# SILICA FUME LATEX MODIFIED CONCRETE BRIDGE DECK OVERLAY

# Tualatin River Bridge #1417N

Final Report Experimental Features Project No. 93-05

By

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Prepared for

Oregon Department of Transportation Research Unit Salem, Oregon 97310

and

Federal Highway Administration Washington, D.C. 20590

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16. Abstract	1,000,000,000,000				
The Oregon Department of Transport chloride permeability in bridge deck oused. The overlay was completed in permeability measurements were low	overlays. On this project a new July of 1993. Three years late	w product combi er, only a few cra	ning both silica fume and	d LMC was	
One unexpected result was lower friction numbers on the deck surfacing. At this time it is unclear if the reduced fri was due to the modified cement or the smaller aggregates used in the mix design. ODOT plans further testing to det the main cause of the reduced friction.					
This product was difficult to mix and	place and has since been disc	continued by the	producer.		
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# ACKNOWLEDGMENTS

The author would like to thank the following Oregon Department of Transportation (ODOT) personnel for their contributions and help in gathering information for this report: Bruce Patterson, Les Harkema, Keith Johnston, and Mike Dunning. In addition, the author thanks Reichhold Chemicals, Inc. for their cooperation supplying data on their product.

# **DISCLAIMER**

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This report does not constitute a standard, specification, or regulation.

# SILICA FUME LATEX MODIFIED CONCRETE BRIDGE DECK OVERLAY

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# 1.0 INTRODUCTION

In September of 1993, The Oregon Department of Transportation overlayed a deteriorated bridge deck. A new material was used to overlay the deck: silica fume latex modified concrete. Both silica fume and latex modified concrete have been used in earlier projects with good results. Silica fume adds strength to the concrete and latex retards cracking. Both materials also reduce chloride permeability. This report documents the construction, permeability testing and the three-year performance of the overlay. Figure 1.1 shows the general conditions of the bridge.



Figure 1.1 The Tualatin River Bridge.

# 2.0 PROJECT DESCRIPTION

# 2.1 PROJECT LOCATION AND ENVIRONMENT

The project is located near Portland, Oregon on Highway 99W. The north bound structure (BR#1417N) of the twin bridges over the Tualatin River at milepost 12.18 received the overlay (Figures 2.1 and 2.2). The climate in this area has mild winters although a few freezing periods are normal in winter months. The average daily traffic of 33,000 is 90% cars with the remaining 10 % percent consisting of trucks with gross weights from 20,000 lbs to 80,000 lbs

The structure, built in 1957, is a 463-foot RCDG with 8 bents. The concrete deck has a slight down grade with a curve to the left. The deck surfacing had spalled and cracked to the point it needed resurfacing.

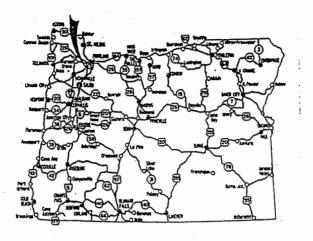


Figure 2.1 The project is located near Portland, Oregon.

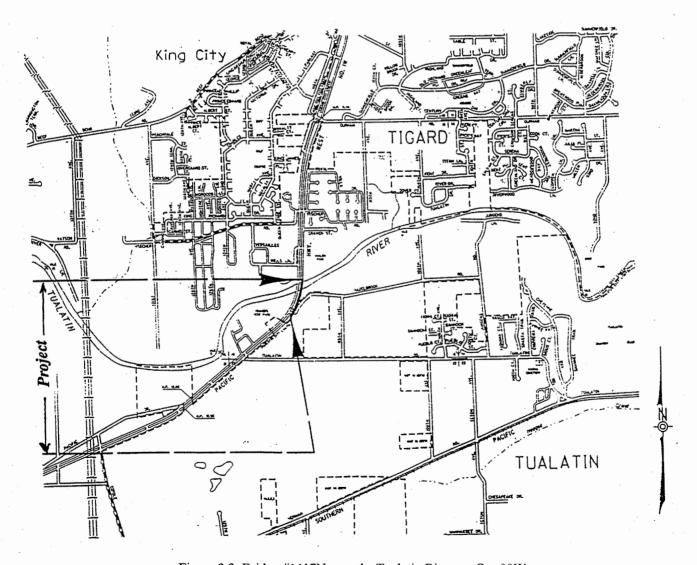


Figure 2.2 Bridge #1417N over the Tualatin River on Ore 99W.

