# OPERATIONAL ANALYSIS OF SHARED LANE MARKINGS AND GREEN BIKE LANES ON ROADWAYS WITH SPEEDS GREATER THAN 35 MPH 

Contract No. BDK 82-977-04
FINAL REPORT
January 2014

Report Prepared by:

Thobias Sando, Ph.D., P.E., PTOE (PI)
School of Engineering
University of North Florida
1 UNF Drive
Jacksonville, FL 32224
Tel: (904) 6201142
Fax: 904-620-1391
Email: t.sando@unf.edu


Report Prepared for:

The Florida Department of Transportation
Research Center
605 Suwannee Street, MS 30
Tallahassee, FL 32399


## DISCLAIMER

The opinions, findings, and conclusions, expressed in this publication are those of the authors and not necessarily those of the State of Florida Department of Transportation.

## APPROXIMATE CONVERSIONS TO SI UNITS

| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| :---: | :---: | :---: | :---: | :---: |
| LENGTH |  |  |  |  |
| in | inches | 25.4 | millimeters | mm |
| ft | feet | 0.305 | meters | m |
| yd | yards | 0.914 | meters | m |
| mi | miles | 1.61 | kilometers | km |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| AREA |  |  |  |  |
| $i n^{2}$ | square inches | 645.2 | square millimeters | $\mathrm{mm}^{2}$ |
| $\mathrm{ft}^{2}$ | square feet | 0.093 | square meters | $\mathrm{m}^{2}$ |
| $\mathrm{yd}^{2}$ | square yard | 0.836 | square meters | $\mathrm{m}^{2}$ |
| ac | acres | 0.405 | hectares | ha |
| $\mathrm{mi}^{2}$ | square miles | 2.59 | square kilometers | $\mathrm{km}^{2}$ |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| VOLUME |  |  |  |  |
| fl oz | fluid ounces | 29.57 | milliliters | mL |
| gal | gallons | 3.785 | liters | L |
| $\mathrm{ft}^{3}$ | cubic feet | 0.028 | cubic meters | $\mathrm{m}^{3}$ |
| $\mathrm{yd}^{3}$ | cubic yards | 0.765 | cubic meters | $\mathrm{m}^{3}$ |
| NOTE: volumes greater than 1000 L shall be shown in $\mathrm{m}^{3}$ |  |  |  |  |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| MASS |  |  |  |  |
| oz | ounces | 28.35 | grams | g |
| lb | pounds | 0.454 | kilograms | kg |
| T | short tons (2000 lb) | 0.907 | megagrams (or "metric ton") | Mg (or "t") |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| TEMPERATURE (exact degrees) |  |  |  |  |
| ${ }^{0} \mathrm{~F}$ | Fahrenheit | 5 $(\mathrm{~F}-32) / 9$ <br> or $(\mathrm{F}-32) / 1.8$  | Celsius | ${ }^{\circ} \mathrm{C}$ |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| ILLUMINATION |  |  |  |  |
| fc | foot-candles | 10.76 | lux | 1 x |
| fl | foot-Lamberts | 3.426 | candela/m ${ }^{2}$ | $\mathrm{cd} / \mathrm{m}^{2}$ |
| SYMBOL | WHEN YOU KNOW | MULTIPLY BY | TO FIND | SYMBOL |
| FORCE and PRESSURE or STRESS |  |  |  |  |
| lbf | pound force | 4.45 | newtons | N |
| $\mathrm{lbf} / \mathrm{in}^{2}$ | pound force per square inch | 6.89 | kilopascals | kPa |

## TECHNICAL REPORT DOCUMENTATION PAGE

|  | 2. Government Accession No |  | 3. Recipient's Catalog No, |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4. Title and Subtitle Operational Analysis of Shared Lane Markings and Green Bike Lanes on Roadways with Speeds Greater Than 35 mph |  |  | 5. Report <br> January 20 <br> 6. Perform | Organization Co |  |
| 7. Author: Sando, T. and Hunter, W |  |  | 8. Performing Organization Re |  |  |
| 9. Performing Organization Name and Address University of North Florida <br> 1 UNF Drive <br> Jacksonville, FL 32224 |  |  | 11. Contract or Grant No. BDK82-977-04 |  |  |
| 12. Sponsoring Agency Name and Address Florida Department of Transportation Research Center 605 Suwannee Street, MS 30 Tallahassee, FL 32399 |  |  | 13. Type of Report and Period Covered Final Report <br> May 18, 2010 to August 31, 2012 |  |  |
| 15. Supplementary Notes |  |  |  |  |  |
| 16. Abstract <br> This study analyzed the effectiveness of shared lane markings (sharrows), wide curb lanes, standard and buffered bike lanes, and green bike lanes on improving operations of bicycle facilities. Three measures of effectiveness were used in this study: lateral separation between the motor vehicle and bicyclist, the distance of bicyclists to the curb or edge of pavement, and the yielding behavior of drivers and cyclists at merge points. Also, motor vehicle speeds before, while, and after passing bicyclists were analyzed. Except for the Bridge of Lions site, the before-and-after data indicate that installation of sharrows led to an increase in lateral separation between motor vehicles and bicyclists. At Riverside Drive, the separation increased by 0.67 feet, while at the North $56^{\text {th }}$ Street site, an increase of 2.55 feet was observed after installing sharrows and increasing the outside lane width. Data also suggested a significant improvement in lateral separation of 0.86 feet at Sunset Drive, which was widened to create a wider outside lane (but had no shared lane markings), and Bailey Road, where a marked buffer between the travel lane and bike lane resulted in an increase in separation between motor vehicles and bicyclists of 0.72 feet. It was also observed that bicyclists rode further from the curb/edge of pavement for the after-period compared to the before-period for Riverside Drive, Bridge of Lions, North $56^{\text {th }}$ Street, and Sunset Drive. P-values less than 0.05 were observed for these five sites suggesting that the treatments were effective in moving bicyclists further from the curb/edge of pavement. Data also indicates that drivers slow down as they pass bicyclists on nonlimited access roadways (before speed of 32.02 mph to 29.97 mph while-passing) and then increase their speeds after overtaking the bicyclists ( 30.80 mph while-passing to 32.82 mph after-passing). The difference between the speeds before-passing and while-passing, and while-passing and after-passing, were both significant with a pvalue less than 0.000 . However, when the before-passing ( 32.02 mph ) and after-passing ( 32.54 mph ), excluding while-passing speeds, were analyzed, no significant difference was found ( p -value $=0.110$ ). For limited access facilities, the difference between the overtaking driver's speed before-passing ( 37.35 mph ) and while-passing $(34.93 \mathrm{mph})$ the bicyclists was significant with a p -value of 0.000 . However, the difference between motor vehicle speeds while-passing bicyclists ( 34.94 mph ) and after-passing ( 35.48 mph ) was not significant ( p -value $=$ $0.150)$. Contrary to the non-limited access streets, the difference between vehicle speeds before- ( 37.33 mph ) and after-passing ( 35.48 mph ) was significant for the limited access facilities ( p -value $=0.017$ ). |  |  |  |  |  |
| 17. Key Word Bicyclist, sharrows, bike lanes, green bike lanes |  |  | 18. Distribution Statement No restrictions. |  |  |
| 19. Security Classif. (of this report) Unclassified. |  | 20. Security (of this page) Unclassified. |  | 21. No. of Pages 91 |  |

## ACKNOWLEDGEMENTS

The author wishes to thank the Florida Department of Transportation (FDOT) for sponsoring this project. Special thanks go to Mary Anne Koos, Project Manager, for working tirelessly to bring this project to fruition. Graduate research assistants, Michelle Angel, Pham Giao Quynh, and Hoang Hung deserve special recognition for their significant contribution in making this study a success.

This research project would not have been possible without the full support of the cyclists, videographers, traffic technicians, and designers who gave their time to ensure that we would have high quality, reliable data that represented all of Florida's communities. A special thank you to the following teams:

Central Office: FDOT: Dennis Felton, Matt Cosgrove, and Shawn Trotman
District 2: FDOT: Chris Ledew, Scott Lent, Jennifer Graham, Rachel Walton, Joshua Reichert, Austin Chapman, Rodney Cooper and Doreen JoynerHoward
North Florida Transportation Planning Organization: Elizabeth De Jesus
City of Jacksonville: James Reed City of St. Augustine: Police Department

District 3: FDOT: Miranda Glass, J.C. Spivey, and Cliff Johnson
District 4: FDOT: John Mark Palacios
Palm Beach Metropolitan Planning Organization: Bret Baronak
Broward County: Mark Horowitz
District 5: FDOT: Joan Carter, Todd Alexander, George Borchik, Steve Tonjes, Jeff Shepard, and Mary McGehee
Revolutions Cyclery: Ginger Twigg
District 6: FDOT: Ken Jeffries, Chris Tavella, Heidi Solaun, Danny Iglesias, Felix Hernandez, Omar Meitin, Aileen Boucle, Hong Benitez, Andre Diaz, and Jorge Rodriguez
Miami-Dade Metropolitan Planning Organization: David Henderson Kimley-Horn and Associates, Inc: John McWilliams, Stewart Robertson, Katie Vila, Doug Cobb, Stephan Mack, Denise Chevrenak, and Kelsey Lewis

District 7: FDOT: Lori Marable, Stephen Benson, Ron Chin, Peter Hsu, Rochelle Garrett, Mary Lou Godfrey, Kelli Bradley, Susan Moore,<br>Katasha Cornwell, Thomas Curley, Britni West, Brit Hardy, Kellie Ziemak, and Sally Prescott,<br>City of Temple Terrace: Robert Gordon, Police Department<br>City of Tampa: Bernadette Corey<br>City Bike Tampa: Kevin Craft, and Jim Schirk

## EXECUTIVE SUMMARY

The main objective of this study was to examine the effectiveness of shared lane markings (sharrows) and bike lanes on higher speed roadways and operational impacts of bicyclists on limited access bridges. Thus, this study collected before-and-after data on selected roadways that were retrofitted by installing sharrows or bike lanes. In addition to studying the performance of the shared lane markings, the effects of green bike lanes in improving awareness and yielding at complex intersections and limited access ramps was evaluated. One site, Sunset Drive in Miami, was retrofitted by widening the outside lane, and was included in the study. A reconstruction of Bailey Road in Ft. Lauderdale provided an opportunity to compare a bike lane section configured with an adjacent buffer zone, and a bike lane without a buffer.

Three main measures of effectiveness were used in this study: lateral separation between the motor vehicle and bicyclist, the distance of bicyclists to the curb or edge of pavement, and the yielding behavior of drivers and cyclists at merge points. Also, motor vehicle speeds before, while, and after passing bicyclists were analyzed.

Due to variability in site characteristics, a separate statistical analysis was conducted for each site. Statistical analyses consisted of determining descriptive statistics and performing an Analysis of Variance (ANOVA).

The analysis of the before-and-after data at Riverside Drive in Jacksonville and North $56{ }^{\text {th }}$ Street in Temple Terrace indicate a significant increase in lateral separation between motor vehicles and bicyclists after installation of sharrows. Only one site, the Bridge of Lions in St Augustine, did not show improvement in lateral space between vehicles and bicyclists, and was not significant. This might be attributed to the type of roadway section. This is a two-lane undivided bridge, and the only site with a single lane in each direction.

This study also compared operational characteristics between sharrows and bike lanes at the North $56^{\text {th }}$ Street site in Tampa. The difference in lateral separation between the sharrows and the bike lane segments was not significant ( p -value $=0.216$ ). However, the results suggest a significant difference for the distance from the bicyclists to the curb or edge of pavement. It was also observed that vehicles did not slow down on the bike lane section as much as they did at the sharrows site. However, this was an anecdotal observation.

Retrofitting Sunset Drive to include a wider outside lane improved the separation between vehicles and bicyclists. The improvement in the lateral clearance was also observed when comparing the buffer and no-buffer bike lane sections at the Bailey Road site. For all sites except Bailey Road, the distance between the bicyclists and the curb or edge of pavement significantly increased in the after-periods.

For limited access facilities, it was observed that bicyclists prefer crossing straight paths rather than less conventional (skewed) crossings, and used the shortest path to cross on- and off-ramps. It was also observed that bicyclists preferred to keep some momentum rather than stopping when crossing the ramps.

