TTI: 9-1002

MASH TEST 3-10 ON 31-INCH W-BEAM GUARDRAIL WITH STANDARD OFFSET BLOCKS

ISO 17025 Laboratory
Testing Certificate # 2821.01

Crash testing performed at:
TTI Proving Ground
3100 SH 47, Building 7091
Bryan, TX 77807

Research Report 9-1002-4

Cooperative Research Program

TEXAS TRANSPORTATION INSTITUTE
THE TEXAS A&M UNIVERSITY SYSTEM
COLLEGE STATION, TEXAS

TEXAS DEPARTMENT OF TRANSPORTATION

in cooperation with the
Federal Highway Administration and the
Texas Department of Transportation
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16. Abstract

The Texas Department of Transportation (TxDOT) initiated a review of their guardrail standards based on the outcome of recent crash test results and a Federal Highway Administration technical memorandum pertaining to guardrail height. TxDOT expressed interest in the use of a generic 31-inch tall guardrail to provide enhanced containment capacity for light trucks. However, some concerns were expressed regarding the increased size of the blockout used in the Midwest Guardrail System (MGS). Consequently, TxDOT requested an evaluation of a 31-inch tall guardrail system that incorporates conventional 8-inch deep offset blocks.

The test reported herein corresponds to American Association of State Highway and Transportation Officials (AASHTO) *Manual for Assessing Safety Hardware* (*MASH*) test 3-10. This is primarily a severity test that assesses risk of injury to the vehicle occupants. This test was considered to be the more critical of the two tests due to the potential for increased vehicle-post interaction resulting from decreasing the depth of the offset blocks from 12 inches to 8 inches. The 31-inch W-beam guardrail with standard offset blocks met all required *MASH* performance criteria for test 3-10.

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DISCLAIMER

This research was performed in cooperation with the Texas Department of Transportation (TxDOT) and the Federal Highway Administration (FHWA). The contents of this report reflect the views of the authors, who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official view or policies of the FHWA or TxDOT. This report does not constitute a standard, specification, or regulation, and its contents are not intended for construction, bidding, or permit purposes. In addition, the above listed agencies assume no liability for its contents or use thereof. The United States Government and the State of Texas do not endorse products or manufacturers. Trade or manufacturers' names appear herein solely because they are considered essential to the object of this report. The engineer in charge of the project was Roger P. Bligh, P.E. (Texas, #78550).

TTI PROVING GROUND DISCLAIMER

The results of the crash testing reported herein apply only to the article being tested.

ACCREDITED
ISO 17025 Laboratory
Testing Certificate # 2821.01

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TABLE OF CONTENTS

Page

LIST OF FIGURES ix

LIST OF TABLES x

CHAPTER 1. INTRODUCTION 1
1.1 INTRODUCTION 1
1.2 BACKGROUND 1
1.3 OBJECTIVES/SCOPE OF RESEARCH 4

CHAPTER 2. SYSTEM DETAILS 7
2.1 TEST ARTICLE DESIGN AND CONSTRUCTION 7
2.2 MATERIAL SPECIFICATIONS 7
2.3 SOIL CONDITIONS 7

CHAPTER 3. TEST REQUIREMENTS AND EVALUATION CRITERIA 11
3.1 CRASH TEST MATRIX 11
3.2 EVALUATION CRITERIA 11

CHAPTER 4. CRASH TEST PROCEDURES 13
4.1 TEST FACILITY 13
4.2 VEHICLE TOW AND GUIDANCE PROCEDURES 13
4.3 DATA ACQUISITION SYSTEMS 13
4.3.1 Vehicle Instrumentation and Data Processing 13
4.3.2 Anthropomorphic Dummy Instrumentation 14
4.3.3 Photographic Instrumentation and Data Processing 14

CHAPTER 5. CRASH TEST RESULTS 15
5.1 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS 15
5.2 TEST VEHICLE 15
5.3 WEATHER CONDITIONS 15
5.4 TEST DESCRIPTION 15
5.5 DAMAGE TO TEST INSTALLATION 18
5.6 VEHICLE DAMAGE 18
5.7 OCCUPANT RISK FACTORS 18

CHAPTER 6. SUMMARY AND CONCLUSIONS 25
6.1 ASSESSMENT OF TEST RESULTS 25
6.1.1 Structural Adequacy 25
6.1.2 Occupant Risk 25
6.1.3 Vehicle Trajectory 26
6.2 CONCLUSIONS 26

TABLE OF CONTENTS (CONTINUED)

Page

CHAPTER 7. IMPLEMENTATION STATEMENT 29

REFERENCES 31

APPENDIX A. DETAILS OF THE TEST ARTICLE 33

APPENDIX B. CERTIFICATION DOCUMENTATION 41

APPENDIX C. SOIL STRENGTH DOCUMENTATION 47

APPENDIX D. TEST VEHICLE PROPERTIES AND INFORMATION 49

APPENDIX E. SEQUENTIAL PHOTOGRAPHS 53

APPENDIX F. VEHICLE ANGULAR DISPLACEMENTS AND ACCELERATIONS 57

LIST OF FIGURES

Figure **Page**

Figure 1.1. Typical Cross Section of the Texas Metal Beam Guard Fence. 3
Figure 2.1. Details of the TxDOT 31-inch Guardrail Installation. 9
Figure 2.2. Test Article/Installation before Test No. 420020-5. 10
Figure 5.1. Vehicle/Installation Geometrics for Test No. 420020-5. 16
Figure 5.2. Vehicle before Test No. 420020-5. 17
Figure 5.3. Position of the Vehicle after Test No. 420020-5. 19
Figure 5.4. Installation after Test No. 420020-5. 20
Figure 5.5. Vehicle after Test No. 420020-5. 21
Figure 5.6. Interior of Vehicle for Test No. 420020-5. 22
Figure 5.7. Summary of Results for *MASH* Test 3-10 on the TxDOT 31-inch W-Beam Guardrail. 23
Figure C1. Test Day Static Soil Strength Documentation. 47
Figure C2. Summary of Strong Soil Test Results for Establishing Installation Procedure. 48
Figure D1. Vehicle Properties for Test No. 420020-5. 49
Figure E1. Sequential Photographs for Test No. 420020-5 (Overhead and Frontal Views). 53
Figure E2. Sequential Photographs for Test No. 420020-5 (Rear View). 55
Figure F1. Vehicle Angular Displacements for Test No. 420020-5. 57
Figure F2. Vehicle Longitudinal Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity). 58
Figure F3. Vehicle Lateral Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity). 59
Figure F4. Vehicle Vertical Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity). 60
Figure F5. Vehicle Longitudinal Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle). 61
Figure F6. Vehicle Lateral Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle). 62
Figure F7. Vehicle Vertical Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle). 63

LIST OF TABLES

Table **Page**

Table 1.1. Summary of *MASH* Crash Tests Performed on Non-Proprietary Strong Post W-Beam Guardrail. 4
Table 6.1. Performance Evaluation Summary for *MASH* Test 3-10 on the TxDOT 31-inch W-Beam Guardrail. 27
Table D1. Exterior Crush Measurements for Test No. 420020-5. 50
Table D2. Occupant Compartment Measurements for Test No. 420020-5. 51

CHAPTER 1. INTRODUCTION

1.1 INTRODUCTION

This project was set up to provide Texas Department of Transportation (TxDOT) with a mechanism to quickly and effectively evaluate high priority issues related to roadside safety devices. Roadside safety devices shield motorists from roadside hazards such as non-traversable terrain and fixed objects. To maintain the desired level of safety for the motoring public, these safety devices must be designed to accommodate a variety of site conditions, placement locations, and a changing vehicle fleet. Periodically, there is a need to assess the compliance of existing safety devices with current vehicle testing criteria.

Under this project, roadside safety issues are identified and prioritized for investigation. Each roadside safety issue is addressed with a separate work plan, and the results are summarized in an individual test report.

1.2 BACKGROUND

The American Association of State Highway and Transportation Officials (AASHTO) (2002) *Roadside Design Guide* defines a guardrail as "a longitudinal barrier used to shield motorists from natural or man-made obstacles located along either side of a traveled way." Guardrail can be generally classified as weak post and strong post systems. Weak post systems are more flexible and have greater dynamic deflection than strong post systems. The weak posts serve primarily to support the rail elements at their proper elevation for contact with an impacting vehicle. The posts are readily detached from the rail element(s) and dissipate little energy as they yield to the impacting vehicle and are pushed to the ground.

In contrast, strong post barriers incorporate larger, stronger posts that absorb significant energy as they rotate through the soil during an impact. The increased post stiffness results in reduced dynamic deflection and increased vehicular deceleration rates. Spacer blocks are used to offset the rail element from the posts to minimize vehicle snagging on the posts. Severe vehicle-post interaction can impart high decelerations to the vehicle and lead to vehicle instability. Strong post systems are more widely used across the country due to their lower deflection and reduced maintenance requirements.

In the mid-1990s, Texas Transportation Institute (TTI) researchers conducted full-scale crash tests of all commonly used guardrail systems in accordance with National Cooperative Highway Research Program (NCHRP) *Report 350* Test 3-11 (*1*) under a pooled fund study administered by Federal Highway Administration (FHWA) (*2*). It was under this testing program that performance issues associated with light trucks impacting the standard strong steel-post W-beam guardrail system, G4(1S), were first identified. Snagging of the pickup truck's wheels on the steel support posts was aggravated by the collapse of the W6×9 steel offset blocks, and precipitated rollover of the truck as it exited the barrier. Subsequent testing demonstrated that a modified G4(1S) system that incorporates 8-inch deep wood or structural plastic offset blocks between the W-beam rail element and W6×9 steel posts in lieu of the original W6×9 steel offset block was able to accommodate the 3/4-ton, 2-door, pickup truck design vehicle (denoted 2000P) and comply with *NCHRP Report 350* guidelines *(3,4,5)*.

The strong wood-post W-beam guardrail system, G4(2W), which utilizes 6-inch × 8-inch wood posts and offset blocks, contained and redirected the 2000P pickup (*2*). However, instability of the pickup truck resulted in the test being classified as marginally acceptable.

Both of these strong-post W-beam guardrail systems are national standards and form the basis for TxDOT's current guard fence designs. Figure 1.1 shows a cross section of a typical TxDOT guard fence. The guard fence is constructed with 12-gauge, W-beam rail mounted at a height of 21 inches to the center on 6-ft long W6×9 steel, 7-inch diameter wood, or 6-inch × 8-inch wood posts spaced at 6 ft-3 inches. The 8-inch deep offset blocks inserted between the rail and posts may be fabricated from wood or an approved alternative.

Recent testing under the new 2009 AASHTO *Manual for Assessing Safety Hardware* (*MASH*) (*6*) has demonstrated that these strong-post W-beam guardrail systems are at or near their performance limits. Under NCHRP Projects 22-14(02) and 22-14(03), a series of crash tests were performed to assess the impact performance of commonly used barrier systems when impacted by the new 1/2-ton, four-door, pickup truck design vehicle (designated 2270P) under the AASHTO *MASH* guidelines. The increase in the weight of the new pickup truck from approximately 4400 lb to 5000 lb (2000 kg to 2270 kg) increases the impact severity of the structural adequacy test (Test 3-11) for longitudinal barriers by 13 percent. Table 1.1 shows a summary of these barrier tests.

A 27 5/8-inch tall, modified G4(1S) steel post W-beam guardrail failed due to rail rupture when impacted by a 5000-lb, 3/4-ton pickup truck. In a subsequent test of the same system with the 5000-lb, 1/2-ton, 4-door *MASH* pickup truck, the guardrail successfully contained and redirected the vehicle (*7*). However, the rail had a vertical tear through approximately half of its cross section, indicating that the modified G4(1S) guardrail is at its performance limits with no factor of safety. In a test of the G4(2W) wood post W-beam guardrail, the rail ruptured and failed to contain the heavier *MASH* pickup truck.

The implications of these tests are being examined by FHWA and AASHTO. Several states are considering or have already implemented the use of alternate strong-post guardrail systems that offer enhanced containment capacity. As an example, a modified guardrail design known as the Midwest Guardrail System (MGS) (*8*) has successfully met the *MASH* guidelines and has been shown to have additional capacity or factor of safety beyond the design impact conditions. The MGS guardrail increases the W-beam rail height from 27 inches to 31 inches, increases the depth of the offset blocks between the rail and posts from 8 inches to 12 inches, and moves the rail splice locations from the posts to mid-span between posts. There are also several proprietary guardrail systems (Gregory GMS, Nucore Nu-Guard, and Trinity T-31) that have successfully met the new *MASH* impact performance guidelines.

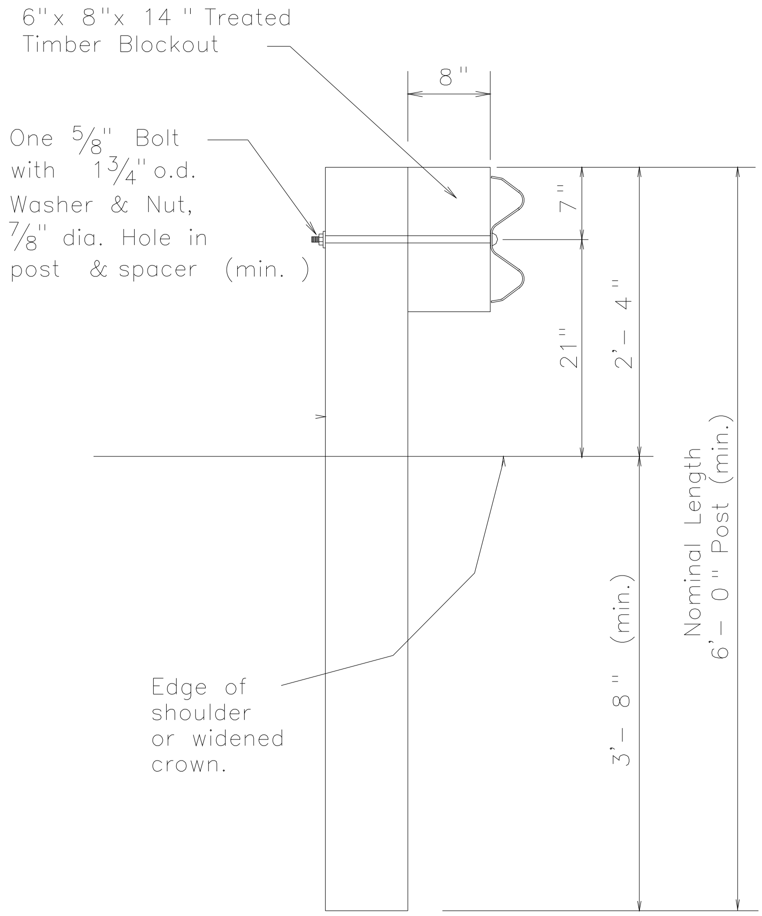


Figure 1.1. Typical Cross Section of the Texas Metal Beam Guard Fence.

On May 17, 2010, FHWA issued a technical memorandum to provide guidance to State DOTs on height of guardrail for new installations on the National Highway System (NHS) (*9*). The memorandum discusses performance issues with the modified G4(1S) guardrail and details the minimum mounting heights of steel post guardrail systems successfully crash tested under both *NCHRP Report 350* and *MASH*. In regard to *NCHRP Report 350*, it states that transportation agencies should ensure the minimum height of newly-installed modified G4(1S) W-beam guardrail is at least 27 3/4 inches to the top of the rail, including construction tolerance. A nominal installation height of 29 inches, ±1 inch, may be specified and is considered acceptable for use on the NHS.

In regard to *MASH*, the memorandum recognizes performance issues with modified G4(1S) guardrail and recommends that transportation agencies consider adopting generic or proprietary 31-inch high guardrail designs (instead of the modified G4(1S) system) as standard for all new installations. It states that these systems have met *MASH* criteria and offer improved crash-test performance and increased capacity to safely contain and redirect higher center-of-gravity vehicles such as pickup trucks and SUVs.

Table 1.1. Summary of *MASH* Crash Tests Performed on Non-Proprietary Strong Post W-Beam Guardrail.

Agency Test No.	Test Designation	Test Article	Vehicle Make and Model	Vehicle Mass (lb)	Impact Speed (mph)	Impact Angle (deg)	PASS/ FAIL
2214WB-1[a]	3-11	Modified G4(1S) Guardrail	2002 GMC 2500 3/4-ton Pickup	5000	61.1	25.6	FAIL[c]
2214WB-2[a]	3-11	Modified G4(1S) Guardrail	2002 Dodge Ram 1500 Quad Cab Pickup	5000	62.4	26.0	PASS[d]
2214MG-1[a]	3-11	Midwest Guardrail System (MGS)	2002 GMC 2500 3/4-ton Pickup	5000	62.6	25.2	PASS
2214MG-2[a]	3-11	MGS	2002 Dodge Ram 1500 Quad Cab Pickup	5000	62.8	25.5	PASS
2214MG-3[a]	3-10	MGS (Max. Height)	2002 Kia Rio	2588	60.8	25.4	PASS
476460-1-5[b]	3-11	G4(2W) W-Beam Guardrail	2007 Chevrolet Silverado Pickup	5009	64.4	26.1	FAIL[c]

a) Test performed at University of Nebraska under NCHRP Project 22-14(2)
b) Test performed at TTI under NCHRP Project 22-14(3)
c) Rail ruptured
d) Rail tore through half its cross section

TxDOT initiated a review of their guardrail standards based on the outcome of these recent studies and the FHWA technical memorandum. TxDOT expressed interest in the use of a generic 31-inch tall guardrail to provide enhanced containment capacity for light trucks. However, some concerns were noted regarding the size of the blockout used in the MGS and the practical aspects of using it on new guardrail installations in Texas. The larger offset block will be more expensive and require more space than the offset blocks currently in use. Ideally, TxDOT desired a crashworthy guardrail system that meets *MASH* evaluation criteria, has improved containment capacity for larger passenger vehicles than the modified G4(1S), and incorporates a conventional 8-inch deep offset block.

