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PHASE I DEVELOPMENT OF A NON-PROPRIETARY, FOUR-CABLE, HIGH TENSION MEDIAN BARRIER

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During the last decade, the use of cable median barriers has risen dramatically. Cable barriers are often utilized in depressed medians with widths ranging from 30 to 50 ft (9.1 to 15.2 m) and with fill slopes as steep as 4H:1V. Although cable barriers have been shown to contain and redirect many heavy trucks, a careful review of accident records has indicated that passenger vehicles do occasionally penetrate through the standard 3-cable median barrier and enter opposing traffic lanes. As a result, the Midwest States Pooled Fund Program sponsored a research and development project to improve the safety performance of existing, non-proprietary, cable median barriers. These safety improvements included increased cable spacing, increased cable height, the use of four cables, increased cable tension, and optimized cable attachment to posts. Three Test Level 3 crash tests were performed on a four-cable, high-tension median barrier placed in a 46-ft (14.0-m) wide, 4H:1V V-ditch. All tests were conducted according to the safety performance guidelines provided in the Manual for Assessing Safety Hardware (MASH). The first test utilized a 2270P pickup truck impacting the barrier placed on a downslope and 12 ft (3.7 m) laterally away from the front slope break point. The vehicle was contained and redirected by the barrier and deceptable according to the MASH guidelines. The second test utilized a 1100C small car impacting the barrier placed 27 ft (8.2 m) laterally away from the front slope breakpoint or 4 ft (1.2 m) up the back slope of the V-ditch. During the second test, the small car impacted the ditch bottom with a soft-soil condition and began to plow into the soil, thus causing moderate decelerations and velocity change before contacting the barrier and a marginal impact performance. The third test utilized a 1100C small car impacting the barrier in the same position as used in the second test. During the third test, the small car impacted the ditch bottom with a hard-soil condition and was contained by the barrier.			
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UNCERTAINTY OF MEASUREMENT STATEMENT

The Midwest Roadside Safety Facility (MwRSF) has determined the uncertainty of measurements for several parameters involved in non-standard testing of roadside safety hardware as well as in standard full-scale crash testing of roadside safety features. Information regarding the uncertainty of measurements for critical parameters is available upon request by the sponsor and the Federal Highway Administration.

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1 INTRODUCTION

1.1 Background and Problem Statement

The use of cable median barriers has risen dramatically during the last several years. These barriers are most frequently utilized in the medians of suburban or rural freeways that have experienced large increases in traffic volumes. Cable barriers are often placed in depressed medians with widths ranging from 30 to 50 ft (9.1 to 15.2 m) and with fill slopes as steep as 4H:1V. Although cable barriers have been shown to contain and redirect many heavy trucks, a careful review of accident records has indicated that passenger vehicles do occasionally penetrate through the standard 3-cable median barrier and enter opposing traffic lanes. A detailed evaluation of accidents involving the low-tension, non-proprietary, cable median barrier seems to indicate that the barrier is most vulnerable when struck from the one cable side. Further, crash testing has demonstrated that cables mounted on the back side of the posts are often ineffective for containing and redirecting an impacting vehicle.

Therefore, the Midwest States Pooled Fund Program sponsored a research study at the Midwest Roadside Safety Facility (MwRSF) to improve the safety performance of existing, low-tension, cable median barriers in an effort to reduce cross-over median crashes as well as to reduce dynamic barrier deflections. For this initial effort, MwRSF reviewed existing low-tension, cable median barriers, identified key design features, and developed several prototype four-cable, low-tension median barrier systems [1]. For this study, three full-scale vehicle crash tests were performed using pickup truck and small sedan test vehicles. For the testing program, each cable barrier system was installed on level terrain with the understanding that the final barrier system later would be tested and evaluated in a depressed median. Although the preliminary testing program resulted in

both unsuccessful and satisfactory outcomes, members of the Midwest States Pooled Fund Program chose to discontinue the R&D effort to develop an improved low-tension, cable median barrier system. Instead, the Pooled Fund members refocused their resources to develop a new, non-proprietary, high-tension, cable barrier system for use on both generally level terrain and in depressed medians.

During the initial R&D effort, MwRSF completed several tasks contributing to the development of new system components. These tasks included: the design and testing of an improved cable-to-post attachment mechanism to satisfy predetermined loading requirements; the identification, modification, and testing of cable end-fittings and splices for use in the new barrier system; and design and component testing of new barrier post to provide similar energy absorption as previous cable system posts [2-3]. Following the completion of these initial component studies, additional research funding was provided to configure, test, and evaluate the prototype high-tension, cable median barrier system when installed in a depressed median.

1.2 Research Objectives

The primary research objective was to develop an improved, non-proprietary, high-tension, cable median barrier system that would provide acceptable safety performance when installed on generally flat terrain as well as when placed at any location within a depressed median with fill slopes equal to or flatter than 4H:1V. The new barrier system should limit dynamic barrier deflections using cable-to-post attachment hardware that maximized the energy dissipated by the support posts as well as to select the appropriate range for cable tension. In addition, the new barrier system should mitigate vehicle underride and/or override by selecting the appropriate number of cables, the upper and lower cable heights, as well as a reasonable cable spacing. LS-DYNA

simulations were to be performed to determine the critical ditch width, cable barrier placement within the depressed median, and the top cable height for use in the full-scale crash testing program. Finally, the cable median barrier system was to be tested and evaluated according to the Test Level 3 (TL-3) safety performance criteria set forth in the *Manual for Assessing Safety Hardware* (MASH) [4].

1.3 Research Scope

The high-tension, cable median barrier system was initially configured using information obtained from MwRSF's prior research studies pertaining to the development of an improved cable median barrier. Next, LS-DYNA simulations were performed to: (1) determine the critical ditch width for use in the crash testing program; (2) determine cable barrier placement that provided the worst-practical condition for the crash testing program using test designation nos. 3-10 and 3-11; and (3) modify the top and bottom cable heights and the resulting cable spacing in order to prevent both vehicle override and underride of the barrier system. Design details were prepared for the prototype, high-tension, four-cable, median barrier system. The cable median barrier was constructed in a 4H:1V V-ditch for use in the testing and evaluation program. Three full-scale vehicle crash tests were conducted. The first test utilized a ¹/₂-ton Quad Cab pickup truck, weighing approximately 5,000 lb (2,268 kg), impacting at a speed and angle of 62.1 mph (100.0 km/h) and 25.0 degrees, respectively. The second and third tests utilized a small car, weighing approximately 2,425 lb (1,100 kg), impacting at a speed and angle of 62.1 mph (100.0 km/h) and 25.0 degrees, respectively. Finally, the test results were analyzed, evaluated, and documented. Conclusions and recommendations were then made that pertain to the safety performance of the cable barrier system.

2 BARRIER DESIGN

2.1 Barrier Components

From the onset of this study, multiple R&D tasks were undertaken to develop various barrier components. One such R&D task involved the design and testing of new cable-to-post attachment hardware. This effort involved the evaluation of various brackets, clips, U-bolts, J-bolts, keyway arrangements, and slots. Each of these options were designed in an attempt to provide significant lateral resistance while at the same time releasing vertically under low loads (e.g., under 1,000 lb). The component test results concluded that an 1/8-in. (3-mm) thick steel bracket attached to the post with shoulder bolts provided the best results [2]. Thus, these brackets were recommended for use in the new high-tension, cable median barrier.

Another R&D study was conducted to optimize the barrier system posts. Previous cable barriers were comprised of steel posts with soil plates welded to the front face to provide additional resistance to rotation through the soil. MwRSF researchers sought to eliminate the need for these plates by extending the embedment depth of the post. Dynamic component tests revealed that an S3x5.7 (S76x85) steel post embedded 43 in. (1,092 mm) into the soil provided similar resistance to the previous post designs [3]. Therefore, the 90-in. (2,286-mm) long, S3x5.7 (S76x85) posts were utilized in the new barrier design.

An analysis was also conducted to select the pretension in the cables. Thermal expansion and contraction due to temperature changes can have a huge impact on the magnitude of pretension in a cable barrier system. Calculations showed that a long run of cable exposed to a temperature swing of 100 deg Fahrenheit (38 deg Celsius) can vary the load by 4,000 lb (17.8 kN). BARRIER VII computer simulations were conducted to evaluate the effect of varying the cable pretensions [5].

Results showed that the cable tension had only a minimal effect on the maximum deflection of the system. Thus, increasing the tension in the cables would not negatively affect the performance of the barrier [6]. However, taking tension out of the system and allowing the cables to sag could not only reduce the barrier's ability to capture errant vehicles through cable-sheet metal interlock but also require additional maintenance after minor impacts. Therefore, a pretension corresponding to 4,000 lb (17.8 kN) at 110 deg Fahrenheit (43 deg Celsius) was prescribed for the new barrier system.

2.2 Barrier Configuration and Critical Locations

One goal for this research study was to develop a new, high-tension, four-cable median barrier system that could be located anywhere in a 4H:1V sloped V-ditch. The difficulty in developing a barrier that can be placed anywhere in a ditch is that the vertical position of an errant vehicle in relation to the ground is no longer constant as it is for vehicles traversing level terrain. Instead, the vertical position of the vehicle can vary between being airborne when traveling off the slope break point and into the V-ditch to being at ground level when landing in the ditch and compressing the vehicle suspension. In order to determine the appropriate cable heights and ensure the capture of errant vehicles, it became necessary to identify both critical barrier locations within the V-ditch as well as a critical ditch width.

The research team believed that the upper and lower limits for cable heights would be controlled by two conditions, vehicle override and underride, respectively. The greatest chance of vehicle override would occur when a pickup truck entering the ditch was airborne and at its maximum height above the sloped ground directly below it. This maximum height would be directly related to a specific lateral offset, or a critical barrier location. Thus, the critical barrier location for override and the height to the upper cable (maximum height of the vehicle) would be determined from the trajectory of the pickup off the slope break point and into the 4H:1V sloped V-ditch.

Using NCHRP Report No. 350 or MASH TL-3 impact conditions and LS-DYNA computer simulations, the pickup truck's trajectory was tracked. From this analysis, the truck's bumper would reach a maximum height of 43 in. (1,092 mm) above the sloped ditch at an offset distance of 12 ft (3.7 m) laterally from the slope break point, as shown in Figure 1. Thus, the upper cable was prescribed a minimum height of 43 in. (1,092 mm) and the critical barrier offset distance was established as 12 ft (3.7m) for the full-scale test evaluating pickup containment and override potential.



Midwest Roadside Safety Facility

Figure 1. Critical Cable Height and Barrier Offset for 2000P Pickup Truck

The lower cable height would be designed to prevent vehicle underride, and the greatest potential for underride occurs when a small car lands in the V-ditch causing the front wheel

suspension to be maximally compressed prior to engaging the lower cable with the vehicle's front bumper. However, vehicle traversals through different ditch widths result in varying levels of compression for the front suspension. As a result, a combination of critical ditch width and critical barrier offset was required to identify the necessary height for the lowest cable.

In evaluating underride potential, two V-ditch widths were evaluated: (1) a relatively common 32 ft (9.8 m) wide section and (2) an extended 46 ft (14.0 m) wide section.NCHRP Report No. 350 or MASH TL-3 impact conditions and LS-DYNA were again utilized during the underride analysis as well as an actual field survey of small car front bumper heights and front-end designs. The 46-ft (14.0-m) wide V-ditch was determined to be more critical as it allowed the vehicle to maximize its airborne flight, thus falling further vertically before impacting the back slope near the ditch bottom. Once striking the ground surface, the vehicle's front suspension began to compress as the vehicle traversed up the back slope. When the maximum suspension compression was observed, the vehicle's leading corner was approximately 4 ft (1.2 m) up the back slope, and the front bumper had bottomed out. Thus, the second critical barrier location (for the small car underride condition) was established as 4 ft (1.2 m) up the back slope of a 46-ft (14.0-m) wide V-ditch. Selected simulation results for the Geo Metro and Dodge Neon at a time just prior to barrier impact are shown in Figures 2 and 3, respectively.

Based on the selected lower and upper limits for cable heights of 13 in. (330 mm) and 43 in. (1,092 mm), respectively, and the desire to utilize four cables within the system, a 10-in. (254-mm) cable spacing was chosen. Thus, the heights of the four cables were selected as 13 in. (330 mm), 23 in. (584 mm), 33 in. (838 mm), and 43 in. (1,092 mm), as measured between the centerline of each cable and ground level.

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Figure 2. Geo Metro Prior to Impact - 46-ft (14.0-m) Wide V-Ditch



Figure 3. Dodge Neon Prior to Impact - 46-ft (14.0-m) Wide V-Ditch

However, several uncertainties existed within the LS-DYNA computer simulations, including: (1) unavailability of accurate models for MASH test vehicles to perform the simulations; (2) actual upper and lower slope break points will vary from the idealized configurations; (3) variability with vehicle and tire behavior while traversing slope break points; and (4) the state of tire and suspension compression possibly aiding the launching of the vehicles when departing the slope break point. Due to these uncertainties in the simulations and to be conservative against vehicle override, each cable height was increased by 2 in. (51 mm) while maintaining the 10-in. (254-mm) cable spacing. As such, the cable heights were revised prior to testing and positioned at 15 in. (381 mm), 25 in. (635 mm), 35 in. (889 mm), and 45 in. (1,143 mm) above grade. However, the 46-ft (14.0-m) wide V-ditch was retained. The critical lateral barrier offset of 12 ft (3.7 m) from the front slope break point was utilized to evaluate the maximum override propensity for pickup trucks. In addition, a critical lateral barrier offset of 4 ft (1.2 m) from the ditch bottom was used to evaluate the maximum underride propensity for small cars.

3 CABLE MEDIAN BARRIER DESIGN DETAILS FOR TEST NO. 4CMB-1

The cable barrier system was constructed using a total length of 608 ft (185.3 m) with the majority of the barrier system placed within a simulated depressed median. The test installation consisted of several distinct components, systems, and features: (1) a depressed V-ditch; (2) wire ropes or cables; (3) steel support posts; (4) cable-to-post attachment brackets; (5) cable splice hardware; (6) breakaway end terminal hardware; (7) reinforced concrete foundations; (8) cable end fittings; (9) turnbuckle assemblies; and (10) load cell end assemblies. Design details were prepared for the cable median barrier system installed in a simulated depressed median, as shown in Figures 4 through 19. Material specifications, mill certifications, and certificates of conformity for the system materials are shown in Appendix A.

A 360-ft (109.7-m) long simulated depressed median V-ditch was constructed using an overall width of 46 ft (14.0 m) in combination with 4H:1V side slopes. As a result, the vertical depth at the center of the ditch was approximately 5 ft - 9 in. (1.75 m). The full ditch cross section was constructed for a length of 280 ft (85.3 m). A 40-ft (12.2 m) long sloped transition region was located at the upstream and downstream ends of the V-ditch, thus allowing for the ditch to be raised to the roadway elevation.

Four ³/₄-in. (19-mm) diameter, Class A galvanized 3x7 (pre-stretched) wire ropes were utilized for the cable rail elements. The cables were either supported or anchored at 40 different locations, as shown in Figure 4. Post nos. 1 and 40 were configured to serve as the upstream and downstream end anchors, respectively, and these locations incorporated breakaway end terminal hardware supported by 2-ft (610-mm) diameter by 10-ft (3,048-mm) long reinforced concrete foundations. Post nos. 2 and 39 consisted of breakaway steel support posts anchored to 1-ft (305-

mm) diameter by 4-ft (1,219-mm) long reinforced concrete foundations. Post nos. 3 through 38 consisted of S3x5.7 (S76x8.5) standard steel line posts measuring 7 ft - 3 in. (2,210 mm) in length. The standard line posts were embedded 39 in. (991 mm) into the compacted soil foundation. The spacing between post nos. 1 and 2 as well as 39 and 40 was 8 ft (2.44 m), while the post spacing between post nos. 2 through 39 was 16 ft (4.88 m). For the standard line posts, the four cables were attached to the posts and placed at 15 in. (381 mm), 25 in. (635 mm), 35 in. (889 mm), and 45 in. (1,143 mm) above the ground surface, as shown in Figures 4 and 6.

Each cable was attached to the line posts using an ¹/₈-in. (3.2-mm) thick ASTM A36 steel keyway bracket, two 5/16-in. (7.9-mm) diameter ASTM A307 hex head shoulder bolts, and the appropriate nuts and washers. Details for the cable-to-post attachment bracket, mounting hardware, and locations can be found in Figures 18 and 19.

It should be noted that the initial barrier configuration had planned to utilize a 42-in. (1,067mm) post embedment depth along with cable locations of 13 in. (330 mm), 23 (584 mm), 33 (838 mm), and 43 in. (1,093 mm). After the steel posts had been fabricated, further analysis led the research team to raise the top cable height from 43 in. to 45 in. as well as to incorporate a similar incremental upward shift for the lower three cables. It was also realized that an insufficient top end distance of approximately 1 in. (25 mm) was available for attaching the upper cable-to-post bracket to the top of each post. As such, each line post was raised 3 in. (76 mm) to accommodate the two late design modifications, thus resulting in a 39-in. (991-mm) post embedment depth with cable heights of 15 in. (381 mm), 25 in. (625) mm), 35 in. (879 mm), and 45 in. (1,143 mm). However, it was the research team's opinion that these changes would not negatively effect the safety performance of the barrier system, and new longer posts were not fabricated. Each of the four wire ropes were spliced together using special cable splice hardware located between post nos. 18 through 21, as shown in Figure 5. At the ends of the cable barrier system, each cable was sloped down to the ground and anchored to a simulated breakaway end terminal system, as shown in Figure 7 and 10 through 13. As noted previously, post nos. 1 and 40 served as the end cable anchors and consisted of a cable anchor bracket, cable release lever, brass keeper rod, special end fittings, and a reinforced concrete foundation. Details for the concrete foundation are provided in Figure 10. The cable anchor bracket was fabricated using six different ASTM A36 steel plates that were welded together, as shown in Figures 11 and 12. The cable release lever was manufactured from three 1¼-in. (32-mm) square steel tubes, a steel plate, and two triangular steel gusset plates, as shown in Figure 13.

As noted previously, posts nos. 2 and 39 served as breakaway steel support posts with attached hanger hardware, as shown in Figure 14. The S3x5.7 (S76x8.5) posts incorporated a steel bracket plate near the top of the post as well as a slipbase connection near the groundline, as shown in Figures 15 and 16. Each post was inserted into steel foundation tube assembly measuring 4 in. x 3 in. x 1/4 in. (102 mm x 76 mm x 6.3 mm) and conforming to ASTM A500 Grade B. A 3/4-in. (19.1 mm) diameter by 6-in. (152-mm) long, ASTM A307 hex head bolt, nut, and two washers were used to support the post and ensure proper embedment, as shown in Figure 17. Each steel tube assembly was embedded within a reinforced concrete foundation.

Near the upstream end of the barrier system, one 50,000-lb (222.4-kN) capacity tension load cell was spliced into each of the cable lines between post nos. 3 and 4, as shown in Figures 6 and 8. Details for the load cells, threaded rods, turnbuckles, end fittings, and rod couplers are provided in Figures 8 and 9.

As discussed in the previous chapter, the pretension in the cables was selected to be 4,000 lb (17.8 kN) at 110 deg Fahrenheit (43 deg Celsius). A cable tensioning chart was developed as a function of the ambient air temperature and for use when installing the barrier system, as provided in Table 1. MASH specifies that all cable systems be tested/evaluated under tensions corresponding to a temperature of 100 deg Fahrenheit (38 deg Celsius). As a result, the cables were pretensioned to a target load of 4,213 lb (18.7 kN).

