLE CAP - NO SKEW, SAME AS SOLUTION 1 EXCEPT
STRINGERS INPUT AT FRACTIONAL STATIONS

TABLE 5 -- MULTI-LANE LOADING SUMMARY

(*--CRITICAL NUMBER OF LANE LOADS)

MOMENT (FT-K) AT STA	DEAD LD EFFECT	LANE ORDER	POSITIVE MAXIMUM	LOAD AT LANE STA	LANE ORDER	NEGATIVE MAXIMUM	LOAD AT LANE STA
14	-1.944E 02	C	0		0	-1.932E 02	1 4
		1	0		1	-1.932E 02	1 4
		2	0		2	0	
		3	0		3	0	
		4	0		4	0	
		C*			0*		
25	1.348E 02	C	2.350E 02	0 18	0	-3.162E 01	0 57
		1	2.125E 02	1 14	1	-1.198E 01	3 72
		2	7.752E 01	2 34	2	-1.933E 00	1 4
		3	7.372E 00	4 102	3	-3.788E-01	4 112
		4	4.022E 00	3 82	4	-1.652E-01	2 44
		2*			0*		
42	4.682E 01	C	1.251E 02	0 30	0	-8.049E 01	0 57
		1	1.125E 02	2 34	1	-3.049E 01	3 72
		2	3.426E 01	1 14	2	-5.641E-01	4 112
		3	1.676E 01	4 102	3	-4.206E-01	2 44
		4	1.024E 01	3 82	4	0	
		3*			0*		
50	-2.727E 02	C	3.420E 01	0 93	0	-1.408E 02	0 31
		1	2.413E 01	4 102	1	-1.405E 02	2 34
		2	1.316E 01	3 82	2	-5.662E 01	1 14
		3	4.964E 00	1 4	3	-3.920E 01	3 72
		4	0		4	-1.240E 00	4 112
		3*			3*		
59	6.064E 01	C	1.461E 02	0 52	0	-9.416E 01	0 23
		1	9.999E 01	2 44	1	-6.643E 01	1 14
		2	2.046E 01	3 72	2	-3.544E 01	2 34
		3	3.413E 00	1 4	3	-6.060E 00	4 102
		4	3.113E-01	4 112	4	-1.704E 00	3 82
		C*			2*		
77	6.064E 01	C	1.461E 02	0 64	0	-9.416E 01	0 93
		1	9.999E 01	3 72	1	-6.643E 01	4 102
		2	2.046E 01	2 44	2	-3.544E 01	3 82
		3	3.413E 00	4 112	3	-6.060E 00	1 14

		4	3.113E-01	1	4	4	-1.704E 00	2	34
		C*				2*			
E6	-2.727E 02	C	3.420E 01	0	23	0	-1.408E 02	0	85
		1	2.413E 01	1	14	1	-1.405E 02	3	82
		2	1.316E 01	2	34	2	-5.662E 01	4	102
		3	4.564E 00	4	112	3	-3.920E 01	2	44
		4	0			4	-1.240E 00	1	4
		3*				3*			
94	4.682E 01	C	1.251E 02	0	86	0	-8.049E 01	0	59
		1	1.125E 02	3	82	1	-3.049E 01	2	44
		2	3.426E 01	4	102	2	-5.641E-01	1	4
		3	1.876E 01	1	14	3	-4.206E-01	3	72
		4	1.024E 01	2	34	4	0		
		3*				0*			
111	1.348E 02	0	2.350E 02	0	98	0	-3.162E 01	0	59
		1	2.125E 02	4	102	1	-1.198E 01	2	44
		2	7.752E 01	3	82	2	-1.933E 00	4	112
		3	7.372E 00	1	14	3	-3.788E-01	1	4
		4	4.022E 00	2	34	4	-1.652E-01	3	72
		2*				0*			
122	-1.944E 02	C	0			0	-1.932E 02	4	112
		1	0			1	-1.932E 02	4	112
		2	0			2	0		
		3	0			3	0		
		4	0			4	0		
		0*				0*			
SHEAR (K)									
AT STA	DEAD LD EFFECT	LANE CRDR	POSITIVE MAXIMUM	LOAD AT LANE STA	LANE ORDER	NEGATIVE MAXIMUM	LOAD AT LANE STA		
12	-6.650E 01	C	0		0	-6.650E 01	1	4	
		1	0		1	-6.650E 01	1	4	
		2	0		2	0			
		3	0		3	0			
		4	0		4	0			
		0*			0*				
16	6.335E 01	C	4.845E 01	1	14	0	-5.749E 00	0	57
		1	4.845E 01	1	14	1	-2.178E 00	3	72
		2	1.405E 01	2	34	2	-6.886E-02	4	112
		3	1.340E 00	4	102	3	-3.004E-02	2	44
		4	7.312E-01	3	82	4	0		
		2*				0*			
34	-1.152E 01	C	4.696E 00	2	37	0	-2.236E 01	0	16
		1	4.696E 00	2	37	1	-2.180E 01	1	14
		2	1.340E 00	4	102	2	-2.178E 00	3	72
		3	7.312E-01	3	82	3	-6.887E-02	4	112