1.3 OBJECTIVES/SCOPE OF RESEARCH

The objective of this test was to evaluate the performance of a 31-inch tall W-beam guardrail with standard offset blocks according to the *MASH* standards for Test Level 3 (TL-3) longitudinal barriers. The test performed was *MASH* test 3-10 involving a 1100C (2420 lb) vehicle impacting the critical impact point (CIP) of the length of need (LON) of the guardrail at a nominal impact speed and angle of 62 mi/h and 25 degrees, respectively. This test was selected to investigate vehicle-barrier interaction to determine if a small passenger car can be successfully contained and redirected without excessive deceleration or unacceptable occupant compartment deformation.

Reported herein are the details of the 31-inch tall W-beam guardrail with standard offset blocks, test conditions, description of the test performed, assessment of test results, and implementation recommendations.

CHAPTER 2. SYSTEM DETAILS

2.1 TEST ARTICLE DESIGN AND CONSTRUCTION

The guardrail incorporates a standard 12-gauge corrugated W-beam rail section mounted at a height of 31 inches on 6-ft long, W6×8.5 steel posts. The posts were spaced on 6 ft-3 inch centers and embedded 40 inches in a compacted road base material. The rail was offset from the posts using 6-inch wide × 8-inch deep × 14-inch long routed wood offset blocks. The rail was attached to the blockout and post using a single 5/8-inch diameter × 10-inch long button head bolt. The rail splices were located midspan between posts.

The length of the W-beam guardrail section was 106.25 ft. A 37.5 ft, steel post ET-PLUS end treatment was attached to each end, making the overall length of the installation 181.25 ft.

Figure 2.1 shows details of the 31-inch W-beam guardrail with standard offset blocks. Figure 2.2 shows photographs of the completed test installation. Appendix A presents detailed drawings of the bridge rail.

2.2 MATERIAL SPECIFICATIONS

The W-beam guardrail conformed to AASHTO M 180, Standard Specification for Corrugated Sheet Steel Beams for Highway Guardrail. The W6×8.5 steel guardrail posts complied with American Society for Testing and Materials (ASTM) A36. The routed wood offset blocks were Grade 1 southern yellow pine. The guardrail post bolts and rail splice bolts complied with ASTM A307 and were galvanized in accordance with ASTM A153. The nuts complied with ASTM A563 and were galvanized in accordance with ASTM A153.

Appendix B contains mill certification sheets and other certification documents for the materials used in the 31-inch W-beam guardrail installation.

2.3 SOIL CONDITIONS

The guardrail and end treatment posts were installed in soil meeting AASHTO standard specifications for "Materials for Aggregate and Soil Aggregate Subbase, Base and Surface Courses," designated M147-65(2004), grading B. In accordance with Appendix B of *MASH*, soil strength was measured the day of the crash test (see Appendix C, Figure C1). During construction of the guardrail installation for the full-scale crash test, two W6×16 posts were installed in the immediate vicinity of the guardrail, utilizing the same fill materials and installation procedures followed for the guardrail system and used in the reference tests (see Appendix C, Figure C2).

As determined from the reference tests shown in Appendix C, Figure C2, the minimum static post load required for deflections of 5 inches, 10 inches, and 15 inches, measured at a height of 25 inches, is 3940 lb, 5500 lb, and 6540 lb, respectively (90 percent of static load for the initial reference installation). On the day of the test, April 14, 2009, load on the test post at deflections of 5 inches, 10 inches, and 15 inches was 7182 lbf, 8484 lbf, and 9424 lbf, respectively, as shown in Appendix C, Figure C1. The strength of the backfill material met minimum requirements.

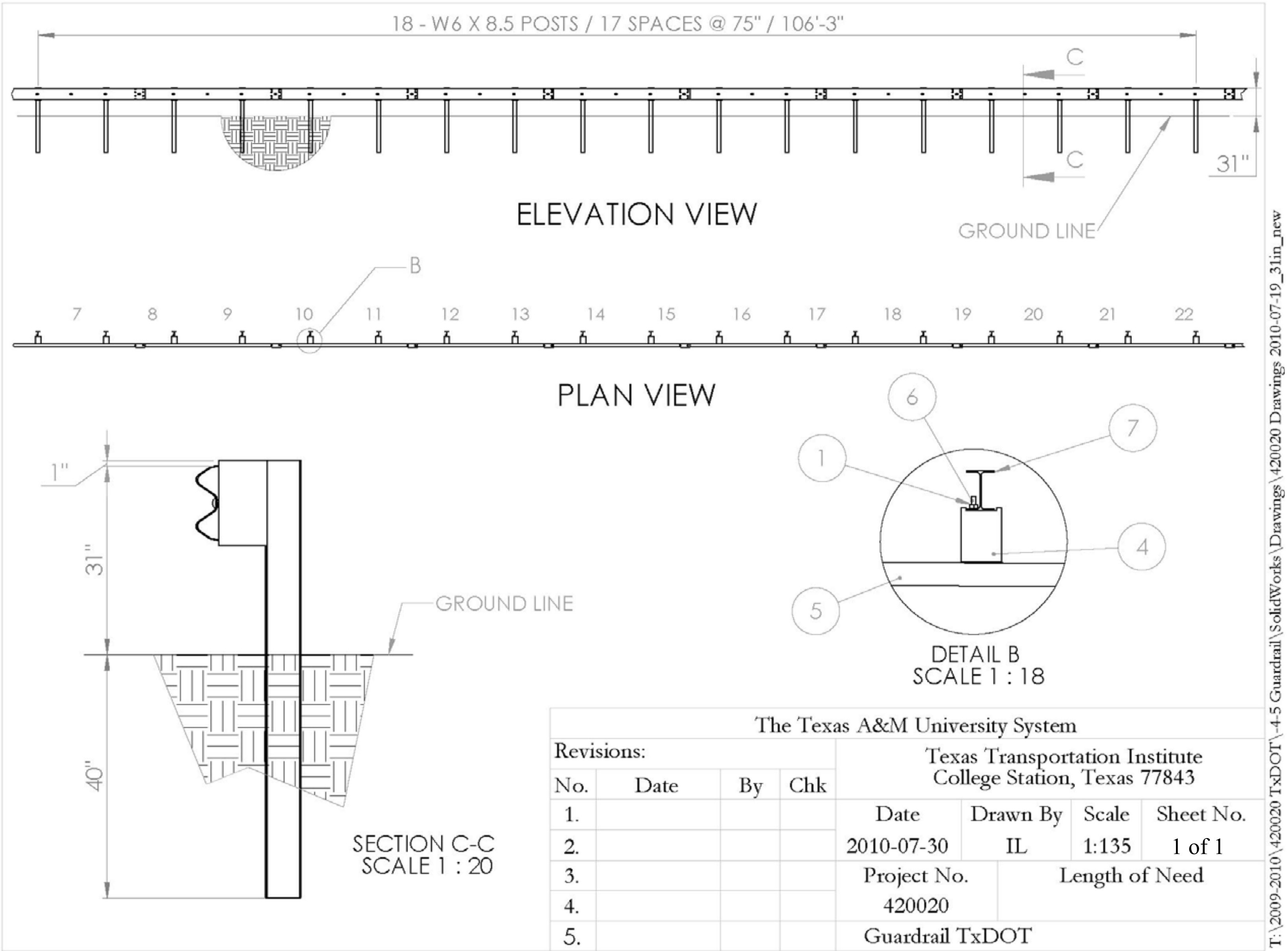


Figure 2.1. Details of the TxDOT 31-inch Guardrail Installation.

Figure 2.2. Test Article/Installation before Test No. 420020-5.

CHAPTER 3. TEST REQUIREMENTS AND EVALUATION CRITERIA

3.1 CRASH TEST MATRIX

Two tests are recommended to evaluate longitudinal barriers to TL-3 in accordance with *MASH*. Details of these tests are described below.

> ***MASH* test 3-10**: An 1100C (2425 lb) vehicle impacting the critical impact point (CIP) of the length of need (LON) of the barrier at a nominal impact speed and angle of 62 mi/h and 25 degrees, respectively. This test investigates a barrier's ability to contain and redirect a small passenger vehicle.

> ***MASH* test 3-11**: A 2270P (5000 lb) vehicle impacting the CIP of the LON of the barrier at a nominal impact speed and angle of 62 mi/h and 25 degrees, respectively. This is a strength test to verify a barrier's capacity for containing light trucks in a stable manner.

The test reported herein corresponds to MASH test 3-10. The CIP was determined to be 9 ft upstream of a post using Figure 2-8 in *MASH*. The target impact point was thus selected to be 9 ft upstream of post 14 or 33 inches upstream of post 13.

The crash test and data analysis procedures were in accordance with guidelines presented in *MASH*. Chapter 4 presents brief descriptions of these procedures.

3.2 EVALUATION CRITERIA

The crash test was evaluated in accordance with the criteria presented in *MASH*. The performance of the guardrail is judged on the basis of three factors: structural adequacy, occupant risk, and post impact vehicle trajectory. Structural adequacy is judged upon the guardrail's ability to contain and redirect the vehicle, or bring the vehicle to a controlled stop in a predictable manner. Occupant risk criteria evaluates the potential risk of hazard to occupants in the impacting vehicle, and to some extent other traffic, pedestrians, or workers in construction zones, if applicable. Post impact vehicle trajectory is assessed to determine potential for secondary impact with other vehicles or fixed objects, creating further risk of injury to occupants of the impacting vehicle and/or risk of injury to occupants in other vehicles. The appropriate safety evaluation criteria from table 5-1 of *MASH* were used to evaluate the crash test. These criteria are listed in further detail under the assessment of the crash test.

CHAPTER 4. CRASH TEST PROCEDURES

4.1 TEST FACILITY

The full-scale crash test reported herein was performed at Texas Transportation Institute (TTI) Proving Ground. TTI Proving Ground is an International Standards Organization (ISO) 17025 accredited laboratory with American Association for Laboratory Accreditation (A2LA) Mechanical Testing certificate 2821.01. The full-scale crash test was performed according to TTI Proving Ground quality procedures and according to the *MASH* guidelines and standards.

The Texas Transportation Institute Proving Ground is a 2000-acre complex of research and training facilities located 10 miles northwest of the main campus of Texas A&M University. The site, formerly an Air Force base, has large expanses of concrete runways and parking aprons well suited for experimental research and testing in the areas of vehicle performance and handling, vehicle-roadway interaction, durability and efficacy of highway pavements, and safety evaluation of roadside safety hardware. The site selected for construction and testing of the TxDOT guardrail evaluated under this project is along the edge of an out-of-service apron. The apron consists of an unreinforced jointed-concrete pavement in 12.5 ft by 15 ft blocks nominally 8 to 12 inches deep. The apron is over 50 years old, and the joints have some displacement, but are otherwise flat and level.

4.2 VEHICLE TOW AND GUIDANCE PROCEDURES

The test vehicle was towed into the test installation using a steel cable guidance and reverse tow system. A steel cable for guiding the test vehicle was tensioned along the path, anchored at each end, and threaded through an attachment to the front wheel of the test vehicle. An additional steel cable was connected to the test vehicle, passed around a pulley near the impact point, through a pulley on the tow vehicle, and then anchored to the ground such that the tow vehicle moved away from the test site. A two-to-one speed ratio between the test and tow vehicle existed with this system. Just prior to impact with the installation, the test vehicle was released to be free-wheeling and unrestrained. The vehicle remained free-wheeling, i.e., no steering or braking inputs, until the vehicle cleared the immediate area of the test site, at which time brakes on the vehicle were activated to bring it to a safe and controlled stop.

4.3 DATA ACQUISITION SYSTEMS

4.3.1 Vehicle Instrumentation and Data Processing

The test vehicle was instrumented with a self-contained, on-board data acquisition system. The signal conditioning and acquisition system is a 16-channel, Tiny Data Acquisition System (TDAS) Pro produced by Diversified Technical Systems, Inc. The accelerometers that measure the x, y, and z axis of vehicle acceleration are strain gauge type with linear millivolt output proportional to acceleration. Accelerometer data are measured with an expanded uncertainty of ±1.7 percent at a confidence factor of 95 percent (k=2). Angular rate sensors, measuring vehicle roll, pitch, and yaw rates, are ultra small size, solid state units designs for crash test service. Rate of rotation data is measured with an expanded uncertainty of 0.7 percent at a confidence factor of 95 percent (k=2).

The TDAS Pro hardware and software conform to the latest Society of Automotive Engineers (SAE) J211, Instrumentation for Impact Test. Each of the 16 channels is capable of providing precision amplification, scaling, and filtering based on transducer specifications and calibrations. During the test, data are recorded from each channel at a rate of 10,000 values per second with a resolution of one part in 65,536. Once recorded, the data are backed up inside the unit by internal batteries to prevent data loss should the primary battery cable be severed. Initial contact of the pressure switch on the vehicle bumper provides a time zero mark and initiates the recording process. After each test, the data are downloaded from the TDAS Pro unit into a laptop computer at the test site. The raw data are then processed by the Test Risk Assessment Program (TRAP) software to produce detailed reports of the test results. Each of the TDAS Pro units is returned to the factory annually for complete recalibration. Accelerometers and rate transducers are also calibrated annually with traceability to the National Institute for Standards and Technology.

TRAP uses the data from the TDAS Pro to compute occupant/compartment impact velocities, time of occupant/compartment impact after vehicle impact, and the highest 10-millisecond (ms) average ridedown acceleration. In addition, maximum average accelerations over 50-ms intervals in each of the three directions are computed. For reporting purposes, the acceleration versus time curves for the longitudinal, lateral, and vertical directions are plotted using a 60-Hz digital filter.

TRAP uses the data from the yaw, pitch, and roll rate transducers to compute angular displacement in degrees at 0.0001-s intervals and then plots yaw, pitch, and roll angles versus time. These displacements are in reference to the vehicle-fixed coordinate system with the initial position and orientation of the vehicle-fixed coordinate systems being initial impact.

4.3.2 Anthropomorphic Dummy Instrumentation

An Alderson Research Laboratories Hybrid II, 50th percentile male anthropomorphic dummy, restrained with lap and shoulder belts, was placed in the driver's position of the 1100C vehicle. The dummy was uninstrumented.

4.3.3 Photographic Instrumentation and Data Processing

Photographic coverage of the test included three high-speed cameras: one overhead with a field of view perpendicular to the ground and directly over the impact point; one placed behind the installation at an angle; and a third placed to have a field of view parallel to and aligned with the installation at the downstream end. A flashbulb activated by pressure-sensitive tape switches was positioned on the impacting vehicle to indicate the instant of contact with the installation and was visible from each camera. The films from these high-speed cameras were analyzed on a computer-linked motion analyzer to observe phenomena occurring during the collision and to obtain time-event, displacement, and angular data. A mini-DV camera and still cameras recorded and documented conditions of the test vehicle and installation before and after the test.

CHAPTER 5. CRASH TEST RESULTS

5.1 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

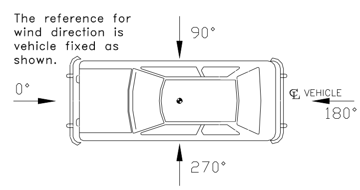
MASH test 3-10 involves an 1100C vehicle weighing 2420 lb ±55 lb impacting the test article at an impact speed of 62.2 mi/h ±2.5 mi/h and an angle of 25 degrees ±1.5 degrees. The target impact point was 33 inches upstream of post 13, near the splice between posts 12 and 13. The 2003 Kia Rio used in the test weighed 2435 lb and the actual impact speed and angle were 60.4 mi/h and 25.6 degrees, respectively. The actual impact point was 38.0 inches upstream of post 13. Impact severity was calculated at 1778 kip-ft, or 0.4 percent below target.

5.2 TEST VEHICLE

A 2003 Kia Rio, shown in Figures 5.1 and 5.2, was used for the crash test. Test inertia weight of the vehicle was 2435 lb, and its gross static weight was 2609 lb. The height to the lower edge of the vehicle bumper was 8.5 inches, and the height to the upper edge of the bumper was 22.75 inches. Figure D1 in Appendix D gives additional dimensions and information on the vehicle. The vehicle was directed into the installation using the cable reverse tow and guidance system, and was released to be free-wheeling and unrestrained just prior to impact.

5.3 WEATHER CONDITIONS

The test was performed on the morning of August 26, 2010. Rainfall recorded prior to the test was 0.38 inches 10 days prior to the test date. Weather conditions at the time of testing were as follows: Wind speed: 7 mi/h; wind direction: 80 degrees with respect to the vehicle (vehicle was traveling in a northwesterly direction); temperature: 89°F, relative humidity: 45 percent.



5.4 TEST DESCRIPTION

The 2003 Kia Rio, traveling at an impact speed of 60.4 mi/h, impacted the 31-inch W-beam guardrail with standard offset blocks 38 inches upstream of post 13 at an impact angle of 25.6 degrees. At approximately 0.015 s after impact, the W-beam rail element began to deflect toward the field side, and at 0.029 s, post 13 began to deflect toward the field side. The left front corner of the bumper of the vehicle contacted post 13 at 0.032 s, and the tire contacted post 13 at 0.039 s. Post 14 began to deflect toward the field side at 0.042 s. At 0.069 s, the vehicle began to redirect, and at 0.076 s, post 15 began to deflect toward field side. The left front corner of the vehicle contacted post 14 at 0.101 s, and post 16 began to deflect toward the field side at 0.179 s. At 0.199 s, the left front corner of the vehicle contacted post 15, and at 0.295 s, the left front corner of the vehicle contacted post 16. The vehicle became parallel with the guardrail at 0.327 s and was traveling at a speed of 37.3 mi/h. At 0.814 s, the vehicle lost contact with the guardrail and was traveling at an exit speed and angle of 29.2 mi/h and 15.0 degrees, respectively. Brakes on the vehicle were applied at 3.5 s, and the vehicle subsequently came to rest 185 ft downstream of impact and 47 ft from the traffic face of the rail toward traffic lanes. Figure E2 and Figure E3 in Appendix E show sequential photographs of the test period.

Figure 5.1. Vehicle/Installation Geometrics for Test No. 420020-5.

Figure 5.2. Vehicle before Test No. 420020-5.