# 3.0 CONSTRUCTION

## 3.1 MIX DESIGN

Before the overlay, a trial mix design was submitted to the ODOT Materials Lab for approval. Trial batches were made and tested by Material Testing Corporation for Hamilton Construction on July 28, 1993. ODOT personnel and representatives from Reichhold Chemical, the SFLMC supplier, witnessed the testing (see Table 3.1).

Table 3.1: Trial Mix Design MTC-93\_11 for One Cubic Yard.

Item	Batching weights in lbs	Absolute volume in cu.ft.
Cement, Tilbury Type I-II	660	3.36
Water (10.0 gallons)	83	1.34
Concrete Sand (SSD)	1,476	9.06
Coarse Aggregate (SSD) 5/8"-3/8"	1,467	8.74
Reichhold Chemical Silica Fume	238	3.43
(25.9 gal/cy)		
Entrained Air	0.0	1.07
Total	3,924	27

The trial mix was tested for slump, air content, unit weight and compressive strength.

All testing met ODOT specifications. The unit weight, however, was lower than the design value.

Results are listed below:

- Slump = 7"
- Air Content = 8%
- Compressive strength (7 day)= 5,170 lbs (design 4,000)
- Unit Weight =139.8 lbs/cu.ft. (design 145.0)
- W/C=0.33

A complete listing is included in Appendix A.

# 3.2 QUALITY CONTROL

Mix proportions were controlled by calibration of the two mobile mixers the same as a latex modified concrete overlay is controlled. Control settings on trucks #2518 and #2519 were calibrated on August 31, 1993. Settings for cement, fine and coarse aggregates, latex emulsion and water gauges were calibrated. Representatives from MTC and ODOT witnessed and approved the calibration shown in table 3.2.

**Table 3.2: Mix Control Summary** 

Item	Truck # 2518	Truck # 2519
Cement Meter Count	7.28	7.13
Time to Discharge	32.42	33.18
F.A. Sand Dial	5.6	5.8
C.A. Stone Dial	4.45	4.95
Latex(Gauge setting)	7.0=6.8 GPM	7.2=Bottom of mark
Water(Flow Meter)	1.9 Maximum	2.0 Maximum

The cement used in the mix was sampled and tested by ODOT. No defects were reported. The TYLAC 680015-00 (Silica Fume Latex Modifier) was accepted on the supplier's certification. Aggregates, both coarse and fine were sampled and tested by ODOT. The first sample of sand had 1.4% light weight particles. The maximum allowable was 1%. A second sample passed with 0.2% light weight particles. The coarse aggregates met all of ODOT specifications. However, a price agreement had to be written to allow crushed material. Construction testing data is included in Appendix B.

## 3.3 DECK PREPARATION

The deck was hydro-blasted before the overlay to remove deteriorated concrete. Some Class 2 preparation was needed near the north end. Rebar was very near the worn surface. Old concrete was removed to a depth of 3/4-inch below the exposed rebar as per ODOT specifications.

## 3.4 DECK OVERLAY

The deck was overlayed in two stages: the right lane was completed on September 3, 1993 and the left lane was completed on September 23, 1993. One travel lane remained open to traffic during the deck pour by placing concrete barriers near the center of the bridge. These barriers also supported the rail for the concrete deck-paving machine.