0	6.550E 01	4	112	0	0
1	6.550E 01	4	112	1	0
2	0			2	0
3	0			3	0
4	0			4	0
C*				0*	

REACTION (K)									
AT	DEAC LD	LANE	POSITIVE	LOAD AT	LANE	NEGATIVE	LOAD AT		
STA	EFFECT	ORDER	MAXIMUM	LANE STA	ORDER	MAXIMUM	LANE STA		
14	1.339E 02								
		0	1.003E 02	1 4	0	-5.749E 00	0 57		
		1	1.003E 02	1 4	1	-2.178E 00	3 72		
		2	1.409E 01	2 34	2	-6.886E-02	4 112		
		3	1.340E 00	4 102	3	-3.004E-02	2 44		
		4	7.312E-01	3 82	4	0			
		2*			0*				
50	1.646E 02								
		0	5.082E 01	2 38	0	-1.141E 01	0 93		
		1	5.082E 01	2 38	1	-8.048E 00	4 102		
		2	3.985E 01	1 14	2	-4.035E 00	3 82		
		3	1.544E 01	3 72	3	-6.204E-01	1 4		
		4	4.135E-01	4 112	4	0			
		3*			2*				
66	1.646E 02								
		0	5.082E 01	3 78	0	-1.141E 01	0 23		
		1	5.082E 01	3 78	1	-8.048E 00	1 14		
		2	3.985E 01	4 102	2	-4.035E 00	2 34		
		3	1.544E 01	2 44	3	-6.204E-01	4 112		
		4	4.135E-01	1 4	4	0			
		3*			2*				
122	1.339E 02								
		0	1.003E 02	4 112	0	-5.749E 00	0 59		
		1	1.003E 02	4 112	1	-2.178E 00	2 44		
		2	1.409E 01	3 82	2	-6.886E-02	1 4		
		3	1.340E 00	1 14	3	-3.004E-02	3 72		
		4	7.312E-01	2 34	4	0			
		2*			0*				

TABLE 6 -- ENVELOPES OF MAXIMUM VALUES

STA	DIST X (FT)	MAX + MOM (FT-K)	MAX - MOM (FT-K)	MAX + SHEAR (K)	MAX - SHEAR (K)
-1	-5.000E-01	0	0	0	0
0	0	0	0	0	0
1	5.000E-01	0	0	0	0
2	1.000E 00	0	0	0	0
3	1.500E 00	0	0	0	0
4	2.000E 00	0	0	0	0
5	2.500E 00	0	0	3.411E-08	-9.095E-08
6	3.000E 00	3.411E-08	-9.095E-08	0	-1.250E-01
7	3.500E 00	0	-1.250E-01	0	-5.417E-01
8	4.000E 00	0	-5.417E-01	0	-5.854E 01
9	4.500E 00	0	-5.867E 01	0	-1.230E 02
10	5.000E 00	0	-1.235E 02	0	-1.302E 02
11	5.500E 00	0	-1.888E 02	0	-1.310E 02
12	6.000E 00	0	-2.546E 02	0	-1.320E 02
13	6.500E 00	0	-3.208E 02	0	-1.330E 02
14	7.000E 00	0	-3.876E 02	2.054E 01	-1.693E 01
15	7.500E 00	0	-3.378E 02	1.269E 02	0
16	8.000E 00	0	-2.884E 02	1.259E 02	0
17	8.500E 00	0	-2.396E 02	1.249E 02	0
18	9.000E 00	1.029E C1	-1.913E 02	1.239E 02	0
19	9.500E 00	6.507E C1	-1.435E 02	1.229E 02	0
20	1.000E 01	1.263E C2	-9.618E 01	1.219E 02	0
21	1.050E 01	1.870E C2	-4.937E 01	1.209E 02	0
22	1.100E 01	2.472E C2	-3.051E 00	1.199E 02	0
23	1.150E 01	3.069E C2	0	1.189E 02	0
24	1.200E 01	3.662E C2	0	1.179E 02	0
25	1.250E 01	4.249E C2	0	6.263E C1	0
26	1.300E 01	4.262E C2	0	1.089E 01	-1.478E 01
27	1.350E 01	4.173E C2	0	1.567E C0	-2.850E 01
28	1.400E 01	4.059E C2	0	5.670E-C1	-2.950E 01
29	1.450E 01	3.939E C2	0	0	-3.050E 01
30	1.500E 01	3.815E C2	0	0	-3.150E 01
31	1.550E 01	3.686E C2	0	0	-3.250E 01
32	1.600E 01	3.552E C2	0	0	-3.350E 01
33	1.650E 01	3.413E C2	0	0	-3.450E 01
34	1.700E 01	3.269E C2	0	0	-3.550E 01
35	1.750E 01	3.120E C2	0	0	-3.650E 01
36	1.800E 01	2.965E C2	0	0	-3.750E 01
37	1.850E 01	2.806E C2	0	0	-3.850E 01
38	1.900E 01	2.642E C2	0	0	-3.950E 01
39	1.950E 01	2.473E C2	0	0	-4.050E 01
40	2.000E 01	2.299E C2	-9.400E 00	0	-4.150E 01
41	2.050E 01	2.127E C2	-2.129E 01	0	-4.250E 01
42	2.100E 01	1.958E C2	-3.367E 01	0	-8.624E 01
43	2.150E 01	1.323E C2	-6.736E 01	0	-1.571E 02
44	2.200E 01	4.853E 01	-1.109E 02	0	-1.791E 02
45	2.250E 01	0	-1.550E 02	0	-1.801E 02
46	2.300E 01	0	-2.041E 02	0	-1.811E 02
47	2.350E 01	0	-2.691E 02	0	-1.821E 02
48	2.400E 01	0	-3.523E 02	0	-1.831E 02
49	2.450E 01	0	-4.360E 02	0	-1.841E 02
50	2.500E 01	0	-5.214E 02	2.004E 01	-3.823E 01
51	2.550E 01	0	-4.629E 02	1.429E 02	0
52	2.600E 01	0	-4.048E 02	1.419E 02	0