5.5 DAMAGE TO TEST INSTALLATION

Damage to the test installation is shown in Figures 5.3 and 5.4. Post 1 was pulled downstream 0.5 inches at ground level, and post 12 was pushed toward the field side 0.25 inches at ground level. Post 13 was leaning downstream and toward the field side 25 degrees, and there were tire marks on the traffic side flange of the post. Posts 14 and 15 were leaning downstream 80 degrees, and post 16 was leaning downstream 30 degrees. Post 30 was pulled upstream 0.25 inches. The W-beam rail element was separated from posts 13 through 17, and the bolt hole at post 2 was torn. Working width was 2.38 ft. Maximum dynamic deflection of the W-beam rail element during the test was 2.38 ft, and maximum permanent deformation was 1.58 ft.

5.6 VEHICLE DAMAGE

The left front and left side of the 1100C vehicle were damaged as shown in Figures 5.5. The left front strut, left front strut tower, left front lower ball joint, left front lower ball joint, left front outer tie rod end, and left inner and outer CV joints were damaged. Also damaged were the front bumper, hood, grill, radiator and radiator support, left front fender, left front door, and left rear door. The left front tire and wheel rim were damaged and the windshield sustained stress cracking from the left lower corner. Maximum exterior crush to the vehicle was 12.5 inches in the side plane at the left front corner at bumper height. No occupant compartment deformation was noted. Figure 5.6 shows photographs of the interior of the vehicle. Exterior crush measurements and occupant compartment measures are provided in Appendix D, Tables D1 and D2.

5.7 OCCUPANT RISK FACTORS

Data from the accelerometer located at the vehicle center of gravity were digitized for evaluation of occupant risk. In the longitudinal direction, the occupant impact velocity was 21.0 ft/s at 0.130 s, the highest 0.010-s occupant ridedown acceleration was 8.8 Gs from 0.188 to 0.198 s, and the maximum 0.050-s average acceleration was −6.8 Gs between 0.058 and 0.108 s. In the lateral direction, the occupant impact velocity was 17.4 ft/s at 0.130 s, the highest 0.010-s occupant ridedown acceleration was 6.8 Gs from 0.162 to 0.172 s, and the maximum 0.050-s average was 5.6 Gs between 0.067 and 0.117 s. Theoretical Head Impact Velocity (THIV) was 29.2 km/h or 8.1 m/s at 0.126 s; Post-Impact Head Decelerations (PHD) was 10.1 Gs between 0.188 and 0.198 s; and Acceleration Severity Index (ASI) was 0.82 between 0.064 and 0.114 s. Figure 5.7 summarizes these data and other pertinent information from the test. Vehicle angular displacements and accelerations versus time traces are presented in Appendix F, Figures F3 through F9.

Figure 5.3. Position of the Vehicle after Test No. 420020-5.

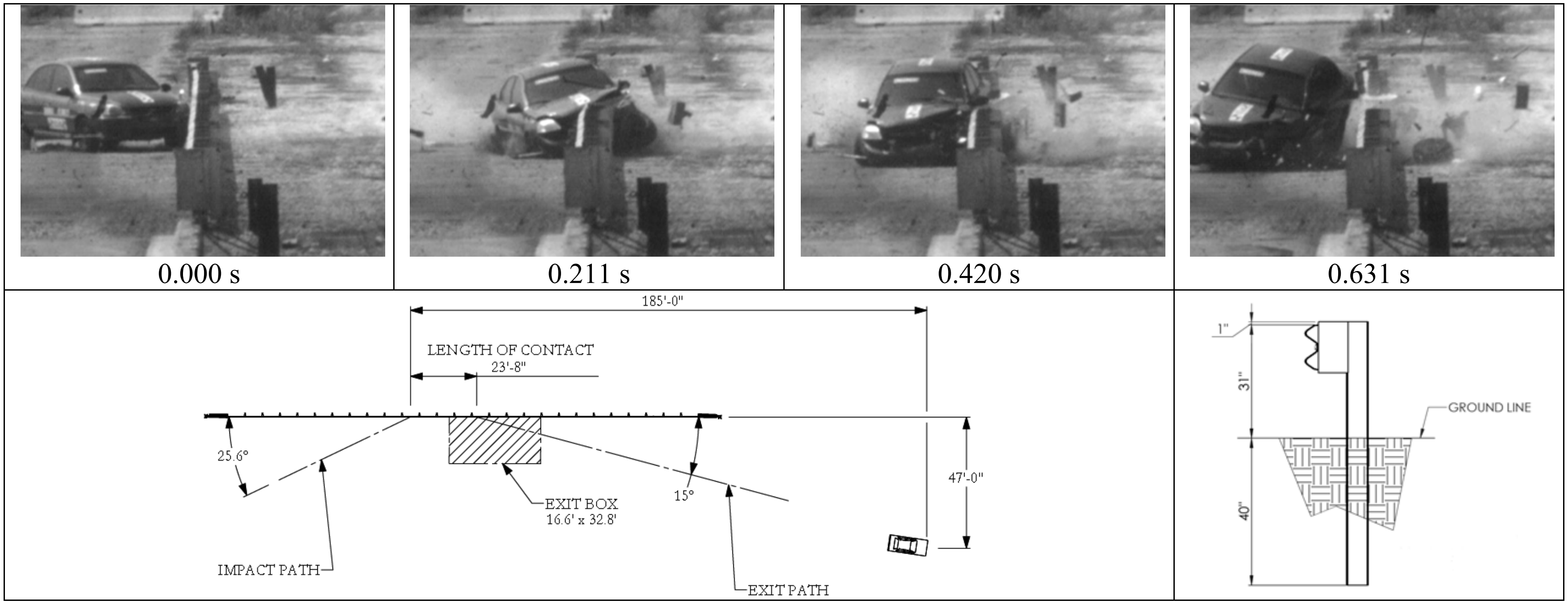
Figure 5.4. Installation after Test No. 420020-5.

Figure 5.5. Vehicle after Test No. 420020-5.

Before Test

After Test

Figure 5.6. Interior of Vehicle for Test No. 420020-5.



General Information	
Test Agency	Texas Transportation Institute (TTI)
Test Standard Test No.	MASH Test 3-10
TTI Test No.	420020-5
Date	2010-08-26
Test Article	
Type	Guardrail
Name	31-inch W-Beam Guardrail with standard offset blocks
Installation Length	181.25 ft
Material or Key Elements	12-ga. W-beam rail, 8-inch deep routed wood blockouts
Soil Type and Condition	Crushed Limestone, Dry
Test Vehicle	
Type/Designation	1100C
Make and Model	2003 Kia Rio
Curb	2387 lb
Test Inertial	2435 lb
Dummy	174 lb
Gross Static	2609 lb

Impact Conditions	
Speed	60.4 mi/h
Angle	25.6 degrees
Location/Orientation	38 inches upstrm
Exit Conditions	Post 13
Speed	29.2 mi/h
Angle	15.0 degrees
Occupant Risk Values	
Impact Velocity	
Longitudinal	21.0 ft/s
Lateral	17.4 ft/s
Ridedown Accelerations	
Longitudinal	8.8 G
Lateral	6.8 G
THIV	29.2 km/h
PHD	10.1 G
ASI	0.82
Max. 0.050-s Average	
Longitudinal	-6.8 G
Lateral	5.6 G
Vertical	-1.8 G

Post-Impact Trajectory	
Stopping Distance	185 ft dwnstrm 47 ft twd traffic
Vehicle Stability	
Maximum Yaw Angle	49 degrees
Maximum Pitch Angle	-11 degrees
Maximum Roll Angle	-16 degrees
Vehicle Snagging	No
Vehicle Pocketing	No
Test Article Deflections	
Dynamic	2.38 ft
Permanent	1.58 ft
Working Width	2.38 ft
Vehicle Damage	
VDS	11LFQ4
CDC	11LDEW3
Max. Exterior Deformation	12.5 inches
OCDI	LF0000000
Max. Occupant Compartment Deformation	0
Impact Severity	1778 kip-ft (-0.4%)

Figure 5.7. Summary of Results for *MASH* Test 3-10 on the TxDOT 31-inch W-Beam Guardrail.

CHAPTER 6. SUMMARY AND CONCLUSIONS

6.1 ASSESSMENT OF TEST RESULTS

An assessment of the test based on the applicable *MASH* safety evaluation criteria is provided below.

6.1.1 Structural Adequacy

A. Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.

Results: The 31-inch W-beam guardrail with standard offset blocks contained and redirected the 1100C vehicle. The vehicle did not penetrate, underride, or override the guardrail. Maximum dynamic deflection of the W-beam rail element during the test was 2.38 ft. (PASS)

6.1.2 Occupant Risk

D. Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.

Deformation of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH (roof ≤4.0 inches; windshield ≤3.0 inches; side windows = no shattering by test article structural member; wheel/foot well/toe pan ≤9.0 inches; forward of A-pillar ≤12.0 inches; front side door area above seat ≤9.0 inches; front side door below seat ≤12.0 inches; floor pan/transmission tunnel area ≤12.0 inches).

Results: The W-beam rail element detached from posts 13 through 17. However, the detached rail did not penetrate or show potential for penetrating the occupant compartment, nor to present hazard to others in the area. (PASS)

No occupant compartment deformation occurred. (PASS)

F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.

Results: The 1100C vehicle remained upright during and after the collision event. Maximum roll and pitch angles were −16 degrees and −1 degrees, respectively. (PASS)

H. Occupant impact velocities should satisfy the following:

Longitudinal and Lateral Occupant Impact Velocity

Preferred	*Maximum*
30 ft/s	*40 ft/s*

Results: Longitudinal occupant impact velocity was 21.0 ft/s, and lateral occupant impact velocity was 17.4 ft/s. (PASS)

I. Occupant ridedown accelerations should satisfy the following:

Longitudinal and Lateral Occupant Ridedown Accelerations

Preferred	*Maximum*
15.0 Gs	*20.49 Gs*

Results: Longitudinal ridedown acceleration was 8.8 G, and lateral ridedown acceleration was 6.8 G. (PASS)

6.1.3 Vehicle Trajectory

For redirective devices, the vehicle shall exit the barrier within the exit box.

Result: The 1100C vehicle exited within the exit box. (PASS)

6.2 CONCLUSIONS

The 31-inch W-beam guardrail with standard offset blocks performed acceptably for *MASH* test 3-10, as summarized in Table 6.1.

Table 6.1. Performance Evaluation Summary for *MASH* Test 3-10 on the TxDOT 31-inch W-Beam Guardrail.

Test Agency: Texas Transportation Institute | Test No.: 420020-5 | Test Date: 2010-08-26

MASH **Test 3-10 Evaluation Criteria**	***Test Results***	***Assessment***
Structural Adequacy *A. Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable*	The 31-inch W-beam guardrail with standard offset blocks contained and redirected the 1100C vehicle. The vehicle did not penetrate, underride, or override the guardrail. Maximum dynamic deflection of the W-beam rail element during the test was 2.38 ft.	Pass
Occupant Risk *D. Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.*	The W-beam rail element detached from posts 13 through 17. However, the detached rail did not penetrate or show potential for penetrating the occupant compartment, nor to present hazard to others in the area.	Pass
Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH.	No occupant compartment deformation occurred.	Pass
F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.	The 1100C vehicle remained upright during and after the collision event. Maximum roll and pitch angles were −16 degrees and −1 degrees, respectively.	Pass
H. Longitudinal and lateral occupant impact velocities should fall below the preferred value of 30 ft/s, or at least below the maximum allowable value of 40 ft/s.	Longitudinal occupant impact velocity was 21.0 ft/s, and lateral occupant impact velocity was 17.4 ft/s.	Pass
I. Longitudinal and lateral occupant ridedown accelerations should fall below the preferred value of 15.0 Gs, or at least below the maximum allowable value of 20.49 Gs.	Longitudinal ridedown acceleration was 8.8 G, and lateral ridedown acceleration was 6.8 G.	Pass
Vehicle Trajectory		
For redirective devices, the vehicle shall exit the barrier within the exit box.	The 1100C vehicle exited the barrier within the exit box.	Pass

CHAPTER 7. IMPLEMENTATION STATEMENT

TxDOT initiated a review of their guardrail standards based on the outcome of recent crash test results and an FHWA technical memorandum pertaining to guardrail height. TxDOT expressed interest in the use of a generic 31-inch tall guardrail to provide enhanced containment capacity for light trucks. However, some concerns were expressed regarding the increased size of the blockout used in the Midwest Guardrail System (MGS). Consequently, TxDOT requested an evaluation of a 31-inch tall guardrail system that incorporates conventional 8-inch deep offset blocks.

MASH recommends two tests to evaluate guardrail systems to TL-3. The tests have the same impact speed and angle, but use different vehicles. *MASH* test 3-10 uses a small passenger car weighing 2420 lb, while *MASH* test 3-11 uses a 5000-lb, 4-door pickup truck.

The test reported herein corresponds to *MASH* test 3-10. This is primarily a severity test that assesses risk of injury to the vehicle occupants. This test was considered to be the more critical of the two tests due to the potential for increased vehicle-post interaction resulting from decreasing the depth of the offset blocks from 12 inches to 8 inches. The 31-inch W-beam guardrail with standard offset blocks met all required *MASH* performance criteria for test 3-10.

There currently is no implementation date for adopting *MASH*. TTI researchers recommend running test 3-11 to complete the *MASH* test matrix if TxDOT desires to adopt a *MASH* compliant 31-inch tall guardrail with standard offset blocks. If the impact performance in both tests is comparable to the impact performance of the MGS, it will provide enhanced justification to use other tested variations of the MGS with standard blockouts as well.

REFERENCES

1. Ross, Jr., H.E., Sicking, D.L., Zimmer, R.A. and Michie, J.D., "Recommended Procedures for the Safety Performance Evaluation of Highway Features," National Cooperative Highway Research Program *Report 350*, Transportation Research Board, National Research Council, Washington, D.C., 1993.

2. Mak, K. K., Bligh, R. P., and Menges, W. L., "Testing of State Roadside Safety Systems—Volume XI: Appendix J—Crash Testing and Evaluation of Existing Guardrail Systems," Research Report No. FHWA-RD-98-046, Federal Highway Administration, Washington, D.C., 1998.

3. Bullard, Jr., D.L., Menges, W.L., and Alberson, D.C., "NCHRP Report 350 Compliance Test 3-11 of the Modified G4(1S) Guardrail with Timber Blockouts," Research Report No. FHWA-RD-96-175, Federal Highway Administration, Washington, D.C., 1996.

4. Bligh, R.P. and Menges, W.L., "Testing and Evaluation of a Modified Steel Post W-Beam Guardrail with Recycled Polyethylene Blockouts," Research Report 400001-MPT, Texas Transportation Institute, College Station, TX, February 1997.

5. Alberson, D.C., Menges, W.L., and Schoeneman, S.K., "NCHRP Report 350 Test 3-11 of the Strong Post W-Beam Guardrail with Trinity Composite Blockout," Research Report 400001-TRB3, Texas Transportation Institute, College Station, TX, May 2001.

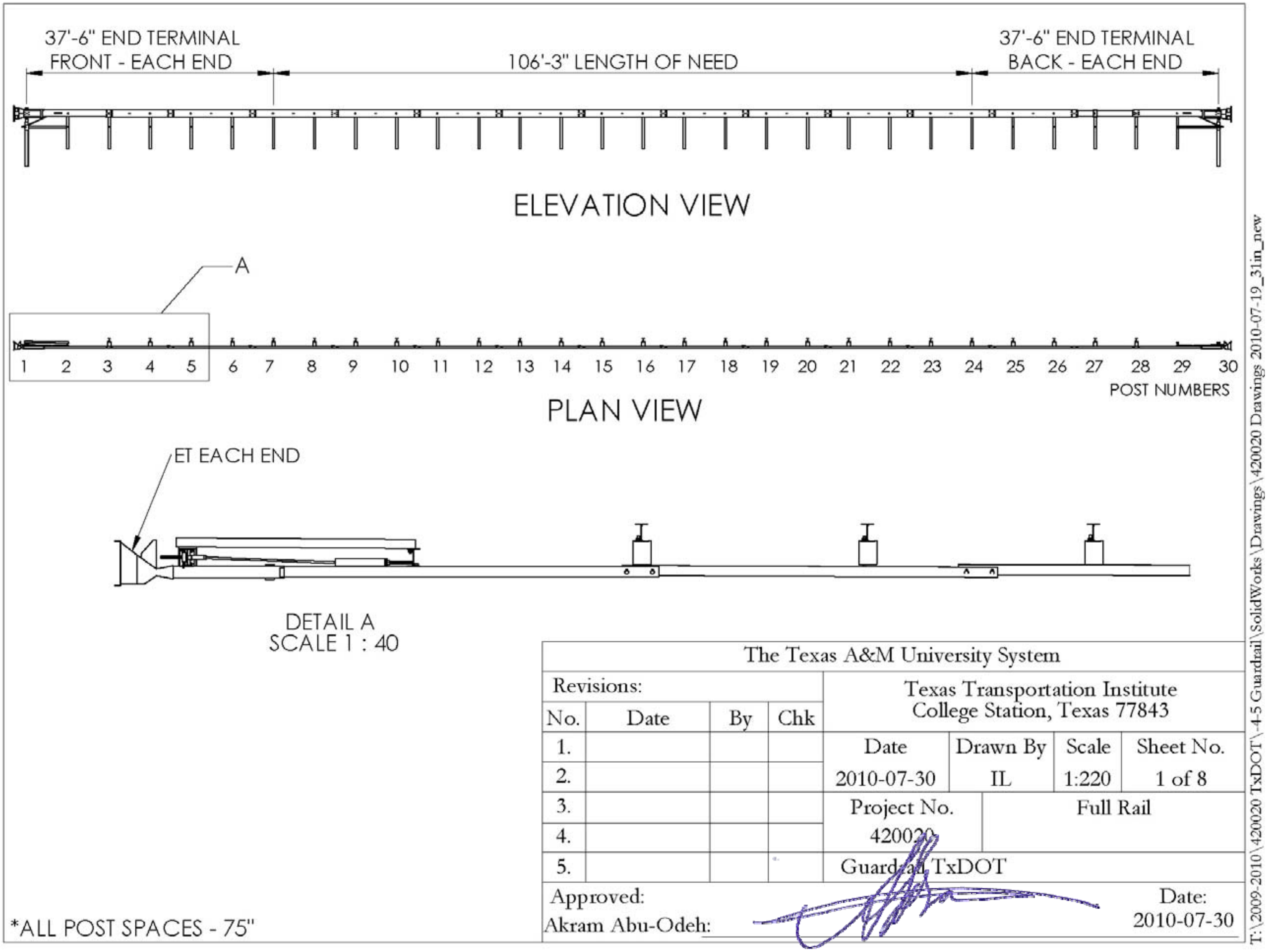
6. AASHTO, *Manual for Assessing Safety Hardware*, First Edition, American Association of State Highway and Transportation Officials, Washington, D.C., 2009.