Weather conditions were reported as good. A complete summary can be found in Appendix C. Traffic was reported as heavy on the last section of the second pour. The contractor believed this contributed to the minor cracking found in this area.

The ODOT material inspector reported problems with the pumps on the mobile mixer on the

jobsite. The pumps on the mobile mixers were not designed for the stiff mix produced by adding silica fume. They clogged up several times, causing some delays and confusion about the amount of mix used because one of the mixers left the deck to be repaired. The gears on the pump were stripped. The inspector said that the silica fume proportioning might have been low before the pump problem was discovered. No testing was performed to determine the silica fume/latex content in the placed mixture.

# 4.0 TEST RESULTS

## 4.1 CONCRETE PROPERTIES

Cylinders were cast from the fresh SFLMC material for testing of concrete properties and also permeability testing. The test results of these are listed below except permeability is discussed in section 4.2.

Table 4.1: Concrete Properties from SFLMC

Item	Date 9/3/93 (right lane)	Date 9/23/93 (left lane)
Slump	3 1/4 inches	7 inches
Air	4.5 %	6.8 %
Unit Weight	145.2 lbs/cu.ft.	140.8 lbs/cu.ft.
7 day compressive strength	6640 lbs/cu.in.	5400 lbs/cu.yd.
Cement content	678 lbs/cu.yd.	658 lbs/cu.yd
W/C (water cement ratio)	0.32	0.31

All tests were within specifications. However, there are some differences between the two pours. Note especially the lower unit weight of the second pour. The lower cement content is reflected in the lower breaking strength.

## 4.2 POST CONSTRUCTION TESTING

After construction, a cracking and de-lamination survey was made. Bond tests were also performed on the deck. All bond tests were acceptable. A few minor cracks, found in the north end spans, were sealed with methacrylate.

The concrete was tested for chloride permeability by the AASHTO T277 test procedure. Cylinders were cast at the time of pour while cores were cut from the deck in January of 1994. All cores and cylinders sampled had very low permeabilities (less than 500 coulombs passed) as shown in table 4.3.

Table 4.2: Permeability of Cores and Cylinders for the SFLMC Deck Overlay

Date Poured	Cores, Coulombs Passed	Cylinders, Coulombs Passed
9/3/93	302	346
9/3/93	466	298
9/23/93	286	367
9/23/93	347	313
AVERAGE	350	331
STANDARD DEVIATION	81	31

The manufacturers' pre-construction laboratory test data for permeability of the SFLMC was about 140 coulombs passed. The causes of the higher values on the project are not known. Some speculate that it is a problem with the AASHTO T277 test method. Also, problems in the mobile mixer due to a pump malfunction could alter the percent of silica fume in the mixture.

# 5.0 THREE-YEAR EVALUATION

The deck was inspected after one year and again after three years. Only a few minor cracks were found at the one year inspection. After three years, only a few new cracks had developed. A survey, by means of chain dragging, found only a few minor delaminations. Overall the deck looked very good. However, some polished aggregate was reported in the wheel paths. The polished aggregates appear to be the flat side of the crushed rock used in the mix design. (See Figures. 5.1 - 5.3).

Friction testing for the first and third years of service reflected this wheel path wear (see Table 5.1).

Table 5.1: Average Friction Numbers for Bridge #1417N.

Date	Friction	Change
7/19/94	38	
4/17/97	32	-6

An average value for several other LMC and MC overlays was about 47. A value of 37 or lower is considered hazardous and requires some remedy.



Figure 5.1 Bridge #1417N, note polished aggregates.



Figure 5.2 Bridge #1417N, most of the deck is in good condition.



Figure 5.3 Bridge #1417N, minor cracking is rare.

# 6.0 CONCLUSIONS AND RECOMMENDATIONS

- 1. The silica fume latex modified concrete reduced permeability. However, the reduction was less than the producer's claim. The measured project mean value of 350 coulombs passed was higher than the 140 claimed by the manufacturer.
- 2. The product could not be mixed and placed by a conventional mobile mixer because of pump seals. Modifications to either the mix or the mixer would be costly. Thus the product was discontinued.
- 3. The wearing qualities of this mixture resulted in low friction values.