53	2.650E 01		0	-3.491E 02	1.409E 02	0
54	2.700E 01		0	-2.966E 02	1.399E 02	0
55	2.750E 01		C	-2.445E 02	1.389E 02	0
56	2.800E 01	1.173E 01	0	-1.929E 02	1.379E 02	0
57	2.850E 01	7.708E C1	0	-1.419E 02	1.369E 02	0
58	2.900E 01	1.422E C2	0	-9.130E 01	1.359E 02	0
59	2.950E 01	2.068E C2	0	-4.123E 01	9.711E 01	0
60	3.000E 01	2.320E C2	0	-1.585E 01	4.243E 01	0
61	3.050E 01	2.309E C2	0	-9.803E 00	2.514E C1	-1.114E 01
62	3.100E 01	2.298E C2	0	-5.067E 00	2.414E C1	-1.214E 01
63	3.150E 01	2.282E C2	0	-8.302E-01	2.314E 01	-1.314E 01
64	3.200E 01	2.267E C2	0	0	2.214E 01	-1.414E 01
65	3.250E 01	2.251E C2	0	0	2.114E 01	-1.514E 01
66	3.300E 01	2.255E C2	0	0	2.014E C1	-1.614E 01
67	3.350E 01	2.263E C2	0	0	1.914E C1	-1.714E 01
68	3.400E 01	2.265E C2	0	0	1.814E 01	-1.814E 01
69	3.450E 01	2.263E C2	0	0	1.714E 01	-1.914E 01
70	3.500E 01	2.255E C2	0	0	1.614E C1	-2.014E 01
71	3.550E 01	2.251E C2	0	0	1.514E 01	-2.114E 01
72	3.600E 01	2.267E C2	0	0	1.414E C1	-2.214E 01
73	3.650E 01	2.282E C2	0	-8.301E-01	1.314E 01	-2.314E 01
74	3.700E 01	2.298E C2	0	-5.067E 00	1.214E 01	-2.414E 01
75	3.750E 01	2.309E C2	0	-9.803E 00	1.114E C1	-2.514E 01
76	3.800E 01	2.320E C2	0	-1.585E 01	0	-4.243E 01
77	3.850E 01	2.068E C2	0	-4.123E 01	0	-9.711E 01
78	3.900E 01	1.422E C2	0	-9.130E 01	0	-1.359E 02
79	3.950E 01	7.708E C1	0	-1.419E 02	0	-1.369E 02
80	4.000E 01	1.173E C1	0	-1.929E 02	0	-1.379E 02
81	4.050E 01	C	0	-2.445E 02	0	-1.389E 02
82	4.100E 01	C	0	-2.966E 02	0	-1.399E 02
83	4.150E 01	0	0	-3.491E 02	0	-1.409E 02
84	4.200E 01	0	0	-4.048E 02	0	-1.419E 02
85	4.250E 01	0	0	-4.629E 02	0	-1.429E 02
86	4.300E 01	0	0	-5.214E 02	3.823E 01	-2.004E 01
87	4.350E 01	0	0	-4.360E 02	1.841E 02	0
88	4.400E 01	0	0	-3.523E 02	1.831E 02	0
89	4.450E 01	0	0	-2.691E 02	1.821E 02	0
90	4.500E 01	0	0	-2.041E 02	1.811E 02	0
91	4.550E 01	0	0	-1.550E 02	1.801E 02	0
92	4.600E 01	4.853E C1	0	-1.109E 02	1.791E 02	0
93	4.650E 01	1.323E C2	0	-6.736E 01	1.571E 02	0
94	4.700E 01	1.958E C2	0	-3.367E 01	8.624E C1	0
95	4.750E 01	2.127E C2	0	-2.129E 01	4.250E C1	0
96	4.800E 01	2.259E C2	0	-9.400E 00	4.150E 01	0
97	4.850E 01	2.473E C2	0	0	4.050E 01	0
98	4.900E 01	2.642E C2	0	0	3.950E C1	0
99	4.950E 01	2.806E C2	0	0	3.850E C1	0
100	5.000E 01	2.965E C2	0	0	3.750E 01	0
101	5.050E 01	3.120E C2	0	0	3.650E 01	0
102	5.100E 01	3.269E C2	0	0	3.550E 01	0
103	5.150E 01	3.413E C2	0	0	3.450E 01	0
104	5.200E 01	3.552E C2	0	0	3.350E C1	0
105	5.250E 01	3.686E C2	0	0	3.250E 01	0
106	5.300E 01	3.815E C2	0	0	3.150E C1	0
107	5.350E 01	3.939E C2	0	0	3.050E 01	0
108	5.400E 01	4.059E C2	0	0	2.950E 01	-5.670E-01
109	5.450E 01	4.173E C2	0	0	2.850E 01	-1.567E 00
110	5.500E 01	4.282E C2	0	0	1.478E C1	-1.089E 01
111	5.550E 01	4.249E C2	0	0	0	-6.263E 01
112	5.600E 01	3.662E C2	0	0	0	-1.179E 02