Ambient Air Temperature (Degrees Fahrenheit)	Cable Tension (lb)
110	4,000
100	4,213
90	4,427
80	4,640
70	4,853
60	5,067
50	5,280
40	5,493
30	5,706
20	5,920
10	6,133
0	6,346
-10	6,560
-20	6,773
-30	6,986
-40	7,200

Table 1. Pre-Stretched Cable Tension Chart



Figure 4. System Layout, Test No. 4CMB-1



Figure 5. Cable Splice Location and Detail, Test No. 4CMB-1



Figure 6. Cable Terminal Detail, Test No. 4CMB-1



Figure 7. Cable Anchor Details, Test No. 4CMB-1



Figure 8. Load Cell and Strongback Configuration, Test No. 4CMB -1



Figure 9. Load Cell Assembly Component Details, Test No. 4CMB-1



Figure 10. Cable Anchor Detail, Post Nos. 1 & 40, Test No. 4CMB-1



Figure 11. Cable Anchor Bracket, Part A, Test No. 4CMB-1

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Figure 12. Cable Anchor Bracket Details, Test No. 4CMB-1


Figure 13. Cable Release Lever, Test No. 4CMB-1



Figure 14. Post Nos. 2 & 39 Details, Test No. 4CMB-1

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Figure 15. Cable Hanger Assembly, Post Nos. 2 & 39, Test No. 4CMB-1



Figure 16. Cable Hanger Assembly, Post Nos. 2 & 39, Test No. 4CMB-1



Figure 17. Foundation Tube Assembly, Post Nos. 2 & 39, Test No. 4CMB-1



Figure 18. Cable Support Post, Post Nos. 3-38, Test No. 4CMB-1



Figure 19. Shoulder Bolt, Nut, Washer, and Keyway Bracket Detail, Test No. 4CMB-1

4 TEST REQUIREMENTS AND EVALUATION CRITERIA

4.1 Test Requirements

Longitudinal barriers, such as cable median barriers, must satisfy the requirements provided

in MASH to be accepted for use on National Highway System (NHS) construction projects or as a

replacement for existing systems not meeting current safety standards [4]. According to Test Level

3 (TL-3) of MASH, the barrier system must be subjected to two full-scale vehicle crash tests. The

two crash tests are as follows:

- 1. Test Designation 3-10 consists of a 2,425-lb (1,100-kg) small car impacting the cable median barrier at a nominal speed and angle of 62.1 mph (100.0 km/h) and 25 degrees, respectively.
- 2. Test Designation 3-11 consists of a 5,000-lb (2,268-kg) pickup truck impacting the cable median barrier at a nominal speed and angle of 62.1 mph (100.0 km/h) and 25 degrees, respectively.

The test conditions for TL-3 longitudinal barriers are summarized in Table 2.

			Im	pact Condi		
Test Article	Test Designation	Test Vehicle	Speed		Angle	Evaluation Criteria ¹
<i>i</i> intere			(km/h)	(mph)	(degrees)	
Longitudinal	3-10	1100C	100.0	62.1	25	A,D,F,H,I
Barriers	3-11	2270P	100.0	62.1	25	A,D,F,H,I

Table 2. MASH Test Level 3 Crash Test Conditions

 $\overline{}^{1}$ - Evaluation criteria explained in Table 3.

4.2 Evaluation Criteria

Evaluation criteria for full-scale vehicle crash testing are based on two appraisal areas: (1) structural adequacy and (2) occupant risk. Criteria for structural adequacy are intended to evaluate

the ability of the cable terminal to contain, redirect, or allow controlled vehicle penetration in a predictable manner. Occupant risk evaluates the degree of hazard to occupants in the impacting vehicle. These two evaluation criteria are defined in Table 3. The full-scale vehicle crash test was conducted and reported in accordance with the procedures provided in MASH.

4.3 Soil Strength Requirements

In order to limit the variation of soil strength among testing agencies, foundation soil must satisfy the recommended performance characteristics set forth in Chapter 3 and Appendix B of MASH. Testing facilities must first subject the designated soil to a dynamic post test to demonstrate a minimum dynamic load of 7.5 kips (33.4 kN) at deflections between 5 and 20 in. (127 and 508 mm). If satisfactory results are observed, a static test is conducted using an identical test installation. The results from this static test become the baseline requirement for soil strength in future full-scale crash testing in which the designated soil is used. An additional post installed near the impact point is statically tested on the day of full-scale crash test in the same manner as used in the baseline static test. The full-scale crash test can be conducted only if the static test results show a soil resistance equal to or greater than 90 percent of the baseline test at deflections of 5, 10, and 15 in. (127, 254, and 381 mm). Otherwise, the crash test must be postponed until the soil demonstrates adequate post-soil strength.

It should be noted that test nos. 4CMB-1 and 4CMB-2 were conducted prior to the official release of MASH, and although the test conditions reflected the anticipated test criteria, static soil tests were not performed.

Evaluation Factors		Evaluation Criteria				
Structural Adequacy	A	A. Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable				
	Ι	D. Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH.				
	F	F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.				
	H. Occupant Impact Velocities (OIV) (see Appendix A, Section A5.3 for calculation procedure) should satisfy the following limits:					
Occupant		Occupant Impact Velocity Limits, ft/s (m/s)				
Risk		Component	Preferred	Maximum		
				Longitudinal and Lateral	30 ft/s (9.1 m/s)	40 ft/s (12.2 m/s)
		Longitudinal	10 ft/s (3.0 m/s)	16.4 ft/s (5.0 m/s)		
	Ι	I. The Occupant Ride Down Acceleration (see Appendix A, Sect A5.3 for calculation procedure) should satisfy the following lim				
		Occu	pant Ridedown Accel	eration Limits (g's)		
		Component	Preferred	Maximum		
		Longitudinal and Lateral	15.0 g's	20.49 g's		

Table 3. MASH Evaluation Criteria for Crash Tests

5 TEST CONDITIONS

5.1 Test Facility

The testing facility is located at the Lincoln Air-Park on the northwest (NW) side of the Lincoln Municipal Airport and is approximately 5 mi. (8 km) NW of the University of Nebraska-Lincoln.

5.2 Vehicle Tow and Guidance System

A reverse cable tow system with a 1:2 mechanical advantage was used to propel the test vehicle. The distance traveled and the speed of the tow vehicle were one-half that of the test vehicle. The test vehicle was released from the tow cable before impact with the barrier system. A digital speedometer was located on the tow vehicle to increase the accuracy of the test vehicle impact speed.

A vehicle guidance system developed by Hinch [7] was used to steer the test vehicle. A guide-flag, attached to the front-right wheel and the guide cable, was sheared off before impact with the barrier system. The %-in. (9.5-mm) diameter guide cable was tensioned to approximately 3,500 lb (15.6 kN) and was supported laterally and vertically every 100 ft (30.48 m) by hinged stanchions. The hinged stanchions stood upright while holding up the guide cable. As the test vehicle was towed down the line, the guide-flag struck and knocked each stanchion to the ground. For tests 4CMB-1, 4CMB-2, and 4CMB-3, the vehicle guidance systems were 961 ft (293 m), 920 ft (280 m), and 798 ft (243 m) long, respectively.

5.3 Test Vehicles

For test no. 4CMB-1, a 2002 Dodge Ram 1500 Quad-Cab pickup truck was used as the test vehicle. The test inertial and gross static weight was 4,988 lb (2,263 kg). The test vehicle is shown

in Figure 20, and vehicle dimensions are shown in Figure 21.

For test no. 4CMB-2, a 2002 Kia Rio sedan was used as the test vehicle. The test inertial and gross static weights were 2,393 lb (1,085 kg) and 2,557 lb (1,160 kg), respectively. The test vehicle is shown in Figure 22, and vehicle dimensions are shown in Figure 23.

For test no. 4CMB-3, a 2002 Kia Rio sedan was used as the test vehicle. The test inertial and gross static weights were 2,411 lb (1,094 kg) and 2,586 lb (1,173 kg). The test vehicle is shown in Figure 24, and vehicle dimensions are shown in Figure 25.

The vertical and longitudinal components of the center of gravity was determined using the measured axle weights. The location of the final centers of gravity are shown in Figures 26 through 28.

Square, black- and white-checkered targets were placed on the vehicle to aid in the analysis of the high-speed film analysis, as shown in Figures 26 through 28. Round, checkered targets were placed on the center of gravity on the driver's side door, on the passenger's side door, and on the roof of the vehicle. The remaining targets were located for reference so that they could be viewed from the high-speed cameras for film analysis.

The front wheels of the test vehicle were aligned for camber, caster, and toe-in values of zero so that the vehicle would track properly along the guide cable. One 5B flash bulb was mounted on both the hood and roof of the vehicle to pinpoint the time of impact with the barrier on the high-speed film and video. The flash bulb was fired by a pressure tape switch mounted on the front face of the bumper. A remote-controlled brake system was installed in the test vehicle so the vehicle could be brought safely to a stop after the test.



Figure 20. Test Vehicle, Test No. 4CMB-1

Vehicle Geometry -- in. (mm)

Date:	10/30/2007	Test Number:	4C MB - 1	Model:	Ram 1500 Q.C.
Make:	Dodge	Vehicle I.D.#:	1D7HA18N0	2S571166	
Tire Size:	265/70 R17	Year:	2002	Odometer: _	71356

*(All Measurements Refer to Impacting Side)



Figure 21. Vehicle Dimensions, Test No. 4CMB-1



Figure 22. Test Vehicle, Test No. 4CMB-2

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Date:	11/16/2007	Test Nur	mber:4	CMB-2	Model:	Rio S	Sedan
Make:	Kia	Vehicle	I.D.#:	KNADC 12	3X26149132		
⊺ire Size:	P175/65R14	Year:		2002	Odometer:	781	03
⊺ire Inflatio *(All Measu	on Pressure: urements Refer to	29 Impacting Side)		_			
		~			Vehicle Geometry	y —— in. (mn	n)
				a	64.0 (1626)	b 56.0	(1422)
			<u> </u>	с	166.0 (4216)	d 38.25	(972)
α			vehicle	e	95.25 (2419)	f 34.5	(876)
			<u> </u>	g	18.0 (457)	h 40.5	(1029)
<u> </u>		<u>(</u>		Ť	9.0 (229)	j <u>21.0</u>	(533)
				k	10.5 (267)	I 21.0	(533)
_ →			•	m	55.25 (1403)	n 56.875	(1445)
				0	22.0 (559)	p <u>3.25</u>	(83)
		-4-	<u> </u>	q	22.625 (575)	r 15.375	(391)
			k l g	s	11.75 (298)	t 64.75	(1645)
_	f h	e d	t		Wheel Center Height Fr	ont 10.625	(270)
-	↓ W _{front} c	√W _{rear}			Wheel Center Height R	ear11.0	(279)
					Wheel Well Clearance (FR) 25.375	(645)
					Wheel Well Clearance (RR) 23.75	(603)
					Frame Height (FR) NA	NA
					Frame Height (RR) NA	NA
Weights					Engine T	ype 4cyl	Gas
lbs (kg)	Curb	Test Inertial	Gross Static		Engine S	Size 1.5	L
W-front	1366 (620)	1024 (464)	1451 (6	58)	Transmission	Туре:	
W-rear	883 (401)	1369 (621)	1106 (5	02)		Automatic	Manual
W-total	2249 (1020)	2393 (1085)	2557 (11	50)	(F	WD RWD	4WD
GVWR Ra	atings		Dumr	ny Data			
	Front	1808		Туре:	Hybrid 2		
	Rear	1742		Mass	166lbs		
	Total	(3315)	Se	at Position:	Driver's side		
Note any	damage prior to test	:		n	one		

Figure 23. Vehicle Dimensions, Test No. 4CMB-2



24. Test Vehicle, Test No. 4CMB-3

Make: Tire Size: Tire Inflation P *(All Measurem	Kia P175/65R1	Vehicle	I.D.#:	KNADC 1	07061070			
Tire Size: Tire Inflation P *(All Measurem	P175/65R1			NINADO I	23201070	28		
Tire Inflation P *(All Measurem		4 Year:	_	2002	_	Odometer	: 57	454
	ressure: ents Refer t	30 p o Impacting Side)	si					
					Vehicle	Geometry	in. (mm)
					a <u>63.75</u>	(1619)	b <u>55.0</u>	(1397)
			ę n t		c 168.875	(4289)	d <u>36.25</u>	(921)
u			vehicle		e 95.25	(2419)	f <u>37.375</u>	(949)
. ↓ - ŧL					g <u>18.0</u>	(457)	h <u>37.125</u>	(943)
0		<u> </u>			6.75	(171)	j20.5	(521)
					k <u>10.0</u>	(254)	23.0	(584)
			1		m 55.75	(1416)	n <u>56.5</u>	(1435)
					o <u>30.0</u>	(762)	p2.5	(64)
					q22.75	(578)	r <u>15.25</u>	(387)
			k		s <u>12.25</u>	(311)	t65.0	(1651)
f	h	e d	T		Wheel Cent	er Height Fro	ont 10.75	(273)
	, W _{front}	c √ ^W rear			Wheel Cen	ler Height Re	ar <u>11.125</u>	(283)
Mass					Wheel Well	Clearance (F	R) 25.0	(635)
Distribution					Wheel Well	Clearance (R	R) 24.25	(616)
LF	731	RF 732			Fra	me Height (F	R) 7.5	(191)
LR	457	RR 491			Fra	me Height (R	R) 16.0	(406)
Weights						Engine Ty	pe 4 C)	Gas .
lbs (kg) Ci	urb	Test Inertial	Gross Static			Engine Si	ze1	.4
W-front 15	16 (687.65)	1463 (663.61)	1550 (7	/03.07)			Type:	
W-rear 88	37 (402.34)	948 (430.01)	1035 (4	69.47)		2	Automatic	Manual
W-total 24	02 (1089.53)	(1093.61)	2586 (11	72.99)		(FW	RWD	4WD
GVWR Ratings			Du	mmy Data				
Fr	ont	1808		Тур	De: Hybrid 2			
R	ear	1742		Ма	ss: 166lbs			
To	otal	(3315)		Seat Positio	on:			

Figure 25. Vehicle Dimensions, Test No. 4CMB-3



Figure 26. Vehicle Target Locations, Test No. 4CMB-1



Figure 27. Vehicle Target Locations, Test No. 4CMB-2



Figure 28. Vehicle Target Locations, Test No. 4CMB-3

5.4 Data Acquisition Systems

5.4.1 Accelerometers

One triaxial piezoresistive accelerometer system with a range of ± 200 Gs was used to measure the acceleration in the longitudinal, lateral, and vertical directions at a sample rate of 3,200 Hz. The environmental shock and vibration sensor/recorder system, Model EDR-3, was developed by Instrumental Sensor Technology (IST) of Okemos, Michigan. The EDR-3 was configured with 256 kB of RAM memory and a 1,120 Hz lowpass filter. "DynaMax 1 (DM-1)" and "DADiSP" were used to analyze and plot the accelerometer data.

A second system consisted of a two-Arm piezoresistive accelerometer that was developed by Endevco of San Juan Capistrano, California. The accelerometer system was used to measure the longitudinal, lateral, and vertical accelerations at a sample rate of 10,000 Hz. Data was collected using a Sensor Input Module (SIM), Model TDAS3-SIM-16M, which was developed by Diversified Technical Systems, Inc. (DTS) of Seal Beach, California. The SIM was configured with 16 MB SRAM memory and 8 sensor input channels with 250 kB SRAM/channel. The SIM was mounted on a TDAS3-R4 module rack. The module rack was configured with isolated power/event/communications, 10BaseT Ethernet and RS232 Communication, and an internal backup battery. Both the SIM and module rack are crash worthy. Computer software programs "DTS TDAS Control", "DADiSP", and a customized Microsoft Excel worksheet were used to analyze and plot the accelerometer data from the DTS unit. All of the accelerometers were mounted near the center of gravity of the test vehicle.

For test no. 4CMB-1, another triaxial piezoresistive accelerometer system with a range of ± 200 Gs was also used to measure the acceleration in the longitudinal, lateral, and vertical directions

at a sample rate of 10,000 Hz. The environmental shock and vibration sensor/recorder system, Model EDR-4M6, was developed by Instrumented Sensor Technology (IST) of Okemos, Michigan and includes three differential channels as well as three single-ended channels. The EDR-4 was configured with 6 MB of RAM memory and a 1,500 Hz lowpass filter. "DynaMax 1 (DM-1)", "DADiSP", and a customized Microsoft Excel worksheet were used to analyze and plot the accelerometer data. Due to technical difficulties with the EDR-4 accelerometer, its data was not used for analysis.

5.4.2 Rate Transducers

An Analog Systems 3-axis rate transducer with a range of 1,200 degrees/sec in each of the three directions (pitch, roll, and yaw) was used to measure the rates of motion of the test vehicle. The rate transducer was mounted inside the body of the EDR-4M6 and recorded data at 10,000 Hz to a second data acquisition board inside the EDR-4M6 housing. The raw data measurements were then downloaded, converted to the appropriate Euler angles for analysis, and plotted. "DynaMax 1", "DADiSP," and a customized Microsoft Excel worksheet were used to analyze and plot the rate transducer data.

5.4.3 High-Speed Photography

For test no. 4CMB-1, four high-speed AOS VITcam digital video cameras, all operating at 500 frames/sec, were used to film the crash test. Four JVC digital video cameras and two Canon digital video cameras, all with a standard operating speed of 29.97 frames/sec, were also used to film the crash test. Camera details and a schematic of all ten camera locations for test no. 4CMB-1 are shown in Figure 29.

For test 4CMB-2, five high-speed AOS VITcam digital video cameras, all operating at 500

frames/sec, were used to film the crash test. Four JVC digital video cameras and two Canon digital video cameras, all with a standard operating speed of 29.97 frames/sec, were also used to film the crash test. Camera details and a schematic of all eleven camera locations for test no. 4CMB-2 is shown in Figure 30.

For test 4CMB-3, four high-speed AOS VITcam digital video cameras, all operating at 500 frames/sec, were used to film the crash test. Five JVC digital video cameras and two Canon digital video cameras, all with a standard operating speed of 29.97 frames/sec, were also used to film the crash test. Camera details and a schematic of all eleven camera locations for test no. 4CMB-3 is shown in Figure 31.

The Locam films, Photron and AOS videos, and E/cam videos were analyzed using the Vanguard Motion Analyzer, ImageExpress MotionPlus software, and Redlake Motion Scope software, respectively. Actual camera speed and camera divergence factors were considered in the analysis of the high-speed film.

5.4.4 Pressure Tape Switches

For tests 4CMB-1, 4CMB-2, and 4CMB-3, five pressure-activated tape switches, spaced at 6.56-ft (2-m) intervals, were used to determine the speed of the vehicle before impact. Each tape switch fired a strobe light which sent an electronic timing signal to the data acquisition system as the left-front tire of the test vehicle passed over it. Test vehicle speed was determined from electronic timing mark data recorded using the Test Point software. Strobe lights and high-speed film analysis are used only as a backup in the event that vehicle speed cannot be determined from the electronic data.

5.4.5 Cable Instrumentation

Electronic sensors were placed near the end terminal anchors used in the four-cable, high tension, median barrier system. Load cell and string potentiometers were used and are described below.

5.4.5.1 Load Cells

Four load cells were installed along the four-cable barrier system. Each load cell was positioned in line with each cable on the upstream end. The load cells were placed between post nos. 3 and 4, as shown in Figure 32.

The load cells were manufactured by Transducer Techniques and conformed to model no. TLL-50K with a load range up to 50,000 lb (222.4 kN). During testing, output voltage signals were sent from the load cells to a Keithly Metrabyte DAS-1802HC data acquisition board, acquired with Test Point software, and stored permanently on a personal computer. The data collection rate for the load cells was 10,000 samples per second (10,000 Hz).

5.4.5.2 String Potentiometers

A string potentiometer (linear variable displacement transducer) was installed on the upstream end terminal anchorage system and used to monitor longitudinal anchor displacement, as shown in Figure 33.

A UniMeasure PA-50 string potentiometer, with a range of 50 in. (1.27 m), was used. A Measurements Group Vishay Model 2310 signal conditioning amplifier was used to condition and amplify the low-level signals to high-level outputs for multichannel, simultaneous dynamic recording on the Test Point software. After each signal was amplified, it was sent to a Keithly Metrabyte DAS-1802HC data acquisition board and then stored permanently on a personal

computer. The data collection rate for of the string potentiometers was 10,000 samples per second (10,000 Hz).