ODOT recommends against any further use of this product. Also, more work needs to be done to determine the cause of the low friction values.

# APPENDIX A TRIAL MIX TESTING



HAMILTON CONSTRUCTION 3140 NW 185th Avenue Portland, Oregon 97229

Attn: Kevin Gothberg

Re: Silica Fume Latex Modified Concrete Mix Design Tualatin River Bridge No. 1417 Section

### Gentlemen:

As per your request, the attached Proposed Concrete Proportions are based upon laboratory trial batches performed by MTC for the above referenced project.

A trial batch was performed on Wednesday, July 28, 1993. Those in attendance included Mike Merrigan, ODOT, Jerry Walters and Steven Lucas, Reichhold Chemical Company representatives. This Mix Design has benn assigned with the number MTC-93-11.

The trial batch and proportions were performed in accordance the most recent version of ACI 211. The testing was performed in accordance with the current applicable ASTM standard.

The concrete proportions shown on the attached pages are based upon a saturated surface dry (SSD) condition for the aggregates. This is an almost dry condition and typically the aggregates will be wetter in the field. Therefore, the batch weights for the aggregates and water must be corrected for the moisture content of the aggregates in the field.

This can be done by taking the SSD batch weight of the aggregate, multiplying by (100 + % moisture) and dividing by (100 + % absorption). Then decrease the amount of water by the total number of pounds the aggregates are increased.

In order to assure a workable and acceptable mix in the field, the total moisture in the fine aggregate cannot exceed 9%.

If you have any questions concerning the mix design process, the concrete proportions as proposed, or if we can be of any further assistance please call us at (503) 238-3824.

Respectfully submitted,

MATERIALS TESTING CORPORATION

John A. Link

Division Manager

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HAMILTON CONSTRUCTION 3140 NW 185th Avenue Portland, Oregon 97229

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In order to assure a workable and acceptable mix in the field, the total moisture in the fine aggregate cannot exceed 9%.

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Respectfully submitted,

MATERIALS TESTING CORPORATION

John A. Link

Division Manager

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# CONCRETE PROPORTIONS ONE CUBIC YARD

Prepared for: HAMILTON CONSTRUCTION

DESIGN STRENGTH: 4,000 psi @ 7 days BASED ON TRIAL BATCH #1 - MTC 93-10

MIXTURE CRITERIA:

Cement

Tilbury Type I-II

Specific Gravity: 3.15

Fine Aggregate

Sand

Specific Gravity: 2.61

Absorption: 3.2 %

Coarse Aggregate

5/8"-3/8"

Specific Gravity: 2.69

Silica Fume

Reichhold

Specific Gravity 1.11

Absorption: 1.4 %

BATCHING DATA RESULTS & REVIEW:

Slump: 7"

Air Content: 8.0%

W/C ratio: 0.325

**EXPECTED COMPRESSIVE STRENGTH:** 

3 Day: 2,000 psi 7 Day: 4,000 psi

BATCH QUANTITIES: Pounds Per Cubic Yard

Ітем	BATCHING WEIGHTS	Absolute Volume
Cement, Tilbury Type I - II	660 Lbs.	3.36
Water (10.0 gallons)	83 Lbs.	1.34
Concrete Sand (SSD)	1,476 Lbs.	9.06
5/8" - 3/8" Coarse Aggregate (SSD)	1,467 Lbs.	8.74
Reichold Chemical Silica Fume (25.9 gal./cy)	238 Lbs.	3.43
Entrained Air	0.0 Lbs.	1.07
TOTAL	3,924 Lbs.	27.00

Mix may need to be field adjusted to provide the desired air content & slump.