113	5.650E 01	3.069E C2	0	0	-1.189E 02
114	5.700E 01	2.472E C2	-3.051E 00	0	-1.199E 02
115	5.750E 01	1.870E C2	-4.937E 01	0	-1.209E 02
116	5.800E 01	1.263E C2	-9.618E 01	0	-1.219E 02
117	5.850E 01	6.507E C1	-1.435E 02	0	-1.229E 02
118	5.900E 01	1.029E C1	-1.913E 02	0	-1.239E 02
119	5.950E 01	0	-2.396E 02	0	-1.249E 02
120	6.000E 01	0	-2.884E 02	0	-1.259E 02
121	6.050E 01	0	-3.378E 02	0	-1.269E 02
122	6.100E 01	0	-3.876E 02	1.693E 01	-2.054E 01
123	6.150E 01	0	-3.208E 02	1.330E 02	0
124	6.200E 01	0	-2.546E 02	1.320E 02	0
125	6.250E 01	0	-1.888E 02	1.310E 02	0
126	6.300E 01	0	-1.235E 02	1.302E 02	0
127	6.350E 01	0	-5.867E 01	1.230E 02	0
128	6.400E 01	0	-5.417E-01	5.854E 01	0
129	6.450E 01	0	-1.250E-01	5.417E-01	0
130	6.500E 01	2.274E-C8	-9.095E-08	1.250E-01	0
131	6.550E 01	0	0	9.095E-08	-2.274E-08
132	6.600E 01	0	0	0	0
133	6.650E 01	0	0	0	0
134	6.700E 01	0	0	0	0
135	6.750E 01	0	0	0	0
136	6.800E 01	0	0	0	0
137	6.850E 01	0	0	0	0

TABLE 7 -- MAXIMUM SUPPORT REACTIONS

STA	DIST X FT	MAX + REACT K	MAX - REACT K
14	7.000E 00	2.482E 02	0
50	2.500E 01	2.961E 02	0
86	4.300E 01	2.961E 02	0
122	6.100E 01	2.482E 02	0

TIME = 8 MINUTES, 20 AND 23/60 SECONDS

TABLE 8 -- SCALES FOR PLOT OUTPUT

DISTANCE	10 INCHES =	100 FT
MOMENT	2 INCHES =	1000 FT-K
SHEAR	2 INCHES =	200 K

TIME = 8 MINUTES, 41 AND 43/60 SECONDS

PROGRAM CAP 14 - DECK 2 - MATLCK - INGRAM REVISION DATE = 30AUG 66

PROB
SCL 3 EXAMPLE CAP - SAME AS SOLUTION 2 EXCEPT
SKEW = 30.0 DEGREES

TABLE 1 -- PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER		
		2	3	4
OPTIONS TO HOLD (IF=1) FROM PRECEDING PROB NUMBER OF ADDITIONAL CARDS FOR CURRENT PRCB	0	1	1	1
		0	0	0
OPTION (IF=1) TO CLEAR ENVELOPES BEFORE LANE LOADINGS				0
OPTION (IF=1) TO PLCT DESIGN VARIABLE ENVELOPES				1
OPTION (IF=-1) TO OMIT OUTPLT TABLE 5				0
ANGLE OF SKEW, DEGREES				3.000E 01

TABLE 2 -- CONSTANTS

USING DATA FROM THE PREVIOUS PROBLEM

TABLE 3 -- LISTS OF STATIONS

USING DATA FROM THE PREVIOUS PROBLEM

TABLE 4 -- CAP STIFFNESS, AND DATA FOR BOTH FIXED AND MOVABLE LOADS

USING DATA FROM THE PREVIOUS PROBLEM PLUS

NONE

PROB (CCNTD)
 SOL 3 EXAMPLE CAP - SAME AS SOLUTION 2 EXCEPT
 SKEW = 30.0 DEGREES

TABLE 5 -- MULTI-LANE LOADING SUMMARY (*--CRITICAL NUMBER OF LANE LOADS)