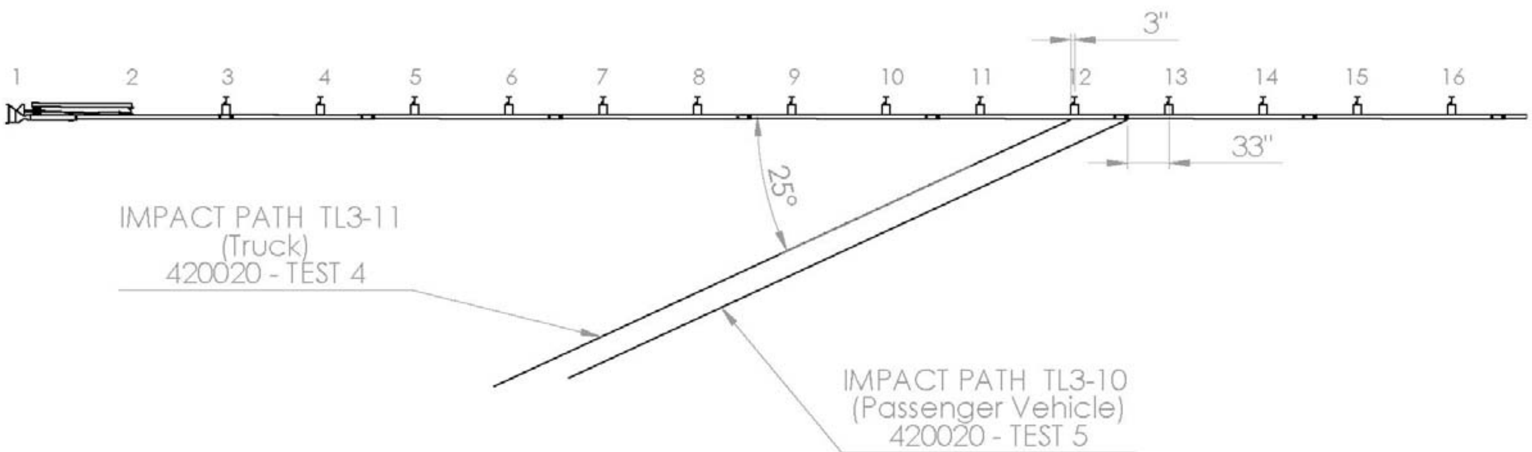
7. Polivka, K. A., Faller, R. K., Sicking, D. L., Rohde, J. R., Bielenberg, B. W., and Reid, J. D., *Performance Evaluation of the Modified G4(1S) Guardrail System – Update to NCHRP 350 Test No. 3-11 with 28" C.G. Height (2214-WB-2)*, Research Report No. TRP-03-169-06, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, NE, October 2006.

8. Polivka, K. A., Faller, R. K., Sicking, D.L., Rohde, J. R., Bielenberg, B. W., and Reid, J. D., *Performance Evaluation of the Midwest Guardrail System – Update to NCHRP 350 Test No. 3-11 with 28" C.G. Height (2214MG-2)*, Research Report No. TRP-03-171-06, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, NE, October 2006.

9. Roadside Design: Steel Strong-Post W-beam Guardrail, May 17, 2010, Memorandum, Office of Safety Design, Federal Highway Administration, U.S. Department of Transportation.

APPENDIX A. DETAILS OF THE TEST ARTICLE





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2.				2010-07-30	IL	1:125	2 of 8
3.				Project No.	Impact Points		
4.				420020			
5.				Guardrail TxDOT			

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#	PART NAME	QTY.
1	Nut, Recessed Guardrail	134
2	Bolt, Button-head 1 1/2"	114
3	Post, 31in. W-6x8.5 SYTP	10
4	Blockout, Wood W-beam Routered	26
5	W-Beam, 4- space 12 gauge	11
6	Bolt, Button-head 10 inch	26
7	Post, W6 x 8.5 SLP	18
8	W-Beam, 9'-4.5" - 12 gauge	2
9	5/16" nut	4
10	5/16" flat washer	8
11	Bolt, 5/16" -18 x 1-1/2" hex	4
12	ET plus head	2
13	Washer, 1" flat	4
14	Nut, 1" -8 hex	4
15	Anchor Bracket, ET Cable	2
16	W-beam, ET	2
17	3/4" Anchor Cable	2
18	Post, CRP Bottom	2
19	CRP top, 31"	2
20	5/16" flat washer	8
21	Bolt, 5/16"-18x2 Hex	4
22	CRP bent plate washer	2
23	Strut, CRP	2
24	Washer, 5/8" flat	6
25	Bolt, 5/8"-11x2" Hex	6

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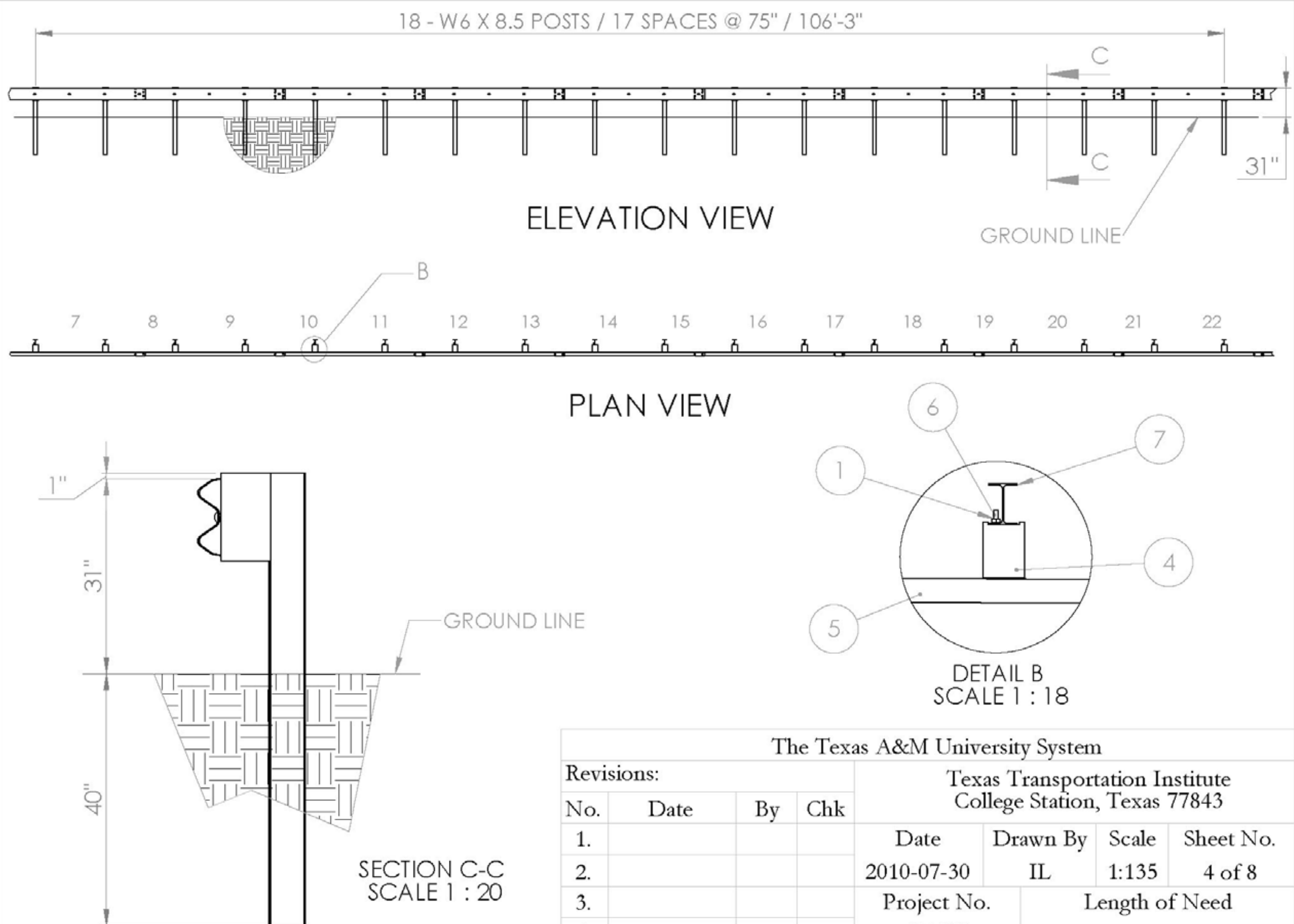

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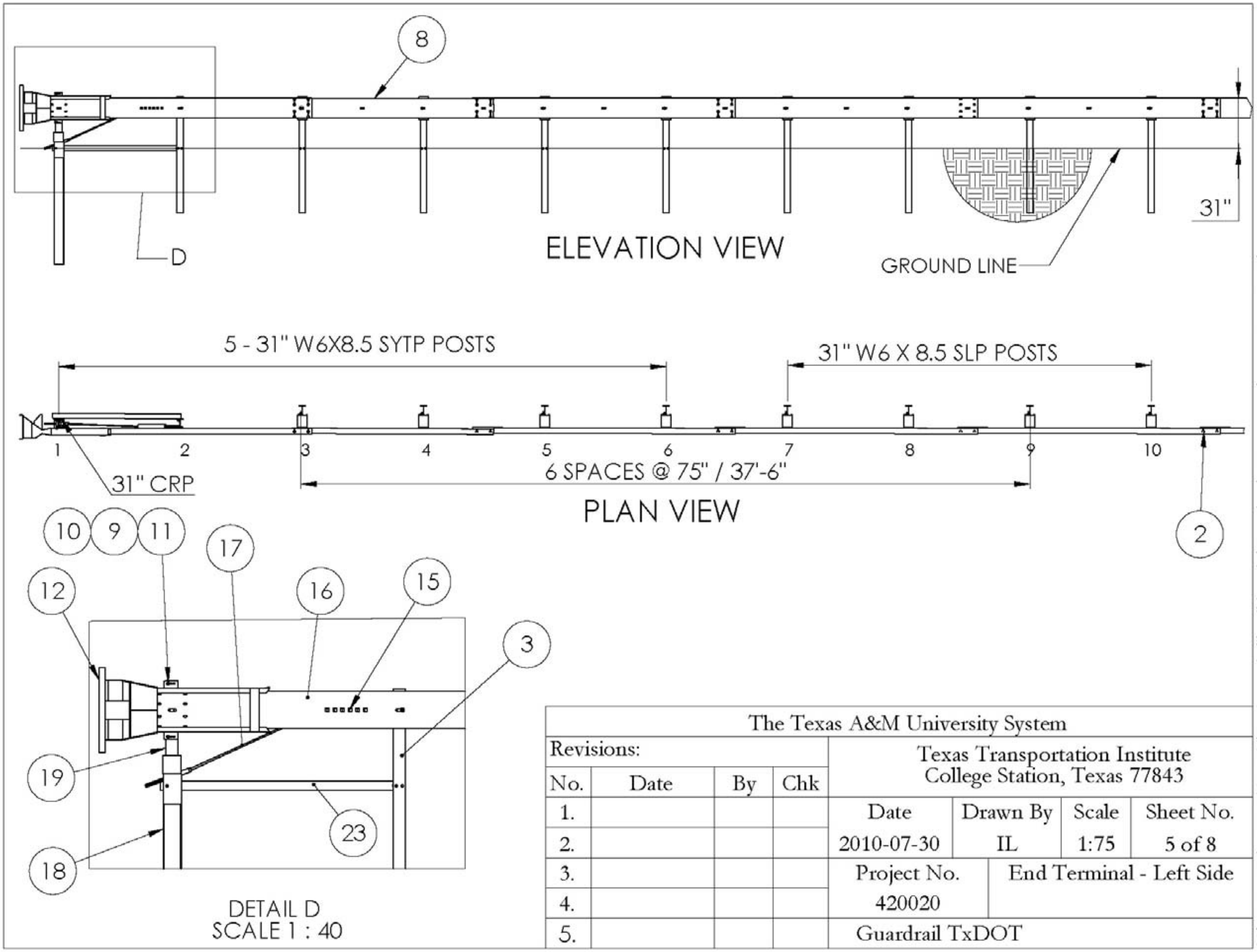

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5.				Guardrail TxDOT			

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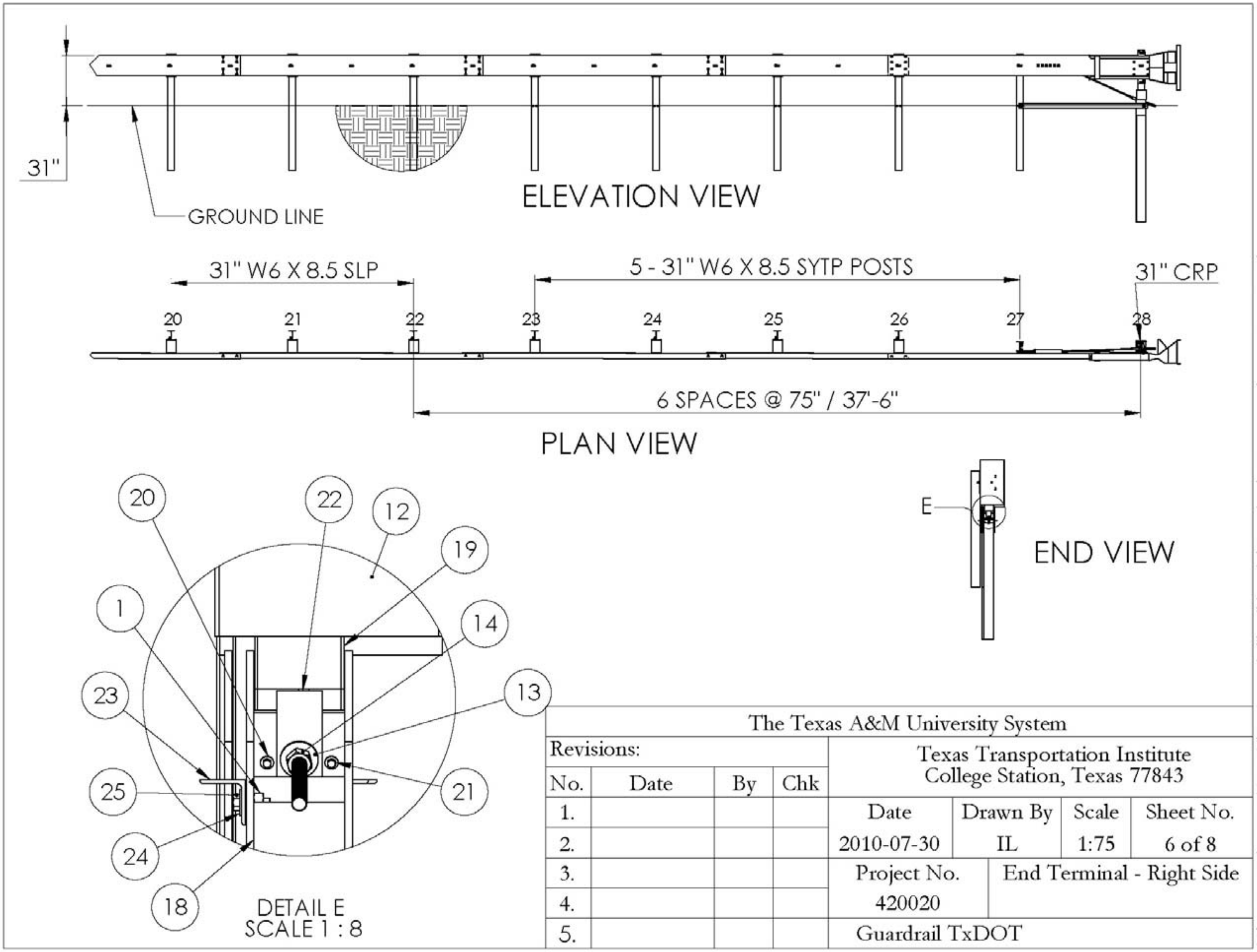

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4.				420020			
5.				Guardrail TxDOT			

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5. 4 Space 12-ga. W-beam
RWM02

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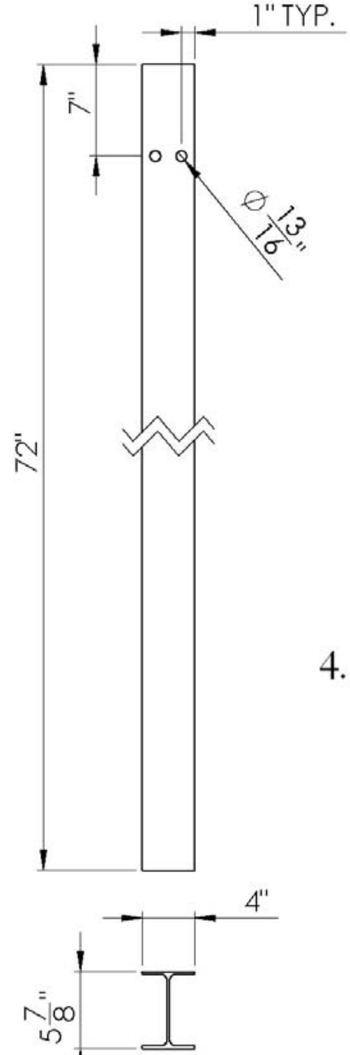

7. Standard Post
W6 x 8.5
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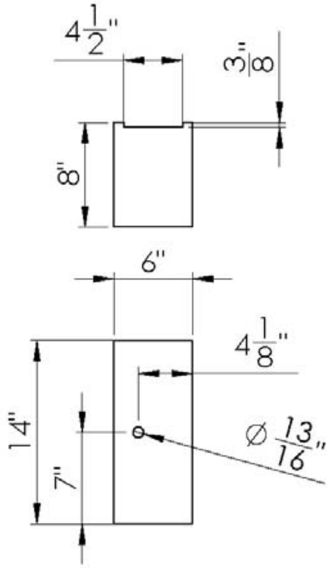
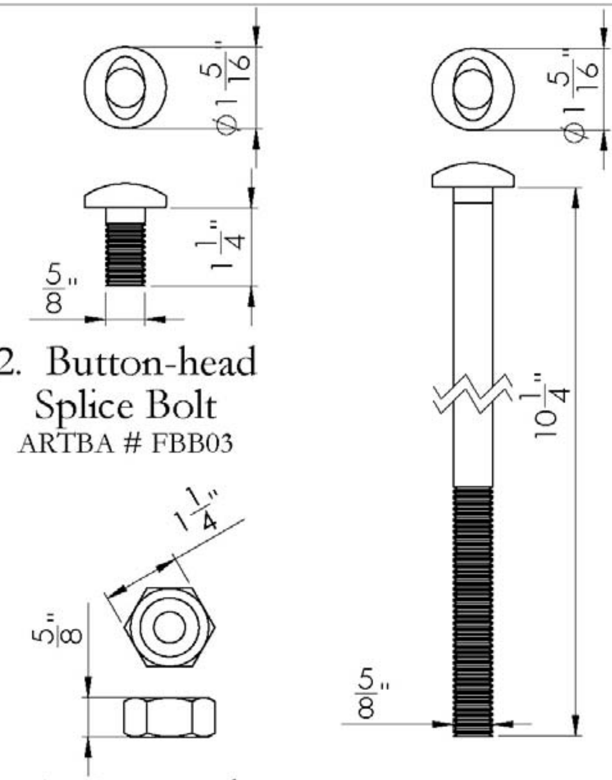


4. 8" Wood Blockout
ARTBA # PDB01b



2. Button-head Splice Bolt
ARTBA # FBB03

1. Recessed Guardrail Nut
ARTBA # FBB01

6. 10" Guardrail Bolt
ARTBA # FBB03

Bolts and Nut
Scale 1:2.5

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Project No.	Posts, etc.
420020	

Guardrail TxDOT

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APPENDIX B. CERTIFICATION DOCUMENTATION

MATERIAL USED

TEST NUMBER	420020-5	Guardrail
DATE	2010-08-26	

DATE RECEIVED	ITEM NUMBER	DESCRIPTION	SUPPLIER	HEAT #
2010-06-11	Parts-10	Guardrail parts	Trinity	see file
2010-06-28	Parts-11	Guardrail parts	Trinity	see file

This Memorandum is an acknowledgement that a Bill of Lading has been issued and is not the original Bill of Lading, nor a copy or duplicate, covering the property named herein, and is intended solely for filing or record.