# **Trial Batch Data** Batch #1- MTC 93 - 11

PROPERTIES:		Fly	Admix	#2	#1	#2			
	Cement	Ash	Latex	Sand	Coarse	Coarse			
Sp. Gr.	3.15		1.11	2.61	2.69				
Absorption	. •			3.2%	1.4%				
Moisture	-			7.3%	1.4%				
Soild Volume Unit Weight	196.6		3.4	162.9	167.9				
Batch Size:	0.111	cubic yd	3.000 a	ıbic feet					
Design % Air:	3.0%				Sand Ratio:	50.1 %			
Sack Content:	7.02				W/C Ratio:	0.33			
% Fly Ash:			Theoretic	al Air Free	Unit Weight:	149.80	lbs/cubic ft		Design Water
				Design	Unit Weight:	145.33	lbs/cubic ft		10.0 gals.
PROPORTIONS:		Fly	Admix	#2	#1	#2	Total	Air	Yield
	Cement	Ash	Latex	Sand	Coarse	Coarse	Water	Content	Totals
SSD Weights per yard:	660		238	1,476	1,467		83	3.0%	3,924 lbs.
Absolute Volume:	3.36		3.43	9.06	8.74		1.33	0.81	26.73 cu.ft.
Estimated Batch Weights (Ibs):	73.30		26.44	164.00	163.00		11.10		437.8 lbs.
Estimated Batch Weights (grams):									
Actual Batch Weights (lbs):	73.30		26.44	170.70	163.00		9.26		442.7 lbs.
Actual Absolute Volumes:	0.37			1.00	0.97		0.15		
ADMIXTURES:	Dosage		Batch		Batch				
	Gal/cu, y	d.	Dosage (ml)		Dosage (fl oz)				
Reichhold Silica Fume	25.94								
*NOTE: Silica Fume is 45% Solids and	55% Wat	er							

**TEST RESULTS:** 

Air Content (PressureMethod):

Unit Weight Bucket Weight: Unit Weight Bucket Volume: 0.2490 cubic foot Concrete Stump:

lbs. inches

7.00

8.0 %

Weight of Bucket & Concrete:

34.77 lbs.

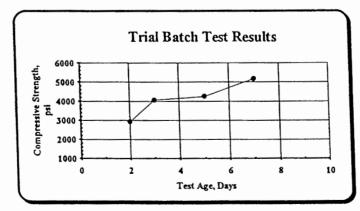
Unit Weight of Concrete:

139.8 lbs/cubic feet

Relative Yield:

Yield: 1.0420 cubic feet

Air Content (Gravametric):				Concrete T	emperature:	76.3 Degrees Farenbeit
•		Comp	pressive Stre	ngth		Average
Test Age, Days:	#1	#2	#3	#4	#5	
2	2920					2920
3	4060					4060
5	4250					4250
7	5350	5160	5180	5070	5100	5170



# LABORATORY RECORD

TUALATIN RIVER BRIDGE NO. 1417n  AGREET NO. 1417n  Pacific Highway West Washington 11349  COUNTY Washington 11349  FAPROJECT NO. X-STP-SO1W(3)  PROJECT MANAGER Dennis Carlson 1802 8-27-93  SCHAMTED BY  AGY. ORG. UNIT TESTNO. VAR. LAB CHARGE	7
Pacific Highway West  Washington 11349  FAFROJECT NO.	,··
Hamilton Construction Co. X-STP-SOlW(3)  ROJECT MANAGER Dennis Carlson 1802 8-27-93  OBMITTED BY AGYORG. UNIT DATE RECEIVED 1807-ORG. UNIT TESTNO. (VAR. LAB CHARGE	
Dennis Carlson 1802 8-27-93  CHAMTTED BY AGYORG. DAILY TEST NO.   VAR.   LAB CHARGE	
The state of the s	
Mike Merrigan 1802 741x \$173.00	:
Hamilton Construction 5-001-1	
MTC-93-11   CLASS   TO BE USED   Silica Fume Latex Overlay	

The Contractor's mix design for the Silica Fume Latex Modified Concrete Overlay was reviewed according to Section 557.13 of the Contract Special Provisions.