M

Digital Video #7

			Temp Teamera eam	initian y	
	No.	Туре	Operating Speed (frames/sec)	Lens	Lens Setting
	1	AOS Vitcam CTM	500	8.5 mm fixed	
ec jh-	2	AOS Vitcam CTM	500	12.5 mm fixed	
Yid Pic	3	AOS Vitcam CTM	500	COMPUTAR 12.5 mm fixed	
- 0/ 2	4	AOS Vitcam CTM	500	Sigma 70-200	70
0	2	JVC - GZ-MC500 (Everio)	29.97		
de	3	JVC - GZ-MC500 (Everio)	29.97		
Ϋ́.	4	JVC - GZ-MC500 (Everio)	29.97		
tal	5	JVC - GZ-MC500 (Everio)	29.97		
igi	7	Canon-ZR90	29.97		
Δ	8	Canon-ZR90	29.97		

4CMB-1 Camera Summary

Figure 29. Camera Locations, Test No. 4CMB-1



	4CMB-2 Camera Summary					
	No.	Туре	Operating Speed (frames/sec)	Lens	Lens Setting	
þé	1	AOS Vitcam CTM	500	8.5 mm fixed		
e e	2	AOS Vitcam CTM	500	12.5 mm fixed		
ide -Sr	3	AOS Vitcam CTM	500	COMPUTAR 12.5 mm fixed		
dg >	4	AOS Vitcam CTM	500	Sigma 70-200	200	
Hi	5	AOS Vitcam CTM	500	Sigma 24-70	28	
0	1	JVC - GZ-MC500 (Everio)	29.97			
de	2	JVC - GZ-MC500 (Everio)	29.97			
Ň	3	JVC - GZ-MC500 (Everio)	29.97			
tal	5	JVC - GZ-MC500 (Everio)	29.97			
igi	7	Canon-ZR90	29.97			
	8	Canon-ZR90	29.97			

50



4CMB-3 Camera Summary

	No.	Туре	Operating Speed (frames/sec)	Lens	Lens Setting
	2	AOS Vitcam CTM	500	8 mm fixed	
-ht eec	3	AOS Vitcam CTM	500	6.5 mm fixed	
Vic Prig	4	AOS Vitcam CTM	500	Sigma 24-70	24
	5	AOS Vitcam CTM	500	Sigma 70-200	100
	1	JVC - GZ-MC500 (E∨erio)	29.97		
eo	2	JVC - GZ-MC500 (Everio)	29.97		
/id	3	JVC - GZ-MC500 (Everio)	29.97		
a /	4	JVC - GZ-MC500 (Everio)	29.97		
Jit a	5	JVC - GZ-MC500 (Everio)	29.97		
ŏ	2	Canon-ZR10	29.97		
	8	Canon-ZR90	29.97		

DV #2



Figure 32. Typical Load Cell Locations



6 FULL-SCALE CRASH TEST NO. 4CMB-1

6.1 Test No. 4CMB-1

The 4,988-lb (2,263-kg) pickup truck impacted the cable median barrier at a speed of 61.8 mph (99.4 km/h) and at an angle of 27.9 degrees. A summary of the test results and sequential photographs are shown in Figure 34. Additional sequential photographs are shown in Figures 35 and 36. Documentary photographs are shown in Figure 37.

6.2 Weather Conditions

Test no. 4CMB-1 was conducted on October 30, 2007 at approximately 3:30 pm. The weather conditions were documented, as shown in Table 4.

Table 4.	Weather	Conditions,	Test No.	4CMB-1
----------	---------	-------------	----------	--------

Temperature	72° F
Humidity	40 %
Wind Speed	21 mph
Wind Direction	180° from True North
Sky Conditions	Overcast
Visibility	14.2 Statute Miles
Pavement Surface	Dry
Previous 3-Day Precipitation	0.00 in.
Previous 7-Day Precipitation	0.00 in.

6.3 Test Description

Initial vehicle impact was to occur between post nos. 15 and 16, or 36 in. (914 mm) downstream from post no. 15, as shown in Figure 38. Table 5 contains a sequential description of the impact events. The final position of the vehicle was determined to be 143 ft (43.6 m) downstream of impact and 4 ft - 6 in. (1.4 m) laterally behind the barrier as shown in Figure 39.

Time (sec)	Event
-0.258	Left-front tire encroached upon the ditch, and the vehicle rolled to the left and pitched downward.
0.000	Vehicle impacted the top cable of the barrier system.
0.012	Post no. 15 began to deflect laterally backward and rotated counterclockwise.
0.028	Vehicle became airborne.
0.040	Post nos. 14 and 16 deflected laterally backward.
0.062	Left-front tire overrode the bottom three cables while the top cable was wrapped around the left-front quarter panel.
0.102	Front of the vehicle contacted post no. 16.
0.116	Post no. 17 deflected laterally backward.
0.120	Vehicle began to redirect downstream.
0.148	Top cable detached from post no. 16, and the right-front side of the grill contacted post no. 15.
0.168	Post no. 18 deflected laterally backward.
0.194	Post no. 19 deflected laterally backward.
0.206	Upper-middle cable disengaged from post no. 15.
0.216	Top cable disengaged from post no. 17.
0.270	Post no. 20 deflected laterally backward.
0.276	Right-front tire overrode the bottom three cables.
0.292	Left-front tire contacted the ground.
0.352	Left-rear tire contacted the ground.
0.392	Top cable disengaged from post no. 18.
0.406	Post no. 21 deflected laterally backward.
0.414	Top cable disengaged from post no. 15.
0.466	Top cable disengaged from post no. 19.
0.478	Post no. 22 deflected laterally backward.
0.508	Post no. 23 deflected laterally backward.
0.514	Right-front tire contacted the ground.
0.620	Truck began to roll to the right.

Table 5. Sequential Description of Impact Events, Test No. 4CMB-1

0.634	Left-front tire became airborne.
0.640	Vehicle yawed to the left and continued to roll to the right.
0.726	Top cable disengaged from post no. 20.
0.876	Vehicle yawed to the right as it exited the barrier.
1.164	Vehicle was parallel to the barrier.
1.622	Top cable disengaged from post no. 21.
1.674	Left-front tire contacted the ground, and the vehicle ceased to roll.

6.4 Barrier Damage

Damage to the barrier was moderate, as shown in Figures 40 through 43. Barrier damage consisted of bent and rotated guardrail posts and detached cables. During the test, the vehicle was in contact with the cable median barrier for 143 ft (43.6 m), which spanned from the point of impact to post no. 24. After the test, the vehicle remained in contact with the system.

The top cable bracket on post no. 3 was removed. The cable and brackets were still attached for post nos. 4 through 14. The top two cable brackets on post no. 15 were removed, and the bottom two cable brackets were slightly bent. The top two cable brackets on post no. 16 were also removed, and the bottom two brackets were deformed. The top two cable brackets on post no. 17 were removed, and the bottom two brackets were slightly bent. The top two cable brackets on post no. 17 were removed, and the bottom two brackets were slightly bent. The top two cable brackets on post no. 18 were removed, the third bracket from the top was slightly bent, and the bottom bracket showed no visible damage. The top two cable brackets on post no. 19 were also removed, and the bottom two brackets were brackets on post no. 20 were removed, the lower bolt for the third bracket was also missing, and the bottom bracket was deformed. All cable brackets for post nos. 21 through 24 were removed, with the exception of the top bracket on post no. 22. The top bracket on post no. 38 was also removed.

Post nos. 8 through 11 had a maximum groundline deflection of 1/4 in. (6 mm) with post no.

8 moving in the downstream direction and post nos. 10 and 11 deflecting backward. Post no. 12 was pushed backward approximately 1% in. (48 mm) with a 2-ft (610-mm) soil crack along the front flange of the post with soil movement forming a 1-ft (305-mm) radius, half-circle crack behind the post. Post no. 13 was deflected backward 3⁵/₈ in. (92 mm) at the groundline. Post no. 14 was deflected backward 15% in. (41 mm) at the groundline and was twisted approximately 45 degrees in a counterclockwise direction. A 2-ft (610-mm) long soil crack was observed along the front flange of the post no. 14, thus creating a 1-ft (305-mm) radius soil crack behind the post. Post no. 15 deflected backward 1³/₄ in. (44 mm) at the groundline and was twisted approximately 45 degrees in the counterclockwise direction. Post no. 16 deflected backward 3 in. (76 mm) and downstream 5 in. (127 mm) at the groundline. Post no. 16 also encountered minor twisting in the counterclockwise direction. Post no. 17 deflected backward 5¹/₂ in. (140 mm) and downstream 2 in. (51 mm) at the groundline. Post no. 18 deflected backward 5 in. (127 mm) and downstream 3 in. (76 mm) at the groundline. Post no. 19 deflected backward 6¹/₂ in. (165 mm) and downstream 5 in. (127 mm) at the groundline. Post no. 20 was deflected both backward and downstream approximately 9¹/₂ in. (241 mm) at the groundline with the top of the post positioned 18 in. (457 mm) above the ground. At the groundline, post no. 21 was pushed backward 8¹/₄ in. (210 mm) and downstream 7 in. (178 mm) with the top of the post positioned 9 in. (229 mm) above the ground. Post no. 22 deflected backward $3\frac{1}{2}$ in. (89 mm) and downstream 5³/₄ in. (146 mm) at the groundline with the top of the post positioned 15 in. (381 mm) above the ground. Post no. 23 was deflected backward 11/8 in. (29 mm) and downstream 4¹/₂ in. (114 mm) at the groundline with the top of the post positioned 14¹/₂ in. (368 mm) above the ground. Post no. 24 deflected backward 2¹/₂ in. (64 mm) and downstream 2 in. (51 mm) at the groundline with the top of the post positioned 12 in. (305 mm) above the ground and held down by the truck. Post no. 25 was pushed backward ½ in. (3 mm) at the groundline, while post nos. 27 and 30 were shifted forward ½ in. (3 mm) at the groundline.

The permanent set of the posts is shown in Figures 41 through 43. The maximum lateral permanent set post deflections were not calculated for this test. The maximum lateral dynamic post deflection was 54 in. (1,372 mm) at post no. 18, as determined from the high-speed video analysis. The maximum lateral cable deflection occurred in the top cable and was determined to be 170.1 in. (4,321 mm) and the working width was determined to be 173.1 in. (4,397 mm), as determined from high speed video analysis.

6.5 Vehicle Damage

Exterior vehicle damage was moderate, as shown in Figures 44 through 46 and Figure 48. Occupant compartment deformation was minimal and did not exceed ½ in. (13 mm) of deformation, as shown in Figure 47.

Driver's side damage consisted of a creased fender near the front door joint; a broken headlight lens cover; major cable scrapes and marks 12 in. (305 mm) up from the bottom of the door; gouging through the sheet metal, fender, and door approximately 6 in. (152 mm) long; and more cable scrapes and marks along the entire length of vehicle at approximately the c.g. height. The passenger side damage consisted of the rear tail light cover removed; scrapes along the entire length of the box, continuing up to the front door handle; smaller scrapes and deformations along the bottom of the door; 3-in. (76-mm) long tears near both door handles; right-front tire disengagement from the rim; and a 1-in. (25 mm) long tear along the tire sidewall. The plastic bumper cover was also removed. The undercarriage showed slight scraping on the right lower control arm, the lower tie rod
was pulled out on the right-front side, the right-rear shock was deformed near the lower connection, and there was slight scraping on the drive shaft and other components.

6.6 Occupant Risk Values

The calculated occupant impact velocities (OIVs) and 0.010-sec average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 6. It is noted that the OIV's and ORA's were within the suggested limits provided in MASH. The calculated THIV and PHD values are also shown in Table 6. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 34. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix B. The EDR-4 data was collected during the test, but researchers believed that the data inaccurately represented the impact event as the data was inconsistent with the other accelerometers. The DTS was mounted in a different location than the EDR-3, resulting in a large difference between the longitudinal OIV values.

Evaluation Criteria		Transducers		
		EDR-3	DTS	
OUV [ft/a (m/a)]	Longitudinal	-5.93 (-1.81)	-8.53 (-2.60)	
OIV [IUS (III/S)]	Lateral	9.06 (2.76)	8.97 (2.73)	
ORA [g's]	Longitudinal	-5.19	-5.33	
	Lateral	3.97	4.61	
THIV [ft/s	s (m/s)]	N/A	10.00 (3.05)	
PHD [g's]	N/A	5.54	

Table 6. Summary of OIV, ORA, THIV, and PHD Values, Test No. 4CMB-1

6.7 Load Cell Results

As previously discussed, tension load cells were installed inline with the cables at the

upstream end of the barrier system in order to monitor the total load transferred to the anchor with respect to time. The load cell results are summarized in Table 7. As noted previously, the target cable tension was 4,213 lb (18.7 kN) at 100 deg Fahrenheit (23.6 deg Celsius). Prior to the testing, the actual cable tension in the top, upper middle, lower middle, and bottom cables was 4,160 lb (18.5 kN), 4,140 lb (18.4 kN), 4,020 lb (17.9 kN), and 4,060 lb (18.1 kN), respectively. These readings were measured using a cable tension meter or cable tensiometer.

The individual cable loads, along with the total combined cable load imparted to the upstream end anchor, were determined and are shown graphically in Figure 49. The total axial force imparted to the upstream anchor was 46.82 kips (208.3 kN) and occurring at 0.459 sec after vehicle impact with the barrier system. The maximum cable force was 23.28 kips (103.6 kN) and occurred in the top cable. The upper middle and lower middle cables experienced a maximum load of 13.67 kips (60.81 kN) and 10.86 kips (48.31 kN), respectively. The bottom cable experienced a maximum load of 7.08 kips (31.49 kN).

Table 7. Load Cell Results, Tes	st No.	. 4CME	3-1
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Cable Legation	Sensor Logation	Maximum Cable Load		Time ¹	
Cable Location	Sensor Location	kips	kN	(sec)	
Combined Cables	Upstream End	46.82	208.3	0.459	
Top Cable	Upstream End	23.28	103.6	0.392	
Upper Middle Cable	Upstream End	13.67	60.80	0.569	
Lower Middle Cable	Upstream End	10.86	48.30	1.484	
Bottom Cable	Upstream End	7.08	31.49	0.204	

¹ - Time determined from initial vehicle impact with the barrier system.

After the crash test, the tension in each cable was again measured using the cable tensiometer. With the pickup truck engaged with the cable barrier system, the cable tension at the

upstream end and in the top, upper middle, lower middle, and bottom cables was 1,580 lb (7.0 kN), 3,120 lb (13.9 kN), 3,320 lb (14.8 kN), and 3,480 lb (15.5 kN), respectively. On the contrary and with the pickup truck engaged with the barrier, the cable tension at the downstream end and in the top, upper middle, lower middle, and bottom cables was 1,540 lb (6.9 kN), 3,120 lb (13.9 kN), 3,360 lb (14.9 kN), and 3,640 lb (16.2 kN), respectively.

With the pickup truck disengaged from the cable barrier system, the cable tension at the upstream end and in the top, upper middle, lower middle, and bottom cables was 880 lb (3.9 kN), 2,460 lb (10.9 kN), 3,140 lb (14.0 kN), and 3,580 lb (15.9 kN), respectively. On the contrary and with the pickup truck disengaged from the barrier, the cable tension at the downstream end and in the top, upper middle, lower middle, and bottom cables was 800 lb (3.6 kN), 2,240 lb (10.0 kN), 3,140 lb (14.0 kN), and 3,640 lb (16.2 kN), respectively.

6.8 Discussion

The analysis of the test results for test no. 4CMB-1 (test designation no. 3-11) showed that the high-tension, four-cable median barrier system adequately contained and redirected the 2270P vehicle when the barrier was placed in a V-ditch with 4H:1V side slopes and 12 ft laterally away from the slope break point. The vehicle remained upright after collision with the barrier system. There were no detached elements nor fragments which showed potential for penetrating the occupant compartment nor presented undue hazard to other traffic. Deformations of, or intrusions into, the occupant compartment that could have caused serious injury did not occur. After collision, the vehicle's trajectory did not intrude into adjacent traffic lanes as it remained within the ditch section. Therefore, test no. 4CMB-1 was determined to be acceptable according to the TL-3 safety performance criteria found in MASH.

	0.000 sec	0.120 sec 0.2	276 sec	0.376 sec	0.618 sec	
	 	27,9° 15 17 18 19 2	-4'-6" [L4 n]			10 (254)
		143' [43	0.6 m]			
	• Test Agency	MwRSF	 Post-Impact Trajectory 			
•	• Test Number	4CMB-1	Vehicle Stabi	ility Satisfactory		10 [254]
	• Date	10/30/07	Stopping Dis	stance $143 \text{ ft} (43.6 \text{ m}) \text{ dow}$	Instream	
•	 NCHRP 350 Update Test Designation 	3-11		4½ ft (1.4 m) latera	lly behind 48	
•	• Appurtenance	Four-Cable High-Tension Median Barrier Installed o	Occupant Impact Veloc	city (EDR-3)	T I	Side
		4:1 Slope	Longitudinal	-5.93 ft/s (1.81 m/s)	0 < 40 ft/s (12.2 m/s)	10 [254]
•	• Median Width	46 ft (14.0 m)	Lateral		< 40 ft/s (12.2 m/s)	
•	• Total Length	$\dots 608 \text{ ft} (1.85 \text{ m})$	 Occupant Ridedown Ac 	cceleration (EDR-3)		
	Placement	\dots 12 ft (3.7 m) down from slope break point	Longitudinal		s	15 (2013
•	• Key Elements - Wire Rope		Lateral	3.97 g's < 20.49 g's	,	15 [381]
		$\dots \frac{34}{4}$ in. (19 mm)	 Occupant Impact Veloc 	eity (EDR-4)		Ground Line
	Size	3x7	Longitudinal		0 < 40 ft/s (12.2 m/s)	
-	Top Cable Height	\dots 45 in. (1,143 mm)	Lateral	6.43 ft/s (1.96 m/s)	< 40 ft/s (12.2 m/s)	
62	Bottom Cable Height	15 in. (381 mm)	 Occupant Ridedown Ac 	cceleration (EDR-4)		
	Incremental Spacing	$\dots 10 \text{ in.} (254 \text{ mm})$	Longitudinal	-3.58 g's < 20.49 g'	s	
	• Key Elements - End Anchor Posts			2.74 g/s < 20.49 g/s	5	
	Post Nos. 1 and 40	\dots 24 in. (610 mm) diameter by 120 in. (3,048 mm) ion	g • Occupant Impact Veloc	city (D1S)		
	Devi Nev. 2 - 120	concrete footer $(22.5.7, (0.76, 0.5))$ (201/ in (714 mm) have it	Longitudinal		40 ft/s (12.2 m/s) 39	
	Post Nos. 2 and 39	$33 \times 57 \times $		8.97 m/s < (2.73 m/s)	s) 40 ft/s (12.2 m/s)	
		S3x5.7 (S76x8.5) by 19 in. (483 mm) 12 in. (305 mi	n) • Occupant Ridedown Ac	$\frac{5}{22} \frac{22}{3} \frac{1}{3} \frac{20}{3} \frac{40}{3} \frac{1}{3}$		
		diameter by 46 in. (1,168 mm) long concrete footer	Longitudinal	-5.33 g s < 20.49 g	s	
•	• Key Elements - Line Posts	\$2.57 (\$76.9 5) by \$7 in (2.210 mm) land	Lateral	4.61 g/s < 20.49 g/s		
	Post Nos. 3-38	16 m (4.0 m)	• THIV (not required)	10.00 ft/s (30.48 m/	s)	
		10 ft (4.9 m)	• PHD (not required)		<u>+</u> L	
	Type of Soft	Grading B - AASHTO M 147-05 (1990)	 Test Article Damage Test Article Deflections 	······································		
	Type/Designation	2270P	Test Afficie Defiections Dormanant 9	et N/A		Ă
	Make and Model	2002 Dodgo Rom 1500 Quod Cob	Dynamia (Da	54 in (1272 mm)		wF
	Curb	5.052 lb (2.201 kg)	Dynamic (Fo	(1,572 mm)	2)	S
	Tost Inortial	4.088 lb (2.261 kg)	Working Wit	$\frac{1}{172} 1 \text{ in } (4.327 \text{ mm})$	1) 2)	T
	Gross Statio	4.088 lb (2.263 kg)	Vehicle Demage	Minimal	1)	Cel
	Impact Conditions	4,988 10 (2,205 kg)	VDS ^[8]	11-L FO-2		IOC
	Speed	61.8 mph (99.4 km/h)	$CDC^{[9]}$	11-LYEN5		т Z н
	Angle	27.9 degrees	Maximum D	eformation ¹ / ₂ in (13 mm)		Jo.
	Impact Location		 Angular Displacements 			T] Cei
	Exit Conditions		Roll	-29 8 deg		nb RP
	Speed	N/A	Pitch	-7 6 deg		-0 er
	Angle	N/A	Yaw			28 3-2
	Exit Box Criterion	N/A	1417	20.0 405		21.2
						2011 3-11

Figure 34. Summary of Test Results and Sequential Photographs, Test No. 4CMB-1



0.642 sec



1.022 sec



1.288 sec



1.560 sec



1.676 sec Figure 35. Additional Sequential Photographs, Test No. 4CMB-1



0.000 sec



0.620 sec



0.214 sec



1.164 sec



0.466 sec



1.674 sec

Figure 36. Additional Sequential Photographs, Test No. 4CMB-1

















Figure 37. Documentary Photographs, Test No. 4CMB-1







Figure 38. Impact Location, Test No. 4CMB-1



Figure 39. Vehicle Final Position and Trajectory Marks, Test No. 4CMB-1



Figure 40. System Damage, Test No. 4CMB-1



Figure 41. Posts Nos. 14-16 Damage, Test No. 4CMB-1



Figure 42. Posts Nos. 17-19 Damage, Test No. 4CMB-1



Figure 43. Post Nos. 20-22 Damage, Test No. 4CMB-1



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Figure 44. Vehicle Damage, Test No. 4CMB-1







Figure 45. Vehicle Damage, Test No. 4CMB-1



Figure 46. Vehicle Damage, Test No. 4CMB-1



Figure 47. Occupant Compartment Damage, Test No. 4CMB-1



Figure 48. Vehicle Undercarriage Damage, Test No. 4CMB-1



Cable Tension Versus Time

Figure 49. Cable Tension Versus Time, Test No. 4CMB-1

7 DESIGN DETAILS AND MODIFICATIONS FOR TEST NO. 4CMB-2

For test no. 4CMB-2, the actual barrier system was nearly identical to the system that was constructed and evaluated in test no. 4CMB-1. However, some minor changes were incorporated into the overall configuration. First, the barrier system was repositioned to a new location on the back slope where the centerline of the steel posts were installed 4 ft (1.2 m) laterally from the bottom of the V-ditch in order to maximize the potential for the small car to underride the bottom cable.