MOMENT (FT-K) AT STA	DEAD LD EFFECT	LANE ORDER	POSITIVE MAXIMUM	LOAD AT LANE STA	LANE ORDER	NEGATIVE MAXIMUM	LOAD AT LANE STA
14	-2.265E 02	0	0		0	-2.231E 02	1 4
		1	0		1	-2.231E 02	1 4
		2	0		2	0	
		3	0		3	0	
		4	0		4	0	
		0*			0*		
25	1.631E 02	0	2.713E 02	0 18	0	-3.651E 01	0 57
		1	2.454E 02	1 14	1	-1.383E 01	3 72
		2	8.951E 01	2 34	2	-2.233E 00	1 4
		3	8.512E 00	4 102	3	-4.374E-01	4 112
		4	4.644E 00	3 82	4	-1.908E-01	2 44
		2*			0*		
42	5.493E 01	0	1.445E 02	0 30	0	-9.294E 01	0 57
		1	1.295E 02	2 34	1	-3.521E 01	3 72
		2	3.956E 01	1 14	2	-1.113E 00	4 112
		3	2.167E 01	4 102	3	-4.857E-01	2 44
		4	1.182E 01	3 82	4	0	
		3*			0*		
50	-3.260E 02	0	3.949E 01	0 93	0	-1.626E 02	0 31
		1	2.786E 01	4 102	1	-1.622E 02	2 34
		2	1.520E 01	3 82	2	-1.116E 02	1 14
		3	5.732E 00	1 4	3	-4.527E 01	3 72
		4	0		4	-1.431E 00	4 112
		3*			3*		
59	6.972E 01	0	1.687E 02	0 52	0	-1.087E 02	0 23
		1	1.155E 02	2 44	1	-7.671E 01	1 14
		2	2.363E 01	3 72	2	-4.093E 01	2 34
		3	3.541E 00	1 4	3	-6.997E 00	4 102
		4	3.595E-01	4 112	4	-1.967E 00	3 82
		0*			2*		
77	6.972E 01	0	1.687E 02	0 64	0	-1.087E 02	0 93
		1	1.155E 02	3 72	1	-7.671E 01	4 102
		2	2.363E 01	2 44	2	-4.093E 01	3 82
		3	3.541E 00	4 112	3	-6.997E 00	1 14

		4	3.595E-01	1	4	4	-1.967E 00	2	34
		0*				2*			
86	-3.260E 02	0	3.949E 01	0	23	0	-1.626E 02	0	85
		1	2.786E 01	1	14	1	-1.622E 02	3	82
		2	1.520E 01	2	34	2	-1.116E 02	4	102
		3	5.732E 00	4	112	3	-4.527E 01	2	44
		4	0			4	-1.431E 00	1	4
		3*				3*			
94	5.493E 01	0	1.445E 02	0	86	0	-9.294E 01	0	59
		1	1.299E 02	3	82	1	-3.521E 01	2	44
		2	3.656E 01	4	102	2	-1.113E 00	1	4
		3	2.167E 01	1	14	3	-4.857E-01	3	72
		4	1.182E 01	2	34	4	0		
		3*				0*			
111	1.631E 02	0	2.713E 02	0	98	0	-3.651E 01	0	59
		1	2.454E 02	4	102	1	-1.383E 01	2	44
		2	8.951E 01	3	82	2	-2.233E 00	4	112
		3	8.512E 00	1	14	3	-4.374E-01	1	4
		4	4.644E 00	2	34	4	-1.908E-01	3	72
		2*				0*			
122	-2.265E 02	0	0			0	-2.231E 02	4	112
		1	0			1	-2.231E 02	4	112
		2	0			2	0		
		3	0			3	0		
		4	0			4	0		
		0*				0*			
SHEAR (K)									
AT	CEAD LD	LANE	PCPOSITIVE	LOAD AT	LANE	NEGATIVE	LOAD AT		
STA	EFFECT	ORDER	MAXIMUM	LANE STA	ORDER	MAXIMUM	LANE STA		
12	-6.720E 01	0	0		0	-6.550E 01	1	4	
		1	0		1	-6.550E 01	1	4	
		2	0		2	0			
		3	0		3	0			
		4	0		4	0			
		0*			0*				
16	6.539E 01	0	4.849E 01	1	14	0	-5.749E 00	0	57
		1	4.849E 01	1	14	1	-2.178E 00	3	72
		2	1.409E 01	2	34	2	-6.886E-02	4	112
		3	1.340E 00	4	102	3	-3.004E-02	2	44
		4	7.312E-01	3	82	4	0		
		2*				0*			
34	-1.227E 01	0	4.696E 00	2	37	0	-2.236E 01	0	16
		1	4.696E 00	2	37	1	-2.180E 01	1	14
		2	1.340E 00	4	102	2	-2.178E 00	3	72
		3	7.312E-01	3	82	3	-6.886E-02	4	112