## TABLE OF CONTENTS

DISCLAIMER ..... ii
APPROXIMATE CONVERSIONS TO SI UNITS ..... iii
TECHNICAL REPORT DOCUMENTATION PAGE ..... iv
ACKNOWLEDGEMENTS ..... v
EXECUTIVE SUMMARY ..... vi
LIST OF FIGURES ..... ix
LIST OF TABLES ..... xii
1 INTRODUCTION ..... 1
2 RESEARCH OBJECTIVES ..... 2
3 LITERATURE REVIEW ..... 3
4 STUDY SITES ..... 10
5 METHODOLOGY ..... 13
5.1 Data Collection ..... 13
5.2 Data Reduction ..... 13
5.3 Data Analysis ..... 14
6 ANALYSIS AND RESULTS ..... 16
6.1 Riverside Drive (Jacksonville) ..... 16
6.1.1 Analysis of Variance ..... 19
6.2 Bridge of Lions (St. Augustine) ..... 19
6.2.1 Analysis of Variance ..... 22
6.3 North $56^{\text {th }}$ Street (Temple Terrace) ..... 23
6.3.1 Sharrows (Site A) ..... 23
6.3.2 Analysis of Variance for Wider Outside Lane and Sharrows (Site A) ..... 25
6.3.3 Bike Lane (Site B) ..... 26
6.3.4 Analysis of Variance for Bike Lane (Site B) ..... 29
6.4 Sunset Drive (Miami) ..... 29
6.4.1 Descriptive Analysis ..... 31
6.4.2 Analysis of Variance ..... 33
6.5 Bailey Road (Fort Lauderdale) ..... 33
6.5.1 Site Description ..... 33
6.5.2 Descriptive Statistics ..... 35
6.5.3 Analysis of Variance ..... 36
6.6 Limited Access Facility Pilot Projects ..... 37
6.6.1 Limited Access Sites ..... 37
6.6.2 Analysis of Driver Characteristics at Crossing Locations ..... 40
6.6.3 Analysis of Bicyclist Characteristics at Crossing Locations ..... 40
6.6.4 Analysis of Pineda Causeway Data ..... 41
6.7 Speed Analysis ..... 43
6.8 Vehicle Lateral Shift ..... 48
7 CONCLUSIONS ..... 49
7.1 Lateral Spacing between Vehicles and Bicyclists ..... 49
7.2 Lateral Spacing between Bicyclists and Curb ..... 49
7.3 Limited Access Crossing. ..... 50
7.4 Speed Analysis ..... 50
REFERENCES ..... 51
APPENDIX A ..... 53
I-195/Julia Tuttle Causeway Fact Sheets ..... 54
APPENDIX B ..... 56
Bicycle Crash Maps for Limited Access Pilot Studies ..... 57
APPENDIX C ..... 60
Bicycles on Limited Access Bridges ..... 61
APPENDIX D ..... 62
Pineda Causeway Fact Sheet ..... 63
APPENDIX E ..... 64
William Lehman Causeway Bike Lane Concept ..... 65
APPENDIX F ..... 68
William Lehman Causeway Fact Sheets ..... 69
APPENDIX G ..... 71
Bridge of Lions Sharrows Plan ..... 72
APPENDIX H ..... 73
Additional Photographs ..... 74

## LIST OF FIGURES

FIGURE 4.1 Locations of Study Sites ..... 11
FIGURE 5.1 Pipe Attachment ..... 13
FIGURE 5.2 Vehicle Lateral Shift into Adjacent Lane Quartiles ..... 14
FIGURE 6.1 Riverside Drive Northbound ..... 16
FIGURE 6.2 Shared Lane Markings: Riverside Drive ..... 17
FIGURE 6.3 Distribution of Bicyclist to Vehicle Separation: Riverside Drive ..... 18
FIGURE 6.2 Distribution of Bicyclist to Curb Distance: Riverside Drive ..... 18
FIGURE 6.5 Shared Lane Markings: Bridge of Lions ..... 20
FIGURE 6.6 Distribution of Bicyclist to Vehicle Separation: Bridge of Lions ..... 21
FIGURE 6.7 Distribution of Bicyclist to Curb Distance: Bridge of Lions ..... 22
FIGURE 6.8 Wider Outside Lanes with Shared Lane Markings: N 56 ${ }^{\text {th }}$ Street (Site A) ..... 23
FIGURE 6.9 Distribution of Bicyclist to Vehicle Separation: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings ..... 24
FIGURE 6.10 Distribution of Bicyclist to Curb Distance: N $56^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings ..... 25
FIGURE 6.11 Bike Lanes: N 56 ${ }^{\text {th }}$ Street (Site B) ..... 27
FIGURE 6.12 Distribution of Bicyclist to Vehicle Separation-Comparing Bike Lane and Sharrows: N 56th Street ..... 28
FIGURE 6.13 Distribution of Bicyclist to Curb Distance-Comparing Bike Lane and Sharrows: N 56th Street ..... 28
FIGURE 6.14 Sunset Drive (before scenario) ..... 30
FIGURE 6.15 Sunset Drive (after scenario) ..... 30
FIGURE 6.16 Distribution of Bicyclist to Vehicle Separation: Sunset Drive. ..... 31
FIGURE 6.17 Distribution of Bicyclist to Edge of Pavement Distance: Sunset Drive ..... 32
FIGURE 6.18 Section with 5-foot Bike Lane: Bailey Rd. ..... 34
FIGURE 6.19 Section with 4-foot Bike Lane and 2-foot Buffer: Bailey Rd. ..... 34
FIGURE 6.20 Distribution of Bicyclist to Vehicle Distance: Bailey Road ..... 35
FIGURE 6.21 Distribution of Bicyclist to Curb Distance: Bailey Road ..... 36
FIGURE 6.22 William Lehman Cswy Westbound Off-ramp ..... 37
FIGURE 6.23 William Lehman Cswy Westbound On-ramp ..... 38
FIGURE 6.24 William Lehman Cswy Eastbound Off-ramp ..... 38
FIGURE 6.25 William Lehman Cswy Eastbound On-ramp ..... 39
FIGURE 6.26 Julia Tuttle Cswy Westbound (Mt. Sinai) On-ramp ..... 39
FIGURE 6.27 Bicyclist Group Riding from US 1 to Pineda Cswy Eastbound After Green Bike Lane Installation. ..... 41
FIGURE 6.28 Bicyclist Crossing from US 1 to Pineda Cswy Eastbound Before Green Bike Lane Installation ..... 42
FIGURE 6.29 Bicyclist Crossing from US 1 to Pineda Cswy Eastbound Before Green Bike Lane Installation ..... 42
FIGURE 6.30 Bicyclist Crossing Ramp from US 1 to Pineda Cswy Eastbound After Green Bike Lane Installation. ..... 43
FIGURE 6.31 Average Vehicle Speed during Passing Events (Non-Limited Access Sites) ..... 44
FIGURE 6.32 Average Vehicle Speed during Passing Events (Limited Access Sites) ..... 45
FIGURE A. 1 Julia Tuttle Cswy Bicycle Improvement Details ..... 54
FIGURE A. 2 Julia Tuttle Cswy Bicycle Improvement Plan ..... 55
FIGURE B. 1 Pineda Cswy Bicycle Crash Map 2005-2010 ..... 57
FIGURE B. 2 Julia Tuttle Cswy Bicycle Crash Map 2005-2010 ..... 58
FIGURE B. 3 William Lehman Cswy Bicycle Crash Map 2005-2010 ..... 59
FIGURE D. 1 Pineda Cswy Bicycle Improvement Details ..... 63
FIGURE E. 1 William Lehman Cswy Bike Lane Plan Sheet 1 of 3 ..... 65
FIGURE E. 2 William Lehman Cswy Bike Lane Plan Sheet 2 of 3 ..... 66
FIGURE E. 3 William Lehman Cswy Bike Lane Plan Sheet 3 of 3 ..... 67
FIGURE F. 1 William Lehman Cswy Bicycle Improvement Details ..... 69
FIGURE F. 2 William Lehman Cswy Bicycle Improvement Plan ..... 70
FIGURE G. 1 Bridge of Lions Sharrows Placement Plan ..... 72
FIGURE H. 1 Driver Yielding Right-of-Way to Bicyclist at Ramp Crossing on Julia Tuttle Cswy74
FIGURE H. 2 Bicyclist Crossing Ramp Using Green Bike Lane on Julia Tuttle Cswy ..... 74
FIGURE H. 3 Cyclist Group Using Wide Shoulder on Julia Tuttle Cswy ..... 75
FIGURE H. 4 Sharrows on Lehman Cswy On-Ramp ..... 75
FIGURE H. 5 Bicyclist using Bike Lane Approaching Lehman Cswy On-Ramp ..... 76
FIGURE H. 6 Cyclist Checking Traffic before Crossing Lehman Cswy On-Ramp ..... 76
FIGURE H. 7 Bicyclist Yielding to Traffic before Crossing Lehman Cswy On-Ramp ..... 77
FIGURE H. 8 Bicyclist Negotiating with Driver before Crossing Lehman Cswy On-Ramp ..... 77
FIGURE H. 9 Bicyclist Crossing Ramp After Yielding to Driver (Fig. H.8) ..... 78
FIGURE H. 10 Bicyclist Crossing Ramp Using Bike Lane at Lehman Cswy On-Ramp ..... 78

## LIST OF TABLES

TABLE 4.1 Roadway Characteristics of Study Sites ..... 12
TABLE 6.1 Lateral Clearance between Bicyclist and Vehicle: Riverside Drive ..... 17
TABLE 6.2 Lateral Clearance between Bicyclist and Curb: Riverside Drive ..... 17
TABLE 6.3 ANOVA Results for Sharrows Effect (Cyclist to Vehicle): Riverside Drive ..... 19
TABLE 6.4 ANOVA Results for Sharrows Effect (Cyclist to Curb): Riverside Drive ..... 19
TABLE 6.5 Lateral Clearance between Bicyclist and Vehicle: Bridge of Lions ..... 20
TABLE 6.6 Lateral Clearance between Bicyclist and Curb: Bridge of Lions. ..... 21
TABLE 6.7 ANOVA Results for Sharrows Effect (Cyclist to Vehicle): Bridge of Lions ..... 22
TABLE 6.8 ANOVA Results for Sharrows Effect (Cyclist to Barrier): Bridge of Lions ..... 22
TABLE 6.9 Lateral Clearance between Motor Vehicle and Bicyclist: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings ..... 24
TABLE 6.10 Lateral Clearance between Bicyclist and Curb: N $56^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings ..... 25
TABLE 6.11 ANOVA Results for Wider Outside Lane with Shared Lane Markings (Cyclist to Vehicle): N 56 ${ }^{\text {th }}$ Street (Site A) ..... 26
TABLE 6.12 ANOVA Results for Wider Outside Lanes with Shared Lane Markings (Cyclist to Curb): N $56^{\text {th }}$ Street (Site A) ..... 26
TABLE 6.13 Lateral Clearance between Motor Vehicle and Bicyclist-Comparing Bike Lane and Sharrows: N 56 ${ }^{\text {th }}$ Street ..... 27
TABLE 6.14 Lateral Clearance between Bicyclist and Curb/Edge of Pavement-Comparing Bike Lane and Sharrows: N $56^{\text {th }}$ Street ..... 27
TABLE 6.15 ANOVA Results for Sharrows vs. Bike Lane (Cyclist to Vehicle): N 56th Street. 29
TABLE 6.16 ANOVA Results for Sharrows vs. Bike Lane (Cyclist to Curb/Edge of Pavement):N 56th Street ......................................................................................................................... 29
TABLE 6.17 Lateral Clearance between Motor Vehicle and Bicyclist: Sunset Drive ..... 31
TABLE 6.18 Lateral Clearance between Bicyclist and Edge of Pavement: Sunset Drive ..... 32
TABLE 6.19 ANOVA Results for Wider Outside Lane Effect (Cyclist to Vehicle): Sunset Drive33

TABLE 6.20 ANOVA Results for Wider Outside Lane Effect (Cyclist to Edge of Pavement): Sunset Drive 33

TABLE 6.21 Lateral Clearance between Motor Vehicle and Bicycle: Bailey Rd. ...................... 35
TABLE 6.22 Lateral Clearance from Bicyclist to Curb: Bailey Rd. ............................................ 36
TABLE 6.23 ANOVA Results for Buffer vs. no Buffer (Cyclist to Vehicle): Bailey Road........ 36
TABLE 6.24 ANOVA Results for Buffer vs. no Buffer (Cyclist to Curb): Bailey Road ............ 37
TABLE 6.25 Operational Characteristics at Crossing Locations ................................................. 40
TABLE 6.26 Average Speed in Terms of Vehicle Types (Non-Limited Access Sites)............... 43
TABLE 6.27 Average Speed in Terms of Vehicle Types (Limited Access Sites) ....................... 44
Table 6.28 Paired T-Test Results for Motor Vehicle Speeds (Non-Limited Access Streets)....... 46
Table 6.29 Paired T-Test Results for Motor Vehicle Speeds (Limited Access Causeways) ........ 47
Table 6.30 Lateral Shift Data for Study Sites ............................................................................... 48
TABLE C. 1 Pilot Projects of Bicycles on Limited Access Bridges ............................................. 61

## 1 INTRODUCTION

Across the United States, a great deal of attention is being focused on promoting energy efficient and environmentally friendly modes of transportation. Bicycling is an integral part of a sustainable transportation system as it is one of the most energy-efficient, cost effective, and environment-friendly modes of transportation. Transportation agencies are looking for ways to better support the growing use of bicycles for transportation and leisure activities in the existing built environment.

In the past, urban design criteria in Florida allowed for the construction of wide outside lanes rather than bike lanes when the corridor was constrained. Wide outside lanes are typically 14 feet wide to allow a shared use by motor vehicles and bicycles. Environmental and historical constraints led to the reconstruction of the Bridge of Lions in St. Augustine with narrow (11") lanes and no bike lanes. Shared lane markings, also known as "sharrows" are a strategy for mitigating the lack of a bike lane. Their intent is to encourage appropriate placement of cyclists in the roadway and sharing of the lane by motorists. According to the Manual on Uniform Traffic Control Devices (MUTCD, 2009) (12), sharrows are known to offer the following benefits:
A. Assist bicyclists with lateral positioning in a shared lane with on-street parallel parking in order to reduce the chance of a bicyclist hitting an open door of a parked vehicle,
B. Assist bicyclists with lateral positioning in lanes that are too narrow for a motor vehicle and a bicycle to travel side by side within the same traffic lane,
C. Alert road users of the lateral location bicyclists are likely to occupy within the traveled way,
D. Encourage safe passing of bicyclists by motorists, and
E. Reduce the incidence of wrong-way bicycling.

## 2 RESEARCH OBJECTIVES

This research is a follow-up study that builds on a previous FDOT research study on the evaluation of restriping roadways to create wider outside lanes for bicyclists. The earlier study, titled Operational and Safety Impacts of Restriping Inside Lanes or Urban Multi Lane Curbed Roadways to 11 Feet or Less to Create Wider Outside Curb Lanes for Bicyclists (BDK82-87701), and can be found on the web page for FDOT's Research Office.

The main objective of this study was to examine the effectiveness of bike lanes and shared lane markings (sharrows) on higher speed roadways, and included FDOT's three pilot projects that allow bicycle travel on limited access highways. Thus, a before-and-after study was conducted on selected roadways that were retrofitted by installing bike lanes and/or sharrows. The study evaluated the effects of green bike lanes in improving awareness and yielding at complex intersections and limited access ramps. In addition to the state highways included in the study, Broward County, Florida participated by constructing a new roadway with two styles of bike lanes (buffered and traditional) that we were able to compare.