Carrier: Trinity Highway Products, LLC [illegible]

Shipper's No. 16-31302

S/O No. 1079892

RECEIVED, subject to the classifications and tariffs in effect on the date of receipt by the carrier of the property described in the Original Bill of Lading, at [illegible] 6/10 20 10 from [illegible]

the property described below, in apparent good order, except as noted (contents and condition of contents of packages unknown) marked, consigned and destined as shown below, which said company (the word company being understood throughout this contract as meaning any person or corporation in possession of the property under the contract) agrees to carry to its usual place of delivery at said destination, if on its own railroad, water line, highway route or routes, or within the territory of its highway operations, otherwise to deliver to another carrier on the route to said destination. It is mutually agreed, as to each carrier of all or any of said property over all or any portion of said route to destination, and as to each party at any time interested in all or any of said property, that every service to be performed hereunder shall be subject to all the conditions not prohibited by law, whether printed or written, herein contained, including the conditions on back hereof, which are hereby agreed to by the shipper and accepted for himself and his assigns.

Consigned to: SAMPLES TESTING TRAINING MTRLS

Destination: TTI ATTN: GARY GERKE, SAFETY & STRUCT SYST DIV — TEXAS A&M RIVERSIDE CAMPUS, 3100 STATE HWY 47, BLDG 7090

Cust. P.O. [illegible]

Load No.: SS-1

Total Weight: 5,016.14

City: BRYAN State: TX Zip: 77807

Ship: 6/10/10

Arrive: 6/11/10 9:00:00AM

Contact: GARY GERKE Phone: 979-845-4681 [illegible] 254963

Delivering Carrier: FedEx Vehicle or Car Initial: ______ No. ______

Collect On Delivery: $______ and remit to: ______

Street ______ City ______ State ______

C.O.D. charge to be paid by Shipper ☐ Consignee ☐

Subject to Section 7 of Conditions of applicable Bill of Lading, if this shipment is to be delivered to the consignee without recourse on the consignor, the consignor shall sign the following statement:
The carrier shall not make delivery of this shipment without payment of freight and all other lawful charges.

TRINITY HIGHWAY PRODUCTS, LLC

Trinity Highway Products, LLC

Per ______ (Signature of Consignor)

If charges are to be prepaid, write or stamp here, "To be Prepaid."

Received $______ to apply in prepayment of the charges on the property described hereon.

Agent or Cashier

Per ______

(The signature here acknowledges only the amount prepaid.)

Charges advanced:

No. Pkgs.	Piece Count	Description of Articles	*Wt.	Class or Rate	✓ Col.
		Upon delivery, all materials subject to Trinity Highway Products, LLC Storage State Policy No. LG-002			
		Project Info: SAMPLES-TESTING THIS ORDER FOR END TERMINALS ONLY			
	4	11G 12/12'6/3'1 SS			
	4	72G 12/12'6/S3 ET2000 ANC [illegible]			
	40	545G 6'0 POST/DB DDR [illegible]			
	4	704A CABLE ANCHOR BRKT ET			
	[illegible]	[illegible]3G [illegible] BEAR [illegible]			
	4	3000G CBL 3/4X6'6/DBL SWG/NOHP			
	384	3340G 5/8" GR HEX NUT			
	384	3360G 5/8"X1.25" GR BOLT			
	8	3900G 1" ROUND WASHER F844			
	8	3910G 1" HEX NUT A563			
	8	4280G 7/16" WASHER F844			
	8	4390G 7/16"X1.5" HEX BOLT GRD 5			
	8	4393G 7/16" LOCK WASHER			
	8	4394G 7/16" HEX NUT A563 DH			
	4	10967G 12/6'3/S1.SS			
	4	15578G 6'0 7'3/7/8 SWEYTP			
	4	15000G 6'0 STT PST/8 [illegible] GR HT			
	2	[illegible]705G SFT YAN STRT 3-HL 6'6			

2-pallets
1-Bundle

GUARDRAIL HWY STEEL
NMFC ITEM 105460
CLASS 50

16-31302

SPECIAL INSTRUCTIONS:

SHIPPER LOAD - CONSIGNEE UNLOAD

Total Weight 5016

3

*If the shipment moves between two ports by a carrier by water, the law requires that the bill of lading shall state whether it is "carrier's or shipper's weight."
NOTE - Where the rate is dependent on value, shippers are required to state specifically in writing the agreed or declared value of the property.
The agreed or declared value of the property is hereby specifically stated by the shipper to be not exceeding ______ per ______

ORIGIN — SHIPPER OR AGENT: I hereby authorize this shipment and make the declaration of values (if any) and agree to the contract terms and conditions hereof. SIGN HERE [signature] DATE 6-10-10

AGENT OR DRIVER: This shipment received subject to exceptions as noted and according to the terms and conditions hereof. (SIGN HERE) ______ DATE ______

DESTINATION — CONSIGNEE OR AGENT: Received the above described property in good condition except as noted on the back hereof and agree to the foregoing contract terms and conditions. SIGN HERE ______ DATE / / TIME A.M. P.M.

DRIVER ______ NO. ______

Permanent post-office address of shipper.

TRI 609-RF (R 10/93)

(This Bill of Lading is to be signed by the shipper and agent of the carrier issuing same.)

CONSIGNEE/CUSTOMER COPY

Certified Analysis

Trinity Highway Products , LLC
2548 N.E. 28th St.
Ft Worth, TX 76111

Order Number: 1072852
Customer PO:
BOL Number: 31302
Document #: 1
Shipped To: TX
Use State: TX

As of: 6/25/10

Customer: SAMPLES,TESTING,TRAINING MTRLS
2525 STEMMONS FRWY

DALLAS, TX 75207

Project: SAMPLES-TESTING THIS ORDER FOR END TERMINALS ONLY!

Qty	Part #	Description	Spec	CL	TY	Heat Code/ Heat #	Yield	TS	Elg	C	Mn	P	S	Si	Cu	Cb	Cr	Vn	ACW
14	11G	12/12'6/3'1.5/S	M-180	A	2	203281	51,200	73,500	30.0	0.200	0.770	0.008	0.003	0.020	0.120	0.00	0.050	0.003	4
			M-180	A	2	102781	52,600	74,900	28.0	0.190	0.750	0.011	0.002	0.020	0.150	0.000	0.060	0.003	4
			M-180	A	2	102782	51,900	73,900	29.0	0.200	0.810	0.009	0.002	0.010	0.140	0.000	0.050	0.003	4
			M-180	A	2	103049	50,300	73,600	28.0	0.200	0.780	0.010	0.003	0.020	0.130	0.000	0.060	0.002	4
			M-180	A	2	203282	50,000	73,500	30.0	0.190	0.760	0.008	0.003	0.030	0.120	0.000	0.050	0.003	4
4	32G	12/12'6/6'3/S ET2000 ANC	M-180	A	2	102476	58,100	77,900	26.0	0.190	0.750	0.009	0.001	0.020	0.130	0.00	0.060	0.002	4
			M-180	A	2	202939	56,800	78,400	25.0	0.190	0.770	0.009	0.004	0.020	0.130	0.000	0.050	0.003	4
40	545G	6'0 POST/DB:DDR	A-36			JK09103532	56,474	72,822	27.9	0.140	0.630	0.011	0.040	0.210	0.390	0.00	0.090	0.001	4
	545G		A-36			1004093	58,119	75,411	28.9	0.140	0.930	0.017	0.031	0.170	0.450	0.00	0.150	0.003	4
	545G		A-36			1004105	54,945	72,078	29.3	0.120	0.940	0.016	0.035	0.180	0.340	0.00	0.150	0.003	4
4	704A	CABLE ANCHOR BRKT	1018			45972C	0	0	0.0	0.170	0.700	0.011	0.020	0.160	0.250	0.00	0.009	0.000	4
5	782G	5/8"X8"X8" BEAR PL/OF	A-36			1005737	46,100	64,600	25.0	0.080	0.640	0.012	0.027	0.210	0.410	0.00	0.170	0.002	4
	782G		A-36			1002736	46,700	67,900	24.0	0.130	0.620	0.015	0.025	0.230	0.450	0.00	0.190	0.003	4
4	14578G	6'0 PST/8.5#/SYTP	A-36			1005535	55,326	72,178	25.8	0.100	0.930	0.007	0.033	0.170	0.320	0.00	0.110	0.002	4
4	15000G	6'0 SYT PST/8.5/31" GR HT	A-36			1004188	55,750	72,733	28.7	0.110	0.920	0.011	0.035	0.180	0.380	0.00	0.150	0.003	4
	15000G		A-36			1004104	54,637	72,751	27.0	0.110	0.920	0.020	0.034	0.180	0.350	0.00	0.170	0.004	4

Certified Analysis

Trinity Highway Products , LLC
2548 N.E. 28th St.
Ft Worth, TX 76111

Customer: SAMPLES,TESTING,TRAINING MTRLS
2525 STEMMONS FRWY

DALLAS, TX 75207

Order Number: 1072852
Customer PO:
BOL Number: 31302
Document #: 1
Shipped To: TX
Use State: TX

As of: 6/25/10

Project: SAMPLES-TESTING THIS ORDER FOR END TERMINALS ONLY!

TL -3 or TL-4 COMPLIANT when installed according to manufactures specifications

Upon delivery, all materials subject to Trinity Highway Products , LLC Storage Stain Policy No. LG-002.

ALL STEEL USED WAS MELTED AND MANUFACTURED IN USA AND COMPLIES WITH THE BUY AMERICA ACT.
ALL GUARDRAIL MEETS AASHTO M-180, ALL STRUCTURAL STEEL MEETS ASTM A36

ALL GALVANIZED MATERIAL CONFORMS WITH ASTM-123, UNLESS OTHERWISE STATED.

BOLTS COMPLY WITH ASTM A-307 SPECIFICATIONS AND ARE GALVANIZED IN ACCORDANCE WITH ASTM A-153, UNLESS OTHERWISE STATED.

NUTS COMPLY WITH ASTM A-563 SPECIFICATIONS AND ARE GALVANIZED IN ACCORDANCE WITH ASTM A-153, UNLESS OTHERWISE STATED.

3/4" DIA CABLE 6X19 ZINC COATED SWAGED END AISI C-1035 STEEL ANNEALED STUD 1" DIA ASTM 449 AASHTO M30, TYPE II BREAKING STRENGTH – 49100 LB

State of Texas, County of Tarrant. Sworn and subscribed before me this 25th day of June, 2010

Notary Public:
Commission Expires:

DANIELLA LEE ROBINSON
Notary Public, State of Texas
My Commission Expires
November 05, 2013

Trinity Highway Products, LLC
Certified By:
Quality Assurance

Certified Analysis

Trinity Highway Products , LLC
2548 N.E. 28th St.
Ft Worth, TX 76111

Customer: SAMPLES,TESTING,TRAINING MTRLS
2525 STEMMONS FRWY

DALLAS, TX 75207

Order Number: 1072852
Customer PO:
BOL Number: 31302
Document #: 2
Shipped To: TX
Use State: TX

As of: 6/25/10

Project: SAMPLES-TESTING THIS ORDER FOR END TERMINALS ONLY!

Qty	Part #	Description	Spec	CL	TY	Heat Code/ Heat #	Yield	TS	Elg	C	Mn	P	S	Si	Cu	Cb	Cr	Vn	ACW
4	10967G	12/9'4.5/3'1.5/S	M-180	A	2	100929	557,000	77,800	26.0	0.190	0.750	0.009	0.001	0.020	0.140	0.00	0.050	0.002	4
			M-180	A	2	100928	63,610	80,920	25.2	0.190	0.750	0.011	0.004	0.030	0.090	0.000	0.040	0.000	4
			M-180	A	2	101800	50,000	73,300	30.0	0.190	0.750	0.012	0.002	0.020	0.120	0.000	0.070	0.002	4
			A-500		2	202248	53,600	75,500	29.0	0.190	0.780	0.011	0.020	0.120	0.120	0.000	0.050	0.002	4
			M-180	A	2	202249	51,800	74,500	30.0	0.190	0.790	0.010	0.002	0.020	0.120	0.000	0.050	0.002	4
2	33795G	SYT-3"AN STRT 3-HL 6'6	A-36			V906151	52,710	75,060	29.5	0.130	0.700	0.011	0.022	0.200	0.240	0.00	0.100	0.021	4

TL -3 or TL-4 COMPLIANT when installed according to manufactures specifications

Upon delivery, all materials subject to Trinity Highway Products , LLC Storage Stain Policy No. LG-002.

ALL STEEL USED WAS MELTED AND MANUFACTURED IN USA AND COMPLIES WITH THE BUY AMERICA ACT.
ALL GUARDRAIL MEETS AASHTO M-180, ALL STRUCTURAL STEEL MEETS ASTM A36

ALL GALVANIZED MATERIAL CONFORMS WITH ASTM-123, UNLESS OTHERWISE STATED.

BOLTS COMPLY WITH ASTM A-307 SPECIFICATIONS AND ARE GALVANIZED IN ACCORDANCE WITH ASTM A-153, UNLESS OTHERWISE STATED.

NUTS COMPLY WITH ASTM A-563 SPECIFICATIONS AND ARE GALVANIZED IN ACCORDANCE WITH ASTM A-153, UNLESS OTHERWISE STATED.

3/4" DIA CABLE 6X19 ZINC COATED SWAGED END AISI C-1035 STEEL ANNEALED STUD 1" DIA ASTM 449 AASHTO M30, TYPE II BREAKING STRENGTH – 49100 LB

State of Texas, County of Tarrant. Sworn and subscribed before me this 25th day of June, 2010

Notary Public:
Commission Expires:

DANIELLA LEE ROBINSON
Notary Public, State of Texas
My Commission Expires
November 05, 2013

Trinity Highway Products, LLC
Certified By: Luis Ortiz
Quality Assurance

This Memorandum is an acknowledgement that a Bill of Lading has been issued and is not the original Bill of Lading, nor a copy or duplicate, covering the property named herein, and is intended solely for filing or record.

RECEIVED, subject to the classifications and tariffs in effect on the date of receipt by the carrier of the property described in the Original Bill of Lading,

at ________ 6/25 20 10 | Carrier | Shipper's No. 1972852

S/O No.

Consigned to: SAMPLES,TESTING,TRAINING MTRLS / TTI - ATTN: GARY GERKE / TX A & M U ... DE CAMPUS

Destination: SAFETY & STRUCT. SYST. DIV / 3100 STATE HWY 47, BLDG 7090

Load No.: VT-1

Total Weight: 1,601.75

6/25/10

6/25/10 8:00:00AM

256820

City: BRYAN State: TX Zip: 77807

Contact: GARY GERKE Phone: 979-845-6185

Delivering Carrier: FedEX Vehicle or Car Initial: No.

Subject to Section 7 of Conditions of applicable Bill of Lading, if this shipment is to be delivered to the consignee without recourse on the consignor, the consignor shall sign the following statement:
The carrier shall not make delivery of this shipment without payment of freight and all other lawful charges.

TRINITY HIGHWAY PRODUCTS, LLC

Per ________ (Signature of Consignor)

If charges are to be prepaid, write or stamp here, "To be Prepaid."

Received $ ________ to apply in prepayment of the charges on the property described hereon.

Agent or Cashier

Per ________ (The signature here acknowledges only the amount prepaid.)