The cable heights, post spacings, and post embedment depths were identical to those used in test no. 4CMB-1. The only difference was that the cable placement relative to the post was reversed. For test no. 4CMB-2, the bottom cable (cable no. 4) and the second cable (cable no. 2) were attached to the back side of the posts, while the third cable (cable no. 3) and the top cable (cable no. 1) were placed on the impacted side of the posts.

The end anchors were installed with a 3-ft (0.9-m) lateral offset away from the ditch bottom and in the direction of the back slope. Therefore, the end anchors were positioned with a 1-ft (0.3 m) lateral offset toward the traffic-side face of the barrier system. This 1-ft (0.3 m) lateral offset was removed by installing post nos. 1 through 9 and 32 through 40 on a taper or through the use of a flared end section. The updated system drawings for test no 4CMB-2 are shown in Figures 102 and 117.

For test no. 4CMB-2, the bottom of the 4H:1V V-ditch was appropriated graded surrounding the cable median barrier system. However, the bottom surface region of the V-ditch was not compacted as firmly as utilized for the soil material which supported the actual steel posts. Instead, the V-ditch bottom surface was constructed in a manner that was believed to allow or accentuate some tire rutting and an increased propensity for underride. However, researchers also understood that there may be a potential for increased loss of vehicle speed prior to striking the barrier system. As such, the ground surface was believed to provide somewhat of a soft-soil condition when the airborne small car would contact the ditch bottom, undergo compression of the front suspension, and have the leading corner of the front bumper gouge the sloped terrain.



Figure 50. System Layout, Test No. 4CMB-2



Figure 51. Cable Splice Location and Detail, Test No. 4CMB-2



Figure 52. Cable Terminal Detail, Test No. 4CMB-2



Figure 53. Cable Anchor Details, Test No. 4CMB-2



Figure 54. Load Cell and Strongback Configuration, Test No. 4CMB-2



Figure 55. Load Cell Assembly Component Details, Test No. 4CMB-2



Figure 56. Cable Anchor Details, Post Nos. 1 & 40, Test No. 4CMB-2



Figure 57. Cable Anchor Bracket, Part A, Test No. 4CMB-2



Figure 58. Cable Anchor Bracket Details, Test No. 4CMB-2



Figure 59. Cable Release Lever, Test No. 4CMB-2



Figure 60. Post Nos. 2 & 39 Details, Test No. 4CMB-2



Figure 61. Cable Hanger Assembly, Post Nos. 2 & 39, Test No. 4CMB-2



Figure 62. Cable Hanger Assembly, Post Nos. 2 & 39, Test No. 4CMB-2



Figure 63. Foundation Tube Assembly, Post Nos. 2 & 39, Test No. 4CMB-2



Figure 64. Cable Support Post, Post Nos. 3-38, Test No. 4CMB-2


Figure 65. Shoulder Bolt, Nut, Washer, and Keyway Bracket Detail, Test No. 4CMB-2

8 FULL-SCALE CRASH TEST NO. 4CMB-2

8.1 Test No. 4CMB-2

The 2,557-lb (1,160-kg) small car impacted the cable median barrier at a speed of 62.7 mph (100.9 km/h) and at an angle of 26.8 degrees. A summary of the test results and sequential photographs are shown in Figure 66. Additional sequential photographs are shown in Figure 67. Documentary photographs are shown in Figures 68 and 69.

8.2 Weather Conditions

Test no. 4CMB-2 was conducted on November 16, 2007 at approximately 3:00 pm. The weather conditions were documented as shown in Table 8.

Table 8.	Weather	Conditions,	Test No.	4CMB-2
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Temperature	60° F	
Humidity	24 %	
Wind Speed	0 mph	
Wind Direction	N/A	
Sky Conditions	Clear	
Visibility	10.0 Statute Miles	
Pavement Surface	Dry	
Previous 3-Day Precipitation	0.00 in.	
Previous 7-Day Precipitation	0.01 in.	

8.3 Test Description

Initial vehicle impact was to occur between post nos. 17 and 18, or 64 in. (1,626 mm) downstream of post no. 17, as shown in Figure 70. Table 9 contains a sequential description of the impact events. The final vehicle position was determined to be 24 ft (7.3 m) downstream and 0.0 ft (0.0 m) laterally from vehicle impact, as shown in Figure 71.

Time (sec)	EVENT
-0.702	Left-front tire encroached upon the ditch, and the vehicle rolled to the left and pitched downward.
-0.022	Left side of the front bumper contacted the ground
0.000	Left side of engine hood contacted the bottom cable.
0.004	Post no. 17 deflected laterally backward.
0.056	Post no. 18 deflected laterally backward.
0.076	Upper- and lower-middle cables were in contact with the windshield, pushing it inward.
0.160	Right-front bumper cover contacted post no. 18.
0.188	Top cable contacted the left A-pillar, and all four cables were in contact with the vehicle.
0.232	Post no. 19 deflected laterally backward, and the top, upper-middle, and lower- middle cables were located at the front left corner of the windshield.
0.282	Vehicle overrode post no. 18.
0.348	Top, upper-middle, and lower-middle cables continued to wrap around the front of the windshield, causing it to crack.
0.372	Post no. 20 deflected laterally backward.
0.470	Post no. 18 lost contact with the bottom of the vehicle.
0.808	Vehicle ceased to travel downstream.

Table 9. Sequential Description of Impact Events, Test No. 4CMB-2

8.4 Barrier Damage

Damage to the barrier was minimal, as shown in Figures 72 through 74. Barrier damage consisted of bent and twisted guardrail posts and detached cables. The length of vehicle contact along the cable median barrier system was about 24 ft (7.3 m) which spanned from post nos. 17 to 19.

Post nos. 11 through 15 raised 4 in. (102 mm), 1 in. (25 mm), ¼ in. (6.4 mm), 7 in. (178 mm), and 6 in. (152 mm) from their original positions, respectively. Post no. 16 raised 5¾ in. (146 mm) from its original position, and rotated 2 in. (51 mm) at the ground line, laterally away from traffic side of the system. Post no. 17 raised 11 in. (279 mm) from its original position, and rotated 1½ in. (38 mm) at the ground line, laterally away from the traffic side of the system. Post no. 18 was completely removed from the soil and its final placement was 12 in. (305 mm) upstream of post no. 19. Post no. 19 rotated 1‰ in. (35 mm) at the ground line, laterally away from its original position, and rotated 3¼ in. (19 mm) at the ground line, laterally away from the traffic side of the system. (12.7 mm) at the ground line, laterally away from the traffic side of the system. 21 rotated ½ in. (12.7 mm) at the ground line, laterally away from the traffic side of the system.

Several cable brackets were detached from posts. The cable brackets are numbered from top to bottom with no. 1 corresponding to the top cable and no. 4 corresponding to the bottom cable. For post no. 18, the 2^{nd} , 3^{rd} , and 4^{th} cable brackets were detached, but the 1^{st} bracket was still attached. The 2^{nd} bracket was found 8 ft - 10 in. (2.7 m) downstream of post no. 19 and 6 in. (152 mm) in front of the system. The 3^{rd} bracket was found 7 ft - 6 in. (2.3 m) upstream of post no. 19 and $3\frac{1}{2}$ ft (1.1 m) behind the system. The 4^{th} bracket was found 2 ft (0.6 m) downstream of post no. 20 and 9 ft - 4 in. (2.8 m) behind the system. For post no. 19, the 2^{nd} and 4^{th} cable brackets were detached. The 2^{nd} bracket was found 19 in. (483 mm) downstream of post no. 19 and 15 in. (381 mm) behind the system. The 4^{th} bracket was found 20 in. (508 mm) downstream of post no. 19 and 2 in. (51 mm) in front of the system. For post no. 20, the 4^{th} cable bracket was missing, and it was found 4 in. (102 mm) downstream of post no. 20 and 9 mm) downstream of post no. 19 and 2 in. (51 mm) in

The permanent set of the posts is shown in Figures 73 and 74. The maximum lateral

permanent set post deflection was not calculated for this test. The maximum lateral dynamic post deflection was 21.2 in. (537 mm) at post no. 18, as determined from the high-speed film analysis. The maximum cable deflection occurred in the bottom cable and was determined to be 33.5 in. (851 mm), and the working width was determined to be 47.6 in. (1,208 mm), as determined from the high-speed film analysis.

8.5 Vehicle Damage

Exterior vehicle damage was moderate, as shown in Figures 75 through 77. Minimal damage was observed in the interior occupant compartment of the vehicle, as shown in Figure 78.

The left-front fender was severely damaged and heavily creased toward the front of the car. It was also torn near the door joint. There was heavy cable contact and loading damage at the bottom of the A-pillar, and scrapes from the cables continued along the entire length of the pillar to the roof line. The left-front steel rim had an 8 in. (203 mm) long by 3 in. (76 mm) deep indent, and the tire was completely disengaged from the rim. The right-rear bumper cover attachment was pulled off, and there was heavy damage to the right-front fender. The right-front window was also broken out. The right-front steel rim was slightly dented, and the tire was deflated and disengaged. There were minor contact marks on the right-front quarter panel, and the entire wheel assembly was pushed in toward the engine.

The face of the front bumper was heavily damaged from cable contact. The radiator support brackets were buckled, crushing other components in the engine compartment. The hood was also deformed on the front end and buckled toward the center. There was major damage along the bottom-left side of the windshield and along the A-pillar where it had disengaged from the frame. Interior damage included a buckled and broken dash. On the left-front suspension, the vertical stabilizer was disengaged at the lower ball joint, the lower A-frame component was bent, and the front wheel drive line component was bent backward. There was also left-front occupant compartment damage on the firewall and floor pan. The maximum deformation of the occupant compartment was 3 in. (76 mm), located at the right-front corner of the left toe pan.

The windshield was severely crushed and deformed inward. Three points were measured using a steel straight edge and an undamaged vehicle for reference. The windshield glass was detached from its adhesive along the A-pillar about 12 in. (305 mm). The maximum intrusion into the occupant compartment was $1\frac{1}{2}$ in. (38 mm) and was located at two points. The coordinates for these points are 47 in. (1,194 mm) from the right side of the windshield and 20¹/4 in. (514 mm) from the top, and 48¹/₂ in. (1,232 mm) from the right side of the windshield and 24³/₄ in. (629 mm) from the top.

8.6 Occupant Risk Values

The calculated occupant impact velocities (OIVs) and 0.010-sec average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 10. It is noted that the OIVs were not within the suggested limits provided in MASH. The calculated THIV and PHD values are also shown in Table 10. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 66. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix C.

Evaluation Criteria		Transducers		
		EDR-3	DTS	
	Longitudinal	-41.32 (-12.6)	-40.45 (-12.33)	
OIV [ft/s (m/s)]	Lateral	-4.48 (-1.37)	-7.27 (-2.21)	
	Longitudinal	-7.94	-7.34	
ORA [g s]	Lateral	-6.25	-6.60	
THIV [ft/s (m/s)]		N/A	41.74 (12.72)	
PHD [g's]		N/A	7.96	

Table 10. Summary of OIV, ORA, THIV, and PHD Values, Test No. 4CMB-2

8.7 Load Cell Results

As previously discussed, tension load cells were installed in line with the cables at the upstream end of the barrier system in order to monitor the total load transferred to the anchor with respect to time. The load cell results are summarized in Table 11. As noted previously, the target cable tension was 4,213 lb (18.7 kN) at 100 degrees Fahrenheit (23.6 degrees Celsius). Prior to testing, the actual cable tension located in the upstream end of the barrier system and in the top, upper-middle, lower-middle, and bottom cables was 4,220 lb (18.8 kN), 4,200 lb (18.7 kN), 4,260 lb (18.9 kN), and 3,820 lb (17.0 kN), respectively. These readings were measured using a cable tension meter or cable tensiometer.

The individual cable loads, along with the total combined cable load imparted to the upstream anchor, were determined and are shown graphically in Figure 79. The total axial force imparted to the upstream anchor was 28.97 kips (128.9 kN) and occurring at 0.363 sec after vehicle impact with the barrier system. The maximum force was 8.41 kips (37.4 kN) and occurred in the bottom cable. The upper middle and lower middle cables experienced a maximum load of 7.15 kips

(31.8 kN) and 8.25 kips (36.7 kN), respectively. The top cable experienced a maximum load of 6.23 kips (27.7 kN).

Cable Legisticn	Sensor Location	Maximum Cable Load		Time ¹
Cable Location		kips	kN	(sec)
Combined Cables	Upstream End	28.97	128.9	0.363
Top Cable	Upstream End	6.23	27.7	0.315
Upper Middle Cable	Upstream End	7.15	31.8	0.163
Lower Middle Cable	Upstream End	8.25	36.7	0.333
Bottom Cable	Upstream End	8.41	37.4	0.405

Table 11. Load Cell Results, Test No. 4CMB-2

¹ - Time determined from initial vehicle impact with the barrier system.

After the crash test, the tension in each cable was again measured. With the small car engaged in the system, the cable tension at the upstream end and in the top, upper-middle, lower-middle, and bottom cables was 3,100 lb (13.8 kN), 2,980 lb (13.3 kN), 3,500 lb (15.6 kN), and 3,380 lb (15.0 kN), respectively. On the contrary and with the small car engaged with the barrier, the cable tension at the downstream end and in the top, upper-middle, lower-middle, and bottom cables was 2,940 lb (13.1 kN), 3,300 lb (14.7 kN), 3,620 lb (16.1 kN), and 3,600 lb (16.0 kN), respectively.

With the small car disengaged from the cable barrier system, the cable tension at the upstream end and in the top, upper-middle, lower-middle, and bottom cables was 3,060 lb (13.6 kN), 2,940 lb (13.1 kN), 2,920 lb (13.0 kN), and 2,720 lb (12.1 kN), respectively. On the contrary and with the small car disengaged from the barrier, the cable tension at the downstream end and in the top, upper-middle, lower-middle, and bottom cables was 3,160 lb (14.1 kN), 3,160 lb (14.1 kN), 3,120 lb (13.9 kN), and 2,720 lb (12.1 kN), respectively.

8.8 Discussion

The analysis of the test results for test no. 4CMB-2 (test designation no. 3-10) showed that the high-tension, four-cable median barrier adequately contained the 1100C vehicle with controlled lateral displacements of the barrier system when placed in a V-ditch with 4H:1V side slopes and 4 ft (1.2 m) laterally away from the ditch bottom and up the back slope. Note that a softer soil condition was intentionally used for the ditch bottom surface. The vehicle remained upright after collision with the barrier system. There were no detached elements nor fragments which showed potential for penetrating the occupant compartment nor presented undue hazard to other traffic. Deformations of, or intrusions into, the occupant compartment that could have caused serious injury did not occur. The test vehicle and front bumper marginally underrode the barrier system although the four cables did contain the vehicle on the engine hood and front windshield. Vehicle roll, pitch, and yaw angular displacements were noted and were deemed acceptable because they did not adversely influence occupant risk safety criteria, nor result in vehicle rollover. After collision, the vehicle's trajectory did not intrude into adjacent traffic lanes as it remained within the ditch section. It should be noted that the longitudinal OIV exceeded the limit provided in MASH of 40 ft/s (12.2 m/s) when considering the loss in vehicle speed during tire rutting and soil plowing just prior to vehicle contact with the barrier. Therefore, test no. 4CMB-2 was determined to be marginally acceptable according to the TL-3 safety performance criteria found in MASH when the barrier was placed near the ditch bottom configured with a soft soil condition.



Figure 66. Summary of Test Results and Sequential Photographs, Test No. 4CMB-2



-0.278 sec



-0.046 sec



0.000 sec



0.050 sec



0.162 sec



0.222 sec



0.344 sec



1.064 sec

Figure 67. Additional Sequential Photographs, Test No. 4CMB-2



Figure 68. Documentary Photographs, Test No. 4CMB-2



Figure 69. Documentary Photographs, Test No. 4CMB-2



Figure 70. Impact Location, Test No. 4CMB-2



Figure 71. Vehicle Final Position and Trajectory Marks, Test No. 4CMB-2



Figure 72. System Damage, Test No. 4CMB-2



Figure 73. Post Nos. 14 and 15 Damage, Test No. 4CMB-3



Figure 74. Post No. 17 Damage, Test No. 4CMB-2



Figure 75. Vehicle Damage, Test No. 4CMB-2



Figure 76. Windshield Damage, Test No. 4CMB-2



Figure 77. Vehicle Damage, Test No. 4CMB-2



Figure 78. Occupant Compartment Damage, Test No. 4CMB-2



Cable Tension - Test No. 4CMB-2

Figure 79. Cable Tensions, Test No. 4CMB-2

9 DESIGN DETAILS AND MODIFICATIONS FOR TEST NO. 4CMB-3

For test no. 4CMB-3, the cable barrier system was nearly identical to the system that was constructed and evaluated in test no. 4CMB-2. The position of the barrier system within the ditch was the same as used for test no. 4CMB-2, or 4 ft (1.2 m) laterally away from the bottom of the V-ditch. However, some moderate changes were incorporated into the overall configuration.

Once again, the bottom of the 4H:1V V-ditch was graded surrounding the cable median barrier system. However, the bottom of the sloped V-ditch near the region of impact was configured with a compacted, strong-soil condition similar to the compacted soil material used to support the steel posts. The soil material conformed to grading B specified in NCHRP Report No. 350. The compacted soil region started 19 ft - 4 in. (5.9 m) upstream from the impact location with the barrier, continued for a length of 36 ft (11 m), and extended 7.7 ft (2.3 m) laterally on either side of the posts. The soil was compacted for a depth of 1 ft (305 mm) using 6 in. (152 mm) lifts. It was believed that the compacted soil pad would reduce the propensity for tire rutting within the ditch bottom, decrease vehicle plowing through the soft soil, and reduce the potential for abrupt changes in velocity, as observed in test no. 4CMB-2.

For this test, the heights of the four cables were modified. The lowest cable (cable no. 4) was positioned 13¹/₂ in. (343 mm) above grade, while the 3rd, 2nd, and 1st cables were positioned at 24 in. (610 mm), 34¹/₂ in. (876 mm), and 45 in. (1,143 mm), respectively. Note, the height to the top cable remained unchanged. The revised cable spacing was 10¹/₂ in. (267 mm). In addition, the embedment depth for post nos. 3 through 38 was increased from 39 in.(991 mm) to 42 in. (1,067 mm). As a result, the post length was increased to 90 in. (2,286 mm). Updated CAD details for the modified barrier system are provided in Figures 80 through 96.



Figure 80. System Layout, Test No. 4CMB-3



Figure 81. Soil Detail View, Test No. 4CMB-3



Figure 82. Cable Splice Location and Detail, Test No. 4CMB-3



Figure 83. Cable Terminal Detail, Test No. 4CMB-3



Figure 84. Cable Anchor Details, Test No. 4CMB-3



Figure 85. Load Cell and Turnbuckle Assembly, Test No. 4CMB-3



Figure 86. Load Cell Assembly, Component Details, Test No. 4CMB-3



Figure 87. Cable Anchor Detail, Post Nos. 1 & 40, Test No. 4CMB-3



Figure 88. Cable Anchor Bracket, Test No. 4CMB-3



Figure 89. Cable Anchor Bracket Details, Test No. 4CMB-3



Figure 90. Cable Release Lever, Test No. 4CMB-3



Figure 91. Second Post Detail, Post Nos. 2 & 39, Test No. 4CMB-3


Figure 92. Cable Hanger Assembly, Post Nos. 2 & 39, Test No. 4CMB-3



Figure 93. Cable Hanger Assembly Details, Post Nos. 2 & 39, Test No. 4CMB-3

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Figure 94. Foundation Tube Assembly, Post Nos. 2 & 39, Test No. 4CMB-3



Figure 95. Cable Support Post, Post Nos. 3-38, Test No. 4CMB-3



Figure 96. Shoulder Bolt, Nut, Washer, and Keyway Bracket Detail, Test No. 4CMB-3

10 FULL-SCALE CRASH TEST NO. 4CMB-3

10.1 Static Soil Test

Before full-scale test No. 4CMB-3 was conducted, the strength of the foundation soil was evaluated with a static test, as recommended by MASH. The static test results, as shown in Appendix D, demonstrated that adequate post-soil resistance was provided above the baseline soil strength limit. Thus, the barrier system was approved for use in the evaluation full-scale crash testing program.