The study will be used by transportation officials in Florida to collect data on the operational performance of the bicycle pilot projects on limited access roadways and refine guidelines on the design of bicycle facilities, including the use of sharrows. Also, this study will provide guidance on suitability of different methodologies used for videotape data collection and reduction.

## 3 LITERATURE REVIEW

Many cities and states have started implementing shared lane markings on roadways to encourage the safe coexistence of bicycles and motor vehicles. However, very few localities have formally evaluated the impact of these markings on safety or operations. In the late 1990s, Pein, Hunter, and Stewart conducted a before-after study of a variant of the bike-in-house marking implemented on a four-lane high volume ( 35,000 vehicles per day) roadway with a 30 $\mathrm{mi} / \mathrm{h}(48 \mathrm{~km} / \mathrm{h})$ speed limit in Gainesville, FL (1). The roadway had wide outside lanes 15 feet to the curb and no on-street parking. The center of the bike-in-house marking was placed 3.5 feet from the curb face.

In the before-period 39 percent of bicyclists were riding in the same direction as traffic. This increased to 45 percent in the after-period, and the increase was statistically significant. Bicyclists rode an average of 1.6 feet from the curb (tire to curb) in the before-period and 1.8 feet from the curb in the after-period-a shift of about 3 inches. This change was statistically significant but not thought to be practically significant. However, examining the distribution of distances showed a larger proportion of bicyclists riding 1.75 to 2.5 feet from the curb, indicating that more riders had additional maneuvering space toward the curb in the event that motor vehicles encroached into their space. This also potentially increased the comfort of bicyclists using the shared lane. Motorists allowed a mean of approximately 1.5 inches additional space when passing bicyclists in the after-period ( 6.1 feet) compared to the before-period ( 6.0 feet); however, this difference was also not thought to be practically significant. The mean and median motor vehicle distance to curb also increased slightly.

The San Francisco Department of Parking and Traffic conducted an evaluation of two shared lane marking designs - a bike-in-house design and a bike-and-chevron design (similar to the sharrow) - on streets with parallel parking (2). The study first conducted assessments to hypothesize an appropriate spacing for bicyclists to be able to avoid the door zone, which is the area where bicyclists risk colliding with an open door of a parked vehicle. By measuring vehicle doors in that locale, they found that the 85th percentile for the door zone extended 9.5 feet from the curb in the study areas. This distance included 7 feet from curb edge to outside of parked vehicle and 2.5 feet occupied by an opened door. From this, they concluded that bicyclists needed to ride at least 2.5 feet, or 30 inches, from parked vehicles to be relatively safe from an opened door. The marking treatments were subsequently implemented with the center of the markings 11 feet from the curb face to suggest a bicycle tracking position. This distance was intended to accommodate the 85th percentile distance of door clearance ( 9.5 feet plus 0.5 feet of shy distance plus half of the average bicycle width of 2 feet.

The San Francisco evaluation used data that were collected on six street segments before-andafter markings were introduced. Curb lane widths, including parking, ranged from about 17 to 19 feet on four 4-lane roads, and the curb lane widths, including parking, were 22 feet on two 2-lane roads. Each of the streets had moderate ( $2,000-4,000$ vehicles per lane per day) to heavy ( $>4,000$ vehicles per lane per day) traffic. In each of these locations, the bike-in-house marking was painted along one side of the road and the bike-and-chevron marking on the other side. Both shared lane markings led to the following results:

- 25 to 35 percent fewer sidewalk riders.
- More space ( 3 to 4 inches) between bicycles and parked vehicles.
- More space (more than 2 feet) between bicycles and passing motor vehicles in travel lanes.
- More space (about 1 foot) between motor vehicles in travel lanes and parked vehicles when no bicycles were present.

There were also reductions in the proportions of wrong-way riders associated with the bike-andchevron design. Due to the bike-and-chevron marking being more readily understood by bicyclists to indicate a preferred travel path, this marking was the preferred choice and ultimately approved for inclusion in the California Manual on Traffic Control Devices (3).

The Transportation Association of Canada (TAC) has now adopted the sharrow for use in Canada, and a paper by Jacobson, Skene, Davidson, and Rawsthorne (4) covers side-by-side, single file, and conflict zones applications. Recommendations for stencil placement and spacing are slightly different from that recommended in the 2009 MUTCD (12). For the conflict zones application, such as a motor vehicle off ramp and straight through bicycle movement, multiple sharrows may be used with a minimum spacing of 1.5 meters. Further research is recommended for stencil elongation as a function of roadway speed, stencil width, minimum sharrow placement from the curb for the full-time parking situation, marking schemes for part-time parking routes, and study of applications and dimensions as related to traffic volume, motor vehicle speed, and vehicle class.

Brady, Loskorn, Mills, Duthie, and Machemehl examined the varying use of sharrows on three different streets in Austin, Texas (5). Sharrows were installed in the middle of the 11 -foot travel lanes on Guadalupe Street, a 4-lane, one-way street with parking on each side. With block lengths of approximately 370 feet, sharrows were installed 40 feet past each intersection, resulting in nominal spacing of 370 feet. Videotape data were collected during peak commuting hours when the parking spaces were rarely filled, thus giving bicyclists the opportunity to ride in the empty parking spaces. After sharrow placement, the average bicyclist lateral position (BLP) from the bicyclist's front wheel to the on-street parking space delineation or the outside of the edge of the parked motor vehicle increased from 3.14 to 3.51 feet, or 4.4 inches. The mode of the BLP observations shifted from 1.1 to 5.5 feet, indicating that an increased number of bicyclists were tracking over the center of the sharrow. The percentage of cyclists riding at a BLP of 4.4 to 6.6 feet, defined as the center of the lane, increased significantly from 31 to 42 percent after sharrow placement. Motorists passing bicyclists also significantly decreased. Bicyclists were significantly less likely to either ride on the sidewalk or in empty parking spaces after sharrows.

In a second evaluation, sharrows were also placed in the center of the lane on E $51^{\text {st }}$ Street, a 2way, 4-lane arterial in a 2,100 foot section where the bike lanes had been dropped. Sharrow spacing was 250 feet in the center of the outside lanes. After sharrow placement, the average BLP increased from 4.0 to 4.75 feet, an increase of 8 inches, and the mode of the BLP increased from 3 to 5 feet. The percentage of cyclists riding at a BLP of 4 to 6 feet, defined as the center of the lane, increased from 44 to 54 percent after sharrow placement ( $p=.069$ ). Sidewalk bicycle riding significantly decreased from 12 to 4 percent.

A third experiment was conducted on Dean Keeton Street, an arterial roadway where space did not allow bike lane placement throughout. Here, sharrows were placed 11 feet from the curb and
next to parked vehicles. The width from center of sharrow to center of the outside lane line was 11 feet. Parking spaces tended to stay filled. Before sharrows, the BLP was evenly distributed between 1.5 and 4.5 feet when motorists passed cyclists. After sharrows, approximately 70 percent of cyclists rode 3 feet from the parked motor vehicles ( $p=0.363$ ). During non-passing events the BLP mode was 4.5 feet. The percentage of cyclists riding within the door zone during a passing event significantly decreased from approximately 80 to 36 percent ( $\mathrm{p}<0.001$ ). During a non-passing event, the percentage of cyclists riding within the door zone significantly decreased from approximately 82 to 68 percent ( $\mathrm{p}<0.001$ ).

Hunter, Thomas, Srinivasan, and Martell performed three separate evaluations of shared lane markings for the FHWA (6). In Cambridge, MA, the evaluation compared a "before" condition with no markings to an "after" condition of sharrows placed at 10 foot spacing from the curb. The objective was to determine whether 10 foot spacing would have a positive effect on where cyclists and motorists were positioned compared with no sharrows. Assuming parked vehicles use 7 feet of space, this placement would result in the center of the sharrows being 3 feet from the parked vehicles. The sharrows were placed 10 feet from the curb for about 2,500 feet on Massachusetts Avenue, which is a 4-lane divided street with approximately 29,000 vehicles per day, parallel parking on both sides, and a speed limit of $30 \mathrm{mi} / \mathrm{h}$.

Results pertaining to the interaction of bicycles and motor vehicles included the following changes from the before-period to the after-period:

- The percentage of bicyclists who were taking the lane decreased from 13 to 8 percent.
- The percentage of bicyclists who kept moving safely (were riding safely and did not need to change speed or direction) increased from 73 to 90 percent.
- The percentage of bicyclists who made slight direction changes decreased from 17 to 6 percent.
- The percentage of bicyclists who yielded (changed direction or speed to give way to a motor vehicle) decreased from 23 to 7 percent.
- When a bicyclist was approaching, existing open vehicle doors decreased from 5 to 2 percent; opening of doors decreased from 4 to 0.3 percent; and motor vehicles pulling in or out of parking spaces decreased from 11 to 4.5 percent. No actual door events occurred in either the before-period or after-period.
- The percentage of motorists who made no movement to change lanes when overtaking a bicycle increased from 27 to 66 percent.
- The percentage of safe overtaking movements by motorists (approached and passed the cyclist without difficulty) increased from 94 to 98 percent.
- The percentage of motor vehicles making no movement (i.e., continuing to follow) when following bicycles increased from 44 to 65 percent.
- The percentage of motorists who yielded (changed direction or speed to give way to a bicycle) increased from 5 to 10 percent.
- The percentage of motorists who made complete lane changes decreased from 12 to 3 percent.
- The percentage of motorists who made slight direction changes decreased from 38 to 22 percent.
- The percentage of motorists who slowed increased from 5 to 10 percent.
- The percentage of motorists who made no change while following a bicyclist increased from 44 to 65 percent.
- The percentage of avoidance maneuvers decreased from 76 to 37 percent.

All of these differences were independent of inbound or outbound direction. Taken together, the results portray a more segregated flow with less lateral movement of bicycles and motor vehicles after sharrow installation.

Results pertaining to the spacing of bicycles and motor vehicles in the presence of a following motor vehicle in the after-period included the following:

- The distance from a bicyclist riding beside a parked motor vehicle increased from 40.1 to 42.3 inches when both directions were combined and increased from 37.4 to 41.5 inches for the inbound direction.
- Outbound spacing was 42.7 inches in the before-period and 43.1 inches in the after-period.
- The percentage of bicyclists who rode within 40 inches (i.e., near the door zone) of parked motor vehicles decreased. Most of the effect was in the inbound direction with a decrease from 58 to 41 percent. Comparable outbound values were 44 percent in the before-period and 38 percent in the after-period.
- The percentage of bicyclists who rode within 30 inches (i.e., within the door zone) remained unchanged at 13 percent.

Results pertaining to the spacing of bicycles and motor vehicles in the absence of a following motor vehicle in the after-period included the following:

- The change in distance between a bicyclist and a parked motor vehicle was negligible (approximately 45 inches before and after).
- The percentage of bicyclists who rode within 40 inches of parked motor vehicles increased from 37.5 to 45 percent, although this may reflect the high percentage of bicyclists who rode over the sharrows.
- When motorists drove past parked motor vehicles, the spacing increased 16 inches (from 77.4 to 93.6 inches) in the inbound direction, 12 inches (from 84.5 to 96.5 inches) in the outbound direction, and 14 inches (from 80.9 to 95.0 inches) combined.

Overall results from Cambridge, MA, indicate the following:

- A total of 94 percent of bicyclists rode over the sharrows.
- There was more operating space for bicycles as motor vehicle spacing from parked motor vehicles increased.
- A number of variables related to the operations of bicycles and motor vehicles showed positive effects.
- Placement of the sharrows 10 ft . from the curb (instead of 11 ft .) was not a problem.

In a second evaluation in Chapel Hill, NC, the sharrows were placed 43.5 inches from the curb along Martin Luther King, Jr. Boulevard (MLK) for 1.25 miles. MLK is a street with a 5-lane cross section (4 travel lanes and a center two-way left turn lane) with no parking, 27,000 vehicles per day, a speed limit of $35 \mathrm{mi} / \mathrm{h}$, and periodic sunken drain grates next to the curb. There was a 3 to 4 percent grade where the videotape data were collected. The street had previously been resurfaced, and the outside lanes were marked nominally as 15 - ft -wide lanes. The spacing of bicycles and motor vehicles from the curb and in situations where motorists passed bicyclists was of primary interest.

Results pertaining to the interaction of bicycles and motor vehicles included the following changes from the before-period to the after-period:

- A total of 91 percent of the bicyclists rode over the sharrows- 97 percent in the downhill direction and 88 percent in the uphill direction. Bicyclists riding uphill traveled slower and tended to ride closer to the curb.
- The percentage of motorists who made no movement to change lanes when overtaking a bicyclist increased from 24 to 32 percent.
- There was no difference in the proportion of bicyclists riding near the curb (approximately 98 percent) or taking the lane (approximately 2 percent).
- The percentage of avoidance maneuvers decreased from 81 to 71 percent.
- The percentage of motorists staying in the lane when following bicyclists increased from 20 to 29 percent.
- There was no change in the percentage of bicyclists or motorists who yielded.