Charges advanced:

Collect On Delivery: $________ and remit to: ________

C.O.D. charge to be paid by Shipper ☐ Consignee ☐

Street ________ City ________ State ________

No. Pkgs.	Piece Count	Description of Articles	*Wt.	Class or Rate	✓ Col.
		Upon delivery, all material subject to Trinity Highway Products,LLC Ret... Goods Policy No. LG-002			
		Project Info: SAMPLES-TESTING THIS ORDER FOR END TERMINAL ONLY!			
	50	3000G CBL 3/4X6'6/DBL SWG/NOH"			
	50	3340G 5/8" GR HEX NUT			
	50	3500G 5/8"X10" GR BOLT A307			
	50	4076B WD BLK RTD 6X6X14			
		2-pallets			

No. Pkgs.	Piece Count	Description of Articles	*Wt.	Class or Rate	✓ Col.
		GUARDRAIL HWY STEEL NMFC ITEM 105460 CLASS 50			

1633547

SPECIAL INSTRUCTIONS:

SHIPPER LOAD - CONSIGNEE UNLOAD

*If the shipment moves between two ports by a carrier by water, the law requires that the bill of lading shall state whether it is "carrier's or shipper's weight."
NOTE - Where the rate is dependent on value, shippers are required to state specifically in writing the agreed or declared value of the property.
The agreed or declared value of the property is hereby specifically stated by the shipper to be not exceeding ________

Total Weight # 1,601 **3**

SHIPPER OR AGENT: I hereby authorize this shipment and make the declaration of values (if any) and agree to the contract terms and conditions hereof. DATE 6/25/10

This shipment received subject to exceptions as noted and according to the terms and conditions hereof. DATE

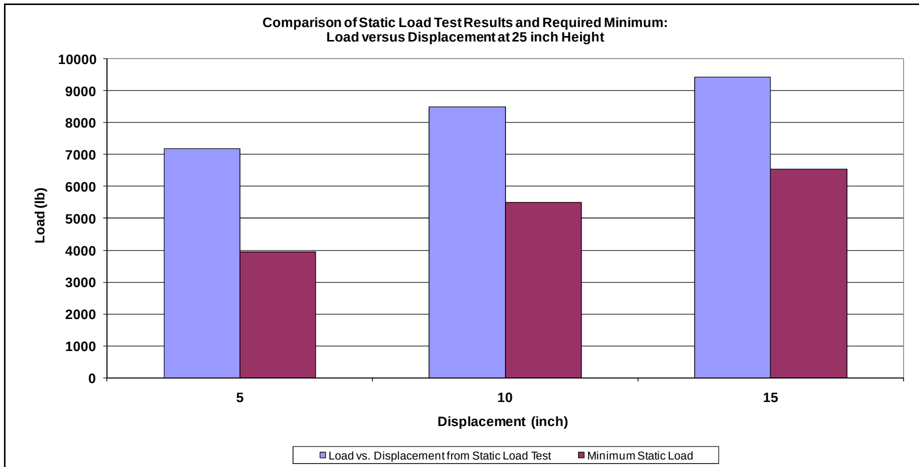
DESTINATION — CONSIGNEE OR AGENT: Received the above described property in good condition except as noted on the back hereof and agree to the foregoing contract terms and conditions.

SIGN HERE ________ DATE / / TIME A.M. P.M.

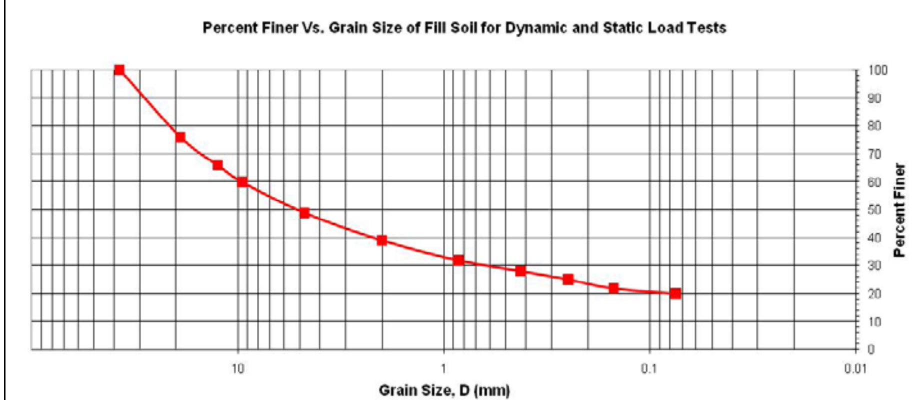
DRIVER ________ NO.

(This Bill of Lading is to be signed by the shipper and agent of the carrier issuing same.)

CONSIGNEE/CUSTOMER COPY



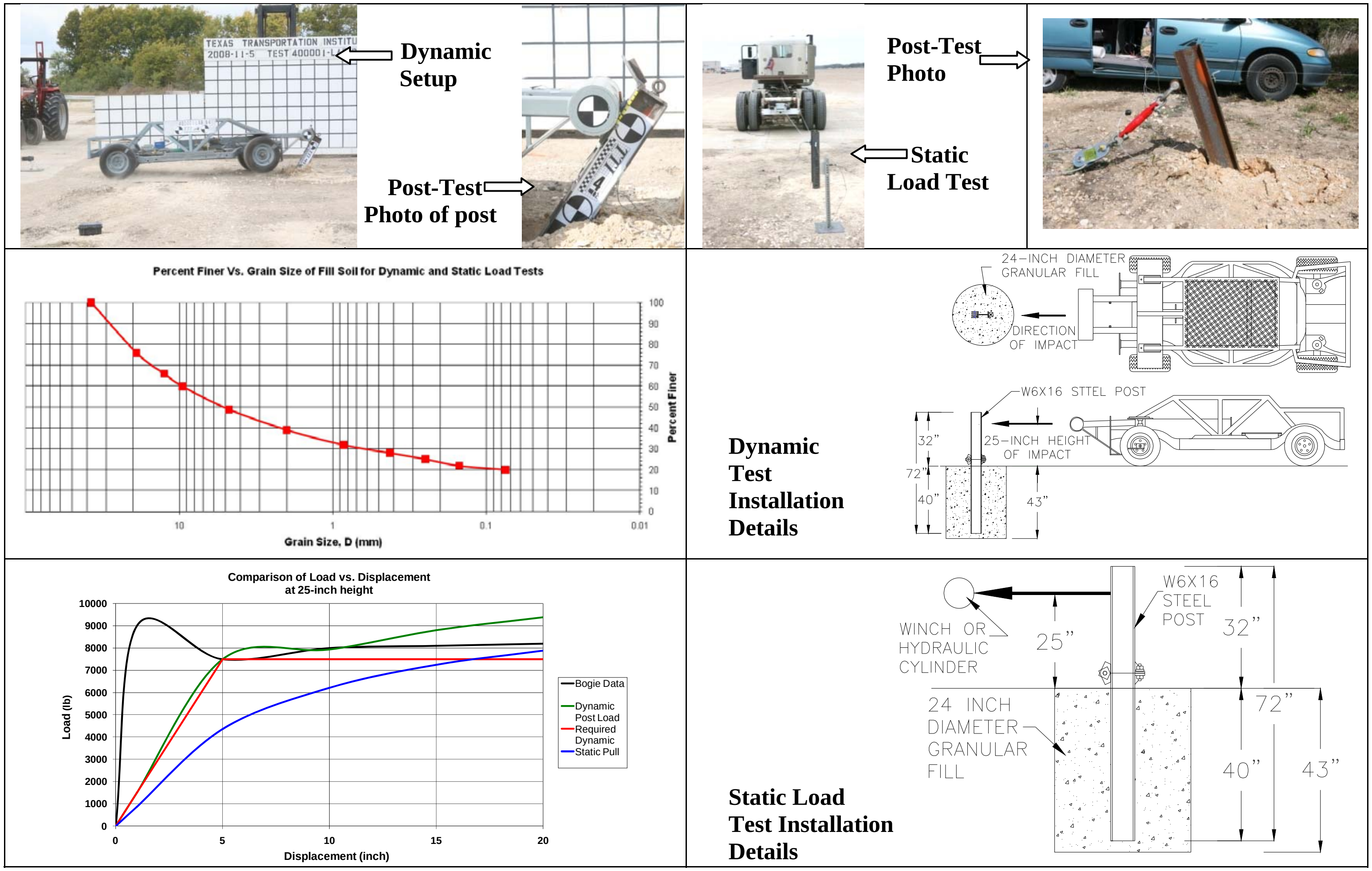
Static Load Setup



Post-Test Photo of Post

Date	2010-08-26
Test Facility and Site Location	TTI Proving Ground – 3100 SH 47, Bryan, Tx
In Situ Soil Description (ASTM D2487)	Sandy gravel with silty fines
Fill Material Description (ASTM D2487) and sieve analysis	AASHTO Grade B Soil-Aggregate (see sieve analysis)
Description of Fill Placement Procedure	6-inch lifts tamped with a pneumatic compactor

Figure C1. Test Day Static Soil Strength Documentation.



Date	2008-11-05
Test Facility and Site Location	TTI Proving Ground, 3100 SH 47, Bryan, TX 77807
In Situ Soil Description (ASTM D2487	Sandy gravel with silty fines
Fill Material Description (ASTM D2487) and sieve analysis	AASHTO Grade B Soil-Aggregate (see sieve analysis above)
Description of Fill Placement Procedure	6-inch lifts tamped with a pneumatic compactor
Bogie Weight	5009 lb
Impact Velocity	20.5 mph

Figure C2. Summary of Strong Soil Test Results for Establishing Installation Procedure.

APPENDIX D. TEST VEHICLE PROPERTIES AND INFORMATION

Date: 2010-08-26 Test No.: 420020-5 VIN No.: KNADC125336223817

Year: 2003 Make: Kia Model: Rio

Tire Inflation Pressure: 32 psi Odometer: 134135 Tire Size: P175/65R14

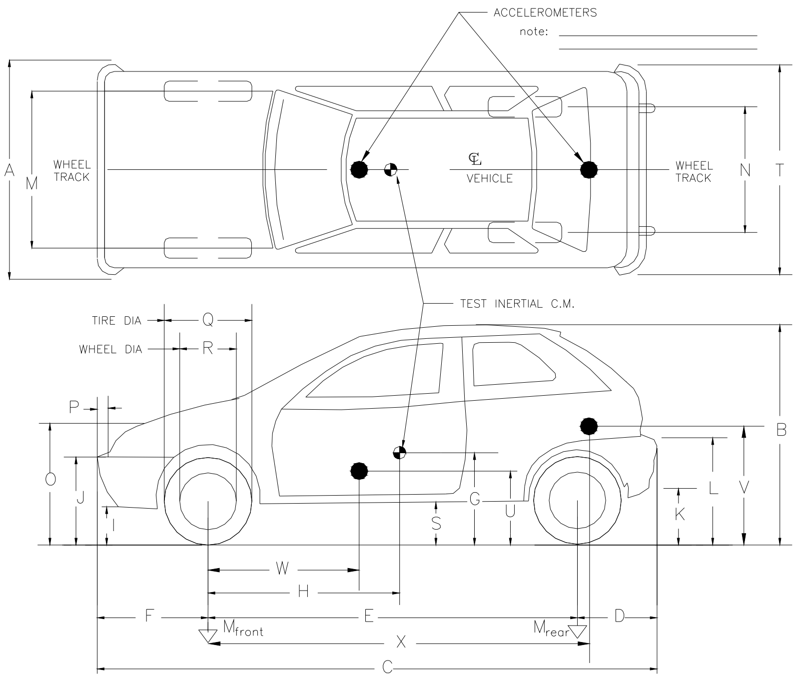
Describe any damage to the vehicle prior to test: ____________

● Denotes accelerometer location.

NOTES: ____________

Engine Type: ____________
Engine CID: ____________
Transmission Type:
x Auto or ____ Manual
x FWD ____ RWD ____ 4WD
Optional Equipment:
____

Dummy Data:
Type: 50th percentile male
Mass: 174 lb
Seat Position: Front Passenger



Geometry: inches

A	62.50	F	32.00	K	12.00	P	3.25	U	15.50
B	56.12	G		L	24.25	Q	22.50	V	20.00
C	164.25	H	34.42	M	56.50	R	15.50	W	39.00
D	37.00	I	8.50	N	57.00	S	8.62	X	103.25
E	75.25	J	22.75	O	28.00	T	63.00		

Wheel Center Ht Front 10.75 Wheel Center Ht Rear 11.125

RANGE LIMIT: A = 65 ±3 inches; C = 168 ±8 inches; E = 98 ±5 inches; F = 35 ±4 inches; G = 39 ±4 inches; O = 24 ±4 inches; M+N/2 = 56 ±2 inches

GVWR Ratings:		**Mass:** lb	Curb	Test Inertial		Gross Static	
Front	1804	M_{front}	1509	1555	Allowable	1640	Allowable
Back	1742	M_{rear}	878	880	Range	969	Range =
Total	3379	M_{Total}	2387	2435	2420 ±55 lb	2609	2585 ±55 lb

Mass Distribution:
lb LF: 779 RF: 776 LR: 433 RR: 447

Figure D1. Vehicle Properties for Test No. 420020-5.

Table D1. Exterior Crush Measurements for Test No. 420020-5.

Date: 2010-08-26 Test No.: 420020-5 VIN No.: KNADC125336223817

Year: 2003 Make: Kia Model: Rio

VEHICLE CRUSH MEASUREMENT SHEET[1]

<table>
<tr><td colspan="2">Complete When Applicable</td></tr>
<tr><td>End Damage</td><td>Side Damage</td></tr>
<tr><td>Undeformed end width ________
Corner shift: A1 ________
A2 ________
End shift at frame (CDC)
(check one)
< 4 inches ________
≥ 4 inches ________</td><td>Bowing: B1 _____ X1 _____
B2 _____ X2 _____
Bowing constant
$\frac{X1+X2}{2}$ = _______</td></tr>
</table>

Note: Measure C_1 to C_6 from Driver to Passenger Side in Front or Rear Impacts – Rear to Front in Side Impacts.

Specific Impact Number	Plane* of C-Measurements	Direct Damage Width** (CDC)	Direct Damage Max*** Crush	Field L**	C_1	C_2	C_3	C_4	C_5	C_6	±D
1	Front plane at bumper ht	12	8.5	45	8.5	6	4.5	3	2	1	0
2	Side plane at bumper ht	14	12.5	49	0	1	3	6.25	10	12.5	+40
	Measurements recorded										
	in inches										

[1]Table taken from National Accident Sampling System (NASS).

*Identify the plane at which the C-measurements are taken (e.g., at bumper, above bumper, at sill, above sill, at beltline, etc.) or label adjustments (e.g., free space).

Free space value is defined as the distance between the baseline and the original body contour taken at the individual C locations. This may include the following: bumper lead, bumper taper, side protrusion, side taper, etc. Record the value for each C-measurement and maximum crush.

**Measure and document on the vehicle diagram the beginning or end of the direct damage width and field L (e.g., side damage with respect to undamaged axle).

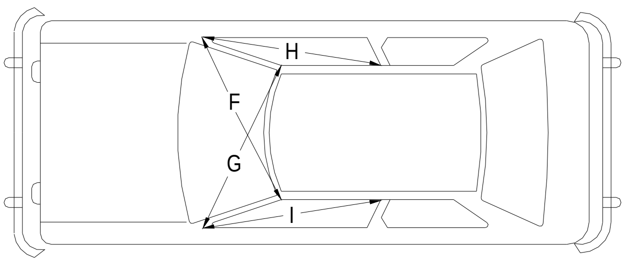
***Measure and document on the vehicle diagram the location of the maximum crush.

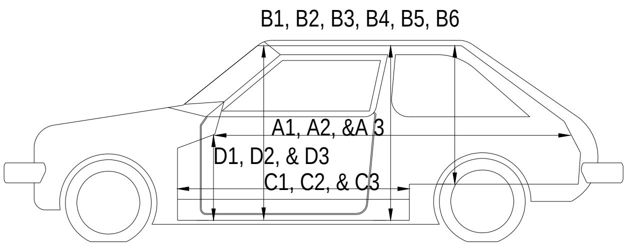
Note: Use as many lines/columns as necessary to describe each damage profile.

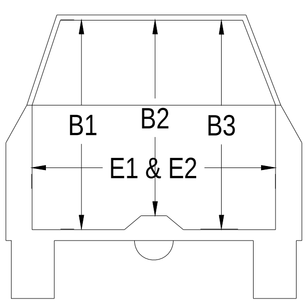
Table D2. Occupant Compartment Measurements for Test No. 420020-5.

Date: 2010-08-26 Test No.: 420020-5 VIN No.: KNADC125336223817

Year: 2003 Make: Kia Model: Rio





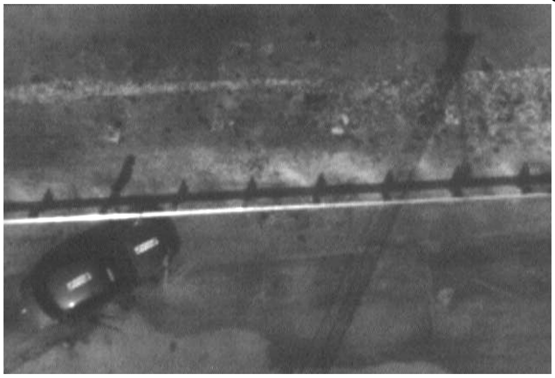


OCCUPANT COMPARTMENT DEFORMATION MEASUREMENT

	Before (inches)	**After** (inches)
A1	67.50	67.50
A2	35.50	35.50
A3	37.25	37.25
B1	39.75	39.75
B2	37.25	37.25
B3	39.12	39.12
B4	34.75	34.75
B5	35.00	35.00
B6	34.75	34.75
C1	26.75	26.75
C2	-----	-----
C3	26.50	26.50
D1	10.25	10.25
D2	-----	-----
D3	8.88	8.88
E1	48.50	48.50
E2	50.75	50.75
F	49.00	49.00
G	49.00	49.00
H	36.50	36.50
I	36.50	36.50
J*	50.25	50.25

*Lateral area across the cab from
driver's side kickpanel to passenger's side kickpanel.