	4	2.758E-01	1	4	4	-2.004E-02	2	44
	3*				2*			
48	-8.699E 01							
	C	1.500E 00	0	93	0	-6.507E 01	0	30
	1	1.340E 00	4	102	1	-6.586E 01	2	34
	2	7.312E-01	3	82	2	-2.315E 01	1	14
	3	2.758E-01	1	4	3	-2.178E 00	3	72
	4	0			4	-6.887E-02	4	112
	3*				2*			
52	7.905E 01							
	0	5.546E 01	0	51	0	-5.507E 00	0	93
	1	5.203E 01	2	44	1	-6.708E 00	4	102
	2	1.326E 01	3	72	2	-2.304E 00	3	82
	3	6.708E 00	1	14	3	-2.446E-01	1	4
	4	2.446E-01	4	112	4	0		
	2*				2*			
68	0							
	0	1.179E 01	0	68	0	-1.179E 01	0	48
	1	1.015E 01	3	72	1	-1.015E 01	2	44
	2	6.708E 00	1	14	2	-6.708E 00	4	102
	3	2.304E 00	2	34	3	-2.304E 00	3	82
	4	2.446E-01	4	112	4	-2.446E-01	1	4
	3*				3*			
84	-7.905E 01							
	C	5.507E 00	0	23	0	-5.546E 01	0	65
	1	6.708E 00	1	14	1	-5.203E 01	3	72
	2	2.304E 00	2	34	2	-1.326E 01	2	44
	3	2.446E-01	4	112	3	-6.708E 00	4	102
	4	0			4	-2.446E-01	1	4
	2*				2*			
88	8.699E 01							
	0	6.507E 01	0	86	0	-1.500E 00	0	23
	1	6.586E 01	3	82	1	-1.340E 00	1	14
	2	2.315E 01	4	102	2	-7.312E-01	2	34
	3	2.178E 00	2	44	3	-2.758E-01	4	112
	4	6.887E-02	1	4	4	0		
	2*				3*			
102	1.227E 01							
	0	2.236E 01	0	100	0	-4.696E 00	3	79
	1	2.180E 01	4	102	1	-4.696E 00	3	79
	2	2.178E 00	2	44	2	-1.340E 00	1	14
	3	6.887E-02	1	4	3	-7.312E-01	2	34
	4	2.004E-02	3	72	4	-2.758E-01	4	112
	2*				3*			
120	-6.539E 01							
	0	5.745E 00	0	59	0	-4.849E 01	4	102
	1	2.178E 00	2	44	1	-4.849E 01	4	102
	2	6.886E-02	1	4	2	-1.409E 01	3	82
	3	2.004E-02	3	72	3	-1.340E 00	1	14
	4	0			4	-7.312E-01	2	34
	C*				2*			
124	6.720E 01							

C	6.55CE 01	4	112	0	0
1	6.55CE 01	4	112	1	0
2	0			2	0
3	0			3	0
4	0			4	0
0*				0*	

REACTION (K) AT STA	DEAD LD EFFECT	LANE ORDER	POSITIVE MAXIMUM	LOAD AT LANE STA	LANE ORDER	NEGATIVE MAXIMUM	LOAD AT LANE STA
14	1.372E 02	C	1.003E 02	1 4	0	-5.749E 00	0 57
		1	1.003E 02	1 4	1	-2.178E 00	3 72
		2	1.409E 01	2 34	2	-6.886E-02	4 112
		3	1.34CE 00	4 102	3	-2.004E-02	2 44
		4	7.312E-01	3 82	4	0	
		2*			0*		
50	1.707E 02	0	5.082E 01	2 38	0	-1.141E 01	0 93
		1	5.082E 01	2 38	1	-8.048E 00	4 102
		2	3.985E 01	1 14	2	-4.035E 00	3 82
		3	1.544E 01	3 72	3	-6.204E-01	1 4
		4	4.135E-01	4 112	4	0	
		3*			2*		
86	1.707E 02	C	5.082E 01	3 78	0	-1.141E 01	0 23
		1	5.082E 01	3 78	1	-8.048E 00	1 14
		2	3.985E 01	4 102	2	-4.035E 00	2 34
		3	1.544E 01	2 44	3	-6.204E-01	4 112
		4	4.135E-01	1 4	4	0	
		3*			2*		
122	1.372E 02	C	1.003E 02	4 112	0	-5.749E 00	0 59
		1	1.003E 02	4 112	1	-2.178E 00	2 44
		2	1.409E 01	3 82	2	-6.887E-02	1 4
		3	1.34CE 00	1 14	3	-2.004E-02	3 72
		4	7.312E-01	2 34	4	0	
		2*			0*		