10.2 Test No. 4CMB-3

The 2,568-lb (1,173-kg) small car impacted the cable median barrier at a speed of 62.0 mph (99.8 km/h) and at an angle of 27.2 degrees. A summary of the test results and sequential photographs are shown in Figure 97. Additional sequential photographs are shown in Figures 98 through 100. Documentary photographs are shown in Figures 101 and 102.

10.3 Weather Conditions

Test no. 4CMB-3 was conducted on August 25, 2008 at approximately 4:15 pm. The weather conditions were documented, as shown in Table 12.

10.4 Test Description

Initial vehicle impact was to occur between post nos. 16 and 17, or 5 ft - 4 in. (1,626 mm) downstream from the centerline of post no. 16, as shown in Figure 103. Table 13 contains a sequential description of the impact events. The final vehicle position was determined to be 28 ft - 5 in. (8.7 m) downstream of impact and 36 in. (914 mm) laterally behind the barrier, as shown in Figure 104.

Table 12	Weather	Conditions,	Test No. 4CMB-3	5
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Temperature	82° F
Humidity	38 %
Wind Speed	7 mph
Wind Direction	120° from True North
Sky Conditions	Clear
Visibility	10.0 Statute Miles
Pavement Surface	Dry
Previous 3-Day Precipitation	0.46 in.
Previous 7-Day Precipitation	0.46 in.

Table 13. Sequential Description of Impact Events, Test No. 4CMB-3

Time (sec)	Event
-0.714	Left-front tire encroached upon the ditch, and the vehicle rolled to the left and pitched downward.
-0.110	Left-front tire contacted the back slope of the ditch, and the vehicle pitched upward and rolled to the right.
-0.054	Left side of the front bumper contacted the ground.
0.000	Left side of the front bumper impacted the bottom cable.
0.030	Left-front corner of the engine hood contacted the lower-middle cable.
0.046	Lower-middle cable crumpled the top-left corner of the engine hood, and post no. 17 deflected laterally backward.
0.064	Post no. 16 deflected laterally backward.
0.072	Center of the engine hood contacted the upper-middle cable.
0.096	Bottom cable disengaged from post no. 17.
0.110	Left side of the front bumper disengaged from the vehicle.
0.116	Right-front bumper cover impacted post no. 17.

0.122	Bottom cable disengaged from post no. 18 as the post deflected laterally backward.
0.132	Upper- and lower-middle cables contacted the left-side mirror, left A-pillar deflected inward, and upper-middle cable disengaged from post no. 17.
0.148	Top cable disengaged from post no. 17.
0.166	Upper- and lower-middle cables disengaged from post no. 18.
0.192	Lower-middle cable disengaged from post no. 19.
0.224	Vehicle ceased to pitch.
0.250	Upper- and lower-middle cables traveled up the left A-pillar, and the windshield fractured along the left edge.
0.264	Top cable overrode the vehicle.
0.332	Vehicle yawed away from the barrier.
0.278	Upper-middle cable disengaged from post no. 19.
0.316	Lower-middle cable disengaged from post no. 20.
0.366	Top of the left A-pillar deformed inward, allowing upper- and lower-middle cables to penetrate the occupant compartment.
0.386	Left-front window shattered.
0.478	Right side of front bumper contacted post no. 18.
0.496	Top cable disengaged from post no. 18.
0.638	the front of the vehicle impacted the ground, the rear end of the vehicle pitches upward, causing the vehicle to decelerate rapidly
0.702	Vehicle yawed away from the barrier.
1.154	Vehicle came to a rest in contact with the system.

10.5 Barrier Damage

Damage to the barrier was minimal, as shown in Figures 105 through 108. Barrier damage consisted of deformed guardrail posts, disengaged cables, and deformed retaining bolts. The length of the vehicle contact along the cable median barrier was about 28 ft - 5 in. (8.66 m), which spanned from the midspan between post nos.16 and 17 through post no. 19.

Post no. 16 rotated in the soil, and there was a ³/₄-in. (19-mm) soil gap at the base of the post. Post no. 17 was bent downstream about the weak axis of bending. A portion of post no. 18 was covered by the vehicle, and the post was primarily bent downstream. Post nos. 19 and 20 rotated slightly toward the non-impact side, and there was a ¹/₈-in. (3-mm) soil gap at the base of the post. All other posts remained undamaged.

The lower-middle cable bracket at post no. 16 released vertically upward. At post no. 17, all four cable brackets released vertically upward. At the lower-middle cable bracket, the upper bolt fractured, while all other bolts were bent. At post no. 18, all four cable brackets released, and the lower-middle cable was held down by the upper bolt of the upper cable bracket. All bolts were bent, with the exception of the upper bolt on the lower-middle cable bracket, which was fractured. At the vehicle's final resting place, the lower-middle cable was pulled above the upper-middle cable. At post no. 19, the lower and lower-middle cables released from the brackets. The upper bolt on the lower-middle cable bracket was fractured, and the lower and lower-middle cable brackets were bent. At post no. 20, the lower-middle cable released, and the lower-middle cable bracket was bent. The lower cable bracket was slightly deformed as the cable pulled away from the post.

The permanent set of the posts is shown in Figures 103 through 105. The maximum lateral and longitudinal dynamic post deflections were 13¹/₂ in. (343 mm) and 23.3 in. (591 mm), respectively, at post no. 17. The maximum lateral cable deflection occurred in the bottom cable and was determined to be 50.3 in. (1,278 mm), and the working width was determined to be 64¹/₂ in. (1,638 mm), as determined from the high-speed film analysis.

10.6 Vehicle Damage

Exterior vehicle damage was severe, as shown in Figures 105 through 108. Damage was

concentrated on the left-front corner of the vehicle. The lower cable was lodged between the tire and the left-front quarter panel. The left-front tire was deflated, the rim was severely bent, and it was turned almost 90 degrees toward the driver's side. The left-side mirror was broken off. The windshield was severely damaged on the left side, and the A-pillar was bent into the occupant compartment. It should be noted that these deformation to the windshield and A-pillar exceeded the occupant compartment deformation limitations described n MASH. The left door was pulled outward at the bottom and inward toward the top. There were orange cable marks and deformation on the roof, caused by the top cable which was painted orange. The left-front window was shattered and there was some vehicle damage from the cables on the B-pillar.

The right-front door was bent slightly out of its frame, and the right-front quarter panel was deformed upward and outward. Post no. 18 was lodged in the right-front bumper and quarter panel. The right-front region of the engine hood was damaged in several places. Blue marks were present, thus indicating that the lower-middle cable caused the damage. The front bumper and both headlights were separated from the vehicle. The lower cable was pulled into the left headlight assembly area. Plastic panels from the undercarriage were broken off of the vehicle.

10.7 Occupant Risk Values

The calculated occupant impact velocities (OIVs) and 0.010-sec average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 14. It is noted that the OIV's and ORA's were within the suggested limits provided in MASH. The calculated THIV and PHD values were not recorded. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 97. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix E.

Evaluation Criteria		Transducers			
		EDR-3	DTS		
OIV [ft/s (m/s)]	Longitudinal	-31.17 (-9.50)	-27.30 (-8.32)		
	Lateral	-2.85 (-0.87)	-4.37 (-1.33)		
	Longitudinal	-6.51	-6.19		
OKA [g [*] s]	Lateral	-6.86	-7.53		
THIV [ft/s (m/s)]		NA	NA		
PHD [g's]		NA	NA		

Table 14. Summary of OIV and ORA Values, Test No. 4CMB-3

10.8 Load Cell Results

As previously discussed, tension load cells were installed inline with the cables and at both ends of the barrier system to monitor the total load transferred to the anchor with respect to time. The load cell results are summarized in Table 15. As noted previously, the target cable tension was 4,213 lb (18.7 kN) at 100 degrees Fahrenheit (23.6 degrees Celsius). Prior to testing, the actual cable tension in the top, upper-middle, lower-middle, and bottom cables located on the upstream end was 4,080 lb (18.1 kN), 4,200 lb (18.7 kN), 4,420 lb (19.7 kN), and 4,380 lb (19.5 kN), respectively. These readings were measured using a cable tension meter or cable tensiometer.

The individual cable loads, along with the total combined cable load imparted to the upstream anchor, were determined and are shown in Figure 112. The total axial force imparted to the upstream anchor was 35.04 kips (155.9 kN) and occurring at 0.121 seconds after vehicle impact with the barrier system. The top cable experienced a maximum load of 5.91 kips (26.3 kN). The upper-middle and lower-middle cables experience maximum loads of 7.51 kips (33.4 kN) and 10.98 kips (48.8 kN), respectively. The bottom cable experienced the highest maximum load of 13.73 kips (61.1 kN).

Cable Legation	Concer Leastion	Maximum	Time ¹		
Cable Location	Sensor Location	kips	kN	(sec)	
Combined Cables	Upstream End	35.04	155.9	0.121	
Top Cable	Upstream End	5.91	26.2	0.139	
Upper Middle Cable	Upstream End	7.51	33.4	0.198	
Lower Middle Cable	Upstream End	10.98	48.8	0.123	
Bottom Cable	Upstream End	13.73	61.1	0.121	

Table 15. Load Cell Results, Test No. 4CMB-3

¹ - Time determined from initial vehicle impact with the barrier system.

After the crash test, the tension in each cable was again measured. With the small car engaged with the system, the cable tension at the upstream end and in the top, upper-middle, lower-middle, and bottom cables was 4,060 lb (18.1 kN), 4,380 lb (19.5 kN), 5,640 lb (25.1 kN), and 5,000 lb (22.2 kN), respectively. On the contrary and with the small car engaged with the barrier, the cable tension at the downstream end and in the top, upper-middle, lower-middle, lower-middle, lower-middle downstream end and in the top, upper-middle, lower-middle, and bottom cables was 4,040 lb (18.0 kN), 4,420 lb (19.7 kN), 6,260 lb (27.8 kN), and 5,040 lb (22.4 kN), respectively.

With the small car disengaged from the cable barrier system, the cable tension at the upstream end and in the top, upper-middle, lower-middle, and bottom cables was 4,720 lb (21.0 kN), 4,960 lb (22.1 kN), 4,280 lb (19.0 kN), and 3,460 lb (15.4 kN), respectively. On the contrary and with the small car disengaged from the barrier, the cable tension at the downstream end and in the top, upper-middle, lower-middle, and bottom cables was 4,620 lb (20.6 kN), 5,000 lb (22.2 kN), 2,560 lb (20.3 kN), and 3,600 lb (16.0 kN), respectively.

10.9 Discussion

The analysis of the test results for test no. 4CMB-3 (test designation no. 3-10) showed that the high-tension, four-cable median barrier adequately contained and redirected the 1100C vehicle

with controlled lateral displacements of the barrier system when placed in a V-ditch with 4H:1V side slopes and 4 ft (1.2 m) laterally up the back slope from the ditch bottom. Note that a strong/stiff soil condition was used for the ditch bottom. The test vehicle did not penetrate nor ride over the barrier system and remained upright during and after the collision. Vehicle roll, pitch, and yaw angular displacements were noted and were deemed acceptable because they did not adversely influence occupant risk safety criteria nor result in vehicle rollover. After collision, the vehicle's trajectory did not intrude into adjacent traffic lanes as it remained within the ditch bottom. However, there were barrier elements which showed potential for penetrating the occupant compartment. Deformations of, or intrusion into, the occupant compartment that could have caused serious injury did occur. The driver's side front window was shattered, and the left A-pillar deformed with cables penetrating the occupant compartment. Therefore, test no. 4CMB-3 was determined to be unacceptable according to the TL-3 safety performance criteria found in MASH when the barrier was placed in a strong/stiff soil condition near the ditch bottom.



Figure 97. Summary of Test Results and Sequential Photographs, Test No. 4CMB-3

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December 28, 2011 MwRSF Report No. TRP-03-213-11



-0.056 sec



0.084 sec



0.156 sec



0.360 sec



0.532 sec



0.756 sec



0.220 sec



1.158 sec

Figure 98. Additional Sequential Photographs, Test No. 4CMB-3



-0.428 sec



0.010 sec



0.084 sec



0.136 sec



0.318 sec



0.400 sec



0.690 sec



0.984 sec

Figure 99. Additional Sequential Photographs, Test No. 4CMB-3



-0.056 sec



0.054 sec



0.100 sec



0.202 sec



0.304 sec



0.420 sec



0.592 sec



1.208 sec

Figure 100. Additional Sequential Photographs, Test No. 4CMB-3



Figure 101. Documentary Photographs, Test No. 4CMB-3



Figure 102. Documentary Photographs, Test No. 4CMB-3



Figure 103. Vehicle Impact Location, Test No. 4CMB-3





Figure 104. Vehicle Final Position and Trajectory Marks, Test No. 4CMB-3



Figure 105. System Damage, Test No. 4CMB-3





Figure 107. Post No. 17 Damage, Test No. 4CMB-3



Figure 108. Post No. 18 Damage, Test No. 4CMB-3



Figure 109. Vehicle Damage, Test No. 4CMB-3



Figure 110. Vehicle Damage, Test No. 4CMB-3

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Figure 111. Windshield Damage, Test No. 4CMB-3

Cable Tension - Test No. 4CMB-3



11 SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS

Under the guidance of the Midwest States Pooled Fund program, MwRSF researchers were asked to develop, test, and evaluate a new high-tension, four-cable barrier system for use in sloped medians as steep as 4H:1V and using a V-ditch configuration. In addition, the new cable barrier system was to accommodate placement anywhere within the sloped median ditch.

Based on this main directive, a new barrier system was developed while considering several other goals. First, the barrier system was designed to be capable withstanding and redirecting errant vehicles on either side of the barrier under the Test Level 3 (TL-3) impact safety standards provided in MASH. The barrier system was configured to limit dynamic barrier deflections as compared to other cable systems using improved cable-to-post attachment hardware to maximize the energy dissipated by the support posts as well as to select an appropriate cable tension. The barrier system was also configured to mitigate concerns for vehicle override and underride with the selection of four cables, an upper cable height of 45 in. (1,143 mm), a lower cable height of 13½ in. (343 mm) and a cable spacing of 10½ in. (267 mm). LS-DYNA computer simulations, in conjunction with field measurements of bumper heights and front-end geometries for small car vehicles, were utilized to determine the critical V-ditch width of 46 ft (14.02 m), a critical lateral barrier offset of 12 ft (3.66 m) for evaluating pickup truck override, and a critical lateral barrier offset of 4 ft (1.22 m) for evaluating small car underride. In addition, the barrier system was intended to maintain an open aesthetic appeal, but it was configured for easy maintenance and repair following impact events.

A full-scale vehicle crash testing program was conducted according to the TL-3 safety performance guidelines found in MASH. Three full-scale crash tests were performed on different cable barrier systems installed at various locations within the 46-ft (14.02 m) wide ditch with 4H:1V

side slopes. A summary of each safety performance evaluation is shown in Table 16.

The first full-scale crash test, test no. 4CMB-1 (override test), consisted of a 4,988-lb (2,263-kg) pickup truck impacting the cable median barrier at a speed of 61.8 mph (99.5 km/h) and at an angle of 27.9 degrees. The system was located 12 ft (3.66 m) laterally away from the front slope break point and down the fore-slope of the ditch. The impact point for this test was 3 ft (914 mm) downstream from the centerline of post no. 15. During the test, the cable barrier system adequately contained and redirected the vehicle. Therefore, the system passed all of the evaluation criteria for test designation no. 3-11 of the MASH impact safety standards.

Test nos. 4CMB-2 and 4CMB-3 (underride tests) were performed on a barrier system positioned on the back slope and 4 ft (1.22 m) laterally from the ditch bottom. The second full-scale test, test no. 4CMB-2, consisted of a 2,557-lb (1,160-kg) small car impacting the cable median barrier at a speed of 62.7 mph (100.9 km/h) and at an angle of 26.8 degrees. A soft soil condition was utilized for the ditch bottom. The impact point for this test was 61 in. (787 mm) downstream from the centerline of post no. 17. During the test, the vehicle made contact with the back slope prior to impacting the barrier system. The vehicle undercarriage and wheels penetrated, gouged, and/or rutted the soft soil, thus causing additional vehicle loss of speed prior to impacting with the cable median barrier. As a result of these occurrences, the vehicle's front end penetrated slightly under the bottom cable. However, the barrier system still contained the vehicle. It should also be noted that the longitudinal OIV exceeded the MASH limit of 40 ft/s (12.2 m/s) when considering the loss in vehicle speed during tire rutting and soil plowing prior to vehicle contact with the barrier. Therefore, test no. 4CMB-2 was determined to be only marginally acceptable according to the TL-3 safety performance criteria found in MASH under test designation no. 3-10 when the barrier was placed

near the ditch bottom configured with a soft soil condition.

For test no. 4CMB-3, the soft soil condition was replaced with a heavily compacted soil to the region surrounding the posts near the vehicle impact location. Test no. 4CMB-3 consisted of a 2,586-lb (1,173-kg) small car impacting the cable median barrier at a speed of 62.0 mph (99.8 km/h) and at an angle of 27.2 degrees. The impact point for this test was 64 in. (1,626 mm)downstream from the centerline of post no. 16. During the test, the vehicle was contained by the cable barrier system. However, some cables were observed to snag on the bolt heads corresponding with the keyway bracket, thus resulting in significant deformations to the A-pillar on the driver's side of the vehicle as well as windshield penetration. Therefore, the cable barrier system did not meet the TL-3 impact standards set forth in MASH for test designation no. 3-10 when the barrier was placed near the ditch bottom configured with a strong/stiff soil condition.

As a result of the unacceptable small car performance observed in test no. 4CMB-3, MwRSF researchers deemed it necessary to redesign the keyway bracket to mitigate concerns for cable snag on the bolt heads. In addition, members of the Midwest States Pooled Fund program expressed some concern with the number of small components and associated complexity of each keyway bracket. Thus, the keyway bracket was recommended for replacement to improve the safety performance of the high-tension cable barrier system as well as provide a simplified cable-to-post attachment mechanism.

Evaluation Factors			Evaluation Criteria				Test No. 4CMB-2	Test No. 4CMB-3
Structural Adequacy	A.	Test artic controlle installatio	cle should contain and redirect the vehicle or bring the vehicle to a ed stop; the vehicle should not penetrate, underride, or override the on although controlled lateral deflection of the test article is acceptable.				S	S
0	D.	D. Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH 08.					S	U
	F.	F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.				S	S	S
	H.	H. Occupant Impact Velocities (OIV) (see Appendix A, Section A5.3 of MASH 08 for calculation procedure) should satisfy the following limits:						
Risk		Occupant Impact Velocity Limits, ft/s (m/s)				S	U/S*	S
			Component	Preferred	Maximum			
			Lateral and Longitudinal	30 ft/s (9.1 m/s)	40 ft/s (12.2 m/s)			
	I.	I. The Occupant Ridedown Acceleration (ORA) (see Appendix A, Section A5.3 of MASH 08 for calculation procedure) should satisfy the following limits:						
		Occupant Ridedown Acceleration Limits (g's)					S	S
				Component	Preferred	Maximum		
			Longitudinal and Lateral	15.0 g's	20.49 g's			

Table 16. Summary of Safety Performance Evaluations Under MASH Guidelines

S - Satisfactory U - Unsatisfactory

* - Results were unsuccessful when contact with ditch back slope prior to barrier contact is considered. Results were successful when beginning of impact is defined as first contact with barrier.

12 REFERENCES

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- Thiele, J.C., Bielenberg, R.W., Faller, R.K., Sicking, D. L., Rohde, J.R., Reid, J.D., Polivka, K.A., and Holloway, J.C., *Design and Evaluation of High-Tension Cable Median Barrier Hardware*, Final Report Submitted to the Midwest States Regional Pooled Fund Program, Transportation Research Report No. TRP-03-200-08, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, February 25, 2008.
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- 5. Powell, G.H., *Barrier VII: A Computer Program for Evaluation of Automobile Barrier Systems*, Report No. FHWA-RD-73-51, Department of Civil Engineering, University of California, Berkeley, California, April 1973.
- 6. Zhu, L., Rhode, J.R., Terpsma, R.J., *Foundation Design for High Tension Cable Guardrails*, Final Report Submitted to the Mid-America Transportation Center, Transportation Research Report No. TRP-03-236-10, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, June, 2010.
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- 8. *Vehicle Damage Scale for Traffic Investigators*, Second Edition, Technical Bulletin No. 1, Traffic Accident Data (TAD) Project, National Safety Council, Chicago, Illinois, 1971.
- 9. *Collision Deformation Classification Recommended Practice J224 March 1980*, Handbook Volume 4, Society of Automotive Engineers (SAE), Warrendale, Pennsylvania, 1985.