Results pertaining to the spacing of bicycles and motor vehicles included the following:

- In the presence of a following motor vehicle in the after-period, bicyclists rode closer to the curb after the sharrows by about 2.5 inches ( 40.1 to 37.7 inches). The effect was more pronounced downhill (4.6 inches closer) versus uphill ( 2.9 inches closer). Similar to Cambridge, MA, this was likely a reflection of bicyclists tracking over the sharrows.
- There were slight increases in the percentages of bicyclists who rode within 30 and 40 inches of the curb. The percentage within 30 inches increased from 12.5 to 15 percent downhill and 47.3 to 50.5 percent uphill.
- When motorists passed bicyclists in the after-period, there was a small decrease in the passing distance overall from 82 to 79 inches. In the downhill direction, motorists passed 7 inches closer to bicycles (from 84.7 to 77.7 inches). There was no change in the uphill direction (from 80.0 to 81.1 inches).
- The percentage of passing motor vehicles within 50 inches showed only small and insignificant differences (from 2.0 to 2.6 percent).
- When the distance of the right front tires of motor vehicles from the curb in the absence of bicycles was examined in the after-period, the spacing increased 8.3 inches in the uphill direction (from 64.4 to 72.7 inches), 4.7 inches in the downhill direction (from 76.6 to 81.3 inches), and 7 inches overall (from 70.5 to 77.0 inches).
- The percentages of motor vehicles within 50 and 60 inches of the curb were also significantly lower in the after-period. The effect was most pronounced in the uphill
direction (from 16 to 4 percent within 50 inches and from 46 to 17 percent within 60 inches).
- Bicyclist sidewalk riding significantly decreased from 43 percent in the before-period to 23 percent in the after-period. In the downhill direction, sidewalk riding decreased from 39 to 10 percent, with no significant change in the uphill direction.
- Wrong-way riding by bicyclists was 11 percent in the before-period and 8 percent in the after-period (non-significant change).

Overall results from Chapel Hill, NC, indicate the following:

- A total of 91 percent of bicyclists tracked over the sharrows and rode at a safe distance from the edge of curb with more of an effect in the downhill direction.
- Motorists moved away from the sharrows, providing more operating space for bicyclists.
- A number of variables related to the operations of bicycles and motor vehicles showed positive effects.
- Bicyclist sidewalk riding decreased in the downhill direction.
- There was no change in the percentage of bicyclist wrong-way riding.

In a third evaluation in Seattle, WA, sharrows were placed in the center of the lane 12.25 feet from the curb on a downhill section of Fremont Street, which is a 2-lane street that has a speed limit of $30 \mathrm{mi} / \mathrm{h}, 10,000$ vehicles per day, 3.6 percent grade, and parking on both sides of the street. The placement was meant to encourage bicyclists to take the lane while traveling downhill. Data were collected in two additional periods following the before-period. The centerline of the street was repositioned to allow a 5 -foot bicycle lane and parking line to be installed on the uphill section of the street (after-period 1). Sharrows were then added in the downhill direction (after-period 2) since there was not enough width for bicycle lanes on both sides of the streets.

Results pertaining to the interaction of bicycles and motor vehicles included the following changes from the before-period to the after-period:

- There was no difference in the safety of the manner in which motorists were following and passing bicyclists. Overall, 97 percent of these maneuvers were considered to be performed safely.
- A total of 15 percent of the bicyclists rode over the sharrow during the after-period 2.
- A significantly higher percentage ( 51 versus 28 percent) of bicyclists shifted toward the center of the lane and took the lane during after-period 1 when the lane was narrowed to accommodate the addition of the bicycle lane in the uphill direction.
- The percentage of bicyclists who yielded (i.e., changed direction or speed to give way to a motor vehicle) decreased from 3.3 percent in the before-period to 2.8 percent in afterperiod 1 and 0.7 percent in after-period 2.
- The percentage of motorists who yielded (i.e., changed direction or speed to give way to a bicycle) decreased from 13 percent in the before-period to 6.5 percent in after-period 1 and 5 percent in after-period 2.

Results pertaining to the spacing of bicycles and motor vehicles included the following:

- In the absence of following motor vehicles, the average spacing between bicycles and parked motor vehicles did not significantly change across periods ( 45.8 inches in the before-period, 47.5 inches in after-period 1, and 44.5 inches in after-period 2).
- The percentage of bicyclist spacing values within 30 inches (i.e., within the door zone) increased from about 6 percent in the before-period to about 12 percent in the two after-periods.
- The percentage of bicyclist spacing values within 40 inches increased from 36 percent in the before-period to 39 percent in after-period 1 and 44 percent in after-period 2 (nonsignificant change).
- When motorists drove past parked motor vehicles in the absence of bicycles in both afterperiods, the average spacing decreased about 18 inches due to the change in the roadway configuration (the lane had been narrowed by 2.5 ft .)

Overall results from Seattle, WA, indicate the following:

- Sharrow placement alone did not seem to result in an increase in the percentage of bicyclists taking the lane.
- Bicyclists were already riding out of the door zone in the before-period and stayed in this location in both after-periods. Sharrows had previously been installed 11 ft . from the curb next to parked cars over a 2,000 -ft, four-lane section of Fremont Street leading into the section studied in the current project.
- It is possible that narrowing the travel lanes and adding the uphill bike lane had more of an effect on operations and spacing than the addition of sharrows.
- The bicyclists riding in the street seemed experienced and showed that it was not necessary to ride in the middle of the lane to control the lane.

Similar operational and spacing measures have been used in studies evaluating operational effects of bicycle lanes and wide curb lanes (without shared lane markings). It has generally been found both in comparative studies $(7,8)$ and before-after studies (9) that the presence of a bicycle lane or shoulder stripe reduces lateral shifting by motor vehicles into an adjacent lane and increases tracking consistency for a given roadway width. The studies also report bicyclist shifts away from the roadway edge or parked vehicles with striping in place $(9,10)$. The van Houten and Seiderman study examined the effects of sequential bicycle lane markings compared with a baseline of only a roadway center line with no bicycle lane marking. This study found that there was less variability in bicycle tracking with the bike lane markings in place (10). The study also reported the overwhelming preference of bicyclists for the bike lanes, as well as the motorists' awareness of them.

Furth, Pulaski, Bussing, and Tavakolian determined that the distance between the curb and a parallel parked car increased as the parking lane width increased from 6 to 9 feet in a study conducted near Boston, Massachusetts (11). As the width of the parking lane increased from 6 to 7 to 8 to 9 feet, the proportion of vehicles parked more than 12 inches from the curb increased from $1 \%$ to $13 \%$ to $44 \%$ to $60 \%$. Thus, a strategy of narrowing parking lanes can provide more operating space for bicyclists.

## 4 STUDY SITES

Locations for data collection were determined by the Florida Department of Transportation (FDOT) with District and local government cooperation, based on the availability of roadway segments of varying characteristics, where higher volumes of bicyclists and pedestrians were known to occur, and where existing shared lane pavement markings (sharrows) for bicycle traffic were not already in place. Sites for the green bike lanes were selected based on pilot projects where the installation of green bike lanes at on-ramps of limited access facilities was underway.

Based on prior experience, it was determined that evaluating existing bicycle traffic would not produce sufficient data for the intended analysis. The FDOT districts were contacted to solicit volunteer riders. For liability reasons, riders had to be employees of participating institutions conducting the research (Florida State University and University of North Florida), the FDOT, or supportive local governments and transportation planning organizations. Research assistants were used only for operating cameras and collecting speed data.

As shown in Figure 4.1, study sites were distributed across the state. Table 4.1 details the roadway characteristics of all study sites. Three sites involved installation of sharrows while one site in Miami, Sunset Drive, was a retrofit from a symmetric multilane to asymmetric section with a wide outside lane. Another site in Fort Lauderdale (Bailey Road) had two different configurations, a conventional bike lane with no buffer and a bike lane with a buffer between motor traffic and bicycles. Three corridors were evaluated for travel by bicyclists on limited access bridges (Pineda, William Lehman, and Julia Tuttle Causeways) and included the installation of standard and green bike lanes. More information about characteristics of the limited access sites can be found in Appendices A to F.


FIGURE 4.1 Locations of Study Sites

TABLE 4．1 Roadway Characteristics of Study Sites

| City | Road Name | State Route No． | From | To | No． of Lanes | Lanes <br> Each Direction | Median Type | Posted Speed | Lane Width（feet） |  |  |  |  |  | Type of Lane Treatment |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  | Lane Treatment Before |  |  | Lane Treatment After |  |  |  |
|  |  |  |  |  |  |  |  |  | 或 | $\begin{aligned} & \stackrel{0}{\#} \\ & i=1 \end{aligned}$ | $\begin{aligned} & \text { む } \\ & 0 \end{aligned}$ | む | $\begin{aligned} & \frac{0}{\bar{z}} \\ & \frac{1}{2} \end{aligned}$ | \＃ |  |
| Jacksonville | Riverside Ave | SR 211 | Post St | Rosselle St | 3 | $\begin{aligned} & \text { 2-NB/ } \\ & \text { 1-SB } \end{aligned}$ | Undivided | 30 | 10 | 10 | 10 | 10 | 10 | 10 | Sharrows added |
| St．Augustine | Bridge of Lions | A1A | Avenida Menendez | N St． Augustine Blvd | 2 | 1 | Undivided | 30 | 11 | －－ | 11 | 11 | －－ | 11 | Sharrows added |
| Temple Terrace | N 56th St | SR 583 | Serena Dr | Mission Hills Ave | 5 | 2 | TWLT | 45 <br> Before／ 35 After | 12 | －－ | 12 | 11 | －－ | 13 | Lane restriping for 11 ft ．inner lane， 13 ft ．outer lane with Sharrows |
|  |  |  |  |  |  |  |  |  |  |  |  | 12 | －－ | 12 | 4 ft ．Bike lane added upstream of Sharrows section |
| Ft <br> Lauderdale | Bailey Rd | Broward County | Woodlands Blvd | NW 64th Ave | 5 | 2 | TWLT | 35 | 11 | －－ | 10.5 |  |  |  | Two Sites：a） 4 ft ．bike lane with 2 ft ．buffer zone，b） 5 ft ．bike lane added |
| Miami | Sunset Dr | SR 986 | SW 72nd Ave | $\begin{aligned} & \text { SW 68th } \\ & \mathrm{Ct} \end{aligned}$ | 4 | 2 | Divided | 40 | 12 | －－ | 12 | 11 | －－ | 14 | Widened pavement and restriped for 11 ft ．inner lane， 14 ft ．outer lane（no curb） |
| ＊Melbourne | Pineda Cswy | SR 404 | US 1 | $\begin{gathered} \text { S Patrick } \\ \text { Dr } \end{gathered}$ | 4 | 2 | Divided | 55 | 12 |  | 12 |  |  |  | Pilot Study：Included bike lanes， raised barriers，and green bike lane |
| ＊Miami | Lehman Cswy | SR 856 | Sullivan Dr | Edward Dr | 4 | 2 | Divided | 25 | 12 | －－ | 12 | 12 | －－ | 12 | 10 ft ．Existing shoulder marked as bike lane |
| ＊Miami | Julia <br> Tuttle <br> Cswy | SR 112 | Alton Rd | Bridge section | 6 | 3 | Divided | 55 | 12 | 12 | 12 | 12 | 12 | 12 | Added green bike lane |

Notes：＊Limited access facilities that are part of a pilot study；Riverside and Bridge of Lions，no gutter pan；Number of lanes equals total through lane width plus any continuous turn lanes．

## 5 METHODOLOGY

### 5.1 Data Collection

The data was collected at various times during the day, often in peak hour durations. In addition to the local cyclists who were traveling through the corridor, volunteer cyclists from FDOT, City, County, and Metropolitan Planning Organization (MPO) staff, as well as FDOT's design consultant were used to provide as many samples as possible of interactions of vehicles sharing the outside lane with bicyclists both before and after bike lanes or sharrow markings were added to the roadway segments. Members of the public who traveled the corridor by bicycle while data was being collected were also included in the analysis.

At each location, cyclists rode along the selected road segment while researchers videotaped their paths and interactions with motor vehicles. Video cameras were strategically located to capture the behavior of motor vehicles as they approached and passed a bicyclist. Efforts were made to conceal the camera and operator from passing traffic as to not influence driver behavior. A plastic pipe, one foot in length, was attached to the front and back of each bicycle as a control measure for determining distances from the video images. Figure 5.1 illustrates the pipe attached to the bicycle.

The lateral separation between the bicyclists and vehicles was measured as the distance from the left shoulder of the bicyclist to the right front passenger door of the vehicle. The distance between the bicyclist and the face of curb or edge of pavement was measured from the bicycle tire (center).


FIGURE 5.1 Pipe Attachment

### 5.2 Data Reduction

The field videos were converted to MPEG-4 video format in the laboratory. Using Adobe Photoshop CS3 software, the videos were analyzed for lateral distances between the curb and the cyclist and the cyclist and the vehicle body. The one-foot plastic pipe served as a control parameter to establish a custom measurement scale using the ruler tool within the software to
determine the lateral distances. Measurements included the lateral distance from the bicycle tire center to the face of curb or edge of pavement and from the cyclist's left shoulder to the passing vehicle's passenger door. The percent of lateral shift by vehicles into the inner lane while passing the bicyclist was determined visually and estimated to within the closest quartile of the adjacent lane width. Figure 5.2 shows examples of quartile assessment during data collection.


FIGURE 5.2 Vehicle Lateral Shift into Adjacent Lane Quartiles
Apart from lateral separation between motor vehicles and bicyclists and vehicle lateral shift into the adjacent lane, other variables were recorded, including vehicle type and speed, and gender, and type of dress of the bicyclist. Restrictions such as the presence of vehicles in the inside lane that may limit lateral shift and lane changing maneuvers for the driver were also recorded. The vehicles were classified into six types: passenger cars, sport utility vehicles (SUVs), pickup trucks, medium trucks, large trucks, and buses. Medium trucks were defined as being larger than a pickup truck but smaller than a tractor-trailer truck. Tractor-trailers were categorized as large trucks. Medium trucks, large trucks, or buses did not appear in the videos often enough to allow for statistical analysis.

Not all videos contributed usable data and some allowed for only certain measurements to be obtained. All the data were entered into a Microsoft Access database and then exported to MINITAB for statistical analysis.

### 5.3 Data Analysis

Due to variability in site characteristics, a separate statistical analysis was conducted for each site. Statistical analyses consisted of determining descriptive statistics and performing an

Analysis of Variance (ANOVA) which use the sum of squares for treatments and sum of squares for error to test the significance of the difference in treatments.