TABLE 6 -- ENVELOPES OF MAXIMUM VALUES

STA	DIST X (FT)	MAX + MOM (FT-K)	MAX - MOM (FT-K)	MAX + SHEAR (K)	MAX - SHEAR (K)
-1	-5.774E-01	C	0	0	0
0	0	0	0	0	0
1	5.774E-01	0	0	0	0
2	1.155E 00	0	0	0	0
3	1.732E 00	0	0	0	0
4	2.309E 00	0	0	0	0
5	2.887E 00	0	0	5.907E-08	-1.181E-07
6	3.464E 00	6.821E-C8	-1.364E-07	0	-1.443E-01
7	4.041E 00	0	-1.667E-01	0	-6.255E-01
8	4.619E 00	0	-7.222E-01	0	-5.872E 01
9	5.196E 00	0	-6.797E 01	0	-1.233E 02
10	5.774E 00	0	-1.431E 02	0	-1.306E 02
11	6.351E 00	0	-2.187E 02	0	-1.316E 02
12	6.928E 00	0	-2.950E 02	0	-1.327E 02
13	7.506E 00	0	-3.720E 02	0	-1.338E 02
14	8.083E 00	0	-4.496E 02	2.121E 01	-1.626E 01
15	8.660E 00	0	-3.907E 02	1.291E 02	0
16	9.238E 00	0	-3.326E 02	1.280E 02	0
17	9.815E 00	0	-2.751E 02	1.268E 02	0
18	1.039E 01	1.454E 01	-2.183E 02	1.257E 02	0
19	1.097E 01	7.874E 01	-1.621E 02	1.245E 02	0
20	1.155E 01	1.503E C2	-1.066E 02	1.234E 02	0
21	1.212E 01	2.212E C2	-5.176E 01	1.222E 02	0
22	1.270E 01	2.914E C2	0	1.210E 02	0
23	1.328E 01	3.610E C2	0	1.199E 02	0
24	1.386E 01	4.298E C2	0	1.187E 02	0
25	1.443E 01	4.981E C2	0	6.328E 01	0
26	1.501E 01	5.022E C2	0	1.138E 01	-1.429E 01
27	1.559E 01	4.858E C2	0	1.902E 00	-2.817E 01
28	1.617E 01	4.768E C2	0	7.476E-01	-2.932E 01
29	1.674E 01	4.631E C2	0	0	-3.048E 01
30	1.732E 01	4.488E C2	0	0	-3.163E 01
31	1.790E 01	4.337E C2	0	0	-3.279E 01
32	1.848E 01	4.180E C2	0	0	-3.394E 01
33	1.905E 01	4.017E C2	0	0	-3.509E 01
34	1.963E 01	3.846E C2	0	0	-3.625E 01
35	2.021E 01	3.669E C2	0	0	-3.740E 01
36	2.078E 01	3.486E C2	0	0	-3.856E 01
37	2.136E 01	3.255E C2	0	0	-3.971E 01
38	2.194E 01	3.058E C2	0	0	-4.087E 01
39	2.252E 01	2.855E C2	0	0	-4.202E 01
40	2.309E 01	2.684E C2	-7.869E 00	0	-4.318E 01
41	2.367E 01	2.476E C2	-2.261E 01	0	-4.433E 01
42	2.425E 01	2.269E C2	-3.801E 01	0	-8.823E 01
43	2.483E 01	1.524E C2	-7.810E 01	0	-1.593E 02
44	2.540E 01	5.444E C1	-1.296E 02	0	-1.814E 02
45	2.598E 01	0	-1.819E 02	0	-1.825E 02
46	2.656E 01	0	-2.401E 02	0	-1.837E 02
47	2.714E 01	0	-3.168E 02	0	-1.848E 02
48	2.771E 01	0	-4.145E 02	0	-1.860E 02
49	2.829E 01	0	-5.128E 02	0	-1.871E 02
50	2.887E 01	0	-6.132E 02	1.982E 01	-3.844E 01
51	2.944E 01	0	-5.441E 02	1.455E 02	0
52	3.002E 01	0	-4.756E 02	1.443E 02	0