APPENDIX E. SEQUENTIAL PHOTOGRAPHS



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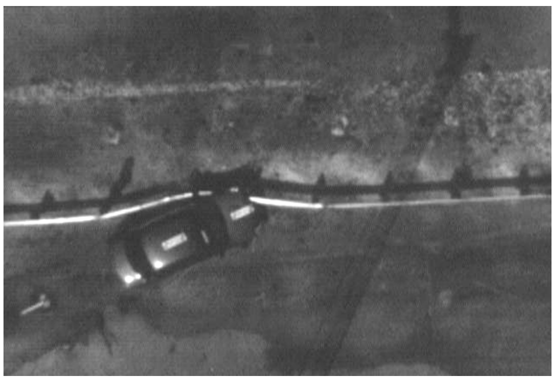

0.106 s

0.211 s

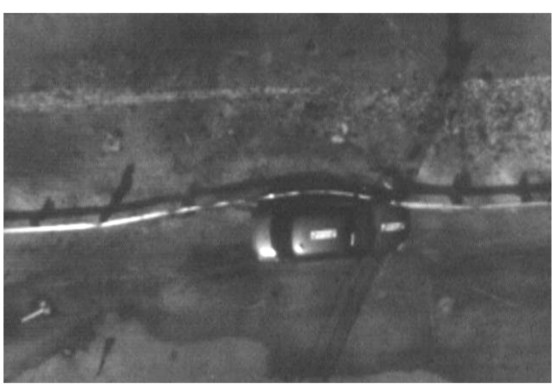

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Figure E1. Sequential Photographs for Test No. 420020-5 (Overhead and Frontal Views).

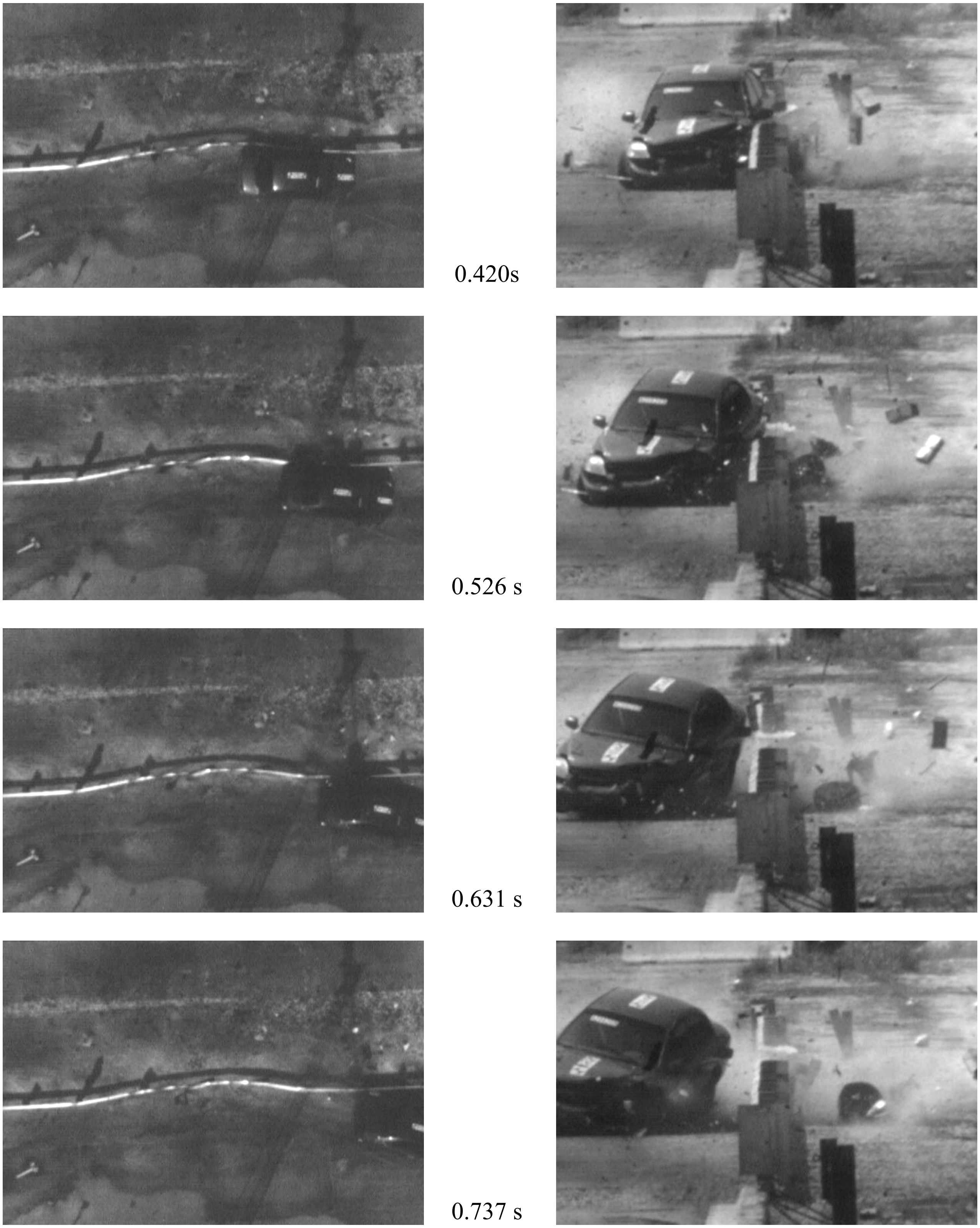
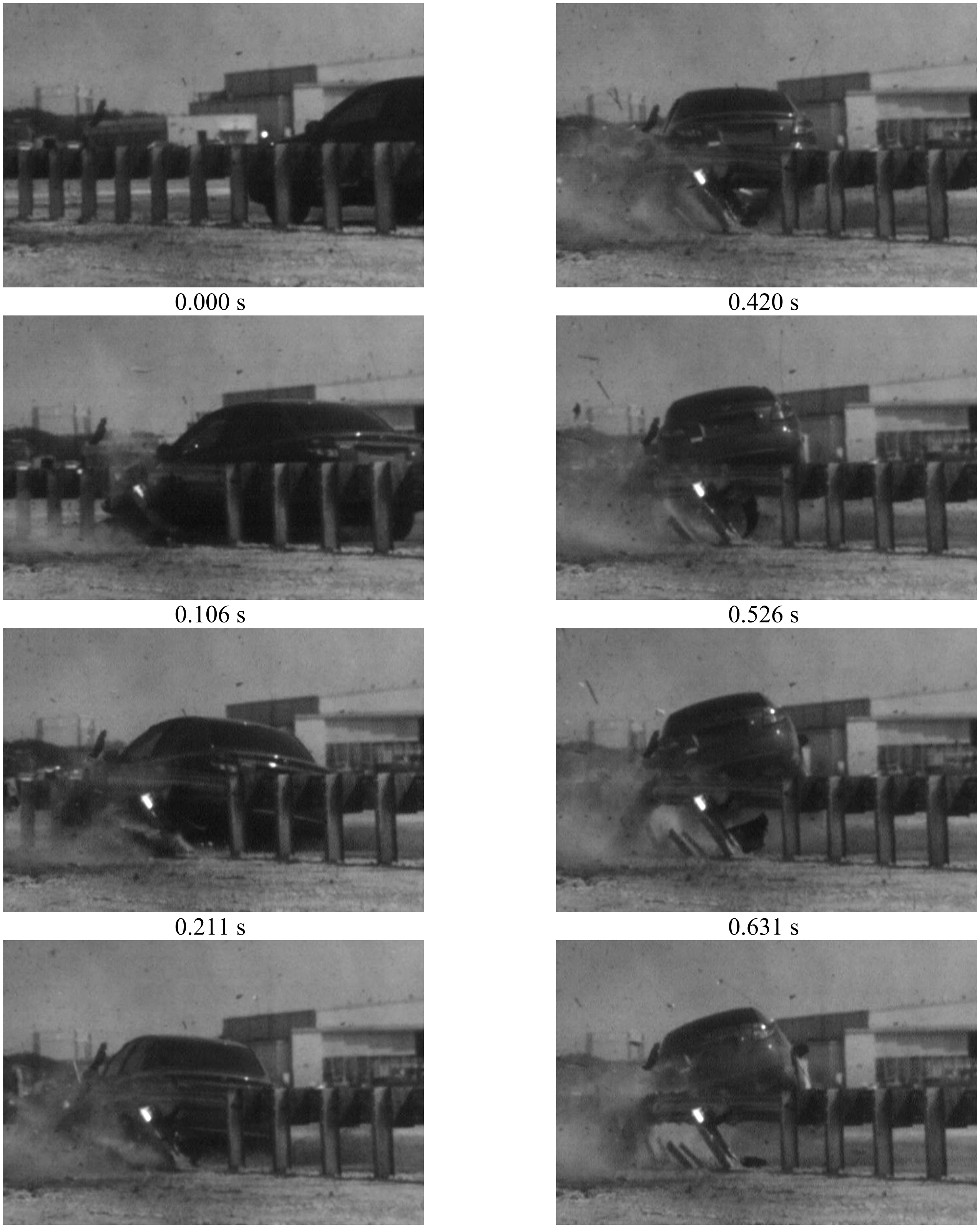


Figure E1. Sequential Photographs for Test No. 420020-5 (Overhead and Frontal Views) (Continued).



0.000 s
0.420 s
0.106 s
0.526 s
0.211 s
0.631 s
0.3174 s
0.737 s

Figure E2. Sequential Photographs for Test No. 420020-5 (Rear View).

APPENDIX F. VEHICLE ANGULAR DISPLACEMENTS AND ACCELERATIONS

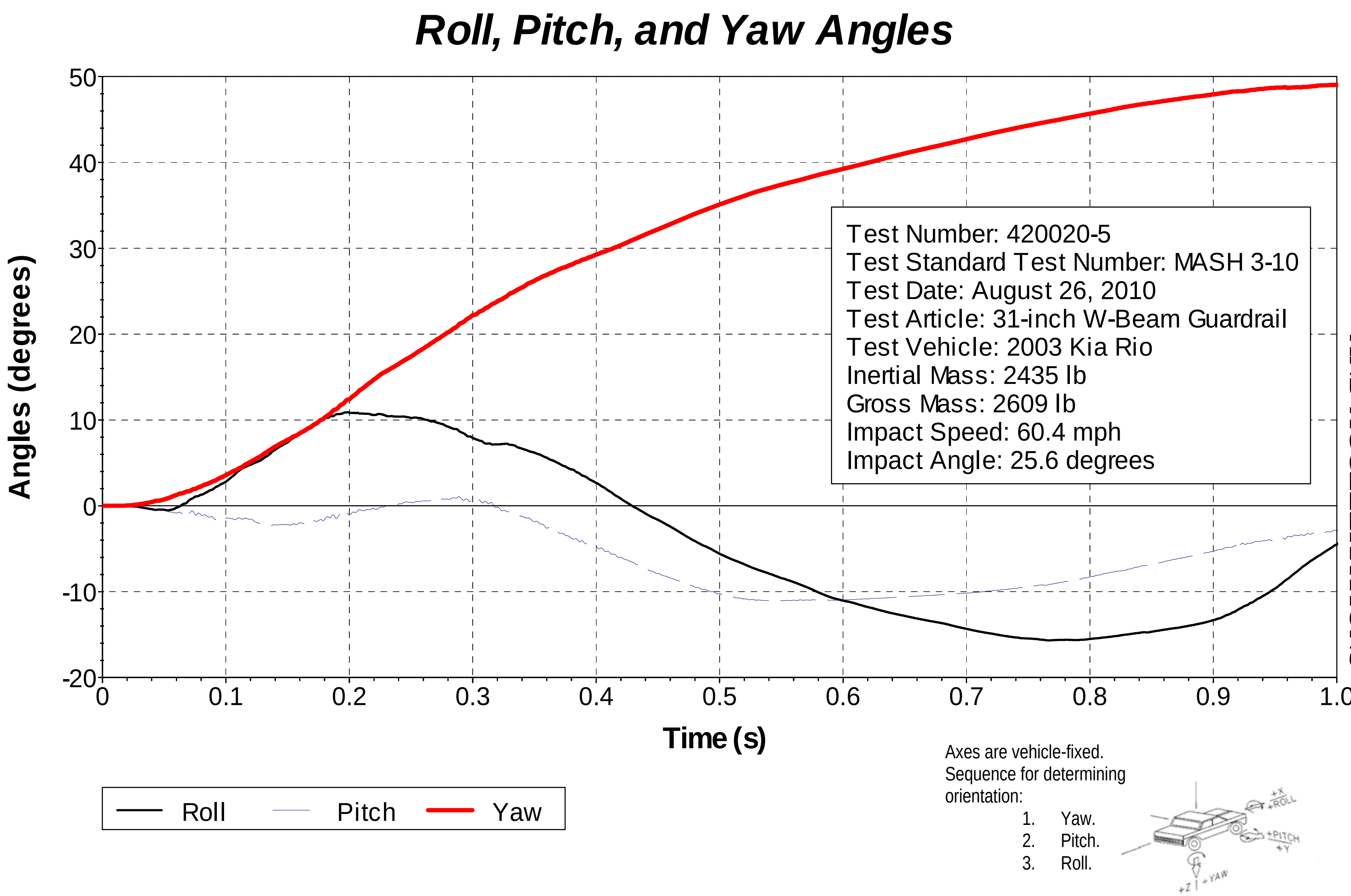


Figure F1. Vehicle Angular Displacements for Test No. 420020-5.

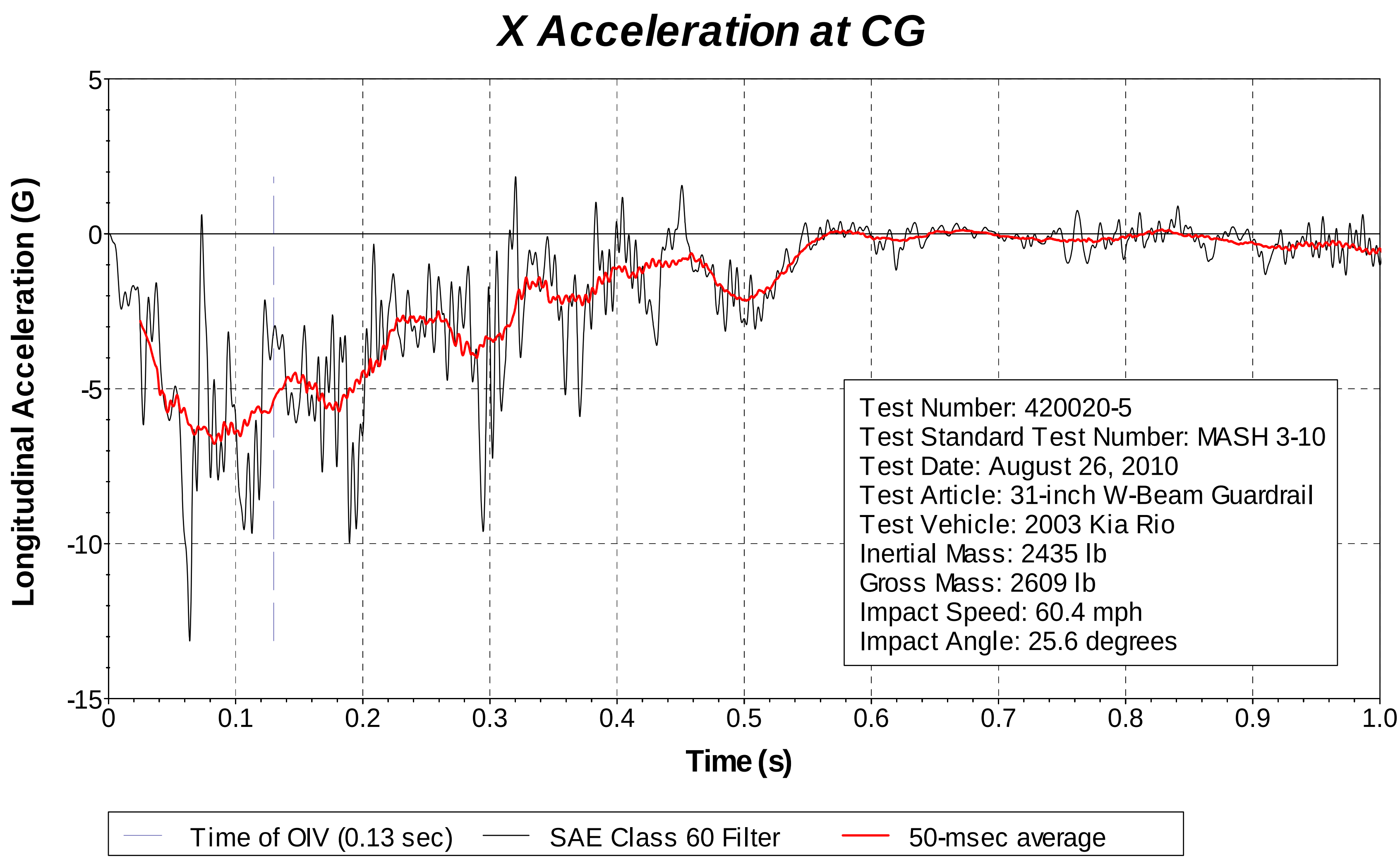


Figure F2. Vehicle Longitudinal Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity).