The mean square for treatments is the measure of variability among the treatment means, while the mean square for error measures the sampling variability within the treatments. The F-statistic is calculated as the ratio of the mean square for treatments (MST) and the mean square for error (MSE) as shown in the equation below.

$$
F=\frac{\mathrm{MST}}{\mathrm{MSE}}
$$

Values of the F-statistic near one indicate that the two sources of variation, between treatment means and within treatments, are approximately equal. In this case, the difference between the treatment means may well be attributable to sampling error, which provides little support for the significance of the difference between the two treatments. To determine whether the value of F statistic exceeds the value of one, enough to suggest a significant difference between the two treatments, a computed F-statistic is compared to the F-value taken from statistical tables and is based on the degrees of freedom and the confidence level. If the obtained p-value is less than the considered level of significance, typically $95 \%$ in traffic studies (i.e., $\alpha=0.05$ ), there would be sufficient evidence to conclude that the two treatment means differ.

## 6 ANALYSIS AND RESULTS

Due to differences in site characteristics and the before-and-after configurations, separate analyses were conducted for each study site. This section discusses the analyses and results for each site.

### 6.1 Riverside Drive (Jacksonville)

The site at Riverside Drive consists of three lanes, two northbound and one southbound, with lane widths of approximately 10 feet. Figures 6.1 and 6.2 display the study segment, located between Post and Rosselle Streets. Sharrows were installed in both directions along the segment as shown in Figure 6.2.


FIGURE 6.1 Riverside Drive Northbound


FIGURE 6.2 Shared Lane Markings: Riverside Drive
Descriptive statistics of the lateral clearance between motor vehicles and bicycles and distance from bicycle to the curb are summarized in Tables 6.1 and 6.2. The lateral vehicle clearance, the lateral separation between the vehicle and the body of the bicyclist, increased from 5.18 feet in the before condition to 5.85 feet after sharrows were installed; a gain of 0.67 feet. The distribution of lateral separation between vehicles and bicyclists, shown in Figure 6.3, indicates the tendency of drivers to shift to the inside lane, and leaving more space for the bicyclist to ride, with the presence of sharrows.

TABLE 6.1 Lateral Clearance between Bicyclist and Vehicle: Riverside Drive

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. <br> (ft.) |  | Minimum <br> (ft.) |  | Maximum <br> (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | After | Before | After | Before | After | Before | After | Before | After |  |
| Jacksonville | Riverside <br> Dr. | 38 | 30 | 5.18 | 5.85 | 1.82 | 1.72 | 1.59 | 1.70 | 8.69 | 8.34 |

TABLE 6.2 Lateral Clearance between Bicyclist and Curb: Riverside Drive

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. <br> (ft.) |  | Minimum <br> (ft.) |  | Maximum <br> (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Before | After | Before | After | Before | After | Before | After | Before | After |
| Jacksonville | Riverside Dr. | 38 | 30 | 2.94 | 4.32 | 0.69 | 2.92 | 2.07 | 1.83 | 4.64 | 12.83 |

The mean lateral distance between the bicycle tire center and face of curb, the lateral bicycle clearance, increased by 1.38 feet from the before-period to the after-period (after sharrows were installed) as listed in Table 6.2. These findings are similar to those found by Brady et al. (5) in relation to the lateral bicycle position to the on-street parking delineation line. The standard location for the shared lane markings was 5.5 feet from the face of curb; however the location of the sharrows varied within the lane as the designers were trying to avoid some rough pavement or utilities. After the sharrows were installed, bicyclists tended to ride further away from the curb, and northbound drivers tended to use the inside lane more frequently. The distribution of the distance from the bicyclist to the curb is depicted in Figure 6.4.


FIGURE 6.3 Distribution of Bicyclist to Vehicle Separation: Riverside Drive


FIGURE 6.2 Distribution of Bicyclist to Curb Distance: Riverside Drive

### 6.1.1 Analysis of Variance

Tables 6.3 and 6.4 show the ANOVA results for the Riverside Drive study segment. The Analysis of Variance indicates a significant difference between the vehicle and bicycle separation from the before-period to the after-period with sharrows by the p-value of 0.039 . Also, the results suggest that there is a significant difference in the way bicyclists ride in relation to the curb after sharrows were installed. A p-value of 0.015 , obtained from the ANOVA, for the distance between bicyclists and the curb for the before- and after-scenario reflects the significance in the difference.

TABLE 6.3 ANOVA Results for Sharrows Effect (Cyclist to Vehicle): Riverside Drive

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 7.45 | 7.45 | 2.36 | 0.039 |
| Error | 77 | 208.07 | 3.15 |  |  |
| Total | 67 | 215.52 |  |  |  |

TABLE 6.4 ANOVA Results for Sharrows Effect (Cyclist to Curb): Riverside Drive

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 26.74 | 26.74 | 6.19 | 0.015 |
| Error | 66 | 349.80 | 4.32 |  |  |
| Total | 67 | 376.54 |  |  |  |

### 6.2 Bridge of Lions (St. Augustine)

The study segment at this location is across the Bridge of Lions, a two lane undivided roadway with lane widths of 11 feet, one in each direction. Sharrow placement at this site is illustrated in Appendix G. Descriptive statistics of the lateral clearance between motor vehicles and bicycles and the distance from bicycle to the curb are summarized in Tables 6.5 and 6.6. The lateral vehicle clearance did not improve significantly at this site. Before sharrows, an average clearance of 3.58 feet was observed. After sharrows were installed, the average clearance improved by only 0.04 feet for an average to of 3.62 feet. Sharrows were placed 5.5 feet of the face of curb due to the narrow lanes. The Bridge of Lions study segment after sharrows were installed is shown in Figure 6.5.


FIGURE 6.5 Shared Lane Markings: Bridge of Lions
These findings may be attributed to high traffic volumes and the crown of the bridge limiting forward sight distance, thus providing little opportunity to overtake bicyclists. Without an adjacent lane in the same direction, oncoming traffic severely limits a driver's ability to shift left and pass a bicyclist. The bridge is also marked as a no passing zone. The distribution of lateral clearance distances shown in Figure 6.6 also depicts the same phenomenon, little difference between the two distributions.

TABLE 6.5 Lateral Clearance between Bicyclist and Vehicle: Bridge of Lions

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum <br> (ft.) |  | Maximum <br> (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Before | After |  |  |  |  |  |  |  |  |
| St. <br> Augustine | Bridge <br> of Lions | 77 | 60 | 3.58 | 3.62 | 1.34 | 1.43 | 0.82 | 1.29 | 6.86 |  |



FIGURE 6.6 Distribution of Bicyclist to Vehicle Separation: Bridge of Lions
Although not much improvement was observed on the lateral clearance between vehicles and bicyclists at the Bridge of Lions site, the lateral distance between the bicyclist and the raised curb increased notably. Before sharrows, the distance from the bicyclist to the raised curb was observed to be 2.74 feet, and 4.24 feet after sharrows; an increase of 1.50 feet (Table 6.6). These findings are further supported by the distribution plots shown in Figure 6.7. This observation suggests that as bicyclists moved further away from the curb after sharrows were installed on the Bridge of Lions, motor vehicles also shifted to the left to maintain the same relative separation. The frequency of aggressive behavior from drivers seemed to decrease, although this is based upon the personal observations of cyclists in the study.

TABLE 6.6 Lateral Clearance between Bicyclist and Curb: Bridge of Lions

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum <br> (ft.) |  | Maximum <br> (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | After | Before | After | Before | After | Before | After | Before | After |  |
| St. <br> Augustine | Bridge <br> of Lions | 77 | 60 | 2.74 | 4.24 | 0.78 | 0.68 | 2.17 | 3.24 | 6.09 |  |



FIGURE 6.7 Distribution of Bicyclist to Curb Distance: Bridge of Lions

### 6.2.1 Analysis of Variance

Tables 6.7 and 6.8 show the ANOVA results for the Bridge of Lions study segment. The Analysis of Variance indicates that there is no significant difference between the vehicle and bicycle separation before and after sharrows were installed, denoted by a p-value of 0.585 . However, the results suggest a significant difference in the distance between bicyclists and the raised curb with a p-value of 0.000 .

TABLE 6.7 ANOVA Results for Sharrows Effect (Cyclist to Vehicle): Bridge of Lions

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 0.57 | 0.57 | 0.30 | 0.585 |
| Error | 135 | 256.77 | 1.90 |  |  |
| Total | 136 | 257.34 |  |  |  |

TABLE 6.8 ANOVA Results for Sharrows Effect (Cyclist to Barrier): Bridge of Lions

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 39.75 | 39.75 | 49.14 | 0.000 |
| Error | 135 | 109.21 | 0.81 |  |  |
| Total | 136 | 148.96 |  |  |  |

### 6.3 North $56^{\text {th }}$ Street (Temple Terrace)

The site at N $56^{\text {th }}$ Street in Temple Terrace included two 4-lane typical sections, one with curb and gutter, a narrow median separator and left turn lane (Site A), and a second with flush shoulders and a grass median (Site B). Through a resurfacing project, lane widths were adjusted in both sections. South of Serena Drive to Mission Hills Avenue (Site A), the four travel lanes were adjusted to 11 feet for the inner lanes, and 13 feet for the outer lanes with sharrows installed 4.0 feet from the face of the curb. Four-foot bike lanes were added in the flush shoulder section North of Serena Drive to Whiteway Drive (Site B), and the four travel lanes transitioned from 12 feet to 11 feet in width. The effects of sharrows and bike lanes were analyzed separately.

Initially at Site A, when the "before" data was collected the study location had a posted speed of 45 mph . As part of the design review, it was determined that the speed limit should be reduced to 35 mph due to the constrained curb and gutter section in Temple Terrace. Figure 6.8 displays the retrofitted design and sharrow placement on N $56^{\text {th }}$ Street from Serena Drive to Mission Hills Avenue.


FIGURE 6.8 Wider Outside Lanes with Shared Lane Markings: N 56 ${ }^{\text {th }}$ Street (Site A)

### 6.3.1 Sharrows (Site A)

Descriptive statistics of the lateral clearance between motor vehicles and bicycles and the distance from bicycle tire to the curb for Site A are summarized in Tables 6.9 and 6.10. A significant improvement in the separation between bicycles and motor vehicles was observed
after adjusting the lane widths, reducing the speed and installing sharrows at this site. The average lateral separation increased from 3.96 feet (before sharrows) to 6.51 feet (after sharrows), an increase of 2.55 feet as shown in Table 6.9 and Figure 6.9.

When a bicyclist was present, most motorists shifted to the inside lane while passing the bicyclist if there was an opportunity to do so and then returned to the outside lane after passing the bicyclist. Before sharrows, there was a tendency of drivers to remain in the outside lane, essentially squeezing between the bicyclist and inner lane, leaving less separation between. Vehicle lateral shift into the adjacent lane is discussed further in Section 6.8.

TABLE 6.9 Lateral Clearance between Motor Vehicle and Bicyclist: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Before | After | Before | After | Before | After | Before | After | Before | After |
| Tampa | $\begin{gathered} \mathrm{N} \\ \text { 56th } \\ \text { St. } \end{gathered}$ | 365 | 171 | 3.96 | 6.51 | 1.36 | 3.17 | 1.40 | 1.42 | 9.71 | 14.3 |



FIGURE 6.9 Distribution of Bicyclist to Vehicle Separation: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings

It was observed that bicyclists rode further from the curb after the lane width was widened, speed reduced and sharrows installed at Site A of the $\mathrm{N} 56^{\text {th }}$ Street study location (Table 6.10 and Figure 6.10). An average distance from bicyclist to curb of 2.32 feet was observed with sharrows present compared to 1.92 feet before sharrows were installed. The majority of cyclists rode from 2.0 to 2.5 feet from the face of curb in the after-period as illustrated in Figure 6.10.

TABLE 6.10 Lateral Clearance between Bicyclist and Curb: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Before | After | Before | After | Before | After | Before | After | Before | After |
| Tampa | $\begin{gathered} \mathrm{N} \\ 56 \mathrm{th} \\ \text { St. } \end{gathered}$ | 365 | 171 | 1.92 | 2.32 | 0.43 | 0.62 | 0.64 | 1.00 | 3.43 | 6.01 |



FIGURE 6.10 Distribution of Bicyclist to Curb Distance: N 56 ${ }^{\text {th }}$ Street (Site A) Wider Outside Lane with Shared Lane Markings

### 6.3.2 Analysis of Variance for Wider Outside Lane and Sharrows (Site A)

As shown in the previous section, descriptive statistics indicate that there was more lateral separation after the outside lane was widened, sharrows installed, and speed reduced. The purpose of conducting an Analysis of Variance (ANOVA) was to determine whether the difference was significant. From the results listed in Tables 6.11 and 6.12 , the p-value of 0.000 shows a significant difference, indicating that the improvements appear to have an influence on
increasing the separation between bicyclists and motor traffic. Likewise, the distance from the bicyclist to the curb shows a significant difference, signifying that bicyclists ride further from the curb in the presence of sharrows.

TABLE 6.11 ANOVA Results for Wider Outside Lane with Shared Lane Markings (Cyclist to Vehicle): $\mathbf{N} 56{ }^{\text {th }}$ Street (Site A)

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 779.82 | 779.82 | 165.58 | 0.000 |
| Error | 534 | 2514.87 | 4.71 |  |  |
| Total | 535 | 3294.69 |  |  |  |

TABLE 6.12 ANOVA Results for Wider Outside Lanes with Shared Lane Markings (Cyclist to Curb): N 56 ${ }^{\text {th }}$ Street (Site A)

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 11.389 | 11.389 | 43.14 | 0.000 |
| Error | 534 | 140.982 | 0.264 |  |  |
| Total | 535 | 152.371 |  |  |  |

### 6.3.3 Bike Lane (Site B)

The N $56^{\text {th }}$ Street (Site B) study location consisted of added bike lanes with adjusted lane widths as shown in Figure 6.11. Descriptive statistics of lateral separation between motor vehicles and bicyclists are listed in Table 6.13. An average of 6.15 feet of separation between vehicles and bicyclists was observed on this segment compared to 6.51 feet observed on the sharrows segment (Site A), a difference of 0.36 feet. As shown in Figure 6.12, the wider distribution curve also indicates that the separation between motorists and bicyclists varied considerably more at Site A than distances observed with bike lanes (Site B). This suggests more willingness by motorists to provide extra space to a bicyclist in the absence of a dedicated bike lane when sharrows are present. In Florida, a minimum separation of 3 feet is required by law.