13 APPENDICES

APPENDIX A

Material Specifications
09/27/2007 10:02 3156893999 BENNETT BOLT WORKS PAGE 01 R 12 Elbridge Street PHONE 315-689-3981 P.O. Box 922 FAX 315-689-3999 Jordan, New York 13080 FACSIMILE TO: DATE: 9.27.07 COMPANY: .+ FAX NUMBER: NUMBER PAGES: OF 402 472 2022 C From: Jim Sincerbeaux Material Test Reports for test assumblies

Figure A-1. Bennett Bolt End-Fitting Material Specifications

PAGE 02 · 09/27/2007 10:02 3156893999 BENNETT BOLT WORKS VETT BOLT WORKS, INC. 12 Elbridge Street P.O. Box 922 PH 315-689-3981 Jordan, New York 13080 FX 315-689-3999 MIDWEST ROADSIDE SAFETY FACILITY SEPT 21,2007 UNIV. OF NEBRASKA 1901 Y STREET BLDG C LINCOLN, NE 68588-0501 (402) 472-9064 ATTN: BOB BIELENBERG CABLE FITTINGS FOR TL3-TL4 GUARDRAIL CABLE CRASH TEST 4 EA CG 198N-H 87M TURNBUCKLE CABLE ASSEMBLY W/ 2 WEDGES 7/8-9 X 11" FLATTENED RODS A449 16 EA CG 184N-H 87M CABLE END ASSEMBLY W/ WEDGE 7/8-9 X 11" FLATTENED ROD A449 NAGER QUALIT YASSURANCE HT NO 734281 7/8-9 x 11" Flattened Rods A449 Mfg. - Southeastern Bolt & Screw, Birmingham, AL Order NO 75410-75590 Malleable Iron Casting ASTM - A47 Grade 32510 Mfg. - Buck Co., Inc., Quarryville, PA Order NO 6002236 Malleable Iron Casting Wedge ASTM - A47 Grade 32510 Mfg. - Buck Co., Inc., Quarryville, PA

Figure A-2. Bennett Bolt End-Fitting Material Specifications

· 09/27/2007 10:02 3156893999

BENNETT BOLT WORKS

PAGE 03 39622

Southeastern Bolt & Scrow, Inc 1037 16th Avenue West Birmingham, AL 35204 (205) 328-4551

MATERIAL TEST REPORT

DATE: July 7, 2004 CUSTOMER: Bennett Bolt Works, Inc. CUSTOMER P.O.: 013218 QUANITY: 57 LAB REPORT NO .: 11065 SPECIFICATION: A449 Type 1 SIZE: 7/8-9 X 48 Double End Rod SURFACE COATING: A158 Class C LOT NO.: L15532 (296489-01) MARKINGS: SBS, Three Radial Lines CHEMISTRY ¢ MN P S \$1 v .47 Сþ CR .75 .010 MO .030 .20 .013 MATERIAL GRADE: 1045 HEAT NO.: 734281 MECHANICAL PROPERTIES

PROOF LOAD

Applied Tensile Force, Ibf Length Measurement Differential, in

AXIAL TENSILE Axial Tensile Load, lbf Failure Location

60,600 Threads

39,250 -0.0005

WEDGE TENSILE 10 Degree Wedge Tensile Load, lbf Failure Location

HARDNESS MEASUREMENTS Rockwell C Scale

28

TEST METHODS: ASTM F606

We certify that the above test results do conform to the requirements of the specifications as shown. These test results relate only to the item tested. This document may be reproduced, but only in its entirety. All material was melted and manufactured in the USA.

alder dell, Quality Assurance Manager Jim Wag

Figure A-3. Bennett Threaded Rod Material Certification

09/27/2007 10:02 31568	93999	BENNETT BOLT WORK	S	PAGE 04
SEP-28-2007 10:13AM FROM-Buc	k Co. HR	717-284-4321	T-131 P.004/	004 F-840
4. 5	BUC	K COMPANY	, INC.	
	897 Lance	aster Pike, Quarryville, PA	17566-9738	
No.	Phone	(717) 284-4114 Fax (717)	284-1321	
	www.hockcompany	greatensing	s@backcompany.com	
	MATERIAI	CERTIFICAT	ION	
Date 8-30-07			Form# CERT-7A	Rev C 4-21-06
CUSTOMER	enneH B	off, Inc		
ORDER NUMBER	75590			
PATTERN NUMBER_	CGBBU	DTH I	REV	
This is to certify that with the drawing or order requirements and / or sup data is on file and availat	the custings listed confe ed requirements. All Qu plementary Quality Assu le upon request.	orm to the following spec- ality Assurance provision trance provisions have be	ifications and comp is and / or Quality / en completed and a	ly in all respects Assurance weepted, SPC
Type Material:	Maileable	Tron		
Specifications: 7	ASTM-14	7		
Grade or Class:(32510			
Heat Number:	Q4			
MECHANICAL PROPI	ERTIES	CHEMICAL ANALY	sis	
Tensile Str. PSI	Such	Total Carbon	3.0	
Yield Str. PSI 40	032	Manganese	.34	
ElongationC	22	Phosphorus	$\overline{\mathcal{O}}$	
PHYSICAL PROPERTY	ES	Chrome	845	-
Brinell Hardness	163	Copper	052	()
PCS SHIPPED	20	DATE SHIPPED	8-30-07	$1 \times /$
of	_	And Sudity	UDUCI Assurance Represen	

Figure A-4. Bennet Bolt End-Fitting Material Specifications

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JEF-20-2001	FROM-Buck Co.	HR	717-284-4321	T-131 P.003/004	F-840
	Sea and a sea	BUCK C	OMPANY INC	~	
		897 Lancaster Pi	ke, Quarryville, PA 17566-9	0738	•
	A REAL	Phone (717) 2	84-4114 Fax (717) 284-4321		
		A TETAT	greatcastings@	buckcompany.com	
		AICKIAL	CERTIFICATIO	DN	
	Date ////4/00	Form Nun	aber CERT-7C	REV. A	
	CUSTOMER:	Denne-H	Bolt Works	>	
	ORDER NUMBER_	_75410	2		
1	PATTERN NUMBER	(GRP	547-		
	This is to carrify th			REV	
c	comply in all respects y	with the drawing o	ed conform to the following r ordered requirements. All	g specifications and Ouality Assurance	
4	Assurance provisions h	lity Assurance req ave been complete	uirements and / or supplem	entary Quality	
a	vailable upon request.	Melted & Manufa	ctured in the USA.		
Т	ype Material:	Malka	beJon		
s	pecifications:	15m-	447		
G	rade or Class:	32510			
H	eat Number:(2P5			
M	ECHANICAL PROI	ERTIES	CHEMICAL ANALY	SIS	
1	ensue str. PSI	4/102	Total CarbonC	253	
Yi	ield Str. PSI	2,284	Manganese	.33	
El	ongation/	<u> </u>	Phosphorus	015	
PH	IVSICAL PROPERT	TES	Chrome, Magnesium	0310	
Br	inell Hardness 1	21	Copper .	11.5	20
PC	S SHIPPED	25_/	DATE SHIPPED_ //	14/0/1	
-	of	- (Quality Assurance		_
		Quali	ry Castings		
		150 90	DE CERTIFIED		
	Ferricic a	and Pearline Malleable Iron	, Gray and Ducrile Iron - Brass - Alumin	6um	

Figure A-5. Bennett Bolt End-Fitting Material Specifications

1955 4			
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\$. 4		
	BUCK	COMPANY, INC.	
	897 Lancaster	Pike, Quarryville, PA 17566-9738	
	Phone (717	284-4114 Fax (717) 284-4321	
	www.buckcompany.com	greatcastings@buckcompany.com	
	MATERIAL C	ERTIFICATION	
	Date 108-07		
Ċ	CUSTOMER BEARE Fort 1	bots Tr	v C 4-21-06
	ORDER NUMBER 6022310		
	PATTERN NUMBER WIWEdge	REV DIG	
	This is to certify that the castings listed conform t with the drawing or ordered requirements. All Quality requirements and / or supplementary Quality Assurance data is on file and available upon request.	o the following specifications and comply in Assurance provisions and / or Quality Assu c provisions have been completed and accept	a all respects rance ptcd. SPC
	Type Material:	Iron	
	Specifications: ASTM-A47		
	Grade or Class: 32510		
	Heat Number: 109		
	MECHANICAL PROPERTIES CL Tensile Str. PSI	HEMICAL ANALYSIS	
	Vield Str. PSI <u>(39, 27, 3</u> M	anganese .30	
	Elongation	osphorus	
	PHYSICAL PROPERTIES Ch	agnesium	
	Brinell Hardness _ 121 Co	pper1.32	\cap
	PCS SHIPPED 10,951 DA	TE SHIPPED 6-8-07	Λ
	of	Buality Assurance Representativ	e.
	Quality (ISO 9001: 2000 Ferritic and Pearlitic Malleable Iron, Gr	CERTIFIED ay and Ductile Iron, Brass, Aluminum	

Figure A-6. Bennett Bolt End-Fitting Material Specifications

09/12/2007 13:41 3156893999 BENNETT BOLT WORKS PAGE 02



Figure A-7. Bennett Bolt End-Fitting Material Specifications

INVOICE DATE INVOICE NO

03/29/07 6133G

INVOICE TIME PAGE #

CUSTOMER #



MIDWEST MACHINERY & SUPPLY CO. Construction Equipment - Supplies - Rentale - Service P.O. Box 81097 • Lincoln, Nebraska 68501-1097

> SHIP TO:

DP07-122

SOLD Midwest Roadside Safety Faciility ^{TO:} 27 Nebraska Hall P.O.Box 880529 Lincoln, NE 68588-0529

TERMS ORDER NO. QRDER DATE SALES REP ORDER NO SHIP VIA 4400004212 7-212 PRICE UNIT ITEM DESCRIPTION ITEMNO EXTENDED PE QUANTITY SHIPPED UNIT 3/4 - 10 x 19" A449 anchor bolts 24 3/4 - 10 HV hex nuts 24 600.00 3/4 Washers 24 PLEASE PAY FROM THIS INVOICE. A MONTHLY FINANCE CHARGE OF 1 1/2 % INTEREST WILL BE ADDED TO ALL ACCOUNTS 30 DAYS PAST DUE. NO STATEMENT WILL BE SENT DAMAGE WAIVER In accepting this equipment by signing below. Customer hereby agrees. (2) The related equipment bas been received in good working condition, and will be returned in the same condition, ordinary wear and tear accepted, on or before the "Due in" time and date. (2) Customer agrees to pay in tull for all damages or loss to renal equipment and further agrees not to loan, sublet or otherwise dispose of equipment or use it at any other location than listed above. (3) Customer agrees to pay in tull for all damages or loss to renal equipment and further agrees not to loan, sublet or otherwise dispose of equipment or use it at any other location than listed above. (4) Customer agrees to pay exponsibility for enter premises of customer at any time to reposses said equipment turners agrees to remover's rights to enter premises of customer at any time to reposses said equipment. Customer hereby walves any rights of action against owner by reason of such taking or entry and agrees to reimburse owner's cost of respossession if any. (5) Customer agrees to reimburse owner for all attorney lees, an annour not less than 25% of all sums due, court cost and expenses incurred by owner to enforce collection or to preserve or enforce owner's rights under this contract. This contract shall be governed by the laws of the state of Nebraska, and the money due hereunder must be paid to the office indicated above in lancaster Courty Nebraska. There are no warranties of merchantability or litness either expressed or implied which extend beyond the description on the face hiered. (6) A day rate consists of 24 hours time out or 8 hours the used, which ever conse litra on machines equipped with hour meters. DAMAGE WAIVER RENTAL CONTRACT 600.00 SALES AMOUNT DISCOUNT FREIGHT SALES TAX .00 600.00 TOTAL PAYMENT REC'D 600.00 BALANCE DUE (6) A day rate consists of 24 hours time out or 8 hours time used, which ever comes first on machines equipped with hou DAMAGE WAIVER does not apply unger the tolawing conditions: A) The irrs 320.90 or 20% of Replacement Cost, whichever is higher of each claim for loss or damage as a result of theft, vandates, or malicious mischief, Accessomes such as air hose, tool steel, electric cord, blades, welding cable, isuid fuel tanks and other similar items are excluded from theft toss. (B) Loss caused by will iterated, mischief, Accessomes such as air hose, tool steel, electric cord, blades, welding cable, licked as the similar to the size of the si Х AUTHORIZED SIGNATURE . when left unaffended.) (F) Use of equipment in violation of any terms of this agreement.

Figure A-8. J-Hook Anchor Stud Material Specifications



Figure A-9. J-Hook Anchor Stud Material Specifications

BER 1981 VAN B TEL (4	AERT COL BERAERT DRIV UREN, AR 729 79) 474-5211	RPORATIC 56 FAX (479) 4	DN Van Bus 74-9075	ren , Arka :	1988	DATE: 06,	/26/2006
Custo Our O Produ Custo MFG S	537439 mer Un rder No 40 ct 3/ mer Part No MP No PS	iversity of N 60060019 4" 3X7 CL A G TAG# 95151124 T904362105	ebraska-Lincoln 0010 ALV GUIDERAIL SH ,95151134,951694	CC QN ORTS 99,952 Cuscomet Spe	stomer Order No y c No ASTM A-74	sample 3	Carriers
Pinishad Tag#	Breaking Strength (1bs.)	Laty Langth (in.)	Adherence Appnarance of Wires	Steel Ductility	Modulus of Elasticity	Pre- Stretched (1bs.)	
95151124	47345	6	Pase	Pase	2450012120	15576	
95151134	47898	6	Pass	Pase	2450012128	15576	

Pass

17789830

15576



95159499 53134

6

Pass

Naterial was melted and made in the U.S.A. The undersigned certifies that the remult are usual results and conform to the specification indicated as contained in the records of this Corporation



Rotary Public Promitision expires

Figure A-10. Cable Material Certification

EXTRA HIGH STRENGTH

	LENGTH	DIAM.	CONST.	CLASS	REEL #	SPECIFICATION	
	1950	3/04	21	CL A	95151134	ASTM A 741	
G	3/-	4" 3X7	CL A GAL	V GUIDERAI	L SHORTS		
		PST	3043	SE10S			
	GROSS \	NT 1	951.16	Lbs.	NET WT	1671.17 Lbs.	
(Zinc	Hot	NON-	FLOODED			
	MADE IN	U.S.A.		MFG. DATE	10/11/2005		
						And a second	

Figure A-11. Cable Material Specifications



Bill To: UNIVERSITY OF NEBRASKA MIDWEST RDSIDE SAFETY FACILITY W328.1 NEBRASKA HALL PO BOX 880529 LINCOLN NE 68588-0529

INVOICE

44224348

Remit to: P.O. Box 29529 Lincoln, NE 68529

Page	3
Account Number	91123
Invoice Date	03/16/07
Invoice Amount	1,712.00
Invoice Number	CI 195253
Amount Paid	

	To insure proper credit, please detach and return top portion of invoice with remittance.									
Invoice	No.: CI 195253 Invoice Date: 03	/16/07 PO No.:			Order:	SP 9	935798 Ship	o#: 1386		
Line	Item Description		Quantity		Unit P	rice	Misc.	Extensic		
1	#11 STOCK REBAR GRADE 60	R1160	1813.00	LB	0	.0000	0.00	0.		
2	#11 REBAR FABRICATED / CUT	11FAB	1813.00	LB	0	.0000	0.00	0.		
	35 PCS #11 X 9-9 STR									
3	#4 STOCK REBAR GRADE 60	R460	1057.00	LB	0	.0000	0.00	0.		
4	#4 REBAR FABRICATED / CUT	4 FAB	1057.00	LB	0	.0000	0.00	0.		
	250 PCS #4 X 6-4 BENT									
	#2 GTOCK PERAR CRADE 60	R360	122.00	LB	0	0000	0.00	0.		
6	#3 DEBAR FABRICATED / CUT	SFAR	122.00	LB	Ő	.0000	0.00	0.		
0	+3 REDAR FABRICATED / COT	JIND	122.00		Ĭ		0.00			
	100 105 #5 1 5 5 5511									
7	GROUP/LOT PRICE - REBAR	RBGP	1	EA	1552	.0000	0.00	1,552.		
8	2-1/2" IND HI CHR PLSTC FT	70216	100	EA		.4800	0.00	48.		
Ĭ	,									
				8						
				3			× ,	2/2 -1		
				8			Les	23010		
L		NEDDACVA					ub Total	1,600.		
Acco	unt: 91123 UNIVERSITY OF	NEBRASKA			F	6	ales Tay	-112-		
	JOD: UNIVERSITI OF	Tay			- H	Tatal	Amount	.1.712		
Tax Co	de: CINOI LINCOIN Sales		1		L	Total	Amount	110		
Retu	Irns: No returns w/o invoice. No returns on unus	able material, seconds, aterials, and fractional		-				160		
	units. All returnable materials subject to 50	% restocking charge.		Ξ						
	No returns accepted after 30 days from date	of purchase.								
Ter	rms: All invoices must be paid within 30 days of i	nvoice. Past due			Conc	rete In	dustries,	Inc.		
	accounts will be charged an interest rate of	1.33% per month		6300 (Cornhusk	er Hwy,	Lincoln, NE	68529-0529		
	which is 10% per year.]	Pho	ne: (402)	434-180	0 Fax: (402)	434-1899		
	n na hann an ann an an ann an ann ann an	Invoice – Cust	omer Copy		www.C	oncrete	industries.co	m		

Figure A-12. Anchor Rebar Material Specifications

-	PURCH	ASE REC	QUISITION	Inca	
ASTRUCTIONS		PO	NUMBER	1151	-
PLEASE PRINT OR TYPE				07 1112 0	242
SEND COMPLETED REQUISITION TO PURC	CHASING	RI	EQ. TRACKING NU	MBER <u>07-1113-0</u>	1343
EITHER BY MAIL OR FAX. DO NOT SEND	TO THE	SUC	GESTED VENDO	R	
GL BUDGET ACCOUNT CODES ARE FOUNI	D IN THE			READ	VMi
CHART OF ACCOUNTS, AVAILABLE FROM	1	NA	ME, COMPANY		1 1111
PROVIDE VENDOR FAX NUMBER TO EXPE	EDITE ORDER:	STR	EET ADDRESS		
EQUIRED DELIVERY DATE (MONTH/DAY	(/YEAR)	CIT	Y, STATE, ZIP		
FOR PURCHASING ONLY:	ORD	ER TYPE	PURC	HASING GRP/BUYER	
			MATERIAL CR	NIR	0
PLANT STORAGE L	OCATION		MATERIAL GRU		Q
FOB	TERMS		INVOICE MATC	HING	
FOB	TERMS QUANTITY	UNIT PRICE	INVOICE MATC TOTAL PRICE	HING GL ACCOUNT CODE	COST CENTE WBS ELEME
FOB	QUANTITY	UNIT PRICE	INVOICE MATC TOTAL PRICE	HING GL ACCOUNT CODE	COST CENTE WBS ELEMEI
FOB	QUANTITY		TOTAL PRICE	HING GL ACCOUNT CODE	
FOB	QUANTITY		INVOICE MATC TOTAL PRICE 212-25	HING	$\frac{\text{COST CENTE}}{\text{WBS ELEME!}}$
FOB	QUANTITY		INVOICE MATC TOTAL PRICE 292.25	HING	COST CENTE WBS ELEMEN 4 Col Voditati proj
FOB			INVOICE MATC	HING	COST CENTE WBS ELEMEN
FOB			INVOICE MATC	GL ACCOUNT CODE	COST CENTE WBS ELEMEN 4-col Vediter proj
FOB			INVOICE MATC	GL ACCOUNT CODE	COST CENTE WBS ELEMEN 4- Col Veditar proj
FOB	QUANTITY QUANTITY 3yd/ 0 0 7		INVOICE MATC	HING	$\frac{COST CENTE}{WBS ELEMET}$ $\frac{4}{Vod_1 tot}$ $\frac{1}{Vod_1 tot}$
FOB	QUANTITY QUANTITY		INVOICE MATC	GL ACCOUNT CODE	PAGER #
FOB	QUANTITY QUANTITY		INVOICE MATC	GL ACCOUNT GL ACCOUNT CODE	COST CENTE WBS ELEMEN
FOB	QUANTITY QUANTITY		INVOICE MATC TOTAL PRICE 292.25 292.25 AMPUS ZIP FAX #	GL ACCOUNT CODE	COST CENTE WBS ELEMEN 4 - Colo Vedita proj
FOB	QUANTITY QUANTITY		INVOICE MATC		COST CENTE WBS ELEME 4 Color Vodital proje

Figure A-13. Concrete Anchor Material Specifications

INVOICE

6

Bennett Bolt Works, Inc.