53	3.060E 01		0	-4.099E 02	1.432E C2	0
54	3.118E 01		0	-3.479E 02	1.420E 02	0
55	3.175E 01		0	-2.866E 02	1.409E 02	0
56	3.233E 01	1.043E C1		-2.259E 02	1.397E C2	0
57	3.291E 01	8.691E 01		-1.659E 02	1.386E 02	0
58	3.349E 01	1.630E 02		-1.066E 02	1.374E 02	0
59	3.406E 01	2.384E C2		-4.792E 01	9.850E 01	0
60	3.464E 01	2.684E C2		-1.785E 01	4.366E 01	0
61	3.522E 01	2.678E 02		-1.019E 01	2.623E 01	-1.006E 01
62	3.580E 01	2.670E C2		-4.145E 00	2.507E 01	-1.122E 01
63	3.637E 01	2.657E C2		0	2.392E C1	-1.237E 01
64	3.695E 01	2.643E C2		0	2.276E 01	-1.353E 01
65	3.753E 01	2.628E C2		0	2.161E 01	-1.468E 01
66	3.811E 01	2.625E C2		0	2.045E 01	-1.583E 01
67	3.868E 01	2.645E C2		0	1.930E 01	-1.699E 01
68	3.926E 01	2.649E C2		0	1.814E 01	-1.814E 01
69	3.984E 01	2.645E C2		0	1.699E 01	-1.930E 01
70	4.041E 01	2.635E C2		0	1.583E 01	-2.045E 01
71	4.099E 01	2.628E C2		0	1.468E 01	-2.161E 01
72	4.157E 01	2.643E C2		0	1.353E 01	-2.276E 01
73	4.215E 01	2.657E C2		0	1.237E C1	-2.392E 01
74	4.272E 01	2.670E C2		-4.145E 00	1.122E 01	-2.507E 01
75	4.330E 01	2.678E C2		-1.019E 01	1.006E 01	-2.623E 01
76	4.388E 01	2.684E 02		-1.785E 01	0	-4.366E 01
77	4.446E 01	2.384E C2		-4.792E 01	0	-9.850E 01
78	4.503E 01	1.630E C2		-1.066E 02	0	-1.374E 02
79	4.561E 01	8.691E C1		-1.659E 02	0	-1.386E 02
80	4.619E 01	1.043E C1		-2.259E 02	0	-1.397E 02
81	4.677E 01		0	-2.866E 02	0	-1.409E 02
82	4.734E 01		0	-3.479E 02	0	-1.420E 02
83	4.792E 01		0	-4.099E 02	0	-1.432E 02
84	4.850E 01		0	-4.756E 02	0	-1.443E 02
85	4.907E 01		0	-5.441E 02	0	-1.455E 02
86	4.965E 01		0	-6.132E 02	3.844E 01	-1.982E 01
87	5.023E 01		0	-5.128E 02	1.871E C2	0
88	5.081E 01		0	-4.145E 02	1.860E 02	0
89	5.138E 01		0	-3.168E 02	1.848E 02	0
90	5.196E 01		0	-2.401E 02	1.837E 02	0
91	5.254E 01		0	-1.819E 02	1.825E 02	0
92	5.312E 01	5.444E 01		-1.296E 02	1.814E 02	0
93	5.369E 01	1.524E C2		-7.810E 01	1.593E 02	0
94	5.427E 01	2.269E C2		-3.801E 01	8.823E 01	0
95	5.485E 01	2.476E C2		-2.261E 01	4.433E 01	0
96	5.543E 01	2.684E C2		-7.869E 00	4.318E 01	0
97	5.600E 01	2.895E C2		0	4.202E 01	0
98	5.658E 01	3.058E C2		0	4.087E 01	0
99	5.716E 01	3.295E C2		0	3.971E 01	0
100	5.774E 01	3.486E C2		0	3.856E 01	0
101	5.831E 01	3.669E C2		0	3.740E 01	0
102	5.889E 01	3.846E C2		0	3.625E 01	0
103	5.947E 01	4.017E C2		0	3.509E 01	0
104	6.004E 01	4.180E C2		0	3.394E 01	0
105	6.062E 01	4.337E C2		0	3.279E 01	0
106	6.120E 01	4.488E C2		0	3.163E 01	0
107	6.178E 01	4.631E C2		0	3.048E 01	0
108	6.235E 01	4.768E C2		0	2.932E 01	-7.476E-01
109	6.293E 01	4.898E C2		0	2.817E 01	-1.902E 00
110	6.351E 01	5.022E 02		0	1.429E 01	-1.138E 01
111	6.409E 01	4.981E C2		0	0	-8.328E 01
112	6.466E 01	4.298E C2		0	0	-1.167E 02

113	6.524E 01	3.610E C2	0	0	-1.199E 02
114	6.582E 01	2.914E C2	0	0	-1.210E 02
115	6.640E 01	2.212E 02	-5.176E 01	0	-1.222E 02
116	6.697E 01	1.503E C2	-1.066E 02	0	-1.234E 02
117	6.755E 01	7.874E C1	-1.621E 02	0	-1.245E 02
118	6.813E 01	1.454E C1	-2.183E 02	0	-1.257E 02
119	6.870E 01	0	-2.751E 02	0	-1.268E 02
120	6.928E 01	0	-3.326E 02	0	-1.280E 02
121	6.986E 01	0	-3.907E 02	0	-1.291E 02
122	7.044E 01	C	-4.496E 02	1.626E 01	-2.121E 01
123	7.101E 01	0	-3.720E 02	1.338E 02	0
124	7.159E 01	0	-2.950E 02	1.327E C2	0
125	7.217E 01	0	-2.187E 02	1.316E 02	0
126	7.275E 01	0	-1.431E 02	1.306E 02	0
127	7.332E 01	C	-6.797E 01	1.233E C2	0
128	7.390E 01	C	-7.222E-01	5.872E 01	0
129	7.448E 01	0	-1.667E-01	6.255E-01	0
130	7.506E 01	3.411E-C8	-1.023E-07	1.443E-C1	0
131	7.563E 01	0	0	8.861E-C8	-2.954E-08
132	7.621E 01	0	0	0	0
133	7.679E 01	0	0	0	0
134	7.736E 01	C	0	0	0
135	7.794E 01	0	0	0	0
136	7.852E 01	0	0	0	0
137	7.910E 01	0	0	0	0

TABLE 7 -- MAXIMUM SUPPORT REACTIONS

STA	DIST X FT	MAX + REACT K	MAX - REACT K
14	8.083E 00	2.516E 02	0
50	2.887E 01	3.022E 02	0
86	4.965E 01	3.022E 02	0
122	7.044E 01	2.516E 02	0

TIME = 11 MINUTES, 8 AND 46/60 SECONDS

TABLE 8 -- SCALES FOR PLCT OUTPUT

DISTANCE	10 INCHES =	100 FT
MOMENT	2 INCHES =	1000 FT-K
SHEAR	2 INCHES =	200 K

TIME = 11 MINUTES, 30 AND 46/60 SECONDS

TIME = 11 MINUTES, 31 AND 20/60 SECONDS
00 HOURS, 12 MINUTES, 09 SECONDS.

END JCB 08064. 14.05.00

TOTAL NUMBER OF PAGES 051