The distance between bicyclists and edge of pavement noted in Table 6.14, depict that bicyclists rode only slightly farther away from the edge of pavement ( 2.51 feet) compared to the sharrows site ( 2.23 feet). The distribution curves in Figure 6.13 illustrate the minor variation between the two treatments.


FIGURE 6.11 Bike Lanes: N 56 ${ }^{\text {th }}$ Street (Site B)

TABLE 6.13 Lateral Clearance between Motor Vehicle and Bicyclist-Comparing Bike Lane and Sharrows: N 56 ${ }^{\text {th }}$ Street

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Sharrows | Bike <br> Lane | Sharrows | Bike Lane | Sharrows | Bike Lane | Sharrows | Bike Lane | Sharrows | Bike <br> Lane |
| Temple Terrace | $\begin{gathered} \mathrm{N} \\ \text { 56th } \\ \text { St. } \end{gathered}$ | 365 | 86 | 6.51 | 6.15 | 3.17 | 2.06 | 1.42 | 3.06 | 14.3 | 13.13 |

TABLE 6.14 Lateral Clearance between Bicyclist and Curb/Edge of Pavement-Comparing Bike Lane and Sharrows: N 56 ${ }^{\text {th }}$ Street

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Bike <br> Lane | Sharrows | Bike <br> Lane | Sharrows | Bike <br> Lane | Sharrows | Bike <br> Lane | Sharrows | Bike <br> Lane |  |
| Temple <br> Terrace | N <br> 56 th <br> St. | 171 | 86 | 2.23 | 2.51 | 0.61 | 0.76 | 1.00 | 0.92 | 6.01 | 5.00 |



FIGURE 6.12 Distribution of Bicyclist to Vehicle Separation-Comparing Bike Lane and Sharrows: N 56th Street


FIGURE 6.13 Distribution of Bicyclist to Curb Distance-Comparing Bike Lane and Sharrows: N 56th Street

### 6.3.4 Analysis of Variance for Bike Lane (Site B)

ANOVA results from the comparison between bicycle treatments (sharrows and bike lane) on North $56^{\text {th }}$ Street are summarized in Tables 6.15 and 6.16. The results indicate that there is no significant difference ( p -value of 0.216 ) between the two treatments in lateral separation between vehicles and bicyclists. However, the results suggest a significant difference (p-value of 0.02) in the distance from the bicyclists to the curb or edge of pavement. It was also observed that drivers did not slow down in the bike lane section as much as they did at the sharrows site. However, this was an anecdotal observation.

TABLE 6.15 ANOVA Results for Sharrows vs. Bike Lane (Cyclist to Vehicle): N 56th Street

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 12.53 | 12.53 | 1.54 | 0.216 |
| Error | 255 | 2072.13 | 8.13 |  |  |
| Total | 256 | 2084.65 |  |  |  |

TABLE 6.16 ANOVA Results for Sharrows vs. Bike Lane (Cyclist to Curb/Edge of Pavement): N 56th Street

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 4.27 | 4.27 | 9.70 | 0.02 |
| Error | 255 | 112.15 | 0.44 |  |  |
| Total | 256 | 116.42 |  |  |  |

### 6.4 Sunset Drive (Miami)

The Sunset Drive study site involved comparing operational characteristics of a four-lane (two lanes in each direction) divided roadway with a curbed median and flush grass shoulders. The pavement width was increased by reducing the median and applying the additional width to the outside lanes. The result was wider outside lanes ( 14 feet lane) and narrower inside lanes (11 feet). No sharrows were installed on this facility, shown in Figures 6.14 and 6.15.


FIGURE 6.14 Sunset Drive (before scenario)


FIGURE 6.15 Sunset Drive (after scenario)

### 6.4.1 Descriptive Analysis

The descriptive statistics for the lateral clearance between vehicles and bicyclists are shown in Table 6.17. Data analysis revealed that more separation between the bicyclists and vehicles was attained after retrofitting the site to include a wider outside lane. An average lateral separation of 5.11 feet was observed after retrofitting compared to an average separation of 4.25 feet that was observed before the widening, illustrated in Figure 6.16.

TABLE 6.17 Lateral Clearance between Motor Vehicle and Bicyclist: Sunset Drive

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | After | Before | After | Before | After | Before | After | Before | After |  |
| Miami | Sunset <br> Blvd. | 65 | 84 | 4.25 | 5.11 | 1.69 | 1.44 | 1.75 | 3.12 | 10.32 | 13.02 |



FIGURE 6.16 Distribution of Bicyclist to Vehicle Separation: Sunset Drive

On Sunset Drive, bicyclists tended to ride closer to the edge of pavement after the outside lane was widened ( 1.92 feet) than before widening ( 2.17 feet), a reduction of .25 feet. These findings are listed in Table 6.18 and illustrated in Figure 6.17.

TABLE 6.18 Lateral Clearance between Bicyclist and Edge of Pavement: Sunset Drive

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Before | After | Before | After | Before | After | Before | After | Before | After |
| Miami | Sunset Blvd. | 65 | 84 | 2.17 | 1.92 | 0.65 | 0.55 | 1.30 | 0.62 | 3.71 | 3.34 |



FIGURE 6.17 Distribution of Bicyclist to Edge of Pavement Distance: Sunset Drive

### 6.4.2 Analysis of Variance

The ANOVA results for Sunset Drive are summarized in Tables 6.19 and 6.20. The results reveal a significant difference in both the separation between motor vehicles and bicyclists ( p -value of 0.001 ) and the lateral distance between bicyclists and the edge of pavement ( $p$-value of 0.008 ). These findings are in agreement with the first phase of this study that solely analyzed the influence of a wide outside lane on bicycle and motor vehicle interactions.

## TABLE 6.19 ANOVA Results for Wider Outside Lane Effect (Cyclist to Vehicle): Sunset Drive

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 27.11 | 27.11 | 11.17 | 0.001 |
| Error | 147 | 356.62 | 2.43 |  |  |
| Total | 148 | 383.73 |  |  |  |

TABLE 6.20 ANOVA Results for Wider Outside Lane Effect (Cyclist to Edge of Pavement): Sunset Drive

| Source | DF | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 2.231 | 2.231 | 7.16 | 0.008 |
| Error | 147 | 45.781 | 0.311 |  |  |
| Total | 148 | 48.011 |  |  |  |

### 6.5 Bailey Road (Fort Lauderdale)

### 6.5.1 Site Description

The analysis of the Bailey Road site in Broward County involved comparing two cross-sections, one with a 5 -foot wide bike lane separated from traffic by a 6 inch lane line (Figure 6.18), and the other with a 4 -foot wide bike lane, separated from adjacent traffic by a 2 -foot striped buffer zone (Figure 6.19 ). Both sections consist of 11 -foot wide outer lanes and 10.5 -foot wide inner lanes.


FIGURE 6.18 Section with 5-foot Bike Lane: Bailey Rd.


FIGURE 6.19 Section with 4-foot Bike Lane and 2-foot Buffer: Bailey Rd.

### 6.5.2 Descriptive Statistics

Tables 6.21 and 6.22 present the descriptive statistics of the buffer and no-buffer segments studied on Bailey Road. Data analysis indicates a significant improvement in the lateral vehicle clearance with the presence of the buffer between the bike lane and adjacent travel lane (Figure 6.20). On average, the lateral separation for the buffered bike lane segment was 5.72 feet, while the separation in the segment with the traditional bike lane was 5.00 feet, a difference of 0.72 feet. The buffered bike lane used six feet of pavement, versus the five feet of width used for the traditional bike lane, but the net gain in separation was only 0.72 feet.

As shown in Figure 6.21, the distance from the bicyclist to the curb also increased from 2.16 feet (with buffer) to 2.31 feet (without buffer). It is interesting to note that while bicyclists rode further from the curb at the site with a buffer, the lateral separation between bicycles and vehicles increased, an indication that a buffered lane plays a role in providing additional separation between motor and bicycle traffic.

TABLE 6.21 Lateral Clearance between Motor Vehicle and Bicycle: Bailey Rd.

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | No <br> Buffer | Buffer | No Buffer | Buffer | No <br> Buffer | Buffer | No <br> Buffer | Buffer | No <br> Buffer | Buffer |
| Ft. <br> Lauderdale | Bailey <br> Rd. | 66 | 61 | 5.00 | 5.72 | 1.01 | 1.10 | 2.24 | 2.03 | 7.30 | 8.54 |



FIGURE 6.20 Distribution of Bicyclist to Vehicle Distance: Bailey Road

TABLE 6.22 Lateral Clearance from Bicyclist to Curb: Bailey Rd.

| City | Road | Sample Size |  | Mean (ft.) |  | Std. Dev. (ft.) |  | Minimum (ft.) |  | Maximum <br> (ft.) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Buffer | No <br> Buffer | Buffer | No <br> Buffer | Buffer | No <br> Buffer | Buffer | No <br> Buffer | Buffer |  |
| Ft. <br> Lauderdale | Bailey <br> Rd. | 66 | 61 | 2.31 | 2.16 | 0.59 | 0.53 | 0.74 | 0.61 | 4.93 | 3.38 |



FIGURE 6.21 Distribution of Bicyclist to Curb Distance: Bailey Road

### 6.5.3 Analysis of Variance

Tables 6.23 and 6.24 show the ANOVA results for the Bailey Road sites. The analysis indicates a significant difference in the vehicle to bicycle separation between the two configurations (buffer and no-buffer) with a p-value of 0.000 . However, the results suggest that there is no significant difference in the way bicyclists ride relative to the curb ( p -value $=0.144$ ).

TABLE 6.23 ANOVA Results for Buffer vs. no Buffer (Cyclist to Vehicle): Bailey Road

| Source | DF what does <br> this mean? | Sum of <br> Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 16.35 | 16.35 | 14.71 | 0.000 |
| Error | 125 | 138.98 | 1.11 |  |  |
| Total | 126 | 155.33 |  |  |  |

TABLE 6.24 ANOVA Results for Buffer vs. no Buffer (Cyclist to Curb): Bailey Road

| Source | DF | Sum of Squares | Mean Square | $F$-Value | $p$-value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Model | 1 | 0.689 | 0.689 | 2.17 | 0.144 |
| Error | 125 | 39.752 | 0.318 |  |  |
| Total | 126 | 40.441 |  |  |  |

### 6.6 Limited Access Facility Pilot Projects

### 6.6.1 Limited Access Sites

The limited access sites included three locations on Florida's Atlantic Coast that are part of FDOT's Bicycles on Limited Access Pilot Project. One site, Pineda Causeway (SR 404), is located in Melbourne, Brevard County, while the other two sites, Julia Turtle Causeway (I 195/SR 112) and William Lehman Causeway (SR 856/192 ${ }^{\text {nd }}$ Street) are located in Miami-Dade County. Figures 6.22 to 6.26 show the crossing locations at these limited access facilities. Refer to Table 4.1 for a description of each site.


FIGURE 6.22 William Lehman Cswy Westbound Off-ramp


FIGURE 6.23 William Lehman Cswy Westbound On-ramp


FIGURE 6.24 William Lehman Cswy Eastbound Off-ramp


FIGURE 6.25 William Lehman Cswy Eastbound On-ramp


FIGURE 6.26 Julia Tuttle Cswy Westbound (Mt. Sinai) On-ramp

### 6.6.2 Analysis of Driver Characteristics at Crossing Locations

It was observed that overtaking drivers slowed down as they approached a bicyclist at a crossing location. When watching the video clips, brake lights indicated the drivers slowing down. In some instances, a reduction in speed was observed without the brake lights illuminated suggesting the driver may have simply released the acceleration pedal to slow the vehicle. On sites with two lanes at the crossing locations, drivers who intended to overtake bicyclists before crossing shifted to the inside lane to provide more room for the bicyclist when passing.

TABLE 6.25 Operational Characteristics at Crossing Locations

| Location | Road <br> Name | Bicyclist reduced speed? |  | Bicyclist visibly checked for overtaking traffic before crossing? |  | Driver reduced speed? |  | Bicyclist used marked crossing location? |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Y | N | Y | N | Y | N | Y | N |
| Miami | Lehman West Off-Ramp <br> (Figure 6.22) | 50.0\% | 50.0\% | 87.5\% | 12.5\% | 100.0\% | 0\% | 25.0\% | 75.0\% |
| Miami | Lehman West On-Ramp <br> (Figure 6.23) | 95.7\% | 4.3\% | 100.0\% | 0\% | 100.0\% | 0\% | 8.7\% | 91.3\% |
| Miami | Lehman East Off-Ramp <br> (Figure 6.24) | 90.0\% | 10.0\% | 100.0\% | 0\% | 100.0\% | 0\% | 10.0\% | 90.0\% |
| Miami | Lehman East On-Ramp <br> (Figure 6.25) | 76.9\% | 23.1\% | 100.0\% | 0\% | 100.0\% | 0\% | 38.5\% | 61.5\% |
| Miami | Julia Tuttle West (On-Ramp) <br> (Figure 6.26) | 81.0\% | 19.0\% | 100.0\% | 0\% | 100.0\% | 0\% | 100.0\% | 0\% |

### 6.6.3 Analysis of Bicyclist Characteristics at Crossing Locations

As shown in Table 6.25, bicyclists slowed down when crossing ramp lanes most of the time except at the William Lehman Causeway Eastbound Off-ramp site. At this location, bicyclists did not see the need to slow down as they could ride on the chevron pavement markings as they waited for an acceptable gap to cross. Bicyclists consistently turned their heads to search for
overtaking traffic before crossing to ensure a safe crossing maneuver. Examples of bicyclist and driver interactions are shown in Appendix H.