Based on the trial batch test results and other information submitted by the Contractor we have determined that the mix design does comply with requirements of 557.13 for new mix designs.

# Mix Proportions as submitted by Hamilton Construction are:

Cement 660 lbs. Tilbury Type 1-2 Coarse Agg. 1467 lbs.(SSD) Absorption: 1.4% 1476 lbs.(SSD) Absorption: 3.2% SF Latex 238 lbs. Reichhold Silica Fume Latex

Water 83 lbs. WC Ratio: 0.33

Air Content: 4.0%

> Air Entraining Agent: None Water Reducer: None

Mix design lab number:

9309287

Our review of this mix design does not relieve the Contractor of his responsibility to produce satisfactory concrete.

NOTE: THIS MIX DESIGN	XDOES.	DOES NOT COMPLY WITH SPECIFICATIONS

# APPENDIX B CONSTRUCTION TESTING

rgan Department of Tran	SAM		A AND LABO FO CONCRETE (	R		REF. (			9 <b>10</b> 598
ROJECT	11.0.	- R	alas /	No 14.	171			C C	89913
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A	DAYS B DAYS		DAYS D	DAYS E		AYS F	DAY:	s¦ G	_DAYS HDAY
CEMENT:	Tilburg	TYPE - 1		L ANALYSIS NO	ADDI	TIVES:	Reid		Latex Silic
MIX DESIGN:	193-09287	DESIGN STRE	1	E SOURCE NO.	GGO	TENT	3-7 IN	! COMA	MAXW/CRATIO
FIELD TES	TOTAL FIELD MOIS	TURE CONTENT	* 5.2 W	WE WE	EMERIT CON	TENT	SLUMP	AIR CONTENT	FIELD W/C RATIO
ADDITIVES	CEMENT 33	15/4/50 *1 218.	FUME CONCRET	E MIX BROPORT	IONS AS BAT		3084	WATER	WATER AT JOB SITE
PRESTRE	SS STEAM HOURS	F.XAM		REO. REL. STR.		ACT. REL		CYLINDER C	JRE HB
CONCRET	E: {	HRS !	الارامة	SECTIC	ZIAL:	HT/W	<u>;                                    </u>	STO	DAYS FIELD DA
FIELD REMAR	RKS: Ambre	+ TIN	p 640	<i>Y</i> ,					
	Mix 9	onp	72° F					•.	
	Silica 1		7 ,	-mil/si	04 /	<b>5</b> 5	5 70	s water	
	Cooler	Tem	D H.	730 /	06	#			
SW	AIR 6.	8 % °	Br St	lace	ra			i	
		<del></del>	/ L	AB USE ONL	Y BELOW	I			
LABOR	RATORY REPORT		<i>:</i>			0	20	-93	9 - 28 - 93
CYL NO.	OATE OF BREAK	AGE DAYS	STRENGTH PSI	OATE REF		LAB RÉ	MARKS: INC	SERVED DEFECT	D, AVG. DIAM., AREA; AND S FOR FAILING CYLINDERS.
A 8	9-30	17	5470	10-1	42	<b> </b>			
c	9-30	7	5390	4		∮	RECE	VED	
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X X X X	PROJECT MANAGER DESCRIPTION GEOLOGIST MATERIALS - PORTLAND MATERIALS - EUGENE R	ENNIS CAI UE D'AGN AS 1	ESE	<b>N</b> . 7		lic	i iy	e	Court
X X		AMILION AL BAIRD	CONSTRUCTIO	N.				ENGINEER OF	TERIALS