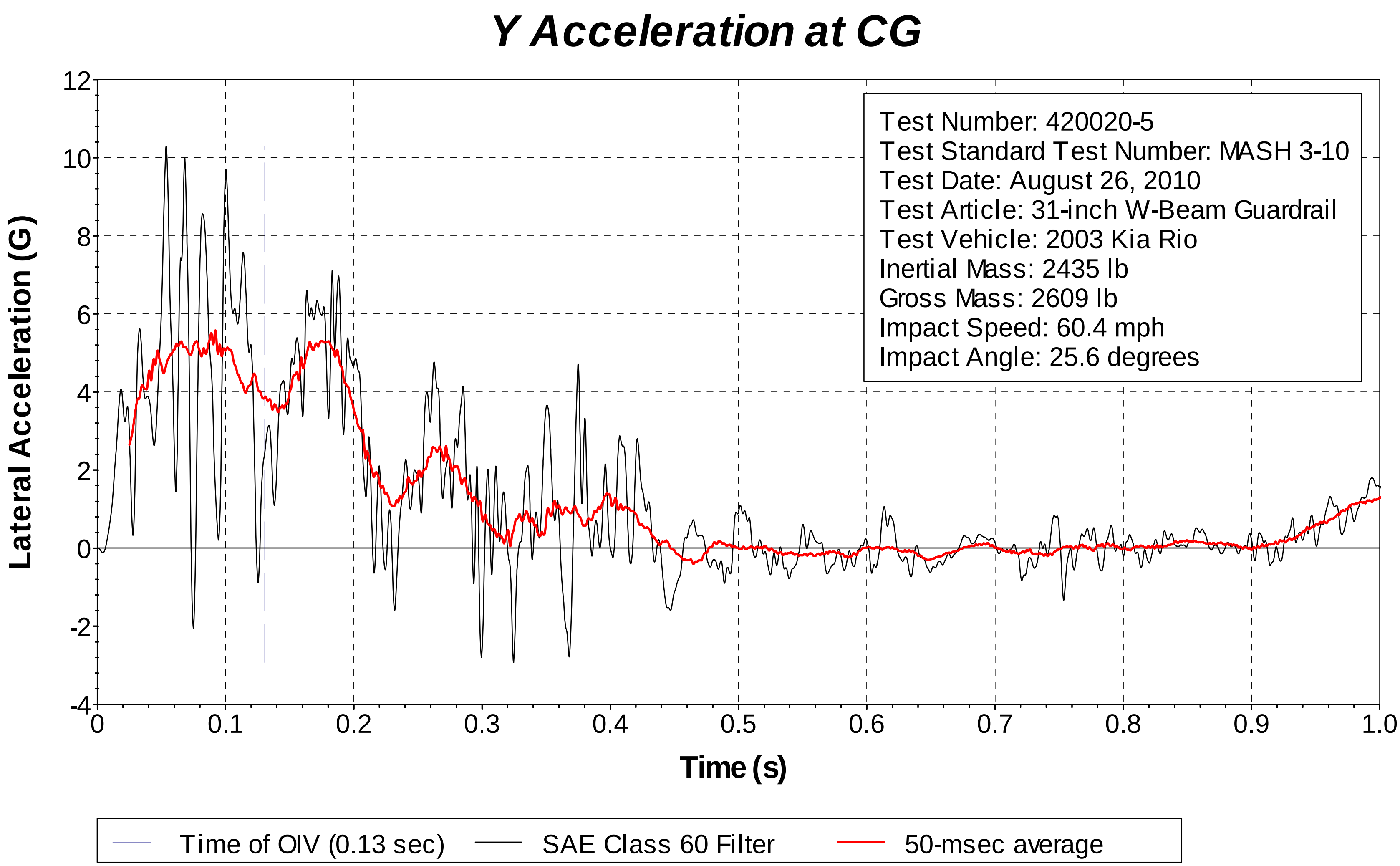


Figure F3. Vehicle Lateral Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity).

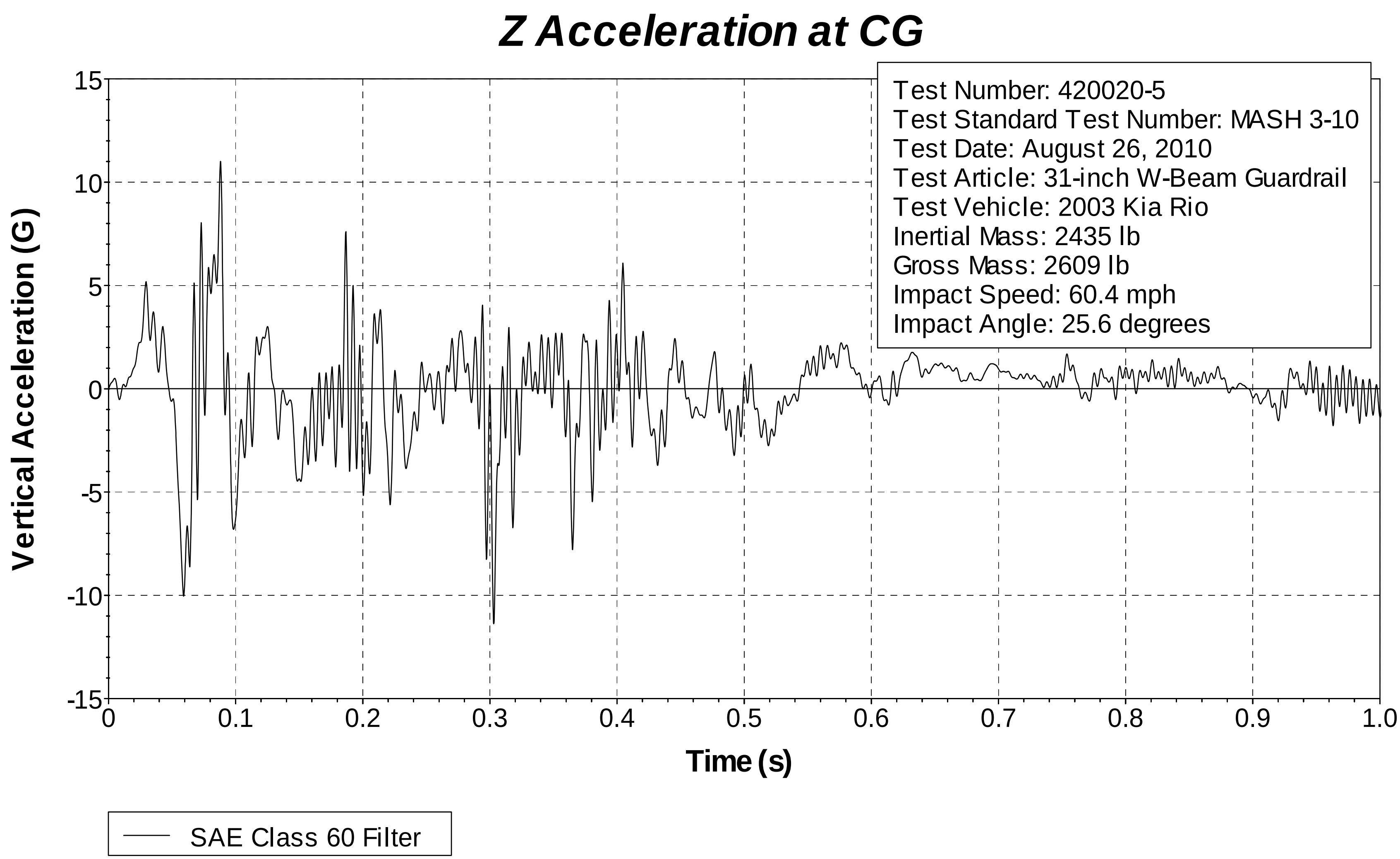


Figure F4. Vehicle Vertical Accelerometer Trace for Test No. 420020-5 (Accelerometer Located at Center of Gravity).

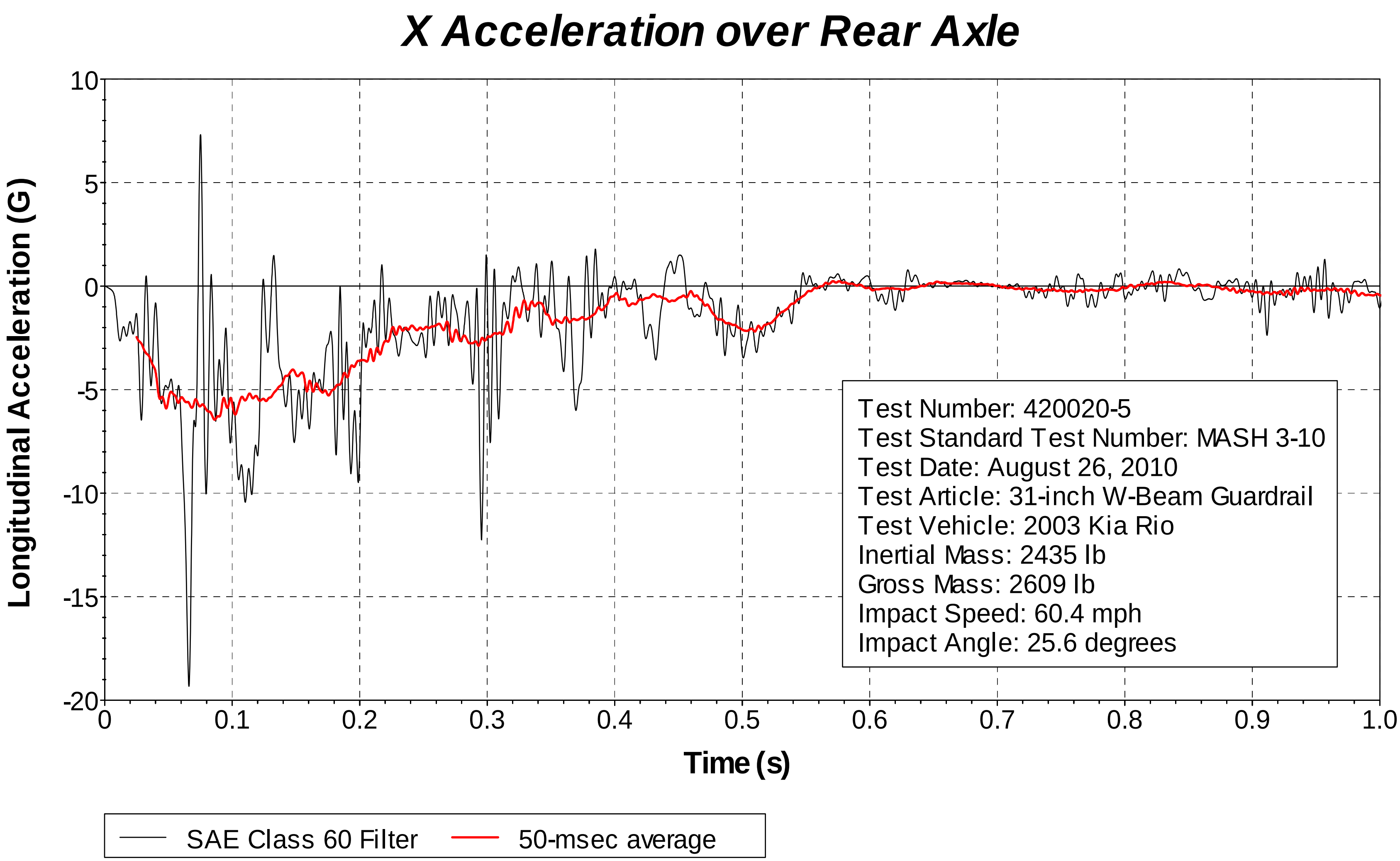


Figure F5. Vehicle Longitudinal Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle).

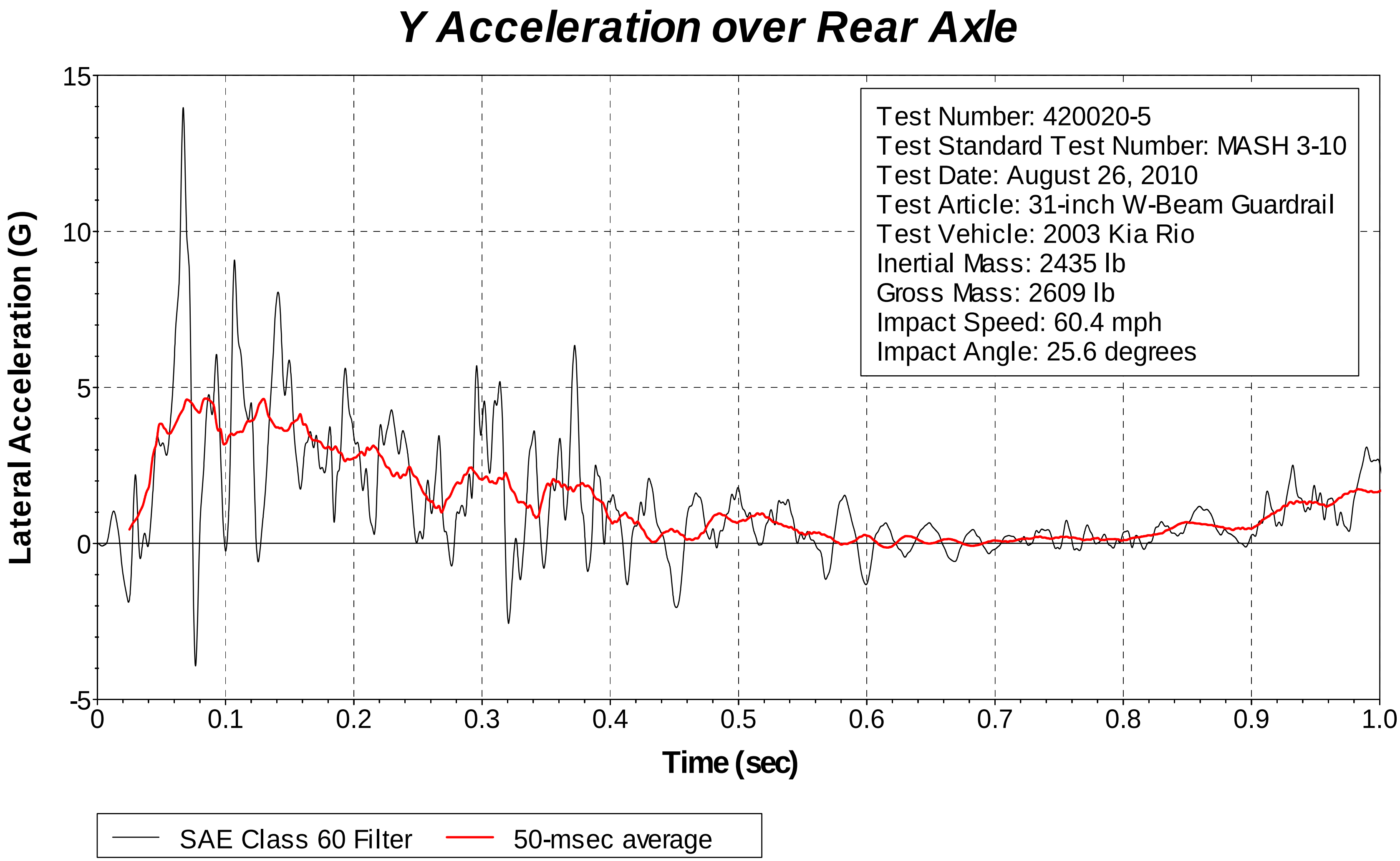


Figure F6. Vehicle Lateral Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle).

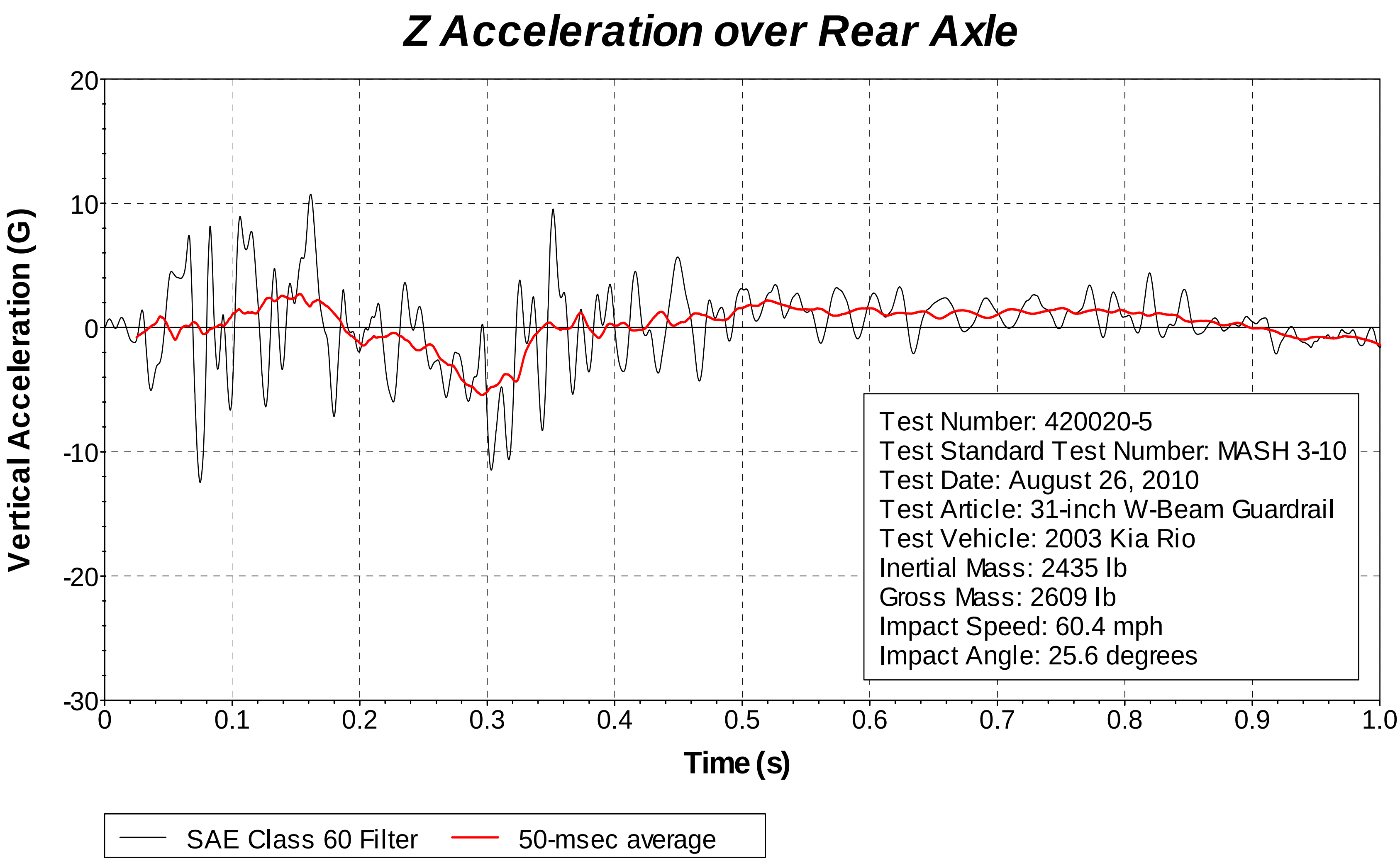


Figure F7. Vehicle Vertical Accelerometer Trace for Test No. 420020-5 (Accelerometer Located over Rear Axle).