### 6.6.4 Analysis of Pineda Causeway Data

The data collection for the effectiveness of green bike lanes in the ramp areas on the Pineda Causeway used local bicyclists. Most of the bicyclists that were observed rode in groups of about 15 cyclists as shown in Figure 6.27. Due to the low number of local cyclists using the Causeway and some riding in groups during the data collection periods, it was difficult to obtain statistically significant sample sizes. This shows the importance of using FDOT teams and volunteers from other agencies to gather sufficient data to allow for inferential statistical analysis. For this section, general observations are discussed with the use of photos for illustration.


FIGURE 6.27 Bicyclist Group Riding from US 1 to Pineda Cswy Eastbound After Green Bike Lane Installation


FIGURE 6.28 Bicyclist Crossing from US 1 to Pineda Cswy Eastbound Before Green Bike Lane Installation


FIGURE 6.29 Bicyclist Crossing from US 1 to Pineda Cswy Eastbound Before Green Bike Lane Installation

Data collected for the before-scenario, before green bike lane installation, indicates that most bicyclists prefer to use the shortest distance to cross the ramp. Figures 6.28 and 6.29 illustrate this phenomenon. After installation of green bike lanes, the same trend was observed. Most bicyclists continued to prefer using a straighter alignment to cross the ramp (Figure 6.30). Cyclists preferred to continue riding in the same alignment they had used on the paved shoulder adjacent to the outside lane as they come down the bridge, as shown in Figure 6.30.


## FIGURE 6.30 Bicyclist Crossing Ramp from US 1 to Pineda Cswy Eastbound After Green Bike Lane Installation

### 6.7 Speed Analysis

Vehicle speed data was collected at three stages during passing events: just before passing, while passing, and after passing the bicyclist. All passing events with missing data from one or more of the three data stages were removed from the sample set. Table 6.26 shows the combined average spot speeds observed for each of the three scenarios for non-limited access study sites. In general, the data reflects a tendency of drivers to reduce speeds while-passing the bicyclist (Figure 6.31). Table 6.27 and Figure 6.32 is similar representation of the observed before, during, and passing average speeds but for the limited access sites.

TABLE 6.26 Average Speed in Terms of Vehicle Types (Non-Limited Access Sites)

| Non-Limited Access |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Type of Vehicle | No. of Observations | Average Speed (mph) |  |  |
|  |  | Before | Passing | After |
| Passenger | 64 | 32.1 | 28.6 | 33.2 |
| SUV | 53 | 32.9 | 30.2 | 34.8 |
| Truck | 22 | 32.5 | 28.9 | 34.5 |
| Van | 13 | 36.0 | 31.1 | 35.2 |
| Bus | 2 | 23.0 | 19.0 | 20.0 |
| Overall | 154 | 32.6 | 29.3 | 34.0 |



FIGURE 6.31 Average Vehicle Speed during Passing Events (Non-Limited Access Sites)

TABLE 6.27 Average Speed in Terms of Vehicle Types (Limited Access Sites)

| Limited Access |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Type of Vehicle | No. of Observations | Average Speed (mph) |  |  |
|  |  | Before | Passing | After |
| Passenger | 44 | 38.2 | 37.1 | 37.9 |
| SUV | 29 | 35.6 | 32.6 | 34.0 |
| Truck | 23 | 37.0 | 34.2 | 34.2 |
| Van | 9 | 36.8 | 32.0 | 32.6 |
| Bus | 4 | 36.3 | 39.0 | 37.8 |
| Overall | 109 | 37.1 | 34.9 | 35.6 |



FIGURE 6.32 Average Vehicle Speed during Passing Events (Limited Access Sites)
Speeds for the three scenarios were therefore compared using a paired $t$-test. This test is appropriate for analyzing samples that have two different treatments, i.e., paired treatments. In this case, paired treatments include before- and while-passing, and while- and after-passing scenarios. The paired $t$-test provides the statistic, which is used to determine if there is a significant difference between the speeds of each motor vehicle for the paired scenarios. The null hypothesis for this test represented the proposition that the spot speeds of the above mentioned scenarios are equal while the alternative hypothesis is that the speeds are not equal. The alternative hypothesis is accepted only when the data suggest sufficient evidence to support it, hence rejecting the null hypothesis. All paired scenarios were tested at the $95 \%$ confidence level.

The MINITAB statistical software results of the paired $t$-test are shown in Tables 6.28 and 6.29 . Table 6.28 summarizes the results for the non-limited access roads studied: Riverside Avenue, Bridge of Lions, N $56^{\text {th }}$ Street, Bailey Road, and Sunset Drive. At a $95 \%$ confidence level, data shows sufficient evidence to indicate that drivers slow down as they approach bicyclists (before speed of 32.02 mph to 29.97 mph while-passing) and then increase their speeds after overtaking the bicyclists ( 30.80 mph while-passing and after speed of 32.82 mph ). The difference between the speeds before-passing and while-passing and while-passing and after-passing were significant with a p-value less than 0.000 . When the before-passing ( 32.02 mph ) and afterpassing ( 32.54 mph ), excluding while-passing data, was analyzed, no significant difference was found ( $p$-value $=0.110$ ). It should be noted that the speed values, e.g., the after-passing speed shown in the comparison to while-passing ( 32.82 mph ) differ from the average after-passing speed of 32.54 mph indicated when comparing before-passing and after-passing because not every pass had all three events (before-, while-, and after-passing) collected.

For limited access facilities (Table 6.29), the difference between the overtaking driver's speed before-passing ( 37.35 mph ) and while-passing ( 34.93 mph ) the bicyclists was significant with a p-value of 0.000 , while the difference between motor vehicle speeds while-passing bicyclists
( 34.94 mph ) and after-passing ( 35.48 mph ) was not significant ( p -value $=0.150$ ). Contrary to the non-limited access streets, the difference between vehicle speeds before- ( 37.33 mph ) and afterpassing ( 35.48 mph ) was significant for the limited access facilities ( p -value $=0.017$ ).

Table 6.28 Paired T-Test Results for Motor Vehicle Speeds (Non-Limited Access Streets)

```
Paired T-Test and Confidence Interval for Before & While-Passing
Paired T for Before - While-Passing
\begin{tabular}{lcccc} 
& N & Mean & StDev & SE Mean \\
Before & 228 & 32.022 & 6.850 & 0.454 \\
While-Passing & 228 & 29.969 & 8.207 & 0.544 \\
Difference & 228 & 2.053 & 5.164 & 0.342
\end{tabular}
```

95\% CI for mean difference: $(1.379,2.727)$
T -Test of mean difference $=0($ vs not $=0)$ : T -Value $=6.00 \mathrm{P}$-Value $=0.000$
Paired T-Test and Confidence Interval for While-Passing \& After
Paired T for While-Passing_1 - After

|  | N | Mean | StDev | SE Mean |
| :--- | :---: | :---: | :---: | :---: |
| While-Passing_1 | 226 | 30.801 | 8.148 | 0.542 |
| After | 226 | 32.819 | 7.312 | 0.486 |
| Difference | 226 | -2.018 | 3.556 | 0.237 |

$95 \%$ CI for mean difference: $(-2.484,-1.552)$
$\mathrm{T}-\mathrm{Test}$ of mean difference $=0($ vs not $=0): \mathrm{T}-$ Value $=-8.53 \mathrm{P}-$ Value $=0.000$
Paired T-Test and Confidence Interval for Before \& After-Passing
Paired T for Before_1-After_1
N Mean StDev SE Mean
$\begin{array}{lllll}\text { Before_1 } & 209 & 32.024 & 6.878 & 0.476\end{array}$
$\begin{array}{lllll}\text { After_1 } & 209 & 32.545 & 7.242 & 0.501\end{array}$
Difference $209 \quad-0.522 \quad 4.699 \quad 0.325$

95\% CI for mean difference: ( $-1.162,0.119$ )
T -Test of mean difference $=0($ vs not $=0):$ T-Value $=-1.60 \mathrm{P}$-Value $=0.110$

Table 6.29 Paired T-Test Results for Motor Vehicle Speeds (Limited Access Causeways)


### 6.8 Vehicle Lateral Shift

Table 6.30 shows the percent of the vehicle lateral shift at each site. Except for the Bridge of Lions and Riverside Drive sites, about $50 \%$ or more of vehicles passed bicyclists without shifting into the adjacent lane. This may be attributed by the fact that Riverside and Bridge of Lions sites have the narrowest lane widths of all the sites ( 10 feet for Riverside Drive and 11 feet for Bridge of Lions). At Riverside, all vehicles shifted to the inside lane when passing bicyclists. When examining the $76 \%$ to $100 \%$ lateral shift quartile, data shows a high level of lateral shift for Riverside ( $82.33 \%$ and $53.33 \%$ before- and after-sharrows, respectively) and a lower level of lateral shift for Bridge of Lions ( $3.71 \%$ and $0.00 \%$ before- and after-sharrows, respectively). This may be attributed to the fact that the Bridge of Lions is a two lane undivided narrow bridge with a no passing zone.

Table 6.30 Lateral Shift Data for Study Sites

| Site |  | Vehicle Lateral Shift into Adjacent Lane |  |  |  |  |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | No Lateral Shift | $1 \%-$ <br> $25 \%$ | $26 \%-$ <br> $50 \%$ | $51 \%-$ <br> $75 \%$ |
| Riverside Dr., <br> Jacksonville | Before | 0.00 | 12.50 | 4.17 | 0.00 | 83.33 |
|  | After | 0.00 | 6.67 | 13.33 | 26.67 | 53.33 |
| Bridge of Lions, <br> St. Augustine | Before | 33.33 | 33.33 | 16.67 | 12.96 | 3.71 |
|  | After | 26.09 | 52.17 | 17.39 | 4.35 | 0.00 |
| N 56th St., <br> Temple Terrace | Before | After | 59.86 | 28.22 | 14.52 | 8.84 |
| Bailey Rd., <br> Ft. Lauderdale | No <br> Buffer | 84.84 | 12.12 | 0.00 | 1.52 | 1.52 |
|  | Buffer | 95.08 | 3.28 | 1.64 | 0.00 | 0.00 |
|  | Before | 53.85 | 29.23 | 6.15 | 4.62 | 6.15 |
|  | After | 84.52 | 11.91 | 0.00 | 1.19 | 2.38 |
| Pineda Cswy., <br> Melbourne | Before | 73.33 | 13.33 | 6.67 | 6.67 | 0.00 |
|  | After | 60.00 | 20.00 | 0.00 | 0.00 | 20.00 |
| Lehman Cswy., <br> Miami | After | 69.72 | 10.56 | 5.63 | 3.53 | 10.56 |
| Julia Tuttle Cswy., <br> Miami | After | 48.28 | 20.69 | 10.34 | 13.79 | 6.90 |

## 7 CONCLUSIONS

This study was conducted to examine the influence of several bicycle facilities on the interaction between motor vehicles and bicyclists. The effectiveness of sharrows, bike lanes, buffers, wide curb lanes, and bicycle crossing pavement markings, were evaluated, including green bike crossings on limited access facilities. Several measures of effectiveness were investigated, including the lateral separation between motor vehicles and bicyclists, the distance from the bicycle tire to the curb, and motor vehicle speeds before-passing, while-passing, and afterpassing a bicyclist. The following sections discuss the results produced from this study.

### 7.1 Lateral Spacing between Vehicles and Bicyclists

The lateral separation between motor vehicles and bicyclists was greater with sharrows compared to without sharrows for all three sites that were evaluated for shared lane markings. For the two sites with narrow outside lanes (Bridge of Lions and Riverside Drive), the lateral separation improved, with the greatest increase at Riverside Drive from an average of 5.18 feet before sharrows to 5.85 feet after sharrows were installed. Increased lateral separation of 2.55 feet also was observed at Site A in Temple Terrace (North $56^{\text {th }}$ Street) after sharrows were installed with an average separation of 3.96 feet and 6.51 feet observed before- and aftersharrows were installed, respectively.

This study also compared operational characteristics between sharrows and bike lanes at North $56^{\text {th }}$ Street (Site B) in Temple Terrace. The separation between motor vehicles and bicyclists in the bike lane segment was of 6.15 feet, compared to the 6.51 feet for the sharrows section. The range of lateral spacing varied from 1.42 feet to 14.30 feet for the sharrows segment, while bike lanes had a narrower range of 3.06 feet to 13.13 feet. Site observations indicated that there were less shifting to the inside lane at the bike lane site compared to the sharrows site. The difference in lateral separation between the sharrows section and the bike lane segment was not significant $(p$-value $=0.216)$.

At the Sunset Drive site in Miami, where the effect of a wide curb lane was examined, lateral vehicle to cyclist separation of 5.11 feet was observed after retrofitting compared to the separation of 4.25 feet before widening the roadway and restriping the lanes to include a wide curb lane, a significant increase ( $p$-value $=0.001$ ).

The influence of a buffer between the vehicle travel lane and bike lane was also evaluated at the Bailey Road site in Broward County. On average, a lateral separation of 5.00 feet was observed for the section without a buffer zone, while the lateral separation between bicyclist and vehicles for the section with a striped buffer was 5.72 feet, a difference of nearly three quarters of a foot.

In general, there was improvement in the amount of lateral spacing drivers provided to bicyclists at all of the study sites. The analyses of variance showed that the difference was significant.