ENGINEER OF MATERIALS

SHOWEER OF MATERIALS

# CYLINDERS CAST FLOW FRESH SFLMC

Client: Oregon Department of Transportation Project: Rapid Chloride Permeability Testing Contact: Mr. Kelth Johnston

Submitter: Mr. Bruce Patterson

CTL Proj. No.: 105295 CTL Proj. Mgr.: M. Morrison Technician: W. Hummerich Approved: R. G. Burg Date: January 24, 1994

# RAPID CHLORIDE PERMEABILITY RESULTS AASHTO T-277

Sample No. (Client ID)	Test Date	Charge Passed (Coulombs)	Relative Chloride Permeability
9-3-93	01-21-94 140 DAYS 01-21-94 126 DAYS 01-21-94 126 DAYS	313 / 429	Very Low Very Low Very Low Very Low

Sample Type: Age Since Casting: Specimen History: 4x8-in. concrete cylinders.

Specimens #1 and #2 140 days, Specimens #3 and #4 126 days.

Specimens were received in dry condition. The specimens were placed in

lime-saturated water maintained at 73±3° F, until prepared for test.

See Table below for interpretation of results.

Chloride Permeability	Charged Passed Coulombs	Representative Concrete Type
High	>4000	High water-cement ratio (0.6)
Moderate	2000-4000	Moderate water-cement ratio (0.4-0.5)
Low	1000-2000	Low water-cement ratio "Iowa" dense concrete
Very low	100-1000	Latex-modified concrete Internally sealed concrete Microsilica concrete
Negligible	<100	Polymer-impregnated concrete Polymer concrete

# CYLINDERS COST FROM FRESH SFLMC

Client: Oregon Department of Transportation

Project: Rapid Chloride Permeability Testing Contact: Mr. Keith Johnston Submitter: Mr. Bruce Patterson

CTL Proj. No.: 105295 CTL Proj. Mgr.: M. Morrison Technician: W. Hummerich Approved: R. G. Burg Date: January 24, 1994

## RAPID CHLORIDE PERMEABILITY RESULTS AASHTO T-277

	Sample No. (Client ID)	Test Date	Charge Passed (Coulombs)	Relative Chloride Permeability
9-3-93 9-17-93	∠ #1 #2 ∠ #3 ∠ #4	01-21-94) 140 DAYS 01-21-94) 01-21-94) 126 DAYS 01-21-94)	346 298 367 6 10 10 313 WAY	Very Low Very Low Very Low Very Low

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# APPENDIX C CONSTRUCTION NARRATIVE

# NEW EXPERIMENTAL FEATURES PROJECT

"SILICA FUME LATEX MODIFIED CONCRETE BRIDGE DECK OVERLAY STUDY"

TUALATIN RIVER BRIDGE (NO. 1417N)

### PACIFIC HIGHWAY WEST

## WASHINGTON COUNTY

# SUMMARY OF CONSTRUCTION INSPECTORS' OBSERVATIONS:

The deck overlay was done in two stages in September 1993. Right from the start of the project there were some concerns that the two end spans on the north end of the bridge were not structurally sound. This was because this section had spalled in some areas to expose the rebar, and the rebar did not seem to have had much cover initially. Removal of the old rail on the left hand side of the bridge prior to construction of a new rail also created some transverse cracks in the longitudnal edge beams at this end.

During class one preparation of the deck, minimal hydroblasting in these same areas exposed several portions of rebar by more than half the bar diameter for more than twelve inches, so hydroblasting was done to 3/4 inch below these rebars as per specification (see picture album for the project).

The following is a summary of some pour data. For more details see attached lab reports.