Branch: 01 Bennett Bolt Works, Inc PO Box 922

Jordan, NY 13080 USA

.

315-689-3981 Bill To:

Customer ID: 11190

UNIVERSITY OF NEBRASKA-LINCOLN MIDWEST ROADSIDE SAFETY FACILI 527 NEBRASKA HALL LINCOLN, NE 68588-0529 USA

INVOIC	CE
500584	7
Invoice Date	Page
8/3/2006 11:14:33	1 of 1
ORDER NUT	MBER
1003718	8

Ship To:

UNIVERSITY OF NEBRASKA-LINCOLN MIDWEST ROADSIDE SAFETY FACILI 527 NEBRASKA HALL LINCOLN, NE 68588-0529 USA

	PO N	umber				Terms Description	Net Due Date	Dis	c Due	Due Date Discount A		unt Amount
	45001	54142				Net 30 Paid in Advance	08/08/06	08	/08/0	8/06 0.00		
Order L	Date	Pick Ticke	t No			Primary Salesrep Name				Taker		
5/2/2006 1	0:55:53	300621	9			Donald C	Bennett			EBENNETT		
	Qua	antities				Itam ID		Pric	ng 1			F
Ordered	Shipped	Remaining	UOM Unit	Size	Disp.	Item Description		U	it Size		Price	Extenaea Price
				Carr	ier:	UPS GROUND	Tr	acking #:		1Z116	09702410	95818
500	500) 0	EA			31CNHH0M		EA			0.08000	40.00
				1.0		5/16-18 HEAVY HEX 1	NUT MG-B695		1			
			Lot N	um.	ber:	52404 Mfg ID:		Qt TF	v: L1411	5-67104	500 EA 1	
500	500) 0	EA			31NWSA0M/MIL		EA			0.40000	200.00
				1.0		5/16 SAE FLAT WASH	IER MIL CARB R	C38-45 MC	1			
			Lot N	ium.	ber:	54399		Qt	v:		500 EA	
						Mfg ID:		WV	/ 2100)45		
Total L	ines: 2								S	UB-TO	OTAL:	240.00
											TAX:	0.00
									MO	UNT	DUE:	240.00

ORIGINAL

Figure A-14. Shoulder Nut and Washer Material Specifications

L

MATERIAL CERTIFICATION

Customer:				Date: 10/17/2005			
BENNETT BOLT WORKS INC.	Customer P.O. Number: 6000494						
P. O. Box 922	Customer Part Nu	mber: 31CNHH	0M				
12 Elbridge Street	Invoice Number:	1539491					
Jordan, NY 13080	Lot Number:	L14115-6	7104				
Description:	Ship Quantity:	141,511	Ship Date:	10/14/05			
NUT HVH 5/16-18 A563 GRA MGL .016	Material:	1026	Heat Number:	7284776			
Specifications:	•						

Telefast Industries, Inc. 777 West Bagley Road Berea, Ohio 44017-2901 216/826-0011 - FAX 216/826-3785

0

Chemical Analysis

C .	Mn 👘	₽ ™	$\mathbf{S}^{(n)} \star \mathbb{P}_{\mathbf{S}}^{(n)} \mathbf{S}^{(n)}$	Si	Ni	Cr	Mo	A
0.260	0.730	0.010	0.006	0.170	0.020		0.020	0.034

Mechanical Properties

Surface Hardness	A57.9 Average
Proof Load	4 Samples Pass
Plating	Mech. Galvanized /pass

We hereby certify that to our actual knowledge the information contained herein is correct. We also certify that all parts substantially conform to SAE, ASTM, or customer specifications as agreed upon. The product has been manufactured and tested in accordance with our Quality Assurance manual. The above data accurately represents values provided by our suppliers or values generated in the TELEFAST INDUSTRIES laboratory. Statistical process control data is on file. All manufacturing processes for these parts occured in the United States of America.

This document may only be reproduced without alteration and only for the purpose of certifying the same or lesser quantity of the product specified here Details and the purpose of certifying the same or lesser of Manager of Manager

Dean Smith Manager of Quality Assurance

Figure A-15. Shoulder Nut Material Specifications

						м	All						
	С ЦАРТЕ										1658	Cold Sprin	ngs Roa
					.1						Saukville	, Wiscons	in 530
- CHA	EEL	121	EEI		CHART	ER STI	EL TES	T REPO	DRT			(262) 2	268-24
		' A Divis	ion of		Reven	se Has	Text A	nd Code	8\$			1-800-4	37-87
		Charte	r Manufa	cturing	Company	, inc.					E	AX (262) 3	268-25
							,	<u></u>	Ču	a. P.O.		••••••••••••••••••••••••••••••••••••••	6050
	Chanc	ller Produ	ucts Div.					Cust Part#			207		
	1491	Chardon								Heat #			411
	Euclid	,OH 441	44117-					Ship Lot #			482		
	Attn:	Ron Sied	ller							Grade#	4	037 A S	K FG I
										TOCESS			
(1			.)	od haarb		(FHR.	511 2129	lastista		!>
and standard	is listed t	elow and	on the re	verse si	de, and th	at it sat	isfies the	se requir	ements.	the specie	ICations		
LAB CODE: 7	388 C	MN	Р	\$	\$1	Nì	CR	MO	CU	SN	v		
Wt%	0.39	0.86	0.014	0.008	0.210	0.04	0.07	0.27	0.08	0.008	0.001		
	AL 0.023	N 0.0060	B 0.0002	TI 0.001	NB 0.002								
OC DEVIATIO	ON EXTGI	reen = n/	R		Test Rei	sults of P	ncessing	Lot # 4824	448				
CC DEVIATIO	ON EXTGI	REEN = N/ %) ROCESSED	R ≠ of Tes 2 2 ⊨ N/R	ts	Test Rea Min Value 76.6 64	sults of P	rocessing Max Valu 76.9 65	Lot # 4824 e	448 Meen Velu 76.8 64	1ġ		TENSILE RA LAB	LAB = = 0358
QC DEVIATIO TENSILE (KSI REDUCTION (QC DEVIATIO Specifications	DN EXTGI OF AREA (DN EXTPF	REEN = N/ %) ROCESSED Meets ci Custome	R 2 2 = N/R ustomer sp er Docume	ts Decification Int =	Test Rea Min Value 76.6 64 ons with ar	tuits of P 9 1y applica	nocessing Max Valu 76.9 65 ble Charte Revision =	Lot # 4824 e r Steel exe Dat	448 Mean Valu 76.8 64 ceptions fo led =	ie or the follow	wing custo	TENSILE RA LAB mer docum	LAB = = 0355 ents:
QC DEVIATIO TENSILE (KSI REDUCTION (QC DEVIATIO Specifications	ON EXTGI	REEN = N/ %) ROCESSED Meets cl Custome	R # of Tes 2 2 = N/R ustomer st er Docume	ts pecification nt =	Test Rei Min Value 76.6 64 ons with ar	ivits of P 9 1y applica	nocessing Max Valu 76.9 65 bla Charte Revision =	Lot # 4824 e T Steel exc Dat	448 Mean Velu 76.8 64 ceptions fo led =	ie or the follow	wing custo	TENSILE RA LAB	LAB = = 0355 ents:
OC DEVIATIO	ON EXTGI OF AREA (ON EXTPF	REEN = N/ %) ROCESSED Meets ci Custome	R # of Tes 2 2 = N/R ustomer st er Docume	tz pecificatio nt =	Test Rau Min Value 76.6 64 ons with ar	sults of P 9 1y applica	rocessing Max Valu 76.9 65 ble Charte Revision =	Lot # 4824 e r Steel exi Dat	448 Mean Valı 76.9 64 64 ceptions fr	ue or the follow	wing custo	TENSILE RA LAB	LAB == = 0356 ents:
OC DEVIATIO TENSILE (KSI REDUCTION OC DEVIATIO Specifications	DN EXTGI DF AREA (DN EXTPF	%) %) &OCESSED Meets a Custome	R # of Tess 2 = N/R ustomer sp er Docume	ts pecificatie nt =	Test Rei Min Valu 76.6 64	sults of P e	racessing Max Valu 76.9 65 65 ble Charte Revision =	Lot # 482 e r Steel exi Dat	448 Mean Valu 76.8 64 Ceptions for led =	ue or the follow	wing custo	TENSILE RA LAB mer docum	LAB = = 0355
OC DEVIATIO	DN EXTGi OF AREA I OF AREA I S:	REEN = N/ %) NOCESSED Meets C Custome	R 2 2 e N/R ustomer sy er Docume	ts Decification nt =	Test Røy Min Valu 76.6 64	iy applica	rocessing Max Valu 76.9 65 65 ble Charte Revision =	cot # 482- e r Steel exe Dat	448 Mean Valu 76.8 64 ceptions fr	ie or the follow	wing custo	TENSILE RA LAB	LAB = = 0355
QC DEVIATIO	DN EXTGI	REN = N/ %} ROCESSED Meets a Customa	R 2 2 = N/R uistomer sj er Docume	ts pecification nt =	Test Rei Min Valu 76.6 64	iy applica	rocessing Max Valu 76.9 65 65 65 66 66 60 80 80 80 80 80 80 80 80 80 80 80 80 80	Lot # 4824 e r Steel exi Dat	448 Mean Valu 76.8 64 ceptions fr led =	ue or the follow	wing custo	TENSILE RA LAB	LAB = = 0356 ents:
OC DEVIATIO	DN EXTGI	REN = N/ %) NOCESSED Meets a Custome	R # of Tess 2 2 N/R ustomer sj er Docume	ts ncificati nt =	Test Rei Min Valu 76.6 64 ons with ar	sults of P e	rocessing Max Valu 76.9 85 ble Charte Revision =	r Steel ex	448 Mean Valu 76.8 64 64 ceptions fo	ue or the follow	wing custo	TENSILE RA LAB	LAB = = 0356
OC DEVIATIO) OF AREA OF AREA ON EXTPF	REEN = N/ %) NOCESSED Meets a Custome	R # of Tess 2 2 = N/R Ustomer st er Docume	ts pecification nt =	Test Ref Min Valu 76.6 64 ons with ar	sults of P	rocessing Max Valu 76.9 65 65 69 69 69 69 69 69 69 69 69 69 69 69 69	Lot # 4824 a T Steel ext Dat	448 Mean Vela 64 Ceptions fr led =	ie or the follow	wing custo	TENSILE RA LAB	LAB = 0355
OC DEVIATIO	I) EXTGI	REEN = N/ %} ROCESSED Meets a Customa	R # of Tess 2 = N/R ustomer \$} er Docume	ts Decification nt =	Test Rei Min Valu 76.6 64	sults of P	racessing Max Valu 76.9 65 65 ble Charte Revision =	r Steel ex	448 Mean Val 76.8 64 Ceptions fr led =	ue or the follow	wing custo	TENSILE RA LAB	LAB = = 0355
OC DEVIATIO	ON EXTGI	REN = N/ %) NOCESSED Meets a Custome	R # of Tess 2 2 N/R ustomer st er Docume	ts Decificatia nt =	Test Rei Min Valu 76.6 64	iy applica	raceselng Max Valu 76.9 65 65	r Steel ex	448 Mean Valu 76.6 64 ceptions fo led =	ue or the follow	wing custo	TENSILE RA LAB	LAB = 035
OC DEVIATIO	I) EXTGI	REEN = N/ %) ROCESSED Meets a Custome	R 2 2 = N/R ustomer \$} er Docume	tx pecificati nt ≠	Test Rei Min Valu 76.6 64	sults of P	rocessing Max Valu 76.9 65 ble Charte Revision =	r Steel ox	448 Meen Vela 64 Coptions fr led =	ue or the follow	wing custo	TENSILE RA LAB	LAB = 035(
OC DEVIATIO) OF AREA I OF AREA I N EXTPF	REN = N/ %) NOCESSED Meets a Custome	R # of Tess 2 2 = N/R ustomer sj er Docume	tz ≫ecificati nt ~	Test Rei Min Valu 76.6 64 ons with ar	y applica	rocessing Max Valu 76.9 65 ble Charte Revision =	Lot # 4824 a r Steel ext Dat	448 Mean Velu 64 64 ceptions fr led =	ie or the follow	wing custo	TENSILE RA LAB	LAB = = 0355

Figure A-16. Post Bracket Material Specifications

					a		Ξ.				
			J	Precisio	n Scre	w & B	olt				
			TNO	Contorn	nance K	eport	20/06	Da	R NO. 073	2006 2	
art No.: evision;	BENNETT BOLT WORKS, INC. Date Ship 31C125BSH5 P.O. No.: Onantity:							550 Í	voice	81497	
evision.	5/16 - SHOULDER I	18 X 1 1/ BOLT	4	1	Manufactu Mfg. Lot.	No.: 060	: 07/ 5129	06/06 1 CP			
	TEST REPOR	RTS STATIN	G STEE	L							
pecificat	ions Requ	ired: SAE	J429 G	R5	•	s	ampli	ng Spec: GM9(ty Tested: 3	050P		
aterial	Size: 40	37 13/3	2 He	at No	Mill: 4	11430	CHAR	TER			
e	Mn	Р	S	Si	Ni		CI	Mo			
.39	.86	.014	.008	.21				.27			
				())			d1				
Core Proof Load Tens		sile	DLINES A	Surface	Y.S.	BL	RA RA				
Hardness Rc 25-34	PSI 85,00	0 PSI 120	Min ,000	Torque	30 N	MI	n %	Min & Min			
30 31	.0000	14	2,748		50.5 51.5						
31	.0000	14	6,393		51.5						
				1	L	a,	1.	6	ſ		
OATING: Type: PF PLAIN FIN			IN	Dej	pth;	Min	2.	7.8			
					1081		5.	10	•		
Decarb:				hreads:			MAG:				
Structure:				atch/Adh:		•	Spec:	9661			
								Speci .			
Comments	Co lested	CONTOLU			AL FADLE	ver obeci	LIUdi	wadarre	~		
perform the f	& Bolt has succes allowing tests: b	ension, hardness	the A2LA, surface di	ISO/IEC 17025 scontinuity, Koc	Accreditation our-drop test,	process. Accre dimensional a	ditation l and gagin	has been granted to Pr og measurements. (Co	ecision Screw &	k Bolt 50-01) The	
ad samples rep	resentative of the	material have b	een tested.	Tests for mecha	nical propertie	s run per AST	M F606.	Chemical analysis reco	orded from mat	terial suppli	
om the date of	shipment. This r To prevent any a	eport shall not be discration or miss	reproduce ise of this c	ertificate, check	il, without the	written approv	al of Pre	cision Screw & Bolt . '	These results re	late only to	
								Precision Scre	w & Rolt		
				· · · · · · · · · · · · · · · · · · ·				Al ila I	h. '	<u>,</u>	
	BENNETT	INC.				Halles Swindlimen					
	P. O. B	OX 922						Quality Assurated	nce Superv	isor H 44117	
	JURDAN	MT 13080						(216) 481-4400 F	ax (216) 481-	4427	

1-1-1 Revised 04/21/04

2 .9 878 P. 2

Page 1 of 1

MAS4:8 8005 .8.43AM

Figure A-17. Post Bolt Material Specifications

APPENDIX B

Accelerometer and Rate Transducer Data Plots, Test No. 4CMB-1



Figure B-1. Longitudinal Deceleration, Test No. 4CMB-1



Figure B-2. Lateral Occupant Impact Velocity, Test No. 4CMB-1



Figure B-3. Longitudinal Occupant Displacement, Test No. 4CMB-1



Figure B-4. Lateral Deceleration, Test No. 4CMB-1



Figure B-5. Lateral Occupant Impact Velocity, Test No. 4CMB-1



Figure B-6. Lateral Occupant Displacement, Test No. 4CMB-1



Figure B-7. Euler Angular Displacements, Test No. 4CMB-1

APPENDIX C

Accelerometer and Rate Transducer Data Plots, Test No. 4CMB-2



Figure C-1. Longitudinal Deceleration, Test No. 4CMB-2



Figure C-2. Longitudinal Occupant Impact Velocity, Test No. 4CMB-2



Figure C-3. Longitudinal Occupant Displacement, Test No. 4CMB-2



Figure C-4. Lateral Deceleration, Test No. 4CMB-2



Figure C-5. Lateral Occupant Impact Velocity, Test No. 4CMB-2



Figure C-6. Lateral Occupant Displacement, Test No. 4CMB-2



Figure C-7. Euler Angular Displacements, Test No. 4CMB-2

APPENDIX D

Soil Static Tests



Figure D-1. Soil Strength, Initial Calibration Tests



Figure D-2. Static Soil Test, Test No. 4CMB-3
APPENDIX E

Accelerometer and Rate Transducer Data Plots, Test No. 4CMB-3



Figure E-1. Longitudinal Deceleration, Test No. 4CMB-3



Figure E-2. Longitudinal Occupant Impact Velocity, Test No. 4CMB-3

205



Figure E-3. Longitudinal Occupant Displacement, Test No. 4CMB-3

206



Figure E-4. Lateral Deceleration, Test No. 4CMB-3



Figure E-5. Lateral Occupant Impact Velocity, Test No. 4CMB-3



Figure E-6. Lateral Occupant Displacement, Test No. 4CMB-3

209



Figure E-7. Euler Angular Displacements, Test No. 4CMB-3

APPENDIX F

Occupant Compartment Deformation Data

VEHICLE PRE/POST CRUSH INFO Set-1

TEST: 4CMB-1 VEHICLE: 2002 Ram 1500 Q.C. 4x2

```
Note: If impact is on driver side need to enter negative number for Y
```

POINT	Х	Y	Z	Χ'	Y'	Z'	DEL X	DEL Y	DEL Z
1	30.75	-27.75	-0.5	30.75	-27.5	-1	0	0.25	-0.5
2	34	-23.75	-1	34	-24	-1.25	0	-0.25	-0.25
3	34.25	-16.75	-1.75	34	-16.25	-1.75	-0.25	0.5	0
4	30	-10.5	0	29.75	-11	0	-0.25	-0.5	0
5	27.5	-28.75	-4.75	27.25	-29.25	-5.25	-0.25	-0.5	-0.5
6	28.5	-22.25	-6	28.5	-22.5	-6.25	0	-0.25	-0.25
7	28.5	- 15.5	-6.25	28.5	-15.75	-6.5	0	-0.25	-0.25
8	27.5	-9.75	-5.5	27.75	-10.25	-5.75	0.25	-0.5	-0.25
9	20	-29.5	-9.5	19.75	-29.5	- 10	-0.25	0	-0.5
10	19.75	-23	-9.5	20	-23.5	-10	0.25	-0.5	-0.5
11	19.75	-17	- 10	19.75	-17.25	-10	0	-0.25	0
12	19.25	-11	-10	19.5	-11	-10.25	0.25	0	-0.25
13	15.25	-3.25	-3	15.25	-3.25	-3.25	0	0	-0.25
14	14	-29.75	-9.5	14.25	-29.75	-9.5	0.25	0	0
15	14.25	-23	-9.25	14.25	-23	-9.5	0	0	-0.25
16	14.25	- 17.5	-9.5	14.25	-17.75	-9.75	0	-0.25	-0.25
17	13.75	-11	-9.75	13.75	-11	-10	0	0	-0.25
18	11.25	-3.5	-3	11.25	-3.5	-3.25	0	0	-0.25
19	9	-29.25	-8.5	9	-29.5	-8.75	0	-0.25	-0.25
20	9	-23.25	-8.75	9	-23.5	-9	0	-0.25	-0.25
21	9	-16.5	-9.25	9	-16.5	-9.25	0	0	0
22	9	-11	-9.5	9	-11.5	-9.5	0	-0.5	0
23	7.25	-1.5	-3.5	7.25	-1.5	-3.5	0	0	0
24	1.25	-28.5	-4.5	1.25	-28.5	-4.75	0	0	-0.25
25	1	-23.25	-4.25	1	-23.25	-4.5	0	0	-0.25
26	1.25	-17	-4.75	1.25	-17	-4.75	0	0	0
27	1	-11.5	-5	1	-11.5	-5	0	0	0
28	1.75	-2	-3	1.75	-2	-3	0	0	0
29			2						
30									