### 7.2 Lateral Spacing between Bicyclists and Curb

While consistent results were obtained for lateral spacing between vehicles and bicycles, mixed results were observed for lateral spacing between bicyclists and the curb or edge of pavement. Distances observed for the Riverside and Bridge of Lions sites showed increases of 1.38 feet and
1.50 feet, respectively, while the spacing at North $56^{\text {th }}$ Street (Site A) in Temple Terrace improved from 1.92 feet to 2.32 feet. The lateral spacing between bicyclists and the edge of pavement on the bike lane section on North $56^{\text {th }}$ Street (Site B) was observed to be greater ( 2.51 feet) than observed at the sharrows segment ( 2.23 feet) and the difference was statistically significant, indicated by a p-value of 0.02 . The results suggested no significant difference in the way bicyclists ride relative to the curb after reconstructing Sunset Drive to a wide curb lane, and when comparing a no-buffer to buffer zone at the Bailey Road site in Fort Lauderdale.

### 7.3 Limited Access Crossing

It was observed that overtaking drivers regularly reduced speeds when approaching a bicyclist at a crossing location. Simultaneously, bicyclists typically slowed down when passing a motorist at a crossing location except at the eastbound off-ramp study site on William Lehman Causeway. At this location, bicyclists did not see the need to slow down as they could ride across the striped chevron pavement markings alongside the ramp lane until an acceptable crossing gap was presented. It is interesting to note that bicyclists consistently turned their heads to watch for approaching traffic before crossing to ensure a safe crossing maneuver.

### 7.4 Speed Analysis

Vehicle speeds were analyzed separately for limited access and non-limited access facilities. Data indicates that drivers slow down as they pass bicyclists on non-limited access roadways (before speed of 32.02 mph to 29.97 mph while-passing) and then increase their speeds after overtaking the bicyclists ( 30.80 mph while-passing to 32.82 mph after-passing). The difference between the speeds before-passing and while-passing, and while-passing and after-passing, were both significant with p-values less than 0.000 . However, when the before-passing ( 32.02 mph ) and after-passing ( 32.54 mph ), excluding while-passing speeds, were analyzed, no significant difference was found ( $p$-value $=0.110$ ).

For limited access facilities, the difference between the overtaking driver's speed before-passing ( 37.35 mph ) and while-passing ( 34.93 mph ) the bicyclists was significant with a p-value of 0.000 . However, the difference between motor vehicle speeds while-passing bicyclists ( 34.94 mph ) and after-passing ( 35.48 mph ) was not significant ( p -value $=0.150$ ). Contrary to the nonlimited access streets, the difference between vehicle speeds before- ( 37.33 mph ) and afterpassing ( 35.48 mph ) was significant for the limited access facilities ( p -value $=0.017$ ).

## REFERENCES

1. Pein, W.E., Hunter, W.W., and Stewart, J.R. Evaluation of the Shared-Use Arrow, Florida Department of Transportation, Tallahassee, Florida, 1999.
2. Alta Planning + Design. San Francisco's Shared Lane Pavement Markings: Improving Bicycle Safety. San Francisco Department of Parking and Traffic, San Francisco, California, 2004.
3. CALTRANS. California Manual on Uniform Traffic Control Devices, Sacramento, CA, 2003.
4. Jacobson, M., Skene, M., Davidson, G. and Rawsthorne, D. An Overview of Shared Use Lane Pavement Markings for Cyclists, Annual Conference of the Transportation Association of Canada, Vancouver, British Columbia, 2009.
5. Brady, J., Loskorn, J., Mills, A., Duthie, J., and Machemehl, R. Operational and Safety Implications of Three Experimental Bicycle safety Devices in Austin, TX. Paper presented at $90^{\text {th }}$ Annual Meeting of the Transportation Board, Washington, DC, 2011.
6. Hunter, W.W., Thomas, L., Srinivasan, R. and Martell, C.A. Evaluation of Shared Lane Markings. Federal Highway Administration Final Report HRT-10-041, Washington, DC: Federal Highway Administration. 2010.
7. Harkey, D.L. and Stewart, J.R. "Evaluation of Shared-Use Facilities for Bicycles and Motor Vehicles," In Transportation Research Record: Journal of the Transportation Research Board, No. 1578, Transportation Research Board of the National Academies, Washington, DC, 1997, pp. 111-118.
8. McHenry, S.R. and Wallace, M.J. Evaluation of Wide Curb Lanes as Shared Lane Bicycle Facilities, Maryland State Highway Administration, Baltimore, Maryland, 1985.
9. Hunter, W.W., Feaganes, J.R., and Srinivasan, R. "Conversions of Wide Curb Lanes: The Effect on Bicycle and Motor Vehicle Interactions," In Transportation Research Record: Journal of the Transportation Research Board, No. 1939, Transportation Research Board of the National Academies, Washington, DC, 2005, pp. 37-44.
10. Van Houten, R. and C. Seiderman. "How Pavement Markings Influence Bicycle and Motor Vehicle Positioning: Case Study in Cambridge, Massachusetts," In Transportation Research Record: Journal of the Transportation Research Board, No. 1939, Transportation Research Board of the National Academies, Washington, DC, 2005, pp. 314, 2005.
11. Furth, P. Dulaski, D., Buessing, M., and Tavakolian, P. "Parking Lane Width and Bicycle Operating Space," In In Transportation Research Record: Journal of the Transportation Research Board, No. 2190, Transportation Research Board of the National Academies, Washington, DC, 2010, pp. 45-50.
12. U.S. Department of Transportation. Manual on Uniform Traffic Control Devices for Streets and Highways [MUTCD], Washington, D.C.: Federal Highway Administration, 2009.

## APPENDIX A

## I-195/Julia Tuttle Causeway Fact Sheets



1-195/Julia Tuttle Causeway
Bicycles on
Linited Access Facities
HIDT riockal

- Program established by State Legslature under House Bill 599 in 2012.
- Permits bicyclists (not pedestriens) to travel on selected limited access facilities.
- Two (2) sites selected in Miami-Dade County. 1-195/Julia Tuttle Causeway and SR 856/William Lehman Causeway.
- Improvements include signing and pevement markings, new crossings, bridge railing, and minor roadway widening
- Facilities proposed to be open to bicycle traffic by March 2013.


Example of marked bicycle lane on a causeway.

- Study examining safety, operations, and demand will be conducted throughout the two (2) year period with a final report due in September 2015.
- Sigging and pevement marking scheme will include shared lane markings (sharrows) from Biscayne Boulevard to the Julia Tuttie Causeway along NE 36th Street (Ensthound) and NE 38th Street (Westbound) and at the SR 907/Alton Road interchange, a marked crossings at the SR 907/Alton Road off-ramp, a green colored bicycle lane at the Mt. Sinai Hospital exit ramp, and designated bicycle lanes along the Causeway.

- Initially, eastbound cyclists will be encourged to utilize

Example of shared lane markings (sharrow) with "bikes may used full lane" signage. the existing shared use path along the south side of the Canseway near the SR 907/Aiton Road eastbound offramp to access SR 907/Alton Road southbound. Once the widening of the eastbound off-ramp is complete, cyclists will be encouraged to utilize the bicycle lane on the new paved shoulder.

- Gyclists traveling from points south and east of the SR 907/Alton Road interchange will be encouraged to travel north on Alton Road ( $E$ ) to Sullivan Drive, left (southbound) onto Alton Road ( W ) to the westbound 1 -195 orramp. See the attached map for more information.
- Cyclists will be directed to enter/exit the Causeway via US 1/Biscayne Boulevard. Bicyclists will not be permitted to travel on $1-195$ west of US 1/Biscayne Boulevard ramps.
- For additional information, please contact Ken Jeffries, the District 6 Bicycle/Pedestrian Coordinator, at 305-$470-5445$ or at kenjeffriesedot state.fl.us.


FIGURE A. 1 Julia Tuttle Cswy Bicycle Improvement Details


FIGURE A. 2 Julia Tuttle Cswy Bicycle Improvement Plan

## APPENDIX B

Bicycle Crash Maps for Limited Access Pilot Studies
FDOT District 5
Bicycle Crashes 2005-2010 Pineda Causeway


Source: FDOT Safety Office Crash Analysis Reporting Database
FIGURE B. 1 Pineda Cswy Bicycle Crash Map 2005-2010

# FDOT District 6 <br> Bicycle Crashes 2005-2010 Julia Tuttle Causeway 



Source: FDOT Safety Office Crash Analysis Reporting Database
FIGURE B. 2 Julia Tuttle Cswy Bicycle Crash Map 2005-2010

## FDOT District 6

Bicycle Crashes 2005-2010
William Lehman Causeway


Source: FDOT Safety Office Crash Analysis Reporting Database
FIGURE B. 3 William Lehman Cswy Bicycle Crash Map 2005-2010

## APPENDIX C

## Bicycles on Limited Access Bridges

TABLE C. 1 Pilot Projects of Bicycles on Limited Access Bridges

| Bicycles on Limited Access Bridges |  |  |  |
| :---: | :---: | :---: | :---: |
| Pilot Projects |  |  |  |
| Location | Brevard Co. | Aventura | Miami Beach/Miami |
| Name | Pineda Causeway | Wm Lehman Causeway | Julia Tuttle Causeway |
| Google Map Link | http://goo.gl/maps | http://goo.gl/maps | http://goo.gl/maps |
| Limited Access Route | SR 404 | SR 856 | 1-195 |
| Waterbody Crossing | Indian \& Banana Rivers | ICWW | Biscayne Bay |
| Interchange 1 | US 1 | W. Country Club | NE 36th / NE 37th |
| Interchange 2 | SR A1A | A1A (Collins Ave) | Alton Road |
| Alt. Crossing Route | SR 518 | SR 826 | Venetian Causeway |
| Distance to Alt. Crossing | $>2$ miles | 1.7-2.0 miles | $>2$ miles |
| Population within 5 miles of Bridge Ends | 96,000 | 311,000 | ? |
| Bike Lane at Interchange 1? | Paved shoulders on Mainline, not on connecting ramp | No, Curb \& Gutter. | Sidewalk. No Paved Shoulder / Bike Lane |
| Bike Lane at Interchange 2? | Paved Shoulders (4'?) | No, Curb \& Gutter. | Sidewalk. No Paved Shoulder / Bike Lane |
| AADT | 33500 to 37500 | 33,500 | 109,000 |
| AADT/Lane | 8375 to 9375 | 5,583 | 18,167 |
| \% Trucks | 4 | 3 | 2.3 |
| Number of Lanes | 4 | 6 | 6 |
| Shoulder Width 1 (FT) | 7 to 8 | $10^{\prime}$ | 10 |
| Max. Grade (\%) | ? | 6 | ? |
| Posted Speed (MPH) | 55 | 45 | 55 |
| Potential Primary Usage | Transportation | Transportation | Both |
| Local Support | YES | YES | Yes |
| Potential Use | Significant | Great | Significant |
| Supports Regional Trails System | Access to Cape Canaveral National Seashore | Don Soffer Trail | Alternate route for East Coast Greenway instead of Venetian Causeway |

## APPENDIX D

Pineda Causeway Fact Sheet


As part of the pilot project, the guard railing on the Pineda Causeway is being retrofitted-making it higher, the roadway restriped, and drainage inlets replaced. Green stripes (above) are being used on the roadway to alert bicyclists and motorists that cyclists are crossing through areas of vehicular traffic. Grated inlets are being replaced with those that have smaller openings, to lower the risk of bike tires slipping in.

The addition of bicycle lanes will not affect the lane width or access for vehicular users on the roadway, nor is any road widening anticipated at this time. No additional right of way is needed. The speed limit will remain the same.

Bicycle lanes are planned for use in March of 2013. The project cost is estimated at $\$ 862,000$ and is covered by state transportation funds.

Table 1: Project Schedule

| Design Begin | May-2012 |
| :--- | ---: |
| Design Complete | June-2012 |
| Project Awarded to Contractor | September-2012 |
| Facility Open to Bike Traffic | March-2013 |

For additional information concerning the project, please contact Jeremy Dilmore, the FDOT Project Manager at (386) 943-5544 or via email at: Jeremy.Dilmore@dot.myflorida.com, or log onto www.cflroads.com.

FIGURE D. 1 Pineda Cswy Bicycle Improvement Details

## APPENDIX E

William Lehman Causeway Bike Lane Concept


FIGURE E. 1 William Lehman Cswy Bike Lane Plan Sheet 1 of 3


FIGURE E. 2 William Lehman Cswy Bike Lane Plan Sheet 2 of 3


FIGURE E. 3 William Lehman Cswy Bike Lane Plan Sheet 3 of 3

## APPENDIX F

William Lehman Causeway Fact Sheets


FIGURE F. 1 William Lehman Cswy Bicycle Improvement Details


FIGURE F. 2 William Lehman Cswy Bicycle Improvement Plan

## APPENDIX G

Bridge of Lions Sharrows Plan


FIGURE G. 1 Bridge of Lions Sharrows Placement Plan

## APPENDIX H

## Additional Photographs



FIGURE H. 1 Driver Yielding Right-of-Way to Bicyclist at Ramp Crossing on Julia Tuttle Cswy


FIGURE H. 2 Bicyclist Crossing Ramp Using Green Bike Lane on Julia Tuttle Cswy


FIGURE H. 3 Cyclist Group Using Wide Shoulder on Julia Tuttle Cswy


FIGURE H. 4 Sharrows on Lehman Cswy On-Ramp


FIGURE H. 5 Bicyclist using Bike Lane Approaching Lehman Cswy On-Ramp


FIGURE H. 6 Cyclist Checking Traffic before Crossing Lehman Cswy On-Ramp


FIGURE H. 7 Bicyclist Yielding to Traffic before Crossing Lehman Cswy On-Ramp


FIGURE H. 8 Bicyclist Negotiating with Driver before Crossing Lehman Cswy On-Ramp


FIGURE H. 9 Bicyclist Crossing Ramp After Yielding to Driver (Fig. H.8)


FIGURE H. 10 Bicyclist Crossing Ramp Using Bike Lane at Lehman Cswy On-Ramp