	STAGE 1 - RIGHT	STAGE 2 - LEFT
Date	9-3-93	9-23-93
Start time	12:15 am	8:07 pm
End time	6:00 am	1:05 am
Air temp (°F)	59	64
Mix temp (°F)	81	72
Wind velo. (mph)	2 approx	2 approx
Precipitation (in)	0	0
Humidity (%)	70 approx	40 approx
Vol (Cu. yd)	43.75	44.1
Cure time (hr)	per specs	per specs

## BOND TESTS ON OVERLAY:

Right lane: Chain dragging the deck detected no delaminations, and there were no cracks after cure. Tensile test results were 168 psi, 344 psi and 283 psi all for failure at the epoxy on the pull cap, and not in the deck. One pull at approximately midspan resulted in a 16 psi value with failing surface being 1/2 at new deck-old deck interface, and 1/2 the surface at about 1/4 inch into the old deck. The break surface suggested a little delamination in the old deck at this location.

Left lane: Two pulls came at 185 psi and 193 psi. Cracks were detected in the northern end spans after cure. The contractor claimed this was due to exposure of the adjacent lane to fast moving rush hour traffic without adequate set of the overlay. The cracks were sealed with methacrylate as per specifications.

Bob Fynn (Inspector)

# PRICE AGREEMENT





HIGHWAY DIVISION

PROJECT NAME (SECTION)  Tualatin River Bridge #1417N		CONTRACT NO. C11349
HIGHWAY	COUNTY	
Pacific Hwy West	Washington	
CONTRACTOR'S NAME AND ADDRESS  Hamilton Construction PO BOX 659  SPRINGFIELD OR 97447		F. A. PROJECT NO.  X-STP-S01W(3)  PRICE AGREEMENT NO.
		1
DESCRIPTION AND LOCATION OF WORK OR ITEMS COVERED BY	THIS AGREEMENT:	

Add to Special Provisions Sec. 02690.20(a):

Coarse Aggregate can be crushed.

PAY ITEM NO.	ITEM DESCRIPTION	EST. QUANTITY	UNIT	AGREED UNIT PRICE	TAUOMA
4001	Aggregate Spec. Change			N/C	
	`. 				

INCREASE -0-ESTIMATED NET COST EFFECT OF THIS AGREEMENT ON THE CONTRACT:

SPECIFICATIONS AND PROVISIONS — THE WORK TO BE DONE UNDER THIS AGREEMENT IS TO BE PERFORMED, MEASURED AND PAID FOR IN ACCORDANCE WITH THE TERMS FOR THE ABOVE CONTRACT EXCEPT AS MODIFIED AS FOLLOWS:

Allow the use of crushed aggregate in the Silica Latex Modified Concrete. See Sec. 02690.20(a)

PLEASE INDICATE YOUR AGREEMENT BY SIGNING, DATING AND RETURNING THE
ORIGINAL TO THE PROJECT MANAGER. WORK SHALL NOT BEGIN UNTIL YOU ARE
NOTIFIED THAT THE PRICE AGREEMENT HAS EITHER BEEN APPROVED OR THAT
WORK MAY COMMENCE UNDER ADVANCE APPROVAL YOUR SIGNATURE FUR-
THER INDICATES AGREEMENT THAT PAYMENTS IN ACCORDANCE WITH THIS
PRICE AGREEMENT CONSTITUTE FULL AND COMPLETE COMPENSATION FOR
ALL COSTS, BOTH DIRECT AND INDIRECT, ARISING OUT OF THE DESCRIBED
ALTERATIONS, EXTRA WORK OR CLAIMS FOR ADDITIONAL COMPENSATION
COVERED BY THIS PRICE AGREEMENT, AND RELEASES AND DISCHARGES THE
STATE FROM ALL SUCH COSTS EXCEPT AS PROVIDED HEREIN.

CONTRACTOR

DATE SUBMITTED BY PROJECT MANAGER 26,/993 يىلى APPROVAL RECOMMENDED OR APPROVED BY REGION

INSTRUCTIONS — AFTER THE CONTRACTOR HAS SIGNED, THE PROJECT MANAGER SUBMITS THE ORIGINAL THROUGH REGION TO CONSTRUCTION.

APPROVAL RECOMMENDED BY LOCAL AGENCY