Figure F-1. Vehicle Crush Info Set 1, Test No. 4CMB-1

VEHICLE PRE/POST CRUSH INFO Set-2

TEST:	4CMB-1
VEHICLE:	2002 Ram 1500 Q.C. 4x2

Note: If impact is on driver side need to enter negative number for Y

	22.5	1.5							
POINT	Х	Y	Z	Χ'	Y'	Z'	DEL X	DEL Y	DEL Z
1	53.25	-26.25	-1.5	53.25	-26	-1.25	0	0.25	0.25
2	56.5	-22.25	-1.5	56.5	-22.5	-1.5	0	-0.25	0
3	56.75	- 15.25	-1.75	56.5	-14.75	-1.75	-0.25	0.5	0
4	52.5	-9	0	52.25	-9.5	0	-0.25	-0.5	0
5	50	-27.25	-5.5	49.75	-27.75	-5.5	-0.25	-0.5	0
6	51	-20.75	-6.5	51	-21	-6.75	0	-0.25	-0.25
7	51	-14	-6.5	51	-14.25	-6.5	0	-0.25	0
8	50	-8.25	-5.75	50.25	-8.75	-5.75	0.25	-0.5	0
9	42.5	-28	- 10.5	42.25	-28	-10.5	-0.25	0	0
10	42.25	-21.5	-10.25	42.5	-22	-10.25	0.25	-0.5	0
11	42.25	-15.5	-10.25	42.25	-15.75	- 10.25	0	-0.25	0
12	41.75	-9.5	-10.25	42	-9.5	-10.25	0.25	0	0
13	37.75	-1.75	-3	37.75	-1.75	-3	0	0	0
14	36.5	-28.25	- 10.5	36.75	-28.25	- 10.25	0.25	0	0.25
15	36.75	-21.5	- 10	36.75	-21.5	-9.75	0	0	0.25
16	36.75	-16	- 10	36.75	-16.25	-10	0	-0.25	0
17	36.25	-9.5	-10	36.25	-9.5	-10	0	0	0
18	33.75	-2	-3	33.75	-2	-3	0	0	0
19	31.5	-27.75	-9.5	31.5	-28	-9.5	0	-0.25	0
20	31.5	-21.75	-9.5	31.5	-22	-9.5	0	-0.25	0
21	31.5	-15	-9.5	31.5	- 15	-9.75	0	0	-0.25
22	31.5	-9.5	-9.75	31.5	- 10	-9.75	0	-0.5	0
23	29.75	0	-3.5	29.75	0	-3.25	0	0	0.25
24	23.75	-27	-5.5	23.75	-27	-5.5	0	0	0
25	23.5	-21.75	-5	23.5	-21.75	-5	0	0	0
26	23.75	-15.5	-5.25	23.75	- 15.5	-5.25	0	0	0
27	23.5	-10	-5.5	23.5	- 10	-5.5	0	0	0
28	24.25	-0.5	-2.75	24.25	-0.5	-3	0	0	-0.25
29									
- 30									



Figure F-2. Vehicle Crush Info Set 2, Test No. 4CMB-1

Occupant Compartment Deformation Index (OCDI)

4CMB-1 Test No. Vehicle Type: 2002 Ram 1500

OCDI = XXABCDEFGHI

XX = location of occupant compartment deformation

A = distance between the dashboard and a reference point at the rear of the occupant compartment, such as the top of the rear seat or the rear of the cab on a pickup

- B = distance between the roof and the floor panel
- C = distance between a reference point at the rear of the occupant compartment and the motor panel
- D = distance between the lower dashboard and the floor panel

E = interior width

F = distance between the lower edge of right window and the upper edge of left window

G = distance between the lower edge of left window and the upper edge of right window

H= distance between bottom front corner and top rear corner of the passenger side window

I= distance between bottom front corner and top rear corner of the driver side window

Severity Indices

- 0 if the reduction is less than 3%
- 1 if the reduction is greater than 3% and less than or equal to 10 %
- 2 if the reduction is greater than 10% and less than or equal to 20 %
- 3 if the reduction is greater than 20% and less than or equal to 30 % 4 if the reduction is greater than 30% and less than or equal to 40 %



1 = Passenger Side

2 = Middle

3 = Driver Side

Location:

Measurement	Pre-Test (in.)	Post-Test (in.)	Change (in.)	% Difference	Severity Index
A1	54.75	54.75	0.00	0.00	Ō
A2	50.25	50.00	-0.25	-0.50	0
A3	56.25	56.25	0.00	0.00	0
B1	47.25	47.25	0.00	0.00	0
B2	42.25	42.50	0.25	0.59	0
B3	48.00	48.00	0.00	0.00	0
C1	66.25	66.50	0.25	0.38	0
C2	46.50	46.50	0.00	0.00	0
C3	63.00	63.00	0.00	0.00	0
D1	23.00	23.00	0.00	0.00	0
D2	13.25	13.25	0.00	0.00	0
D3	23.00	23.00	0.00	0.00	0
E1	66.00	66.00	0.00	0.00	0
E3	65.00	64.75	-0.25	-0.38	0
F	56.25	56.50	0.25	0.44	0
G	56.00	56.00	0.00	0.00	0
Н	37.00	37.25	0.25	0.68	0
	37.50	37.25	-0.25	-0.67	n

Note: Maximum sevrity index for each variable (A-I) is used for determination of final OCDI value

XXABCDEFGHI Final OCDI: LF 0 0 0 0 0 0 0 0 0

Figure F-3. Occupant Compartment Deformation Index, Test No. 4CMB-1

VEHICLE PRE/POST CRUSH INFO

TEST: 4CMB-2 VEHICLE: 2002 Kia Rio Sedan Note: If impact is on driver side need to enter negative number for Y

POINT	Х	Y	Z	X'	Y'	Z'	DEL X	DEL Y	DEL Z
1	26	-20	-0.25	23	-18.75	-0.75	-3	1.25	-0.5
2	29	-15	-1	26.5	-13.25	0	-2.5	1.75	. 1
3	31	-6.75	-1.5	30.25	-6.5	- 1.25	-0.75	0.25	0.25
4	31.25	-2	0	31.5	-2	0	0.25	0	0
5	23.75	-20	-5.5	22	-19	-4.75	-1.75	1	0.75
6	27.5	-9.75	-6.5	27.75	-8.75	-6.25	0.25	1	0.25
7	27.5	-3.25	-7	27.5	-2.75	-6.75	0	0.5	0.25
8	28	-0.25	-6.5	28	-0.5	-6.5	0	-0.25	0
9	19.75	- 19.5	-9	19.25	-18.75	-9.25	-0.5	0.75	-0.25
10	20.75	-7.25	-9.5	20.75	-7.25	-9.25	0	0	0.25
11	20.5	-1.75	- 10	20.5	-1	- 10	0	0.75	0
12	18	0.25	-9.75	17.75	1	-9.75	-0.25	0.75	0
13	14.25	-20.25	-8.5	14.5	- 19.5	-8.75	0.25	0.75	-0.25
14	14.5	-10.25	-9	14.5	-9.75	-9	0	0.5	0
15	14.25	-1.5	-9.5	14.25	-1.5	-9.5	0	0	0
16	11.75	0	-9.25	11.75	0.5	-9.5	0	0.5	-0.25
17	11.5	4	-5	11.5	4	-5.25	0	0	-0.25
18	9.25	-20.5	-8.25	9.25	-20.25	-8.25	0	0.25	0
19	9.5	-9.75	-8.5	9.25	-9.25	-8.75	-0.25	0.5	-0.25
20	10	-1.75	-9.25	10	-1.5	-9.5	0	0.25	-0.25
21	8.25	6	-5	8.5	6.25	-5	0.25	0.25	0
22	5	-25	-3.5	5	-25	-3.75	0	0	-0.25
23	1.5	-20.25	-4.75	1.5	-20.25	-5	0	0	-0.25
24	1.25	-13	-5.75	1.25	-13	-6	0	0	-0.25
25	1.25	-6	-6	1.25	-6	-6.25	0	0	-0.25
26	1.25	0.75	-5.25	1.25	1	-5.25	0	0.25	0
27	1.25	6.75	-4.5	1.25	7	-4.5	0	0.25	0
28							0	0	0
29									
30					-	2			



Figure F-4. Vehicle Crush Info, Test No. 4CMB-2

Occupant Compartment Deformation Index (OCDI)

Test No.4CMB-2Vehicle Type:2002 Kia Rio Sedan

OCDI = XXABCDEFGHI

XX = location of occupant compartment deformation

A = distance between the dashboard and a reference point at the rear of the occupant compartment, such as the top of the rear seat or the rear of the cab on a pickup

B = distance between the roof and the floor panel

 $\mathsf{C}=\mathsf{distance}$ between a reference point at the rear of the occupant compartment and the motor panel

D = distance between the lower dashboard and the floor panel

E = interior width

F = distance between the lower edge of right window and the upper edge of left window

G = distance between the lower edge of left window and the upper edge of right window

 $\ensuremath{\mathsf{H}}\xspace$ distance between bottom front corner and top rear corner of the passenger side window

I= distance between bottom front corner and top rear corner of the driver side window

Severity Indices

0 - if the reduction is less than 3%

1 - if the reduction is greater than 3% and less than or equal to 10 %

2 - if the reduction is greater than 10% and less than or equal to 20 % 3 - if the reduction is greater than 20% and less than or equal to 30 %

4 - if the reduction is greater than 30% and less than or equal to 40 %



where, 1 = Passenger Side

2 = Middle

3 = Driver Side

Location:

Measurement	Pre-Test (in.)	Post-Test (in.)	Change (in.)	% Difference	Severity Index
A1	48.75	47.50	-1.25	-2.56	Ó
A2	45.25	44.75	-0.50	-1.10	0
A3	47.25	47.75	0.50	1.06	0
B1	41.00	41.50	0.50	1.22	0
B2	37.50	37.50	0.00	0.00	0
EG	40.75	41.00	0.25	0.61	0
C1	55.00	52.00	-3.00	-5.45	1
C2	60.25	60.25	0.00	0.00	0
C3	55.25	55.00	-0.25	-0.45	0
D1	20.00	20.00	0.00	0.00	0
D2	18.75	17.50	-1.25	-6.67	1
D3	22.50	22.50	0.00	0.00	0
E1	52.75	52.50	-0.25	-0.47	0
E3	53.50	53.50	0.00	0.00	0
F	44.00	44.00	0.00	0.00	0
G	44.00	42.75	-1.25	-2.84	0
Н	38.50	38.50	0.00	0.00	0
E	38.00	38.00	0.00	0.00	0

Note: Maximum sevrity index for each variable (A-I) is used for determination of final OCDI value

XXABCDEFGHI Final OCDI: LF 0 0 1 1 0 0 0 0 0

Figure F-5. Occupant Compartment Deformation Index, Test No. 4CMB-2

VEHICLE PRE/POST CRUSH INFO

TEST: 4CMB-3 VEHICLE: 2002 Kia Rio Sedan Note: If impact is on driver side need to enter negative number for Y

POINT	Х	Y	Z	X'	Y'	Z	DEL X	DEL Y	DEL Z
1	26.25	-25	0	22	-22.5	1.75	-4.25	2.5	1.75
2	29	-19.5	-2	27.75	-18.5	-1	-1.25	1	1
3	31.5	-14	-2.75	30.25	-13.5	-2	-1.25	0.5	0.75
4	31	-4.75	-4.5	31	-4.5	-4.25	0	0.25	0.25
5	24.25	-25	-4	21.75	-23	-2.75	-2.5	2	1.25
6	27	-19.25	-6	25.75	-18.25	-5	-1.25	1	1
7	29	-13.5	-7.25	28.75	-13	-6.25	-0.25	0.5	1
8	28.25	-4.75	-8.5	28.75	-5	-8	0.5	-0.25	0.5
9	17	-25.5	-5.5	16.75	-25.5	-5.75	-0.25	0	-0.25
10	17.75	- 18.5	- 10.25	17.75	-18.25	- 10	0	0.25	0.25
11	17.5	-13.75	-10.5	17.25	-13.5	- 10	-0.25	0.25	0.5
12	17.5	-9.5	-11.25	17.25	-9.25	-10.75	-0.25	0.25	0.5
13	16	-5	-11	16	-4.75	-11	0	0.25	0
14	12	-0.75	-7	12	-1	-7	0	-0.25	0
15	13	-25.5	-5.25	12.75	-25.5	-5.5	-0.25	0	-0.25
16	12.75	- 19.5	-9.75	12.75	-19.25	- 10	0	0.25	-0.25
17	13	-13.25	- 10.25	12.75	-13	- 10	-0.25	0.25	0.25
18	13.25	-7.25	-11	13	-7	-10.75	-0.25	0.25	0.25
19	9.75	-1	-6.75	9.5	- 1	-6.75	-0.25	0	0
20	9.25	-25.5	-4.75	9.25	-25.25	-5	0	0.25	-0.25
21	10.25	-21.5	-9.5	10.5	-21.25	-9.5	0.25	0.25	0
22	10	-17	-9.5	10	-16.75	-9.5	0	0.25	0
23	10.25	-9.5	-10.75	10.25	-9.25	-10.5	0	0.25	0.25
24	7.75	-1.75	-6.75	7.75	-2	-6.5	0	-0.25	0.25
25	1	-24.25	-5	1	-24	-5.25	0	0.25	-0.25
26	1	- 19	-6.25	1	-19	-6.5	0	0	-0.25
27	1	-13	-7	1	-13	-7	0	0	0
28	0.75	-4.5	-6.75	0.75	-4.5	-6.75	0	0	0
29									
30									



Figure F-6. Vehicle Crush Info, Test No. 4CMB-3

Occupant Compartment Deformation Index (OCDI)

Test No. 4CMB-3 Vehicle Type: 2002 Kia Rio Sedan

OCDI = XXABCDEFGHI

XX = location of occupant compartment deformation

A = distance between the dashboard and a reference point at the rear of the occupant compartment, such as the top of the rear seat or the rear of the cab on a pickup

 ${\sf B}={\sf distance}\ {\sf between}\ {\sf the}\ {\sf roof}\ {\sf and}\ {\sf the}\ {\sf floor}\ {\sf panel}$

 $\mathsf{C}=\mathsf{distance}\ \mathsf{between}\ \mathsf{a}\ \mathsf{reference}\ \mathsf{point}\ \mathsf{at}\ \mathsf{the}\ \mathsf{rear}\ \mathsf{of}\ \mathsf{the}\ \mathsf{occupant}\ \mathsf{compartment}\ \mathsf{and}\ \mathsf{the}\ \mathsf{motor}\ \mathsf{panel}$

D = distance between the lower dashboard and the floor panel

E = interior width

F = distance between the lower edge of right window and the upper edge of left window

G = distance between the lower edge of left window and the upper edge of right window

H= distance between bottom front corner and top rear corner of the passenger side window

I= distance between bottom front corner and top rear corner of the driver side window

Severity Indices

0 - if the reduction is less than 3%

1 - if the reduction is greater than 3% and less than or equal to 10 %

2 - if the reduction is greater than 10% and less than or equal to 20 % 3 - if the reduction is greater than 20% and less than or equal to 30 %

4 - if the reduction is greater than 30% and less than or equal to 40 %



where, 1 = Passenger Side

2 = Middle

3 = Driver Side

Location:

Measurement	Pre-Test (in.)	Post-Test (in.)	Change (in.)	% Difference	Severity Index
A1	49.50	49.00	-0.50	-1.01	0
A2	49.00	48.50	-0.50	-1.02	0
A3	50.25	50.25	0.00	0.00	0
B1	42.00	36.50	-5.50	-13.10	2
B2	37.50	36.00	-1.50	-4.00	1
B3	41.75	41.75	0.00	0.00	0
C1	60.75	59.00	-1.75	-2.88	0
C2	43.50	43.25	-0.25	-0.57	0
C3	60.50	50.00	-10.50	- 17.36	2
D1	21.00	20.75	-0.25	-1.19	0
D2	19.50	19.75	0.25	1.28	0
D3	22.25	22.25	0.00	0.00	0
E1	52.75	52.50	-0.25	-0.47	0
E3	53.75	54.00	0.25	0.47	0
F	47.00	41.25	-5.75	-12.23	2
G	47.75	48.25	0.50	1.05	0
Н	39.50	39.50	0.00	0.00	Ó
	39.75	39.00	-0.75	-1.89	0

Note: Maximum sevrity index for each variable (A-I) is used for determination of final OCDI value

XXABCDEFGHI Final OCDI: LF 0 2 2 0 0 2 0 0 0



APPENDIX G

Vehicle Center of Gravity Determination

	4CMB-1		Vehicle:	2002 Dodge F	Ram 1500QC	
			Vehicle C	G Determinatio	on	
VEHICLE	Equipment	Weight	Long CG	Vert CG	HOR M	Vert M
+	Unbalasted Truck	5052	61.75	28.3	311961	142971.6
+	Brake receivers/wires	5	116	51	580	255
+	Brake Frame	13	34	31	442	403
+	Brake Cylinder	29	74	29	2146	841
+	Strobe Battery	6	74	30	444	180
+	Hub	27	0	14.75	0	398.25
+	CG Plate (EDRs)	8	54	32	432	256
-	Battery	-40	-7	45	280	-1800
-	Oil	-11	8	19	-88	-209
-	Interior	-58	44	24	-2552	-1392
-	Fuel	-140	111	20	-15540	-2800
-	Coolant	-13	-18	35	234	-455
-	Washer fluid	0	-15	35	0	0
BALLAST	Water	110	111	20	12210	2200
	Misc.		0	0	0	0
	Misc.		0	0	0	0
					310549	140848.9
	TOTAL WEIGHT	4988			62.25922	28.23754
wheel hase	140.25					
wheel base	NCHRP 350 Targete				Difference	1
	Test Inertial Weight	5000		4988	-12 N	
	I ong CG	62		62.26	0 25922	
	Vert CG	28		28.24	0.23754	

Vert CG2828.24Note, Long. CG is measured from front axle of test vehicle

Curb Weight			
	Left		Right
Front		1471	1362
Rear		1079	1140
FRONT		2833	
REAR		2219	
TOTAL		5052	

Actual test inertial weight							
	Left		Right				
Front		1426	3	1336			
Rear		1065		1161			
FRONT		2762					
REAR		2226					
TOTAL		4988					

Figure G-1. Vehicle Mass Distribution, Test No. 4CMB-1

	4CMB-2		Vehicle:	2002 Kia Rio	
	_		Vehicle C	G Determination	
VEHICLE	Equipment	Weight	Long CG		HOR M
+	Unbalasted Car	2249	37.39695		84105.75
+	Brake receivers/wire	5	130.5		652.5
+	Brake Frame	5	29.5		147.5
+	Brake Cylinder	22	65		1430
+	Strobe Battery	6	65.5		393
+	Hub	17	0		0
+	CG Plate (EDRs)	8	35		280
-	Battery	-35	-9		315
-	Oil	-8	8		-64
-	Interior	-34	44		-1496
-	Fuel	-15	78		-1170
-	Coolant	-8	-18		144
-	Washer fluid		0		0
BALLAST	Water	95	78		7410
	DTS Unit	20	60		1200
	Misc.	66	52		3432
					96779.75
	TOTAL WEIGHT	2393			40.44285

wheel base 95.25

00.20			
NCHRP 350 Targets		CURRENT	Difference
Test Inertial Weight	2420 (+/-)55	2393	-27.0
Long CG	39 (+/-)4	40.44	1.44285
2023			

Note, Long. CG is measured from front axle of test vehicle

Curb Weight				
	Left		Right	
Front		701		665
Rear		420	18	463
FRONT		1366		
REAR		883		
TOTAL		2249		

Dummy = 166lbs.					
Actual test inertial weight					
	Left		Right		
Front		677		692	
Rear		512		512	
FRONT		1369			
REAR		1024			
TOTAL		2393			

Figure G-2. Vehicle Mass Distribution, Test No. 4CMB-2

	4CMB-3		Vehicle:	2002 Kia Rio	
			Vehicle C	G Determination	
VEHICLE	Equipment	Weight	Long CG		HOR M
+	Unbalasted Car	2402	35.17		84486.75
+	Brake receivers/wire	5	130.5		652.5
+	Brake Frame	10	29.5		295
+	Brake Cylinder	22	65		1430
+	Strobe Battery	6	65.5		393
+	Hub	17	0		0
+	CG Plate (EDRs)	8	35		280
-	Battery	-35	-9		315
-	Oil	-6	8		-48
-	Interior	-50	44		-2200
-	Fuel	-15	78		-1170
-	Coolant	-10	-18		180
-	Washer fluid	-3	0		0
BALLAST	Water	50	78		3900
	Misc.		0		0
	DTS	20	65		1300
					89814.25
	TOTAL WEIGHT	2421			37.098

wheel base 95.25

NCHRP 350 Targets		CURRENT	Difference
Test Inertial Weight	2420 (+/-)55	2421	1.0
Long CG	39 (+/-)4	37.10	-1.90200

Note, Long. CG is measured from front axle of test vehicle

Curb Weight				
	Left		Right	
Front		770		745
Rear		426	13	461
FRONT		1515		
REAR		887		
TOTAL		2402		

Dummy = 166lbs.					
Actual test inertial weight					
	Left		Right		
Front		731	l –	732	
Rear	-	457		491	
FRONT		1463			
REAR		948			
TOTAL		2411			
LIOTAL		2411			

Figure G-3. Vehicle Mass Distribution, Test No. 4CMB-3

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